

Data Collected to Support Monitoring of Constructed Emergent Sandbar Habitat on the Missouri River Downstream from Gavins Point Dam, South Dakota and Nebraska, 2004–06

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Abstract

The U.S. Army Corps of Engineers has constructed emergent sandbar habitat on sections of the Missouri River bordering South Dakota and Nebraska downstream from Gavins Point Dam to create and enhance habitat for threatened and endangered bird species. Two areas near river miles 761.3 and 769.8 were selected for construction of emergent sandbar habitat. Pre- and postconstruction data were collected by the U.S. Geological Survey, in cooperation with the U.S. Army Corps of Engineers, to evaluate the success of the habitat management techniques. Data collected include pre- and postconstruction channel-geometry data (bathymetric and topographic) for areas upstream from, downstream from, and within each construction site. Water-velocity data were collected for selected parts of the site near river mile 769.8. Instruments and methods used in data collection, as well as quality-assurance and quality-control measures, are described. Geospatial channel-geometry data are presented for transects of the river channel as cross sections and as geographical information system shapefiles. Geospatial land-surface elevation data are provided for part of each site in the form of a color-shaded relief map. Geospatial water-velocity data also are provided as color-shaded maps and geographical information system shapefiles.

Introduction

The U.S. Army Corps of Engineers (USACE) authorized a project to alter part of the Missouri River channel downstream from Gavins Point Dam in South Dakota and Nebraska. Channel alterations involved construction of emergent sandbar habitat (ESH) for endangered least terns and threatened piping plovers as outlined in the 2000 Biological Opinion (U.S. Fish and Wildlife Service, 2000) and the amended 2003

Biological Opinion (2003 BiOp) (U.S. Fish and Wildlife Service, 2003). The 2003 BiOp also mandates an intensive program to monitor and evaluate the success of habitat management techniques used by the USACE to meet specific habitat goals. Therefore, to assess the physical and ecological effects of the channel alterations, accurate information was required to define the Missouri River channel geometry prior to and after construction of the project. Two areas were selected by the USACE for creation and enhancement of ESH (fig. 1). The U.S. Geological Survey (USGS), in cooperation with the USACE, collected data to document the pre- and postconstruction channel geometry within the two areas. The channel alterations were completed in the fall of 2004 and spring of 2005 for the downstream and upstream ESH sites, respectively. Preconstruction data were collected during the fall of 2004 and postconstruction data were collected during November 2005 and February 2006.

Purpose and Scope

The purpose of this report is to describe the methods used in data collection and to present pre- and postconstruction channel-geometry data collected from hydrographic (bathymetric and topographic) surveys and postconstruction water-velocity data for ESH on the Missouri River downstream from Gavins Point Dam, located on the South Dakota and Nebraska State line, for 2004–06. Pre- and postconstruction channel-geometry data are provided in both raw and processed forms for each of the two ESH sites. Geospatial channel-geometry data are presented for transects of the river channel as cross sections and as geographical information system (GIS) shapefiles. Geospatial land-surface data are provided for part of each site in the form of a color-shaded relief map. Postconstruction geospatial water-velocity data for selected transects also are provided as color-shaded maps and GIS shapefiles.

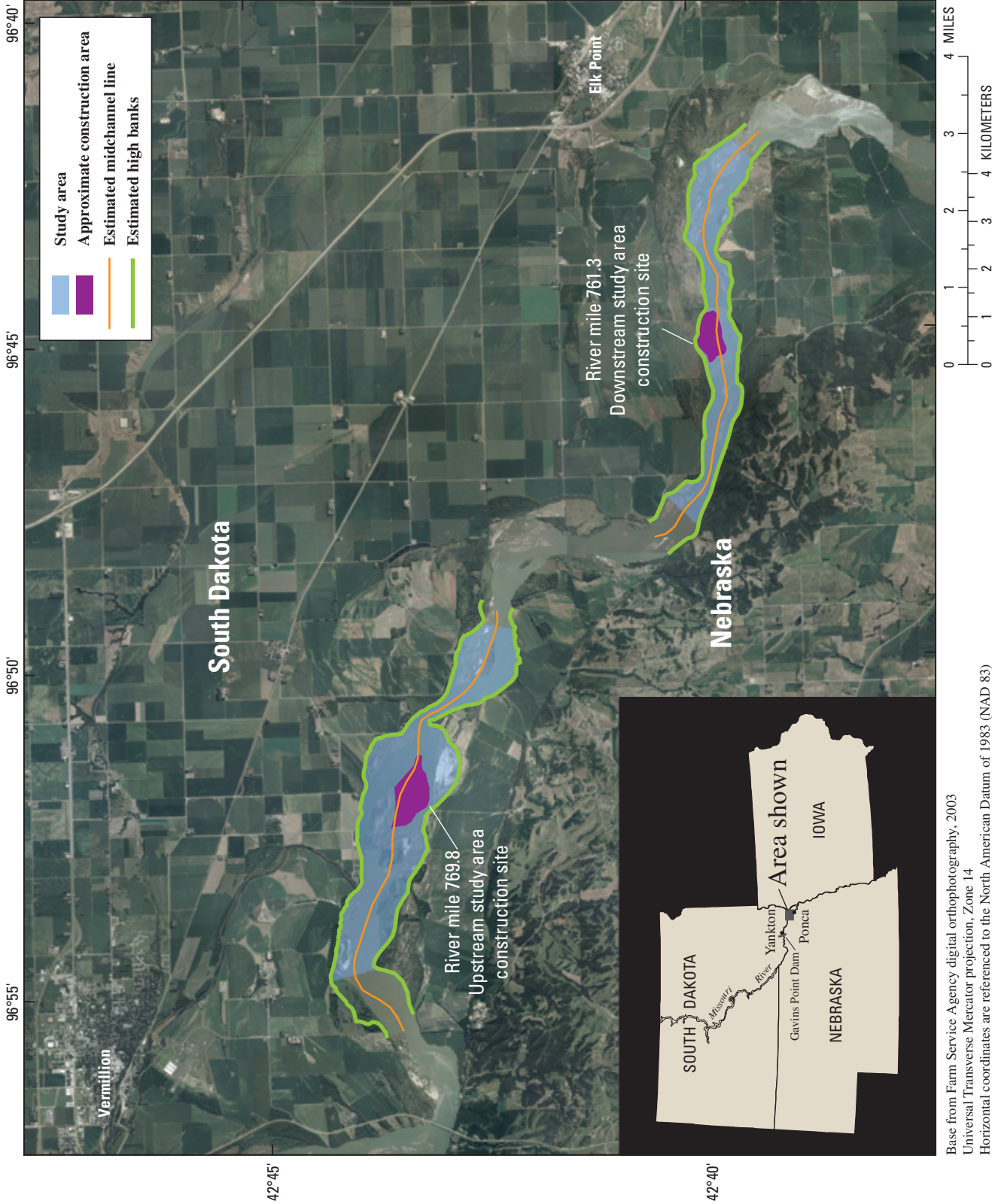


Figure 1. Location of study areas.

Description of Study Areas

Gavins Point Dam is located approximately 3 miles (mi) west of Yankton, South Dakota, on a part of the Missouri River that forms the South Dakota-Nebraska State line (fig. 1). The part between Gavins Point Dam and Ponca, Nebraska, is known as the lower district of the Missouri National Recreation River, where river conditions remain in a relatively natural state. The areas selected for creation and enhancement of ESH are located near river mile (RM) 761.3 (fig. 2) and RM 769.8 (fig. 3) near the South Dakota towns of Elk Point and Vermillion, respectively (fig. 1).

The USACE specified that for each ESH site, preconstruction channel-geometry information was required for a reach extending from 2 mi upstream to 2 mi downstream from the site boundaries. Thus, each of the study areas is composed of a construction area and the 2-mi upstream and downstream buffers adjacent to the construction area.

Despite the impoundments located upstream from Gavins Point Dam, the Missouri River between Gavins Point Dam and Ponca, Nebraska, still possesses several of the characteristics of a “wild” river. As such, certain natural phenomena can be assumed, including shifting sandbars, a primary channel with swift currents, and islands that erode and re-deposit downstream. A major tributary, the Vermillion River, enters the Missouri River in the upstream study area (fig. 3), and several other draws and sloughs contribute flow on a seasonal

basis. The width of the channel varies from approximately 750 feet (ft) to more than 6,000 ft, and water depths during data collection occasionally reached 30 ft. Although flows were less than 12,000 cubic feet per second (ft³/s) during data collection, considerable shifting of sandbars was observed in this dynamic system.

Methods of Study

For preconstruction data collection, a series of transects were laid out every 500 ft approximately perpendicular to the general flow direction of the river. A slightly revised set of transects was used for postconstruction channel-geometry data collection. To better define channel geometry near the ESH sites, the postconstruction transects were spaced more widely at the upstream and downstream ends of the study areas, and additional transects were added between the original transects throughout each construction area. All transects for which data are presented in this report are shown in figures 2 and 3 and listed in table 1. Channel-geometry data were collected approximately every 2 ft along the length of each transect from high bank to high bank and deviated no more than 10 ft on either side of the transect. The coordinates of transect end points, in Universal Transverse Mercator (UTM) feet, Zone 14, and the North American Datum of 1983 (NAD 83) are listed in table 2.

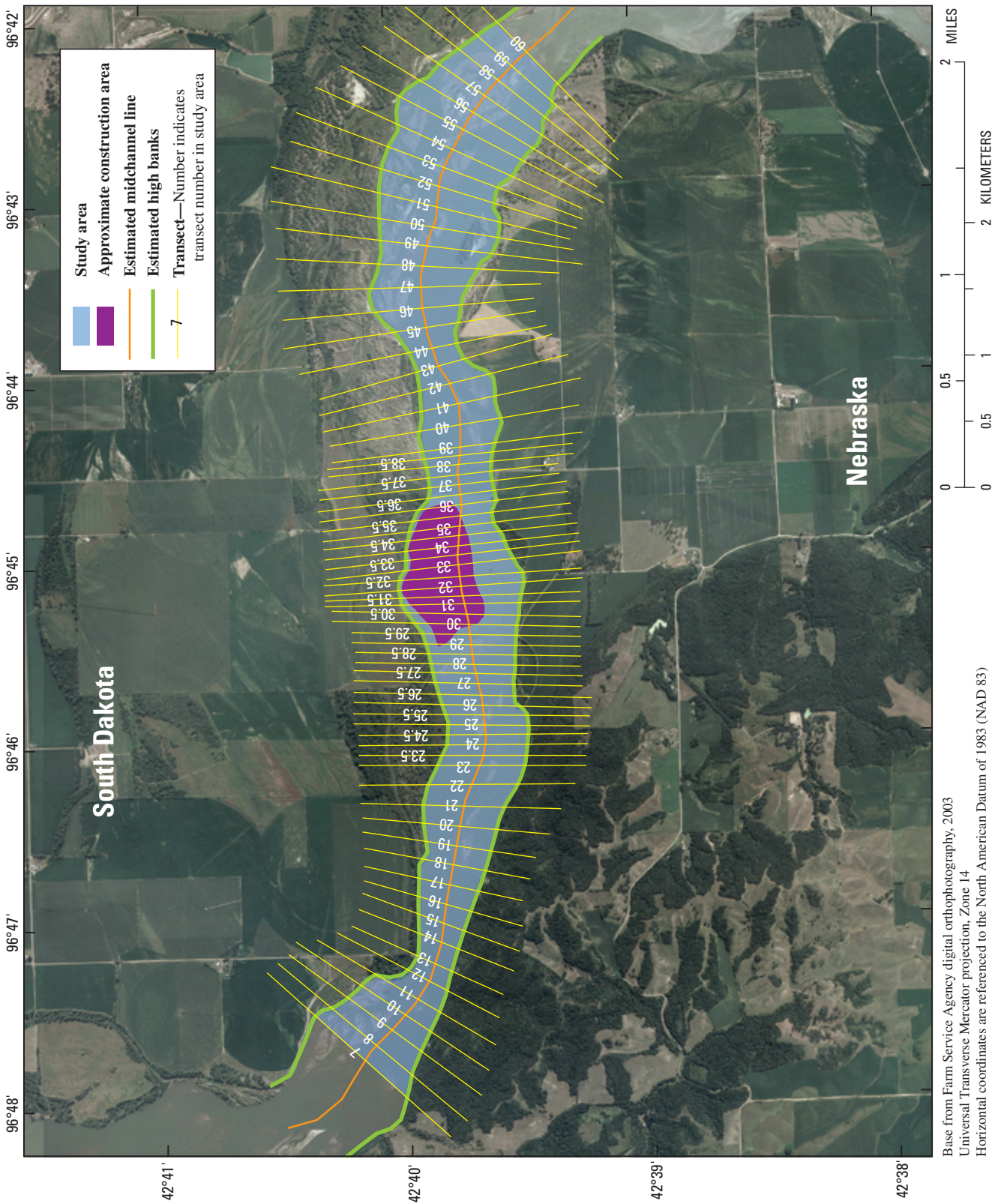


Figure 2. Location of data-collection transects in downstream study area (for the construction area at river mile 761.3).

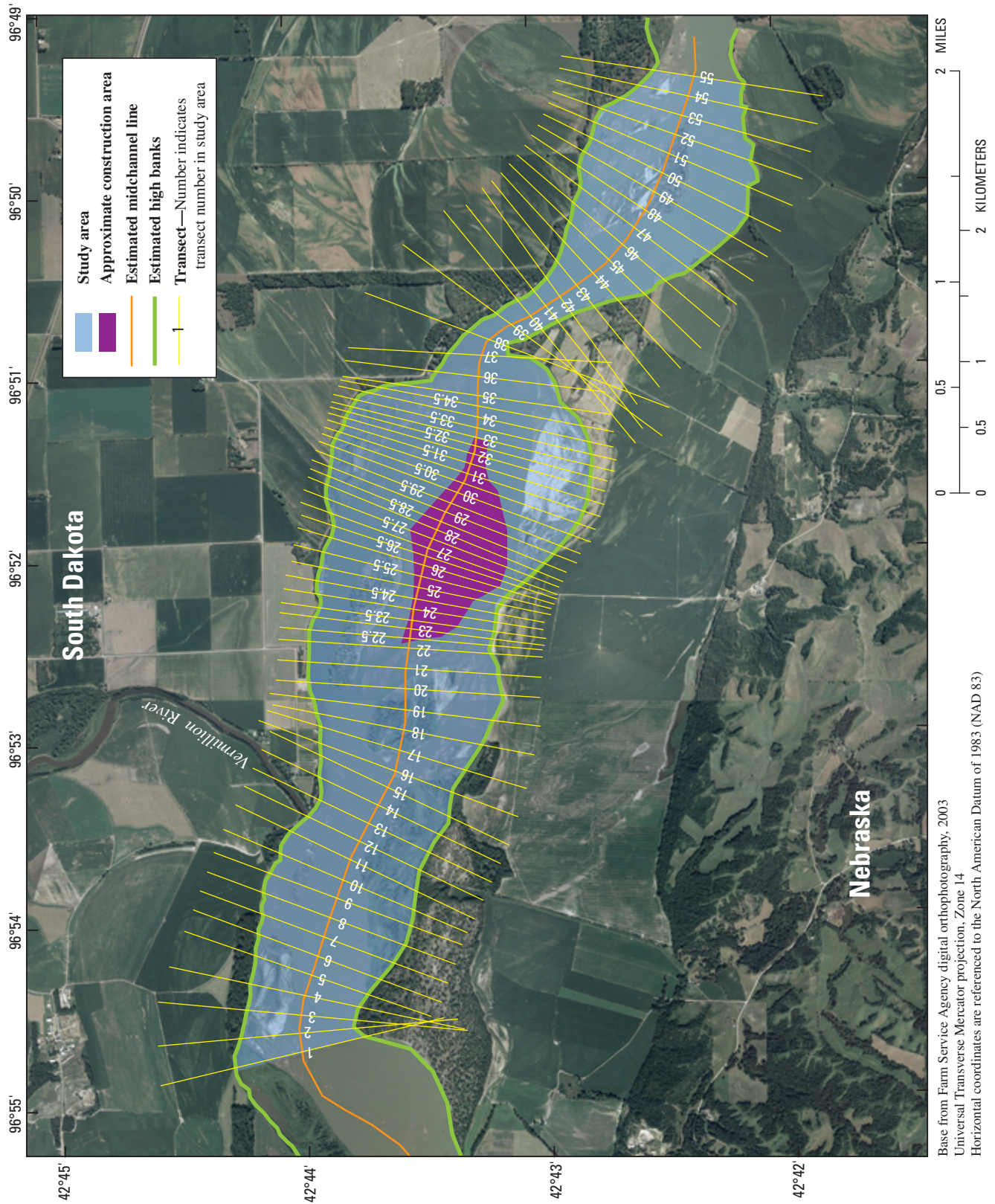


Figure 3. Location of data-collection transects in upstream study area (for the construction area at river mile 769.8).

6 Data Collected on Constructed Sandbar Habitat below Gavins Point Dam, South Dakota and Nebraska

Table 1. Transects specified for pre- and postconstruction data collection.

[RM, river mile]

Preconstruction	Postconstruction	Preconstruction	Postconstruction
Downstream study area (encompassing RM 761.3)		34	31.5
7	7	35	32
8	10	36	32.5
9	12	37	33
10	13	38	33.5
11	14	39	34
12	15	40	34.5
13	18	41	35
14	20	42	35.5
15	21	43	36
16	22	44	36.5
17	23	45	37
18	23.5	46	37.5
19	24	47	38
20	24.5	48	38.5
21	25	49	39
22	25.5	50	40
23	26	51	41
24	26.5	52	42
25	27	53	43
26	27.5	54	45
27	28	55	47
28	28.5	56	50
29	29	57	53
30	29.5	58	56
31	30	59	58
32	30.5	60	60
33	31	Total: 54	54

Table 1. Transects specified for pre- and postconstruction data collection.—Continued

[RM, river mile]

Preconstruction	Postconstruction	Preconstruction	Postconstruction
Upstream study area (encompassing RM 769.8)		28	30.5
1	1	29	31
2	4	30	31.5
3	7	31	32
4	10	32	32.5
5	13	33	33
6	15	34	33.5
7	17	35	34
8	19	36	34.5
9	20	37	35
10	21	38	36
11	22	39	37
12	22.5	40	38
13	23	41	39
14	23.5	42	40
15	24	43	41
16	24.5	44	43
17	25	45	46
18	25.5	46	49
19	26	47	52
20	26.5	48	55
21	27	49	
22	27.5	50	
23	28	51	
24	28.5	52	
25	29	53	
26	29.5	54	
27	30	55	
		Total: 55	48

8 Data Collected on Constructed Sandbar Habitat below Gavins Point Dam, South Dakota and Nebraska

Table 2. Coordinates of transect end points, in Universal Transverse Mercator feet, Zone 14, and North American Datum of 1983.

[ft, feet; RM, river mile]

Transect number	End point in South Dakota		End point in Nebraska	
	Easting (ft)	Northing (ft)	Easting (ft)	Northing (ft)
Downstream study area (encompassing RM 761.3)				
7	2,235,057.57	15,509,263.54	2,230,986.84	15,504,700.72
8	2,235,281.23	15,508,995.13	2,231,389.43	15,504,298.13
9	2,235,504.91	15,508,726.73	2,232,015.71	15,503,940.25
10	2,235,818.04	15,508,547.79	2,232,641.96	15,503,761.35
11	2,235,862.75	15,508,010.98	2,233,268.24	15,503,582.41
12	2,236,041.69	15,507,518.92	2,233,939.23	15,503,492.94
13	2,236,220.63	15,507,161.05	2,234,520.76	15,503,403.47
14	2,236,623.22	15,506,982.14	2,235,102.29	15,503,135.07
15	2,237,025.84	15,506,892.67	2,235,594.38	15,503,000.85
16	2,237,338.96	15,506,847.92	2,236,220.63	15,502,911.42
17	2,237,696.84	15,506,847.92	2,236,846.90	15,502,911.42
18	2,238,144.15	15,506,847.92	2,237,383.68	15,502,732.48
19	2,238,546.77	15,506,892.67	2,237,920.49	15,502,598.26
20	2,238,904.64	15,506,892.67	2,238,502.02	15,502,240.39
21	2,239,262.48	15,506,937.39	2,239,128.30	15,501,972.01
22	2,239,709.83	15,506,982.14	2,239,754.58	15,501,614.14
23	2,240,201.88	15,506,982.14	2,240,201.88	15,501,345.74
23.5	2,240,451.88	15,506,982.14	2,240,451.88	15,501,345.74
24	2,240,693.94	15,506,982.14	2,240,738.69	15,501,301.02
24.5	2,240,943.94	15,506,984.11	2,240,988.69	15,501,302.99
25	2,241,230.75	15,507,116.33	2,241,186.04	15,501,256.27
25.5	2,241,480.75	15,507,114.42	2,241,436.03	15,501,254.36
26	2,241,767.56	15,507,116.33	2,241,633.34	15,501,211.55
26.5	2,242,017.50	15,507,110.65	2,241,883.28	15,501,205.87
27	2,242,304.34	15,507,116.33	2,242,170.15	15,501,435.20
27.5	2,242,554.27	15,507,110.43	2,242,420.08	15,501,429.30
28	2,242,751.68	15,507,071.61	2,242,662.21	15,501,479.92
28.5	2,243,001.65	15,507,067.61	2,242,912.18	15,501,475.92
29	2,243,243.72	15,507,161.05	2,243,154.27	15,501,479.92
29.5	2,243,493.71	15,507,157.11	2,243,404.24	15,501,475.99
30	2,243,780.55	15,507,653.14	2,243,646.37	15,501,479.92
30.5	2,244,030.49	15,507,647.71	2,243,896.31	15,501,474.49
31	2,244,138.43	15,507,832.07	2,244,272.61	15,501,479.92
31.5	2,244,276.34	15,507,855.50	2,244,522.56	15,501,485.20
32	2,244,406.83	15,507,832.07	2,244,854.14	15,501,435.20

Table 2. Coordinates of transect end points, in Universal Transverse Mercator feet, Zone 14, and North American Datum of 1983.—Continued

[ft, feet; RM, river mile]

Transect number	End point in South Dakota		End point in Nebraska	
	Easting (ft)	Northing (ft)	Easting (ft)	Northing (ft)
32.5	2,244,656.22	15,507,849.51	2,245,103.53	15,501,452.64
33	2,244,809.42	15,507,787.32	2,245,525.17	15,501,479.92
33.5	2,245,057.83	15,507,815.51	2,245,773.57	15,501,508.11
34	2,245,301.48	15,507,832.07	2,245,972.48	15,501,524.67
34.5	2,245,550.08	15,507,858.52	2,246,221.07	15,501,551.12
35	2,245,748.82	15,507,876.79	2,246,464.54	15,501,703.61
35.5	2,245,997.16	15,507,905.85	2,246,712.87	15,501,732.40
36	2,246,240.88	15,507,966.26	2,247,001.35	15,501,793.08
36.5	2,246,489.01	15,507,996.83	2,247,249.47	15,501,823.64
37	2,246,777.69	15,507,966.26	2,247,448.69	15,502,016.73
37.5	2,247,026.12	15,507,994.28	2,247,697.11	15,502,044.75
38	2,247,359.22	15,507,742.61	2,247,940.74	15,501,658.86
38.5	2,247,544.72	15,507,770.73	2,248,189.61	15,501,682.65
39	2,247,717.09	15,507,653.14	2,248,477.56	15,501,435.20
40	2,248,119.68	15,507,697.86	2,249,148.55	15,501,524.67
41	2,248,477.56	15,507,876.79	2,249,819.55	15,501,479.92
42	2,248,880.15	15,507,966.26	2,250,401.08	15,501,793.08
43	2,249,282.74	15,507,966.26	2,250,893.14	15,502,195.67
44	2,249,909.02	15,508,458.32	2,251,116.82	15,502,329.85
45	2,250,535.30	15,508,860.91	2,251,474.66	15,502,598.26
46	2,251,251.01	15,508,860.91	2,251,787.82	15,502,643.01
47	2,251,966.76	15,508,995.13	2,252,145.69	15,502,464.07
48	2,252,771.93	15,509,039.85	2,252,414.06	15,502,016.73
49	2,253,532.40	15,509,039.85	2,252,637.75	15,501,435.20
50	2,254,203.40	15,509,174.07	2,252,995.63	15,501,658.86
51	2,255,053.33	15,509,174.07	2,253,264.00	15,501,748.33
52	2,255,679.61	15,508,682.01	2,253,532.40	15,501,569.39
53	2,256,395.36	15,508,547.79	2,253,756.09	15,501,435.20
54	2,257,021.60	15,508,055.73	2,254,024.46	15,501,435.20
55	2,257,558.41	15,507,429.45	2,254,158.68	15,500,898.39
56	2,258,050.47	15,506,892.67	2,254,561.27	15,501,077.33
57	2,258,542.53	15,506,355.86	2,254,874.39	15,501,032.61
58	2,259,079.34	15,505,908.52	2,254,784.93	15,500,451.09
59	2,259,571.40	15,505,505.93	2,255,232.27	15,500,540.52
60	2,259,884.52	15,504,834.94	2,255,679.61	15,500,630.00

10 Data Collected on Constructed Sandbar Habitat below Gavins Point Dam, South Dakota and Nebraska

Table 2. Coordinates of transect end points, in Universal Transverse Mercator feet, Zone 14, and North American Datum of 1983.—Continued

[ft, feet; RM, river mile]

Transect number	End point in South Dakota		End point in Nebraska	
	Easting (ft)	Northing (ft)	Easting (ft)	Northing (ft)
Upstream study area (encompassing RM 769.8)				
1	2,200,344.39	15,532,854.37	2,202,014.17	15,525,827.45
2	2,201,318.43	15,532,923.92	2,201,979.39	15,525,479.58
3	2,202,445.89	15,532,923.92	2,201,701.08	15,525,236.08
4	2,203,266.49	15,532,645.64	2,201,735.85	15,525,305.63
5	2,204,136.14	15,532,263.00	2,202,048.94	15,526,140.50
6	2,204,658.68	15,532,046.40	2,202,396.09	15,525,887.16
7	2,205,128.30	15,531,873.86	2,202,865.74	15,525,714.62
8	2,205,597.95	15,531,701.35	2,203,335.39	15,525,542.11
9	2,206,067.60	15,531,528.81	2,203,805.01	15,525,369.57
10	2,206,640.80	15,531,288.95	2,204,101.37	15,525,236.08
11	2,207,241.35	15,530,993.41	2,204,444.11	15,525,057.86
12	2,207,693.94	15,530,780.13	2,204,896.71	15,524,844.57
13	2,208,146.54	15,530,597.90	2,205,349.30	15,524,631.29
14	2,208,599.13	15,530,353.55	2,205,801.89	15,524,418.00
15	2,209,051.72	15,530,140.27	2,206,254.48	15,524,204.71
16	2,209,319.37	15,530,419.30	2,206,953.89	15,523,983.75
17	2,209,562.87	15,530,210.57	2,207,649.62	15,523,775.02
18	2,209,702.04	15,530,071.43	2,208,470.94	15,523,575.84
19	2,209,980.32	15,530,140.99	2,209,180.23	15,523,427.16
20	2,210,307.82	15,529,985.18	2,209,878.85	15,523,437.59
21	2,210,807.06	15,529,952.44	2,210,377.70	15,523,404.85
22	2,211,306.31	15,529,919.69	2,210,876.98	15,523,372.10
22.5	2,211,611.01	15,529,902.23	2,211,054.03	15,523,377.54
23	2,211,963.16	15,529,897.48	2,211,163.06	15,523,322.79
23.5	2,212,211.33	15,529,867.28	2,211,332.52	15,523,288.03
24	2,212,554.53	15,529,793.12	2,211,545.74	15,523,357.57
24.5	2,212,886.71	15,529,745.91	2,211,729.66	15,523,326.13
25	2,213,221.43	15,529,599.35	2,211,913.65	15,523,169.31
25.5	2,213,633.20	15,529,546.84	2,212,065.58	15,523,151.95
26	2,214,050.36	15,529,445.25	2,212,171.89	15,523,079.29
26.5	2,214,354.80	15,529,360.22	2,212,301.97	15,523,015.10
27	2,214,676.54	15,529,375.70	2,212,415.39	15,523,009.70
27.5	2,214,976.87	15,529,298.02	2,212,588.12	15,522,952.89

Table 2. Coordinates of transect end points, in Universal Transverse Mercator feet, Zone 14, and North American Datum of 1983.—Continued

[ft, feet; RM, river mile]

Transect number	End point in South Dakota		End point in Nebraska	
	Easting (ft)	Northing (ft)	Easting (ft)	Northing (ft)
28	2,215,302.69	15,529,201.75	2,212,728.48	15,522,870.56
28.5	2,215,499.41	15,529,136.28	2,212,960.07	15,522,776.40
29	2,215,720.14	15,529,097.39	2,213,180.71	15,522,522.70
29.5	2,215,953.35	15,529,007.31	2,213,413.92	15,522,432.62
30	2,216,172.37	15,528,923.47	2,213,702.50	15,522,174.83
30.5	2,216,357.87	15,528,850.12	2,214,006.44	15,522,057.11
31	2,216,520.24	15,528,784.30	2,214,328.68	15,521,931.32
31.5	2,216,693.79	15,528,713.27	2,214,628.51	15,521,845.60
32	2,216,868.11	15,528,679.97	2,214,954.82	15,521,826.96
32.5	2,217,017.27	15,528,638.62	2,215,195.61	15,521,759.74
33	2,217,146.39	15,528,610.38	2,215,441.83	15,521,583.46
33.5	2,217,328.30	15,528,563.97	2,215,710.93	15,521,534.57
34	2,217,529.06	15,528,436.43	2,216,033.23	15,521,618.23
34.5	2,217,676.66	15,528,389.79	2,216,407.64	15,521,609.22
35	2,217,772.57	15,528,332.07	2,216,833.30	15,521,687.82
36	2,218,155.21	15,528,262.51	2,217,389.89	15,521,826.96
37	2,218,477.58	15,528,200.67	2,218,108.49	15,521,649.37
38	2,219,824.96	15,527,775.51	2,217,007.25	15,520,226.77
39	2,221,007.73	15,526,836.27	2,216,311.51	15,520,400.72
40	2,221,842.60	15,525,827.45	2,216,137.56	15,521,061.64
41	2,221,981.74	15,525,270.85	2,216,485.43	15,520,887.72
42	2,222,392.11	15,524,871.94	2,217,081.92	15,521,017.42
43	2,222,575.74	15,524,634.53	2,217,510.43	15,520,463.45
44	2,222,773.51	15,524,408.75	2,217,971.88	15,519,936.64
45	2,222,984.69	15,524,195.46	2,218,464.62	15,519,438.94
46	2,223,208.48	15,523,995.43	2,218,986.77	15,518,972.21
47	2,223,444.04	15,523,809.41	2,219,536.41	15,518,538.19
48	2,223,690.50	15,523,638.11	2,220,111.47	15,518,138.45
49	2,223,946.93	15,523,482.14	2,220,709.86	15,517,774.54
50	2,224,212.42	15,523,342.08	2,221,329.32	15,517,447.77
51	2,224,418.49	15,523,218.49	2,221,967.51	15,517,159.35
52	2,224,766.48	15,523,111.80	2,222,622.13	15,516,910.40
53	2,225,053.00	15,523,022.40	2,223,290.70	15,516,701.84
54	2,225,344.47	15,522,950.65	2,223,970.72	15,516,534.39
55	2,225,639.74	15,522,896.81	2,224,659.72	15,516,408.73

Data collected along each transect included (1) water depth (bathymetry) in areas with depths generally greater than 3 ft; and (2) land-surface elevation (topographic) on the high bank and shore and in sandbar areas and riverbed elevation in areas with water depths generally less than 3 ft. In addition, water-velocity data were collected on selected transects in the upstream study area. Water-surface elevation data also were collected at many points throughout both study areas. Specific information regarding instruments used, data-collection methods, and quality-assurance/quality-control (QA/QC) measures are described in the following sections.

The hydrographic surveys consisted of bathymetric and topographic surveys along each transect. The transects were bathymetrically and/or topographically surveyed from high bank to high bank using global positioning system (GPS) guidance to remain on the pre-determined transect line. The topographic surveys were conducted where bathymetric data collection was not possible. Transects generally were perpendicular to the direction of flow. Data acquisition, processing, accuracy, and QA/QC conformed to the recommended standards in "Other General Surveys and Studies" (U.S. Army Corps of Engineers, 2002, table 3-1, p. 3-1) and USGS guidelines.

Bathymetric Survey

Bathymetric data were collected by navigating a survey vessel along the data-collection transects. Navigational maps of the two study areas were developed by creating a georeferenced drawing interface file (DXF file) from digital orthophoto quadrangles (DOQs) of the study areas. The DXF files for approximate high bank and midchannel locations (fig. 1) were imported into Hypack hydrographic software (Coastal Oceanographics, 2006) where channel design and boundaries were used to create border files (.BRD) and planned line files (.LNW) used for data-collection activities. The planned line system used a cross-section spacing of 500 ft, generally oriented perpendicular to channel flow, extending roughly 2 mi upstream and 2 mi downstream from each construction area. Individual cross sections extended from high bank to high bank across the river channel. Prior to postconstruction data collection, additional transects were added between original transects in selected parts of the study areas.

Instrumentation

All instrumentation and equipment used during this project met the requirements noted in "Engineering and Design – Hydrographic Surveying" (U.S. Army Corps of Engineers, 2002; chapter 9). Streambed geometry data were collected using a survey grade Innerspace Technology 456 echosounder coupled with a 200-kilohertz (kHz) 3-degree beam transducer. The transducer was mounted off the starboard side of a 14-ft USGS boat. A 12-channel

differentially corrected Trimble Ag132 GPS (DGPS) was used for horizontal positioning for the bathymetric and water-velocity data sets collected by boat. The DGPS antenna was mounted directly above the transducer (fig. 4). An Innerspace Technology 443A Velocity Profiler was used to measure and record speed-of-sound profiles in the water column. Speed-of-sound profiles for the water column then were used to calibrate the echosounder. All digital bathymetric data were logged into a ruggedized field laptop computer running Hypack hydrographic software.



Figure 4. Configuration of instruments used in bathymetric and water-velocity data collection.

Methods

Equipment setup, calibration, and data-collection procedures complied with the guidelines described by the USACE (2002, chapter 9). To map the streambed geometry of the study area, a continuous series of acoustic pulses were emitted from the bathymetric transducer. The pulses traveled through the water column, reflected off of the streambed, and returned to the transducer. A time-of-travel equation (U.S. Army Corps of Engineers, 2002, chapter 9) was applied to the returning acoustical pulse to determine water depth and streambed geometry. Because bathymetric data collection on rough water was avoided, errors associated with pitch and roll of the survey vessel were not corrected. Digital bathymetric data and horizontal positioning were logged concurrently on a computer running Hypack software. Digital data were processed within Hypack to remove erroneous readings in the data set. Instrument draft and water depth at each horizontal location were then subtracted from a water-surface elevation determined using real-time kinematic (RTK) GPS to calculate streambed elevation.

Quality Assurance/Quality Control

Data accuracies for the bathymetric survey satisfied the requirements set forth in “Other General Surveys and Studies” (U.S. Army Corps of Engineers, 2002, table 3–1, p. 3–1). Equipment setup and calibration required an initial measurement of the transducer draft, a speed-of-sound velocity profile, and a bar check (fig. 5). Approximately 10 percent of the cross sections were re-run with a different deployment of the equipment. This duplicate data allowed an assessment of repeatability of both horizontal location and elevation to assure the quality of the data. In addition, the bathymetric analog data were captured and downloaded for future referencing.



Figure 5. Apparatus used to conduct bar check of echosounder transducer.

Topographic Survey

GPS instruments utilize satellite signals to determine the position of a point on the earth. For the topographic survey, GPS instruments were used to collect land-surface elevation data on the dry-land parts of the transects and stream-bottom

elevation data on areas of the transects too shallow for the bathymetric vessel. The GPS data then were combined with the bathymetric data to form cross sections along each transect. GPS instruments also were used to record water-surface elevations throughout the study area and for horizontal location determination of water-velocity data points.

Instrumentation

GPS instruments used to survey shallow water habitat and banks of the study areas on the Missouri River near RM 761.3 and RM 769.8 included Ashtech Z-Xtreme receivers and Geodetic IV revision A antennae. The Geodetic IV revision A GPS antenna is a dual-frequency (L1/L2) model. The horizontal accuracy of the Z-Xtreme was explained in the reference manual as 5 millimeters (mm) + 1 part per million (ppm) when collection type is set to static and the data are post-processed. The accuracy was 1.6 centimeters (cm) + 2 ppm for RTK points with five satellites and position dilution of precision (PDOP) less than 4 (Ashtech Precision Products, 2001).

To minimize distance-related errors, base stations were established near the area to be surveyed rather than relying on benchmarks distant from the site. Most established bases were set using static GPS surveying procedures and post-processed against three first-order vertical and high-order horizontal National Geodetic Survey (NGS) benchmarks. The accuracy for these bases was determined by the accuracy of the NGS benchmark plus the quality of the data collected. Table 3 shows the NGS benchmark relative accuracy and an estimated spatial accuracy (Doyle and McKay, 2000). Post-processing a network with a time span of at least 2 hours usually produced positions with a 95-percent error of less than 0.02 meter (m) horizontal and vertical. If time did not allow for NGS benchmarks to be located, static sessions were processed through the NGS Online Positioning Users Service (OPUS, at <http://www.ngs.noaa.gov/OPUS/>), which processed the session against three continuously operating reference stations. A 2- or 3-hour session processed through OPUS usually produced a position with a peak-to-peak error of less than 0.02-m horizontal and less than 0.03-m vertical.

Table 3. National Geodetic Survey benchmark relative accuracy and estimated spatial accuracy.

[cm, centimeters]

Order	Relative accuracy	Estimated spatial accuracy
A	1:10,000,000	3 cm
B	1:1,000,000	5 cm
1	1:100,000	10 cm
2	1:50,000	20 cm
3	1:10,000	50 cm

Thus, accuracy in each RTK point was the accuracy of the established base plus $1.6 \text{ cm} + 2 \text{ ppm}$. During surveys of the ESH sites, RTK data usually were collected within 6 mi of the base station, so each RTK point had an approximate horizontal error of 3 cm and a vertical error of 10 cm when collected by backpack mount (fig. 6).



Figure 6. Configuration of real-time kinematic global positioning system using backpack mount.

Methods

For the topographic survey, the surveyed transects were set up using DOQs in Hypack software as described previously in the Bathymetric Survey section. The spacing was established by the USACE and modified between pre- and postconstruction data collection. The preconstruction transects were spaced uniformly at 500 ft. For postconstruction data collection, transects were added in the construction areas between the existing transects. The spacing was reduced to 250 ft at the center, then remained at 500 ft, then increased to 1,000 ft and 1,500 ft at the ends of each reach. The transects were set up perpendicular to the channel, usually along the thalweg (figs. 2 and 3).

Elevation measurements were collected using RTK GPS with a backpack mount (fig. 6). Transects were traversed to measure overbank, shore/bank, sandbar, and riverbed elevations for shallow parts of the cross sections not able to be navigated by boat (sections that were typically 3 ft deep or less). Horizontal distances between elevation measurements along the transects generally were less than 2 ft. Horizontal deviation did not exceed 10 ft on either side of a given transect. GPS measurements were only recorded when the error associated with each point was within 0.03 m horizontal and 0.05 m vertical.

Quality Assurance/Quality Control

QA/QC activities for the topographic survey were conducted on a daily basis. Accuracy of the RTK GPS horizontal-position and elevation data was assessed by having each surveyor occupy (stand with the GPS antenna approximately over) points of known horizontal position and elevation a minimum of two times per day (before and after data-collection activities). For example, the standard deviations of the northing and easting and the elevation of 40 occupations at the upstream checkpoint were 0.30, 0.47, and 0.27 ft, respectively. The standard deviations of the northing and easting and the elevation of 32 occupations at the downstream checkpoint was 0.44, 0.35, and 0.26 ft, respectively. Approximately 10 percent of the transects were surveyed in duplicate to provide additional QA/QC information to verify the accuracy and precision of data-collection activities.

Water-Velocity Data Collection

Water-velocity data were collected along selected transects of the upstream study area (RM 769.8) using two methods. In deep water (generally greater than 3 ft), water-velocity profile data were collected using an Acoustic Doppler Current Profiler (ADCP) tethered to the survey vessel. In shallow water (generally less than 3 ft), point water-velocity data were collected using a handheld Acoustic Doppler Velocimeter (ADV).

Instrumentation

The ADCP used to collect water-velocity profiles was a 1200-kHz Teledyne Rowe Deines Instruments (RDI) Rio Grande. The ADCP was mounted directly in front of the bathymetric transducer, off the starboard side near the bow of the survey vessel. ADCP data were logged concurrently with the bathymetric data into a separate ruggedized field laptop computer running WinRiver software (RD Instruments, 2003). The ADCP derived its horizontal location from the same DGPS antenna as the bathymetric transducer.

The handheld ADV used to collect point water-velocity data was a SonTek/Yellow Springs Instruments (YSI) FlowTracker. The ADV was mounted on a top-setting wading rod and used to measure the velocity at a point six-tenths of the water depth. Horizontal positioning of the FlowTracker was recorded with either a Total Station or RTK GPS.

Methods

ADCP water-velocity profiles were obtained on 47 predetermined transects at the upstream study area (RM 769.8) only. Because of instrument limitations and vessel draft, the ADCP was used only in depths greater than approximately 3 ft. Equipment setup, calibration, and data-collection procedures complied with the guidelines described by Oberg and others (2005). Velocity data were collected using Mode 12 with a 10-cm bin cell size and a blanking distance of 20–25 cm. Multiple bins arranged in a vertical column or “ensemble” occurred every 1.45 seconds. Boat speed was maintained at or near 2.0 knots. Data acquisition based on boat speed and DGPS coverage occurred approximately every 5 ft. Velocity and DGPS data were logged on a separate computer running WinRiver ADCP software.

FlowTrackers were used to collect point water-velocity data in parts of the selected transects that were too shallow for the ADCP (less than approximately 3 ft). Point water-velocity data were collected approximately every 50 ft along the transect and included a velocity and a compass direction of flow. The average velocity measured over a 40-second time interval was recorded. Horizontal positioning of the point velocity data was accomplished with a Total Station set up on the transect or by utilizing RTK GPS. Total Station data and FlowTracker data were later combined by point name. When RTK GPS was used for positioning, the velocity and compass direction were used as the point description stored in the GPS.

Quality Assurance/Quality Control

The ADCP was calibrated before data collection on a daily basis and consisted of a measurement of equipment draft, an internal diagnostics test, and a compass calibration. Each FlowTracker was calibrated daily before data collection by submersing in water and running an internal diagnostics test.

Bathymetric, Topographic, and Water-Velocity Data

Raw data collected as described previously were processed and corrected, where applicable. The resulting data sets were used to generate GIS shapefiles, which then were used to develop the maps and cross sections that help visualize

the data. The specific processes used are described in the following sections.

Bathymetric and Topographic Data

The processed bathymetric data were output in an XYZ format, where X and Y are the horizontal locations of a specific point and Z is the riverbed elevation at that point. The XYZ file was used to generate a geospatial data set, and GIS software programs were used to change the data to a different projection and/or visualize the data. Although vertical data were collected in the North American Vertical Datum of 1988, they are presented here in the National Geodetic Vertical Datum of 1929 to facilitate comparison with historical USACE and USGS data. Topographic data collected with GPS were output in a similar format although each point with an XYZ location also has a label to provide an abbreviated description of the point. For example, a label of LEW denotes left edge of water, and TRB denotes top of right bank. A more complete list of label descriptors is found in the metadata associated with the GIS shapefiles.

Transect Cross Sections

For data visualization purposes, a series of cross-section plots were generated for each transect. These plots are located in Supplemental Information Section A at the end of this report. ArcMap software (Harlow and others, 2000) was used to select from the geospatial bathymetric and topographic data sets all data points that were within 10 ft of a transect. The resulting data points were output into an XYZ text file and sorted by transect number, northing, and point identifier. The sorted file was imported into a database using Geosoft's Oasis Montaj version 6.3.0 (Geosoft, 2006) and organized by transect number. In Oasis Montaj, the projection was defined as UTM Zone 14, NAD 83, feet, and data were once again sorted by northing and easting. A non-linear filter (despike) was applied using `nlfilt.gx` with a width of 1 (fiducials) and a tolerance of 1. This defines the magnitude of the filter as 1 ft. The filtered database then was loaded into Encom's Profile Analyst version 6.00 (Rockware, 2006) (expert-user forced entry), where curve profiles were generated.

Because of various field conditions encountered during data collection, small data gaps occasionally are present in cross sections. Every effort was made to navigate the bathymetric vessel such that the data would be collected within 10 ft of the transect. Most of the time, this was achieved, but swift currents, wind, and obstacles sometimes made this impossible. Submerged timber, beaver dams, and log jams occasionally prevented the bathymetric vessel from staying within 10 ft of the transect. Similarly, steep or unstable banks sometimes prevented collection of RTK GPS data every 2 ft. Log jams and beaver dams in shallow water also occasionally affected GPS data collection. In high bank and island areas, thick tree and

brush cover sometimes made it difficult to stay on the transect. Some of the same areas had a vegetative canopy sufficiently thick to interfere with satellite reception. In such areas, it was difficult to maintain continuous lock on the satellites, and occasionally the signal was blocked completely. Because the areas with thick canopy were always on the bank or on the larger islands, they were generally at or above the high-bank elevation.

Surface Visualization Maps

A color-shaded relief map was generated for the post-construction land surface from transect 23 to 39 at the downstream study area near RM 761.3 (fig. 7) and from transect 22 to 35 at the upstream study area near RM 769.8 (fig. 8). Comparisons of the hydrographic data collected in 2004 before ESH construction and in 2005 and 2006 following ESH construction were made to document changes in channel and overbank geometry. This was accomplished by subtracting the postconstruction surface from the preconstruction surface. The resulting maps show the elevation difference between pre- and postconstruction surfaces at the downstream (fig. 9) and upstream (fig. 10) study areas.

Water-Velocity Data

ADCP water-velocity data collected in WinRiver software were output in an XYZ comma-delimited file format.

The data then were sorted in Microsoft Excel by transect number and northing, and easting and saved as a new file. The sorted file was imported into a database using Geosoft's Oasis Montaj version 6.3.0 (Geosoft, 2006) and organized by transect number. Depth (D1) and velocity (R1) fields were loaded as arrays. In Oasis Montaj, the projection was defined as UTM Zone 14, NAD 83, feet, and data once again were sorted by northing and easting. An average velocity channel (column) was generated from the velocity array (R1), and the final database was saved.

The final database was loaded into Encom's Profile Analyst version 6.00 (Rockware, 2006), where water-velocity profiles were generated as section plots (by D1 and R1). A data map was generated using the Look Up Table (LUT) editor, displaying values at 0 as transparent and a linear gradation of colors between -0.5 and 4.0 feet per second (ft/s). Color-shaded velocity cross sections are presented in Supplemental Information Section B for all transects with water-velocity data.

Point water-velocity data from the handheld ADV units were pooled from the two collection sources (GPS and Total Station) and used to generate a GIS shapefile. Because the water velocity was already a vertical average for the depth of flow (as opposed to the vertical profile of velocities collected by the ADCP), point water-velocity data can be visualized by color-shaded points on a plan view (fig. 11).

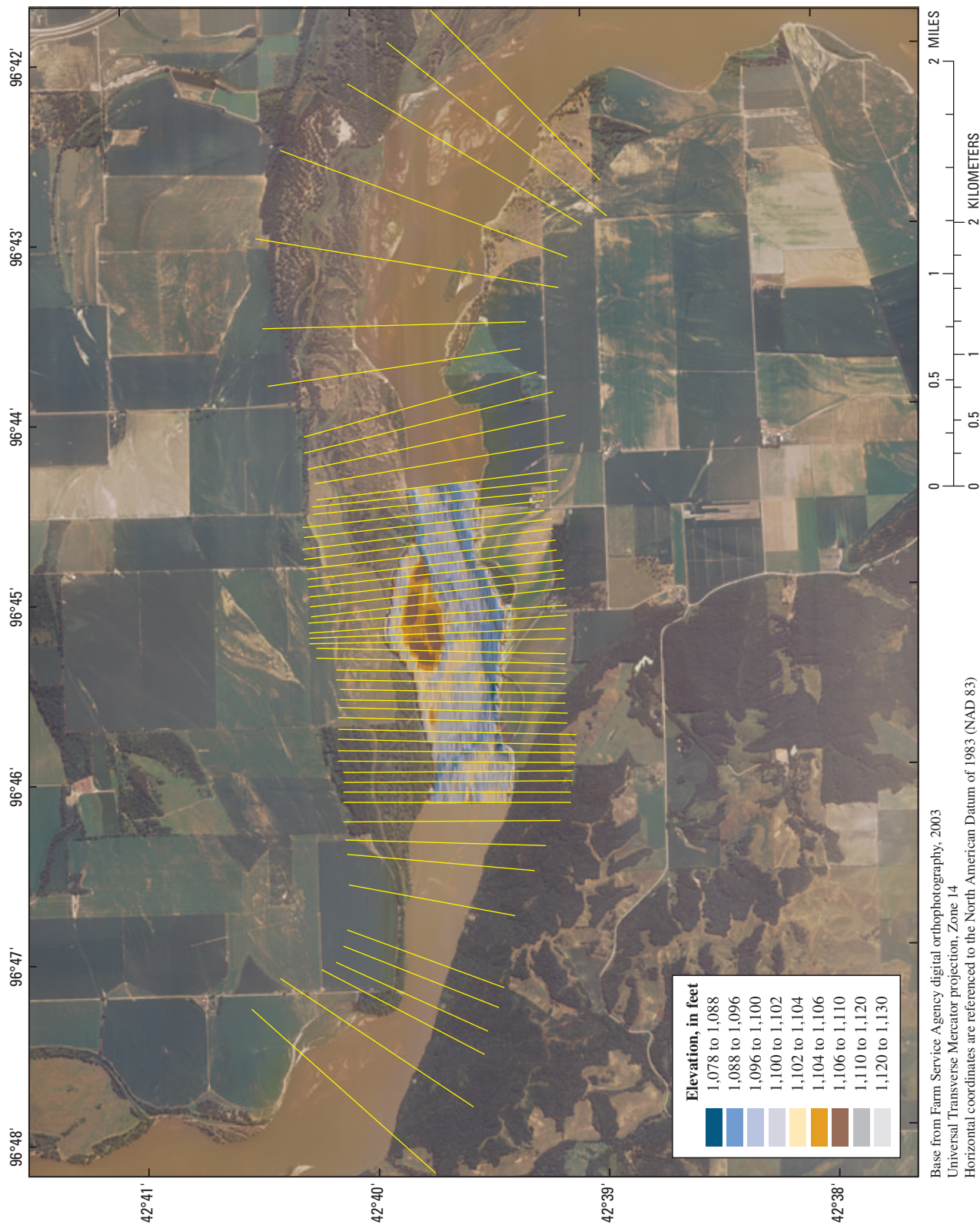


Figure 7. Postconstruction land-surface elevation from transect 23 to 39 at the downstream study area near river mile 761.3.



Figure 8. Postconstruction land-surface elevation from transect 22 to 35 at the upstream study area near river mile 769.8.

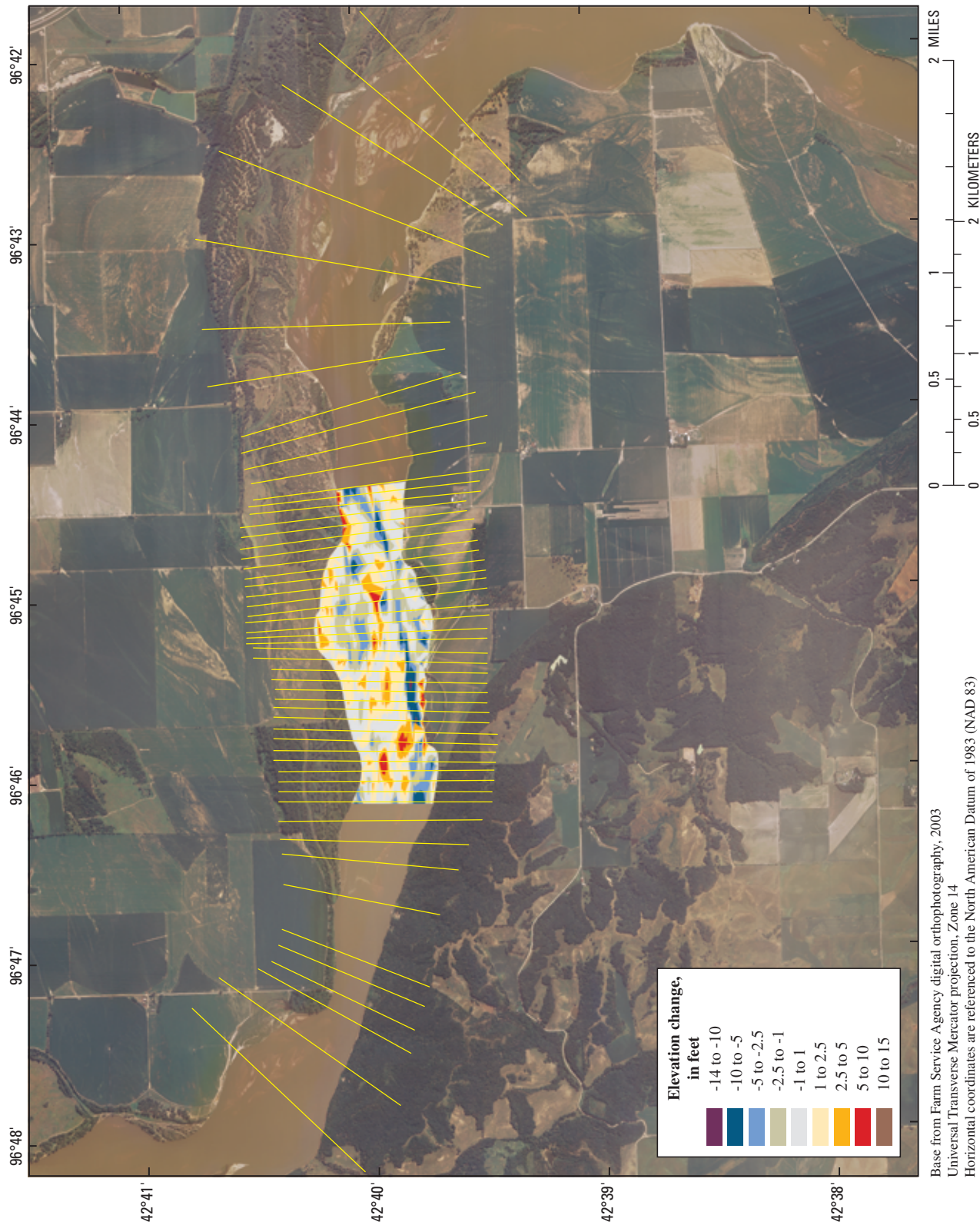


Figure 9. Difference between pre- and postconstruction land-surface elevation from transect 23 to 39 at the downstream study area near river mile 761.3.

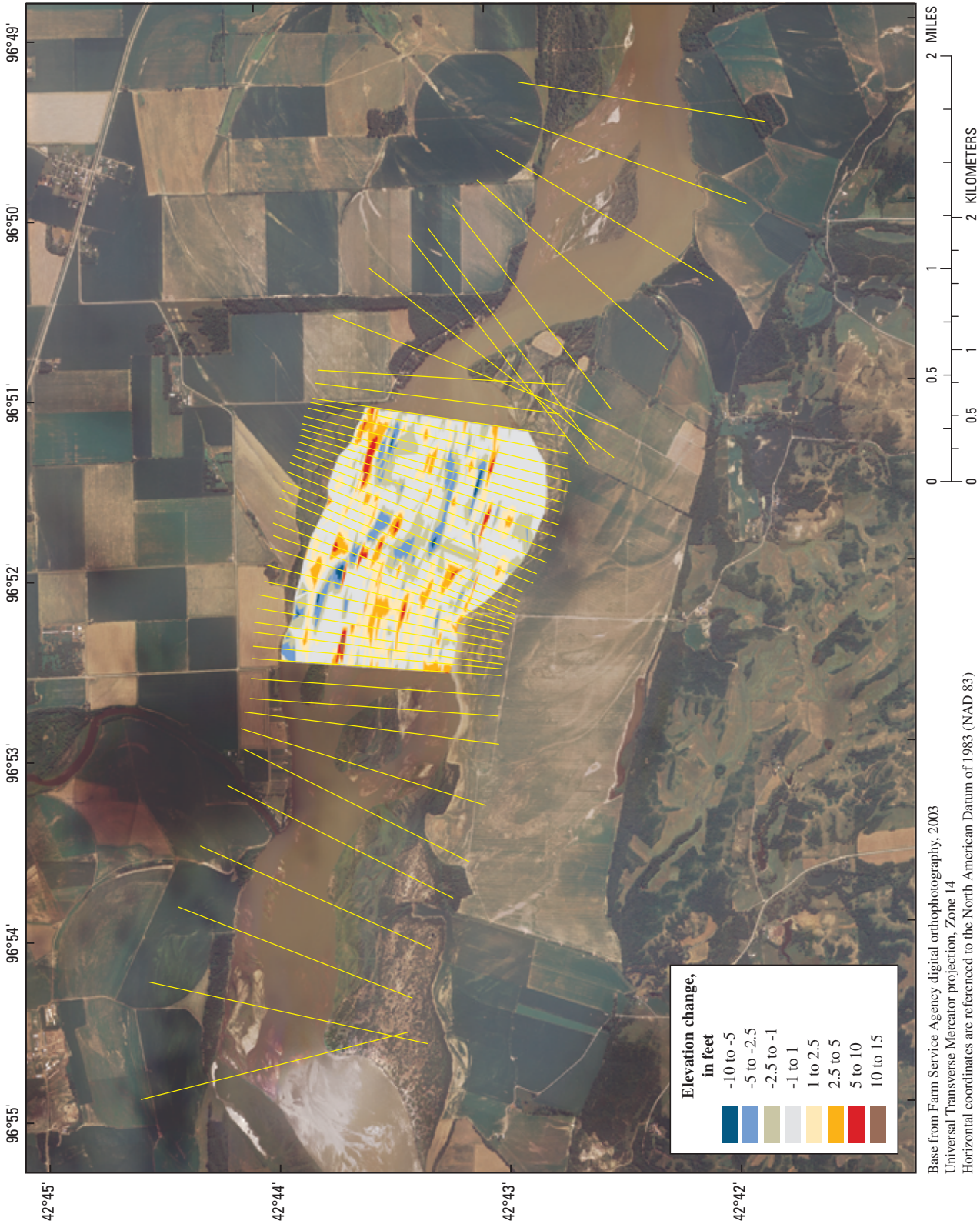


Figure 10. Difference between pre- and postconstruction land-surface elevation from transect 22 to 35 at the upstream study area near river mile 769.8.

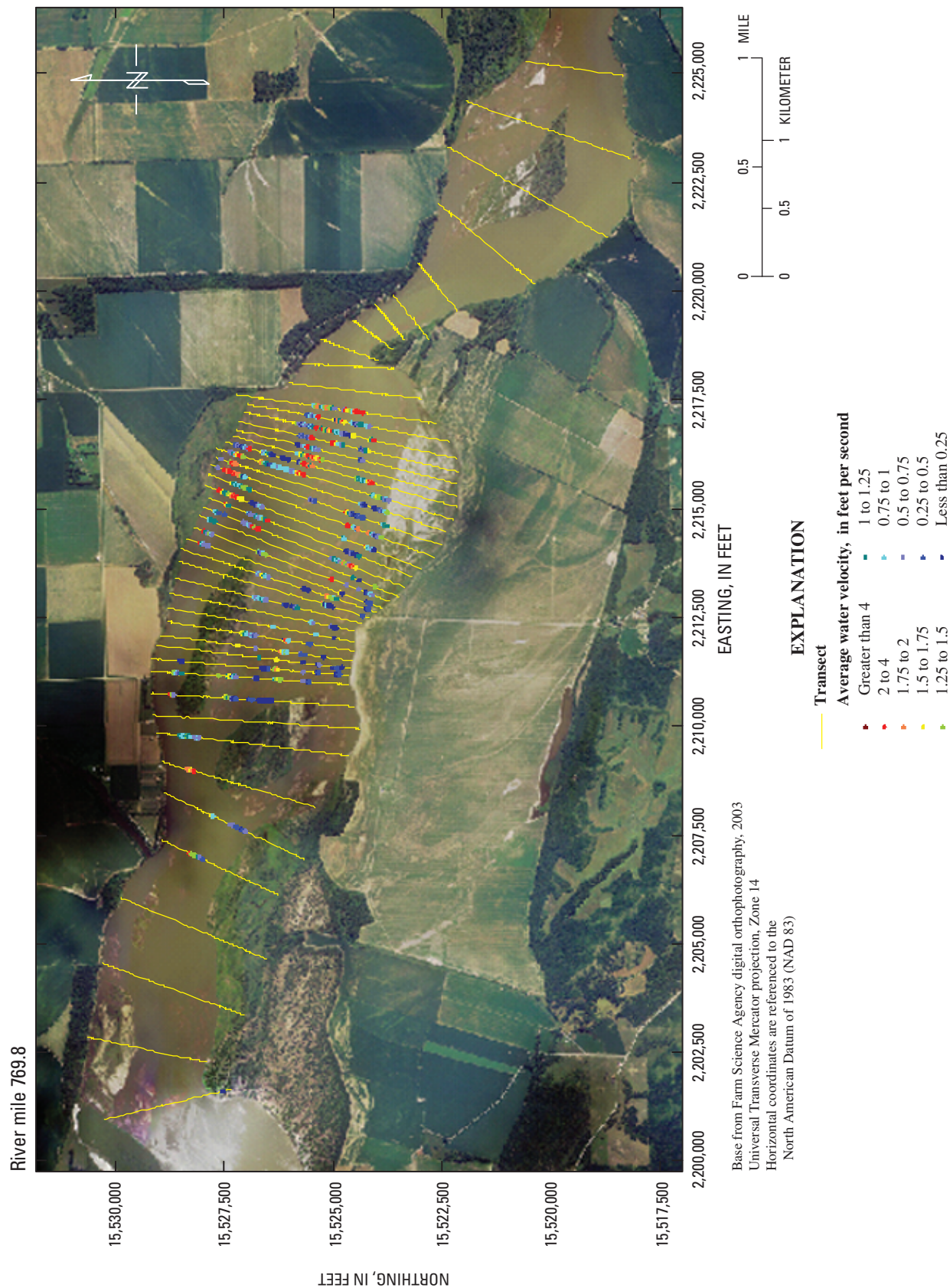


Figure 11. Point water-velocity data in the upstream study area near river mile 769.8.

Summary

The U.S. Army Corps of Engineers (USACE) authorized a project to alter part of the Missouri River channel downstream from Gavins Point Dam in South Dakota and Nebraska. Channel alterations involved construction of emergent sandbar habitat (ESH) for endangered least terns and threatened piping plovers, as outlined in the 2000 Biological Opinion and the amended 2003 Biological Opinion (2003 BiOp). The 2003 Biological Opinion also mandates an intensive program to monitor and evaluate the success of habitat management techniques used by the USACE to meet specific habitat goals. Therefore, to assess the physical and ecological effects of the channel alterations, accurate information was required to define the Missouri River channel geometry prior to and after construction of the project. Two areas were selected by the USACE for creation and enhancement of ESH. The U.S. Geological Survey, in cooperation with the USACE, collected data to document the pre- and postconstruction channel geometry within the two areas. This report describes the methods used in data collection and presents channel-geometry (bathymetric and topographic) and water-velocity data.

The areas selected for creation and enhancement of ESH are located near river mile 761.3 and river mile 769.8 near the South Dakota towns of Elk Point and Vermillion, respectively. The USACE specified that for each ESH site, preconstruction channel-geometry information was required for a reach extending from 2 miles upstream to 2 miles downstream from the site boundaries. Thus, each of the study areas is composed of a construction area and the 2-mile upstream and down-

stream buffers adjacent to the construction area. Data were collected approximately every 2 feet along the length of each transect from high bank to high bank and deviated no more than 10 feet on either side of the transect. Data collected along each transect included (1) water depths in areas with depth generally greater than 3 feet; and (2) land-surface elevation on the high bank and shore and sandbar areas and riverbed elevation in areas with water depths generally less than 3 feet. In addition, water-velocity data were collected on selected transects in the upstream study area.

Bathymetric data were collected by navigating a survey vessel along the data-collection transects. Navigational maps of the two study areas were developed by creating a georeferenced drawing interface file from digital orthophoto quadrangles of the study areas. Global positioning system instruments were used to collect land-surface elevation data on the dry-land parts of the transects and stream-bottom elevation data on areas of the transects too shallow for the bathymetric vessel. The global positioning system data then were combined with the bathymetric data to form cross sections along each transect. Water-velocity data were collected on selected transects of the upstream study area (river mile 769.8) using two methods. In deep water (generally greater than 3 feet), water-velocity profile data were collected using an Acoustic Doppler Current Profiler tethered to the survey vessel. In shallow water (generally less than 3 feet), point water-velocity data were collected using a handheld Acoustic Doppler Velocimeter. Raw data were processed and corrected, where applicable, and the resulting data sets were used to generate geographic information system shapefiles, which then were used to develop maps and cross sections.

References Cited

- Ashtech Precision Products, 2001, Operation and Reference Manual Z-Xtreme: Magellan Corporation, Santa Clara, Calif., 54 p.
- Coastal Oceanographics, 2006, Hypack user's manual: Middletown, Conn., accessed October 23, 2006, at <ftp://ftp.hypack.com/Documents/English/HYPACK%20Manual/PDF/HYPack.pdf>
- Doyle, D., and McKay, E., 2000, NGS survey marker accuracy: accessed March 14, 2006, at <http://gpsinformation.net/main/ngs-accuracy.html>
- Geosoft, 2006, Geosoft Oasis Montaj product description: accessed October 23, 2006, at <http://www.geosoft.com/pinfo/oasismontaj/index.asp>
- Harlow, Melanie; Pfaff, Rhonda; Minami, Michael; Hatakeyama, Alan; Mitchell, Andy; Booth, Bob; Payne, Bruce; Eicher, Cory; Blades, Eleanor; Sims, Ian; Baily, Jonathan; Brennan, Pat; Stephens, Sandy; and Woo, Simon, 2000, ArcGIS 9 using ArcMap: Redlands, Calif., Environmental Systems Resources, Incorporated, 585 p.
- Oberg, K.A., Morlock, S.E., and Caldwell, S.W., 2005, Quality-assurance plan for discharge measurements using acoustic doppler current profilers: U.S. Geological Survey Scientific Investigations Report 2005–5183, 435 p.
- RD Instruments, 2003, WinRiver Application, Version 10.06: San Diego, Calif.
- Rockware, 2006, Encom Profile Analyst product description: accessed October 23, 2006, at <http://www.rockware.com/catalog/pages/profioleanalyst.html>
- U.S. Army Corps of Engineers, 2002, Engineering and design—hydrographic surveying: U.S. Army Corps of Engineers Manual No. 1110–2–1003, 579 p.
- U.S. Fish and Wildlife Service, 2000, Biological opinion on the operation of the Missouri River main stem reservoir system, operation and maintenance of the Missouri River Bank Stabilization and Navigation Project, and operation of the Kansas River reservoir system: Minneapolis, Minn., U.S. Fish and Wildlife Service, 286 p.
- U.S. Fish and Wildlife Service, 2003, U.S. Fish and Wildlife Service 2003 amendment to the 2000 biological opinion on the operation of the Missouri River main stem reservoir system, operation and maintenance of the Missouri River Bank Stabilization and Navigation Project, and operation of the Kansas River reservoir system: Minneapolis, Minn., U.S. Fish and Wildlife Service, 298 p.

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