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UNITED STATES DEPARTMENT OF THE INTERIOR

**INVESTIGATIONS OF METHODS AND
EQUIPMENT USED IN STREAM GAGING**

**PART 1. PERFORMANCE OF CURRENT METERS
IN WATER OF SHALLOW DEPTH**

GEOLOGICAL SURVEY WATER-SUPPLY PAPER 868-A

UNITED STATES DEPARTMENT OF THE INTERIOR
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INVESTIGATIONS OF
METHODS AND EQUIPMENT USED
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PART I. PERFORMANCE OF CURRENT METERS
IN WATER OF SHALLOW DEPTH

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INVESTIGATIONS OF METHODS AND EQUIPMENT USED IN STREAM GAGING

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ABSTRACT

The investigation of the performance of current meters in measuring the velocity of water in shallow depths, recently made by the Geological Survey at the National Hydraulic Laboratory, National Bureau of Standards, Washington, D. C., was arranged primarily for the purpose of determining coefficients to be applied as correction factors to velocities obtained by current meters when used under the adverse conditions of very shallow water. The scope of the investigation covered measurements of discharge in the 12-foot flume with standard-size current meters and with cup-type pygmy current meters that have a bucket wheel 2 inches in diameter. The investigation was limited to water between a minimum depth of 0.2 foot and a maximum depth of 1.5 feet. The velocity ranged from 0.1 foot to 1.5 feet per second. Coefficients for the 0.6-depth method and for the 0.2- and 0.8-depth method, where standard-size current meters were used, were determined for the entire range of velocities. For the 0.5-depth method where standard-size current meters were used and for all methods where pygmy meters were used coefficients were determined for the entire range of velocities except for those below 0.2 foot per second. The depths of water in which measurements were made by the various methods are shown by the depths for which coefficients are given in the diagrams.

The depth of water in the flume was regulated by needle gates a short distance below the place of measurement, the needle gates being adjusted so as to obtain the desired depth and velocity for a given discharge. An 8-inch or a 4-inch venturi meter calibrated in place was used for determining discharges less than 3.5 second-feet. Discharges greater than 3.5 second-feet were measured by a sharp-crested weir calibrated in place. The discharge as measured by the current meter was compared with the weir or venturi-meter discharge measurement to obtain the correction factor for the current-meter measurement. Conditions of beds of smooth concrete, $\frac{3}{4}$ -inch gravel, and coarse gravel were investigated. The coarse gravel was run-of-bank gravel, all retained on a 1-inch screen but passing through a 5-inch screen.

Other phases of the investigation, such as studies of pulsations, vertical velocity curves, distribution of velocities near the side walls of the flume, and performance of current meters when used near the water surface and near the flume walls, were incidental to the main purpose of the investigation. The information obtained from these studies was used in analyzing and interpreting the results of the discharge measurements.

INTRODUCTION

Current meters used in measuring river discharge must be operated under a great variety of conditions. The depth of water at the place of measurement may be from 50 to 100 feet in large rivers and less than 1 foot in small streams. Some measurements are made where the velocity is 15 feet per second or more; others, where the velocity is as low as 0.1 foot per second. Not only are large differences in depth and velocity encountered in measuring rivers and streams in widely separated regions, but also large variations between flood stages and minimum low water in the same river. Measurements are made from cableways, bridges, and boats; by wading; and through ice cover. Structures from which current-meter measurements are made may be designed and built for that particular purpose and may be placed at a selected reach of the stream where, for ordinary stages, the conditions are favorable for accurate measurements of velocity with a current meter. It has often been found, however, that the conditions at low water are not so favorable for current-meter measurements as the conditions at the same place at stages of medium and high water. It may be desirable at low water to select a place where measurements can be made by wading at a section other than at the cableway or bridge used for the measurements made at higher stages.

In selecting a section for a wading measurement particular consideration is given to the distribution of velocities as indicated by the appearance of the water surface. Sections containing cross-currents, eddies, or pools of extremely low velocity are avoided if possible. At times of low water the most favorable conditions with respect to distribution of velocity are generally found at sections of comparatively shallow depth, possibly 1 foot or less. Depths greater than 1 foot but not exceeding about 2.5 feet for wading measurements are desirable for the use of the 0.2- and 0.8-depth method, if the velocities at those depths are not too low. Velocities as low as 0.1 foot per second can be measured with the current meter, but the precision of the measurements is generally better for velocities of 0.5 foot per second and higher than it is for extremely low velocities.

In wading measurements the current meter is usually held in position by means of a rod and a double-ended sliding support that can be readily adjusted to the desired position for the depth of observation. (See pl. 1.) Measurements of small canals and ditches and of low-water flow in small streams where the width is not too great may be made from a small footbridge or similar structure with the same equipment that is used in wading measurements. This procedure eliminates the necessity of standing in the water in the near vicinity of the meter. In current-meter measurements in shallow depths care should be used in placing the meter at the selected position

in the vertical, as small errors in placement of the meter may have considerable effect on the accuracy of the results.

Current meters are ordinarily rated in a still-water rating flume by drawing the meter through the water at a known rate of travel and holding it far enough below the surface of the water and from the side walls of the flume so that the performance of the meter is not affected by proximity to either the water surface or the side walls. Under those conditions, the static water pressure being about the same on all sides of the current meter, the rating that is obtained may be assumed to correspond to a condition where the meter is held in a fixed position in water moving steadily in straight lines of uniform velocity on all sides.

It has been found by experiments that when a vertical-axis cup-type current meter is used in water of very shallow depth in positions where the current-meter cups are in the vicinity of the water surface the action of the meter is retarded, and it is said to "underregister." Likewise, when the meter is placed very close to the stream bed its action may be affected by its proximity to the bed and by irregularities in the boundary surface.

For flow in open channels it has been demonstrated by many experiments that the distribution of velocities in a vertical section may be represented by a segment of a parabolic curve, in which the axis of the parabola is parallel to the water surface. With such a distribution the average of the velocities at 0.2- and 0.8-depth is very nearly the mean velocity in the vertical. It has been found also, that the velocity at 0.6-depth is likewise very near the average velocity for the vertical. Discharge measurements that are made by observations of velocities at 0.2- and 0.8-depth below the water surface are said to be made by the "0.2- and 0.8-depth" method; those made by observations at 0.6-depth below the water surface are said to be made by the "0.6-depth" method. Most discharge measurements are made by one or the other of these methods if the depths and velocities are such as to permit measurements of velocities at those points. The 0.2- and 0.8-depth method is generally preferred because of its greater reliability. Unusual or abnormal conditions in the movements of the water have a less serious effect on the accuracy of the measurement if the observations are taken at the two points at 0.2- and 0.8-depth instead of only at 0.6-depth.

In very shallow water it is not always practicable to use the current meter at the 0.2- and 0.8-depth positions because of the proximity of those positions to the water surface or the stream bed, and under those circumstances the 0.6-depth method may be preferable. In streams where the water may be so shallow that the 0.6-depth method is not practicable, the current meter must be placed at middepth,

where the velocity is ordinarily somewhat greater than the average in the vertical.

Coefficients other than unity may be necessary for current-meter measurements of velocity in very shallow depths for two reasons. First, the distribution of velocity in a vertical may be such that the actual velocity at the point of observation—for instance, at 0.5 depth—is not the mean for the vertical; second, the registration of the current meter may be affected by its proximity to the water surface or the stream bed. Under some conditions the errors from these two sources may have opposite effects and therefore be compensating; under other conditions both errors may be in the same direction and therefore not of a compensating nature.

Because of the lack of definite information in regard to the performance of current meters under the adverse conditions of very shallow water and in order to obtain information as to coefficients that should be applied to measurements made under those conditions, arrangements were made to investigate this problem in the National Hydraulic Laboratory, National Bureau of Standards, Washington, D. C.

ADMINISTRATION AND PERSONNEL

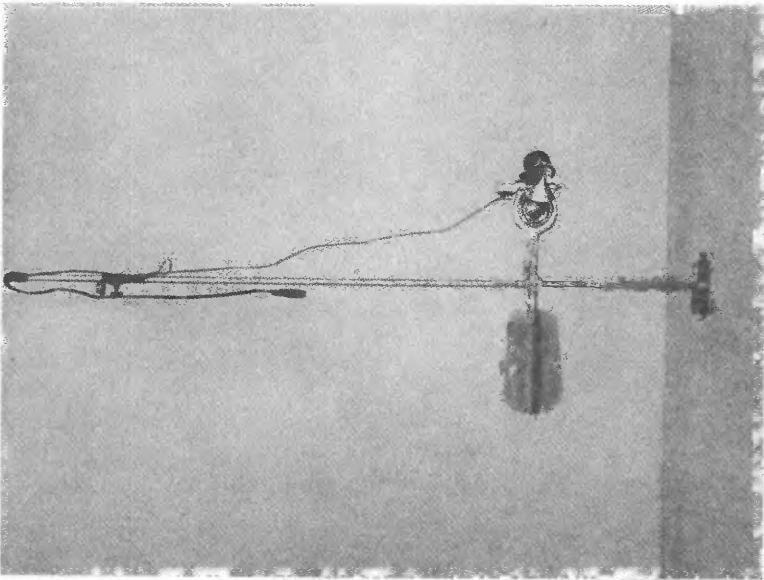
The methods and procedure used in conducting the investigation were arranged by the hydraulic laboratory committee of the water-resources branch of the Geological Survey, consisting of Lasley Lee, C. H. Pierce, and O. W. Hartwell under the administrative direction of N. C. Grover, chief hydraulic engineer, and C. G. Paulsen, chief of the division of surface water. The instrumental observations and current-meter measurements were made by W. S. Eisenlohr, Jr., A. H. Frazier, H. E. Cox, and A. D. Ash. The analyses of data and the computations of coefficients were made by C. H. Pierce, assisted by Mr. Eisenlohr, Mr. Ash, and H. C. Woster.

COOPERATION AND ACKNOWLEDGMENTS

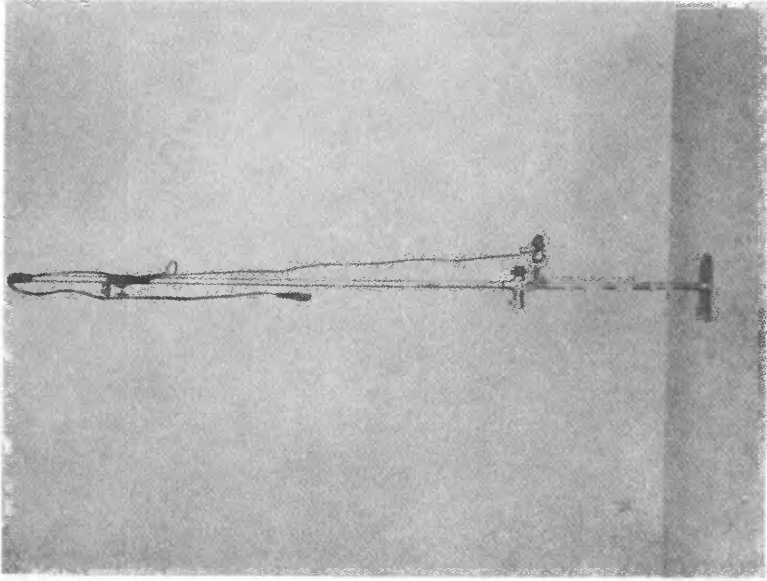
The facilities of the National Hydraulic Laboratory were provided by the National Bureau of Standards. The laboratory staff, under the direction of H. N. Eaton, assisted in the conduct of the work and furnished the pitot static tube that was used for comparisons with the current-meter observations. The various current meters used in the investigation were rated several times during the progress of the work by the section of engineering instruments and mechanical appliances, division of mechanics and sound, National Bureau of Standards. The Ott Xb-type meter was loaned by that section.

PURPOSE AND SCOPE OF THE INVESTIGATION

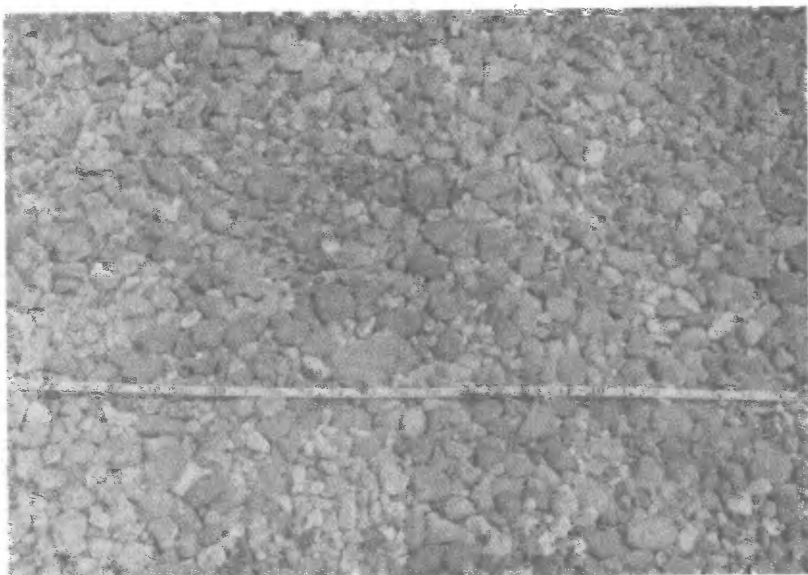
The investigation was made primarily for the purpose of determining coefficients for use as correction factors to velocities obtained in



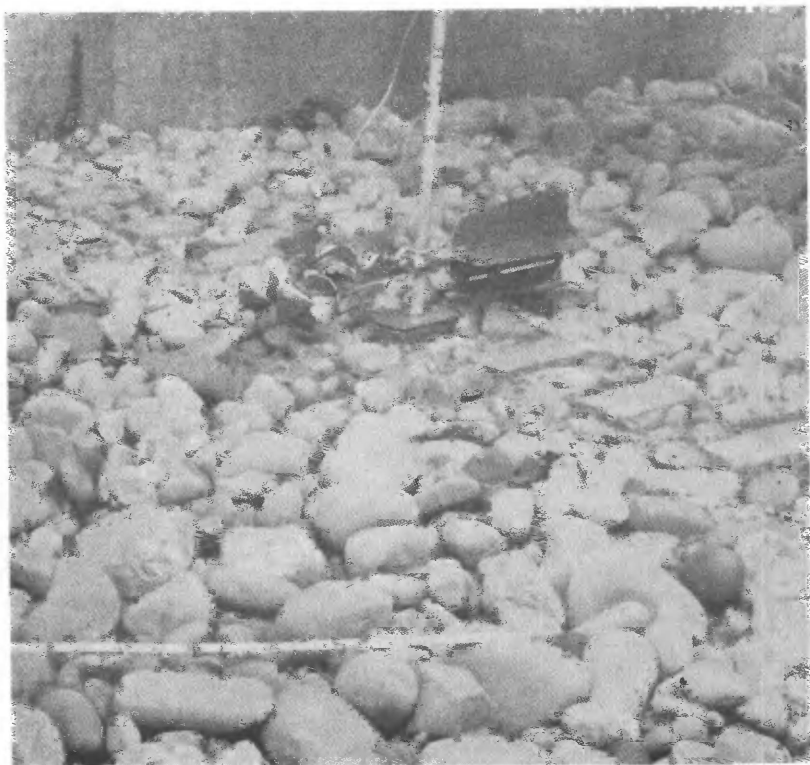
4. TYPE A SMALL PRICE CURRENT METER.



B. CUP-TYPE PYGMY CURRENT METER.



A. FLUME WITH BED OF $\frac{3}{4}$ -INCH GRAVEL.



B. FLUME WITH BED OF COARSE GRAVEL.



A. SECTION OF 12-FOOT FLUME AND PLATFORM FROM WHICH MEASUREMENTS WERE MADE.



B. PART OF THE MEASUREMENT SECTION AND GAGE.

measurements of streams of shallow depths by small Price current meters such as are generally used by engineers of the Geological Survey. A Geological Survey type A small Price current meter, which was used in this investigation, is shown in plate 1, *A*. Descriptions of the individual current meters are given on pages 15-18.

Measurements of discharge in the flume were made under the different conditions of smooth bed, bed of $\frac{3}{4}$ -inch gravel, and bed of coarse gravel in order to obtain information regarding the extent to which the performance of the current meter was affected by the roughness of the bed.

The pygmy current meters, designed and constructed by the Geological Survey, were in an experimental stage and therefore not in general use in stream-gaging work at the time of this investigation, but they were included for the purpose of obtaining information respecting their performance in measurements of velocity in shallow depths and for determining coefficients that would be applicable to measurements made with such meters. One of the pygmy meters used in this investigation is shown in plate 1, *B*. These meters are described on page 18.

Other phases of the investigation, such as studies of pulsations, vertical velocity curves, distribution of velocities near the side walls of the flume, and performance of the current meters when used near the water surface and near the flume walls, were incidental to the main purpose of the investigation. The information obtained from these studies was used in analyzing and interpreting the results of the discharge measurements.

The scope of the investigation covered measurements of discharge in the 12-foot flume with standard-size current meters and with cup-type pygmy current meters that have a bucket wheel 2 inches in diameter for the determination of coefficients applicable to the current-meter measurements by comparison with the discharge as measured by other methods. The depth of water for this investigation was limited to a minimum of 0.2 foot and a maximum of 1.5 feet. The velocity was limited to a range of 0.1 foot to 1.5 feet per second. Coefficients for the 0.6-depth method and for the 0.2- and 0.8-depth method, were determined for the entire range of velocities, using standard-size small Price-type current meters. For the 0.5-depth method where standard-size current meters were used and for all methods where pygmy meters were used coefficients were determined for the entire range of velocities except for those below 0.2 foot per second. The depths of water in which measurements were made by the various methods are shown by the depths for which coefficients are given in the diagrams (pls. 7-13).

METHODS OF MEASUREMENT

The depth of water in the flume was regulated by needle gates a short distance below the place of measurement, the needle gates being adjusted so as to obtain the desired depth and velocity for a given discharge. An 8-inch or a 4-inch venturi meter calibrated in place was used for determining discharges less than 3.5 second-feet. Discharges greater than 3.5 second-feet were measured by a sharp-crested weir calibrated in place. The discharge as measured by the current meter was compared with the weir or venturi-meter discharge measurement to obtain the correction factor for the current-meter measurement. The concrete bed of the flume was assumed to represent a smooth stream bed, and measurements made on it were used in determining the relations for a smooth bed. A layer of $\frac{3}{4}$ -inch gravel was then placed in the flume, as shown in plate 2, *A*, and a series of measurements was made for this condition. The third condition investigated was that of coarse gravel, as shown in plate 2, *B*. The coarse gravel was run-of-bank gravel, all retained on a 1-inch screen but passing a 5-inch screen.

Measurements were made with several different type A small Price current meters, such as are shown in plate 1, *A*. One current meter of the so-called combination type or 623 type, formerly in general use by the Geological Survey (see description on p. 18), was used in the investigation. Measurements were made also with cup-type pygmy meters having bucket wheels 2 inches in diameter (see pl. 1, *B*). The meters were held on a $\frac{1}{2}$ -inch round rod with a base plate by means of a double-ended sliding support inserted between the yoke and the tailpiece and were operated from a platform above the water. The section of the flume and the platform from which the measurements were made are shown in plate 3, *A*. An Ott Xb-type meter, a pitot static tube, and propeller-type pygmy meter were used for purposes of comparison. Observations were also made with a Bentzel velocity tube. Observations of velocity were made at 0.6-depth, at 0.2- and 0.8-depth, at middepth, and at other points in the vertical at intervals of 0.5 foot across the flume and at 0.25 foot from the flume walls. In special studies in which the pitot static tube and the pygmy meters were used, determinations of velocity were made as close as 0.1 foot to the flume wall.

The depths of water at the points where observations of velocity were made were taken from a standard cross section that was developed from numerous soundings and by use of an engineer's level, the standard cross section being referred to a gage at the measurement section (see pl. 3, *B*). Except as noted elsewhere for the condition of 0.2-foot depth of water on the concrete bed, the current meters were placed in the vertical to the nearest 0.01 foot of the correct position. The period of time for each observation of velocity by the current

meter was generally between 50 and 90 seconds, with most of the periods longer than 60 seconds. In studies of the effects of pulsations, the observations extended over periods of several minutes in order to obtain information regarding the effects of pulsations on the velocity measurements. The results of the individual current-meter measurements of discharge are listed in the tables, pages 20-32.

POSITION OF THE CURRENT METER

The current-meter measurements of velocity were made with the current meter supported on a rod at definite, predetermined distances below the surface of the water. The positions of the current meter were those ordinarily used in the 0.5-depth, the 0.6-depth, and the 0.2- and 0.8-depth methods of measurement. The minimum depth of water for which the 0.2- and 0.8-depth method was investigated was 1.0 foot. The minimum depth of water for the investigation of the 0.6-depth method was 0.4 foot for standard-size current meters and 0.2 foot for pygmy meters. For the 0.5-depth method the minimum depth of water was 0.2 foot. Other positions of the current meter for measurements of velocity in shallow depths, such as the 0.4-depth method and a modified form of the 0.2- and 0.8-depth method, were investigated to some extent but were found to give results no better than those obtained by the use of the more generally accepted methods.

The current-meter cups could not be placed exactly at middepth when the depth of water was 0.2 foot on the concrete bottom of the flume. For this condition, when the bottom of the meter yoke rested on the concrete the center of the meter cups was at a depth of 0.07 foot below the water surface, and the top edges of the cups were out of the water. With the current meter in this position the indicated mean velocities were too large for velocities less than 0.7 foot per second (coefficients less than unity) and too small for mean velocities greater than 0.7 foot per second (coefficients greater than unity), the range of coefficients for this condition being from 0.882 at a velocity of 0.2 foot per second to 1.49 at a velocity of 1.5 feet per second. (See pl. 7.)

With 0.2-foot depth of water on the bed of $\frac{3}{4}$ -inch gravel, the current-meter cups were placed at middepth by forcing the bottom of the meter yoke into the gravel until the tops of the meter cups were just submerged (practically at the water surface). This position of the current meter permitted observations to be made at middepth, the diameter of the meter cups being slightly less than the depth of water. The indicated mean velocities of measurements made under this condition were too small, and therefore the coefficients to be applied to the indicated velocities were greater than unity, increasing from

1.01 at a velocity of 0.2 foot per second to 1.52 at a velocity of 1.4 feet per second. (See pl. 8.)

In water 0.3 foot deep the center of the current-meter cups was placed at middepth, and in this position the bottoms of the meter cups were about 0.07 foot above the bed, and the tops of the cups were the same distance below the surface of the water. The coefficients for individual current-meter measurements made in the 0.3-foot

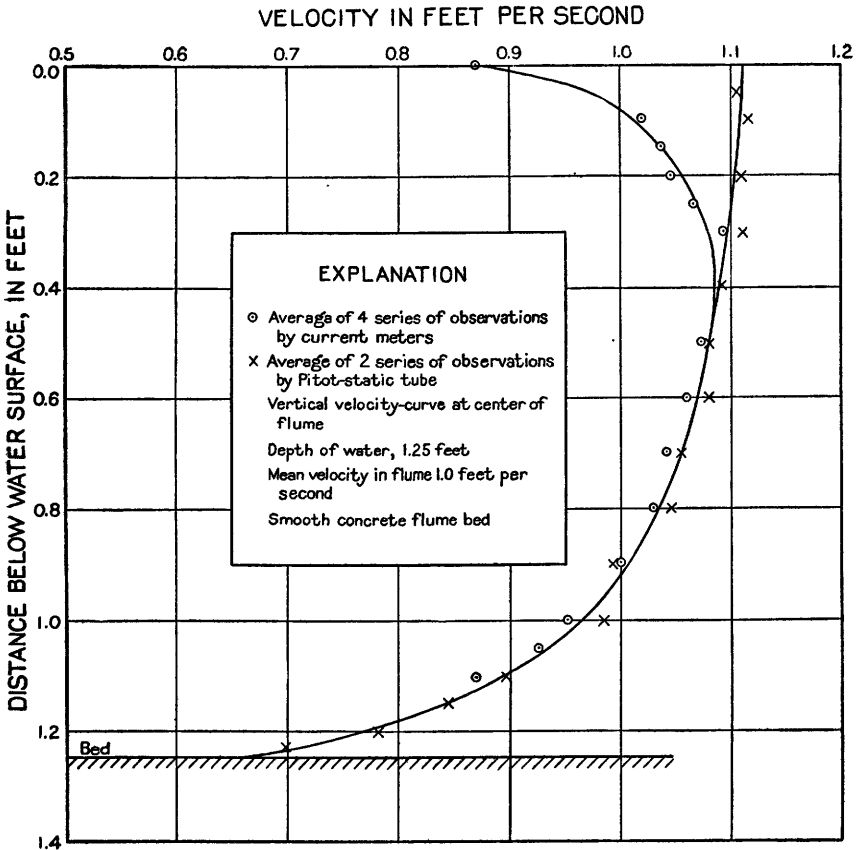


FIGURE 1.—Underregistration of current meters near the water surface.

depth of water showed large variations, not only for the different conditions of beds of smooth concrete and $\frac{3}{4}$ -inch gravel but also for the same conditions of bed. No current-meter measurements were made in water 0.2 or 0.3 foot in depth on the coarse-gravel bed.

The cup-type current meter has a tendency to underregister if the position of the meter is such that the tops of the cups are at or just below the surface of the water. Under those circumstances a vertical velocity curve plotted from current-meter observations of velocity may indicate velocities at the upper end of the curve that are less than the true velocities. This condition is shown in figure 1, where a vertical

velocity curve plotted from the average of four series of observations made with current meters is shown in comparison with the average of two series of observations made with the pitot static tube. In some measurements of velocity errors due to underregistration of the current meter near the water surface may be compensated by the effects of other conditions; but the effect of proximity of the current meter to the surface in water of shallow depth is one of the principal reasons for the use of coefficients to measurements made under such conditions.

DISTRIBUTION OF VELOCITIES IN THE 12-FOOT FLUME

The horizontal distribution of velocity was fairly uniform across the 12-foot flume between distances of about 1 foot from each wall, except for the ordinary variations in velocity between the water surface and the flume bed. Within distances of about 1 foot from each wall the velocity decreased with increasing nearness to the wall until at a distance of 0.1 foot from the wall it was from 40 to 80 percent of the average velocity in the flume. This relation varied somewhat with the depth of water and the discharge in the flume.

The distribution of velocities in the flume was studied by means of measurements made with the pitot static tube and the current meters. Figure 2 shows the horizontal distribution of velocities for half the width of the flume, based on observations at 0.2 depth and 0.8 depth. The averages of the velocities at equal distances from the side walls on the two sides of the flume were used in preparing this diagram. Although distributed fairly uniformly across the flume between distances of about 1 foot from each wall, the velocities were slightly greater on the left side than on the right. This irregularity of flow, which was caused by conditions upstream from the measuring section, was smoothed out to some extent by taking averages of the velocities at equal distances from the two side walls, although the plotted points still show some minor variations. The diagram in figure 2 corresponds to a depth of water of 1.25 feet and a discharge of 7.51 second-feet flowing on a bed of $\frac{3}{4}$ -inch gravel, the average velocity in the flume being 0.50 foot per second, and is typical of the conditions that existed for other depths and velocities. The results of observations made with the pitot static tube and those made with a current meter are indicated by different symbols.

VERTICAL VELOCITY CURVES

The vertical velocity curves in plate 4 show the distribution of velocity in vertical sections near the side walls and at the center of the flume. The velocity measurements were made with the pitot static tube under the same conditions as those described above. The individual curves for sections on the two sides of the flume shown in plate 4 indicate slightly higher velocities for the sections on the left side of the flume than for those on the right.

EFFECTS OF NONUNIFORM VELOCITIES NEAR THE FLUME WALLS

In this investigation it was found that vertical-axis cup-type current meters, if used near the sides of the flume where the velocity was not uniformly distributed across the width of section in front of

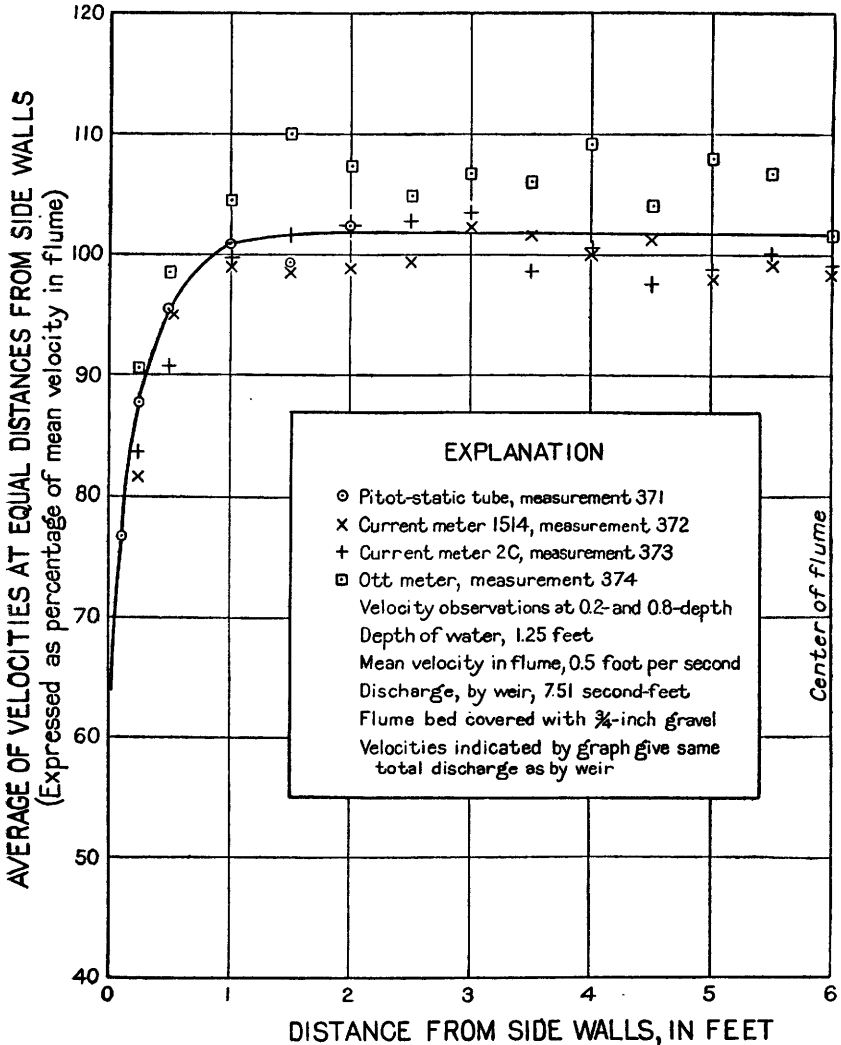


FIGURE 2.—Horizontal distribution of velocities.

the meter (see fig. 2), generally indicated velocities in excess of the true velocities on that side of the flume where the position of the resultant of the nonuniform velocities acting on the meter fell on the side of the axis toward which the cups were turning. Similarly, the current meter indicated velocities less than the true velocities on that side of the flume where the resultant of the nonuniform velocities

acting on the meter fell on the side of the axis from which the cups were turning.

A study of the effects of underregistration and overregistration of the current meter when used near the flume walls, where the velocity distribution was not uniform, was made by comparisons of measurements made with various instruments. These comparisons showed that, for the conditions of depth and velocity at which the measurements were made, the underregistration near one wall of the flume and the overregistration near the opposite wall were so nearly equal that no consistent effects either of uncompensated underregistration or of uncompensated overregistration could be detected. It is recognized that if the rate of change in the velocity distribution had been materially greater on one side of the flume than on the other, there might have been some uncompensated underregistration or overregistration.

In studying the possibility of any uncompensated underregistration or overregistration of the current meter because of the nonuniform velocities in the vicinity of the flume walls, the writer compared the discharge as measured by the current meter in the two 2-foot widths adjacent to the walls and the discharge as measured by the current meter in the 8-foot width between the two 2-foot widths, and determined the ratios of the discharge in each of these parts to the total discharge in the flume as measured by the current meter. These ratios, which are shown in the tables on page 12, are in close agreement with ratios obtained in a similar manner from measurements made by other types of current meters and by the pitot static tube under similar conditions of depth and velocity.

In the comparisons of measurements of discharge in the two 2-foot widths and of the discharge in the central 8-foot width, several different current meters were used. One of these, meter 2C, was arranged with the bucket wheel in reverse position, so that the cups revolved in a clockwise direction when viewed from above instead of in the usual counterclockwise direction. The change in position of the bucket wheel caused no change in the rating of the meter, but its performance with respect to underregistration and overregistration near the side walls was the opposite from that of the cup-type meters using bucket wheels that rotated in the counterclockwise direction. The preceding statement in regard to the effect of nonuniform distribution of velocities and the position of the resultant of the nonuniform velocities acting on the current-meter cups with respect to the axis of the meter is applicable irrespective of the direction of rotation of the meter cups. A pygmy current meter 2 inches in diameter, an Ott propeller-type meter, and a pitot static tube were also used in these comparisons. The comparisons showed no consistent differences in the percentage of discharge in the two 2-foot

widths and in the central 8-foot width that could be attributed to uncompensated underregistration or overregistration of the current meters in the vicinity of the flume walls.

Comparison of measurements by different current meters and the pitot static tube

Flume bed covered with ¼-inch gravel

Measurement No.	Meter ¹	Depth of water (feet)	Method of measurement (fractional depth below surface)	Average velocity in flume (feet per second)	Total discharge (second-feet)		Coefficient for meter measurement	Meter discharge (second-feet)		Percentage of total meter discharge	
					Weir	Meter		In two 2-foot widths	In one 8-foot width	In two 2-foot widths	In one 8-foot width
132	1175	1.25	0.6	0.50	7.53	7.38	1.020	2.44	4.94	33.06	66.94
371	Pitot	1.25	0.6	.50	7.51	7.32	1.026	2.42	4.90	33.06	66.94
375	1514	1.25	0.6	.50	7.51	7.54	.996	2.45	5.09	32.49	67.51
376	2C	1.25	0.6	.50	7.51	7.56	.993	2.49	5.07	32.94	67.06
377	Ott X b	1.25	0.6	.50	7.51	7.81	.962	2.60	5.21	33.29	66.71
378	A3	1.25	0.6	.50	7.51	7.64	.983	2.54	5.10	33.25	66.75
132	1175	1.25	0.2 and 0.8	.50	7.53	7.31	1.030	2.32	4.99	31.74	68.26
372	1514	1.25	0.2 and 0.8	.50	7.51	7.34	1.023	2.34	5.00	31.88	68.12
373	2C	1.25	0.2 and 0.8	.50	7.51	7.39	1.016	2.37	5.02	32.07	67.93
374	Ott X b	1.25	0.2 and 0.8	.50	7.51	7.85	.957	2.54	5.31	32.36	67.64
378	A3	1.25	0.2 and 0.8	.50	7.51	7.50	1.001	2.42	5.08	32.27	67.73
77	1196	.50	0.6	1.00	6.03	5.86	1.029	1.92	3.94	32.76	67.24
363	1514	.50	0.6	1.00	6.01	5.67	1.060	1.83	3.84	32.28	67.72
364	Ott X b	.504	0.6	.995	6.01	6.08	.988	1.95	4.13	32.07	67.93
365	2C	.50	0.6	1.00	6.01	5.72	1.051	1.86	3.86	32.52	67.48
370	Pitot	.50	0.6	1.00	6.01	6.08	.988	1.98	4.10	32.57	67.43
379	A3	.50	0.6	1.00	6.01	6.01	1.000	1.96	4.05	32.61	67.39

Smooth concrete flume bed

308	1514	1.25	0.6	1.01	15.13	15.13	1.000	4.99	10.14	32.98	67.02
309	2C	1.25	0.6	1.01	15.13	15.56	.972	5.14	10.42	33.03	66.97
314	1175	1.25	0.6	1.00	15.06	14.97	1.006	4.92	10.05	32.87	67.13
315	2C	1.25	0.6	1.00	15.06	15.25	.988	4.92	10.33	32.26	67.74
330	1514	1.25	0.6	1.00	15.00	15.01	.999	4.90	10.11	32.64	67.36
331	2C	1.25	0.6	1.00	15.00	15.33	.978	4.94	10.39	32.22	67.78
332	Ott X b	1.25	0.6	1.00	15.00	15.34	.978	4.92	10.42	32.07	67.93
333	A3	1.25	0.6	1.00	15.00	15.03	.998	4.91	10.12	32.67	67.33
354	Pitot	1.25	0.6	1.00	14.98	15.25	.982	4.94	10.31	32.39	67.61
308a	1514	1.25	0.2 and 0.8	1.01	15.13	14.95	1.012	4.84	10.11	32.37	67.63
309a	2C	1.25	0.2 and 0.8	1.01	15.13	15.11	1.001	4.83	10.28	31.96	68.04
334	1514	1.25	0.2 and 0.8	1.00	15.02	14.78	1.016	4.78	10.00	32.34	67.66
335	2C	1.25	0.2 and 0.8	1.00	15.02	15.01	1.001	4.84	10.17	32.24	67.76
336	Ott X b	1.25	0.2 and 0.8	1.00	15.02	15.19	.989	4.90	10.29	32.26	67.74
337	A3	1.25	0.2 and 0.8	1.00	15.02	14.81	1.014	4.74	10.07	32.01	67.99

¹ See descriptions of current meters, pp. 15-18.

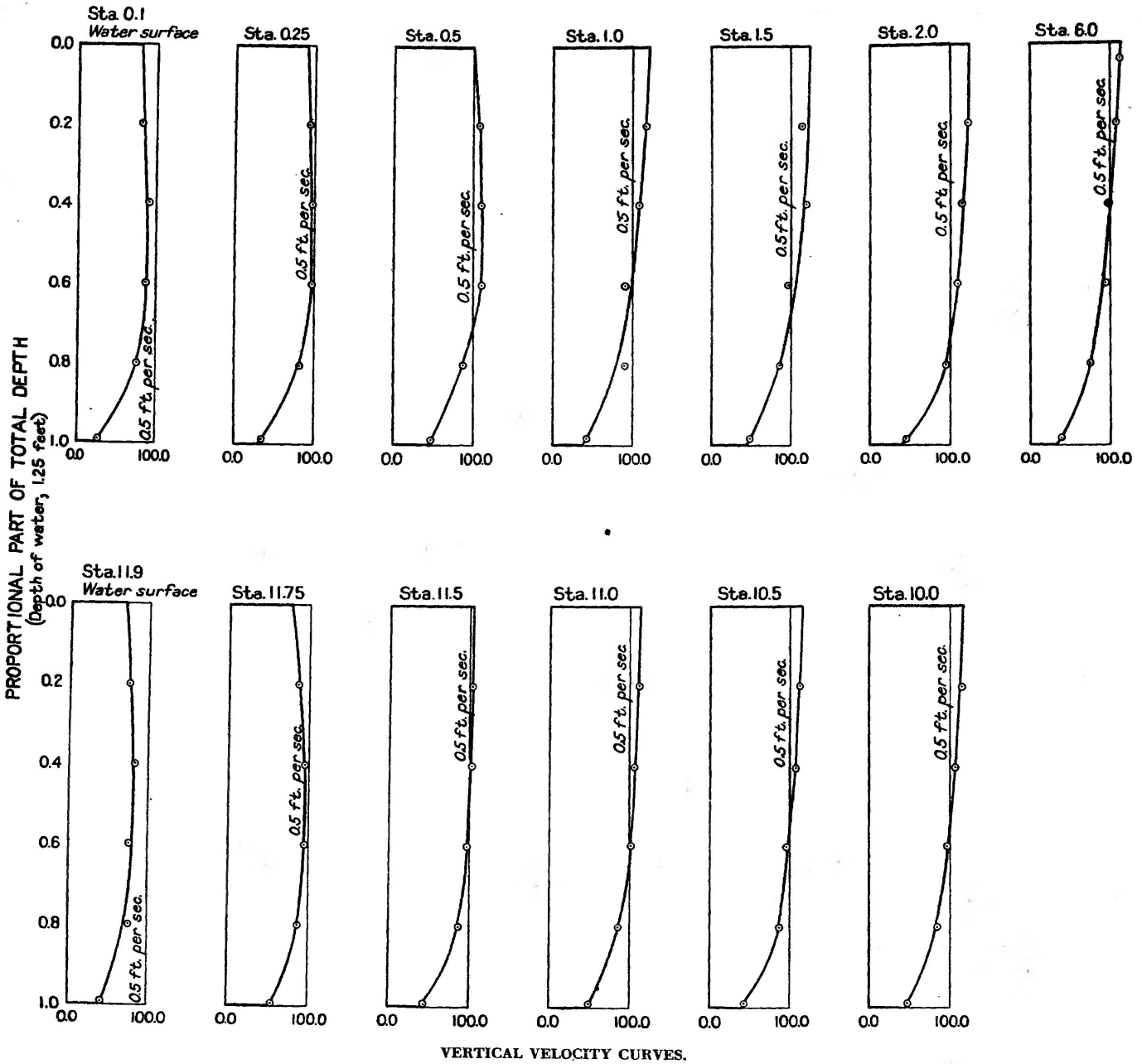
NOTE.—Comparison of measurements for other conditions of depths and velocity showed equally consistent relations of the discharge in the two 2-foot widths at the sides of the flume and the discharge in the 8-foot width as measured by different types of current meters, although the proportional parts of the total discharge carried by the two 2-foot widths and the 8-foot width were somewhat different for the other conditions of depth and velocity.

PULSATIONS

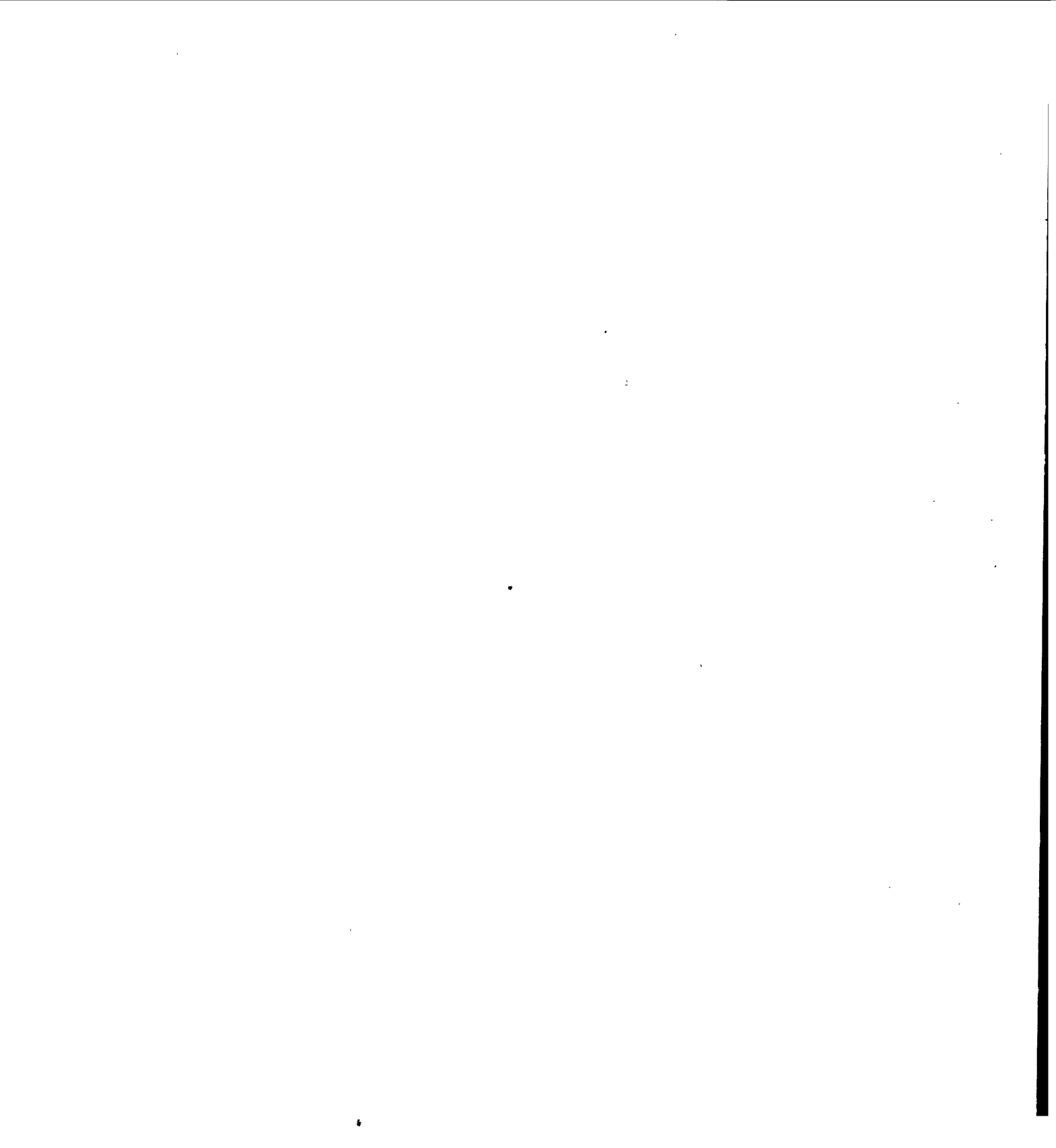
Because of the possibility that the results obtained in the investigations of current-meter performance might have been affected by pulsations, studies were made to obtain information for comparisons of pulsations in different velocities and the effects of pulsations on the observations of velocities for different periods.

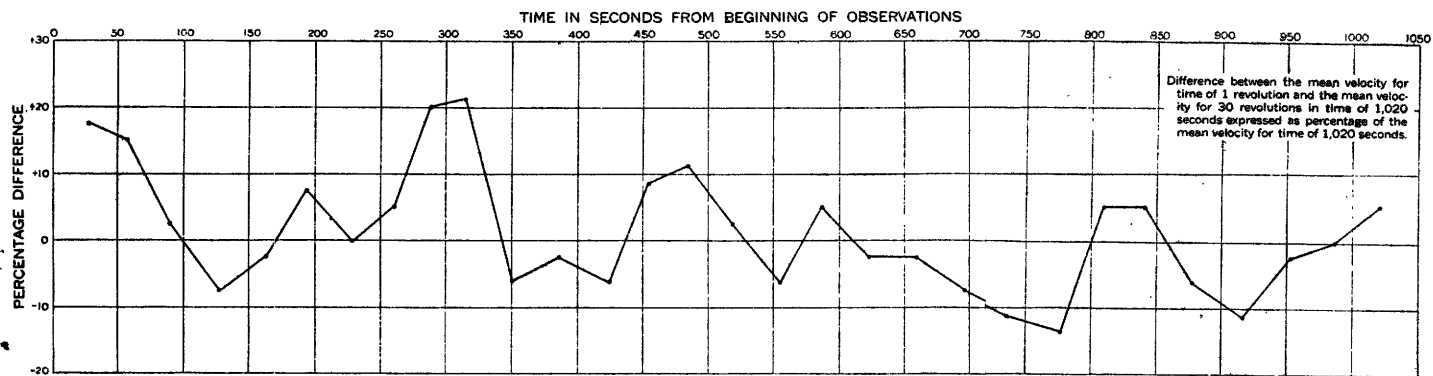
Fluctuations in the velocity of flowing water have been observed and commented on by many writers ¹ and may readily be perceived

¹ See U. S. Geol. Survey Water-Supply Paper 95, pp. 28-32, 1904.

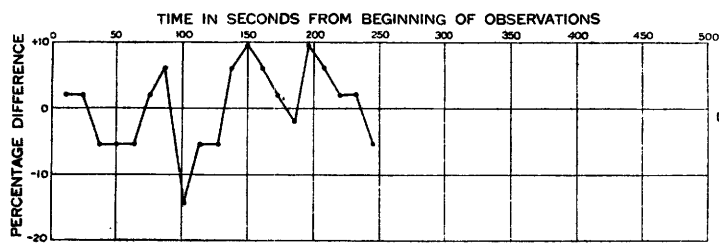


VERTICAL VELOCITY CURVES.

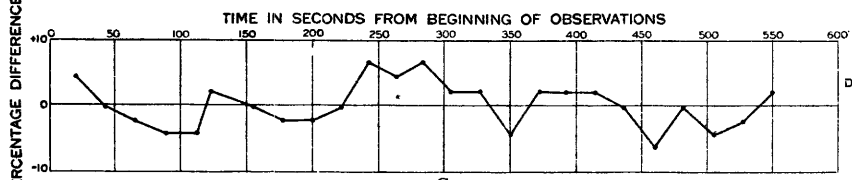




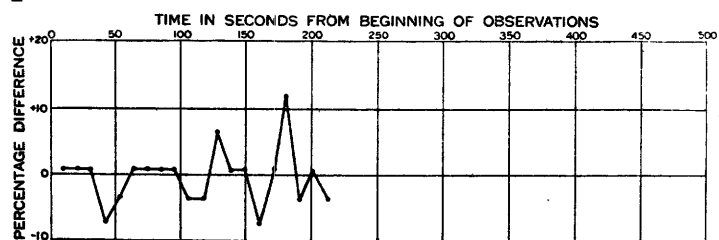
A



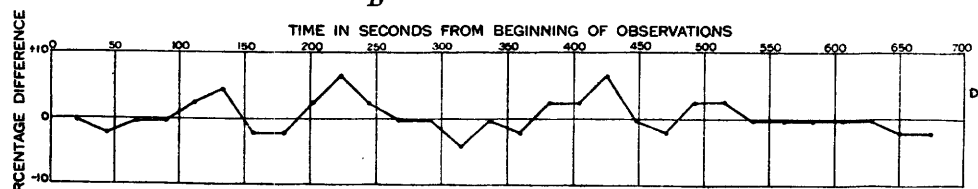
B



C

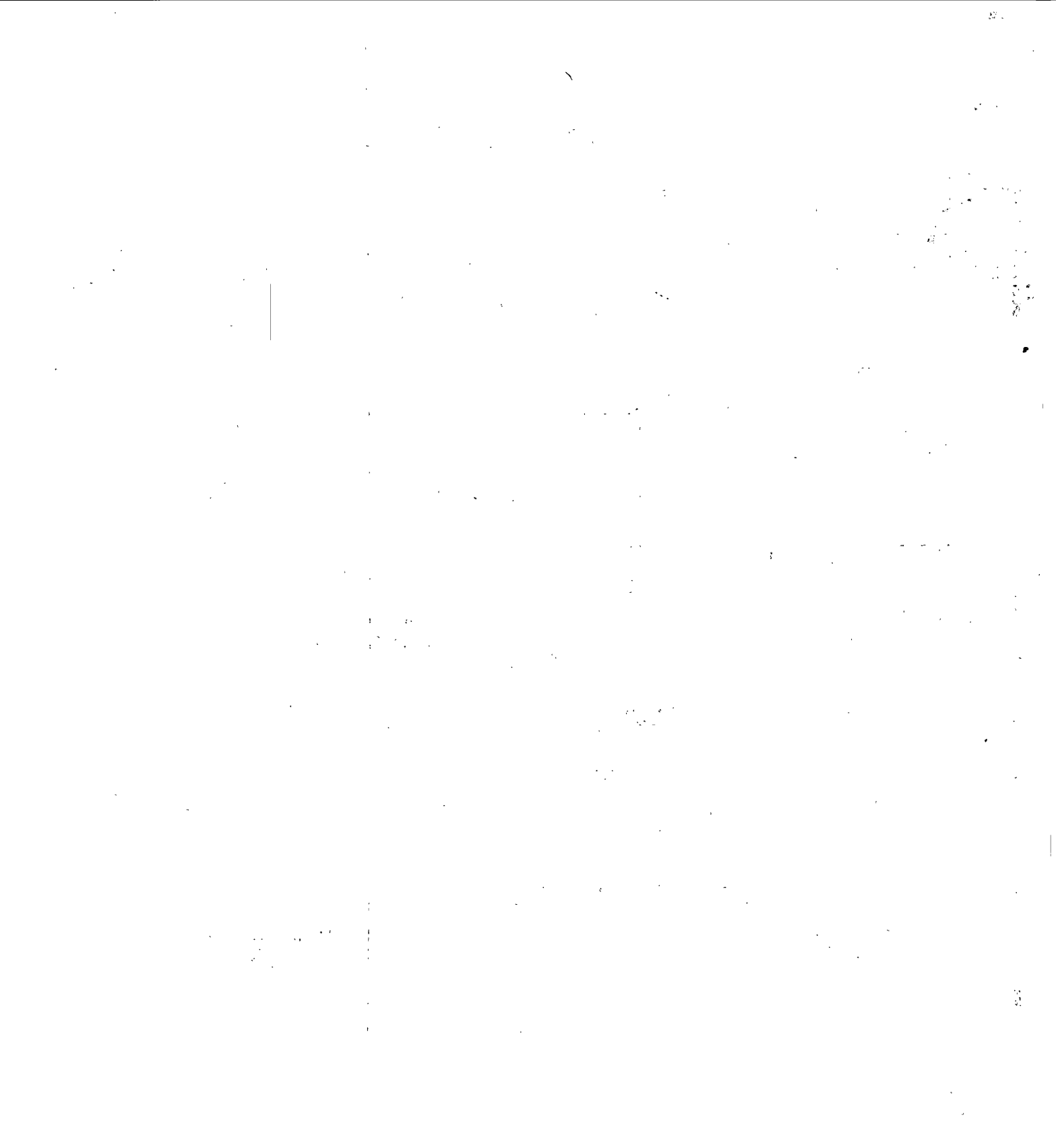


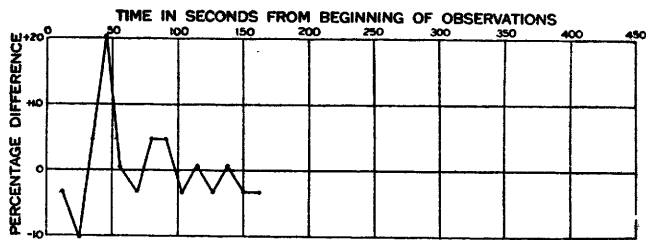
D



E

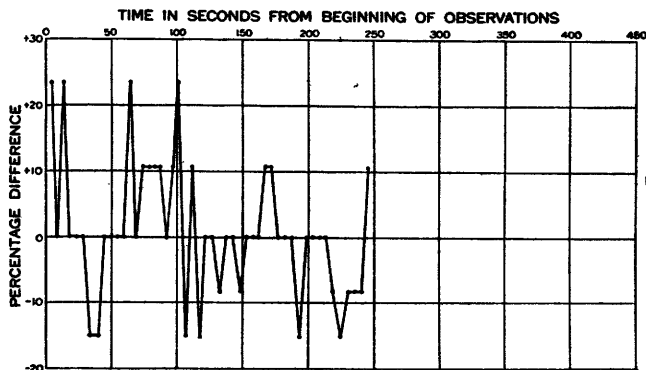
COMPARISONS OF PULSATIONS IN DIFFERENT VELOCITIES.





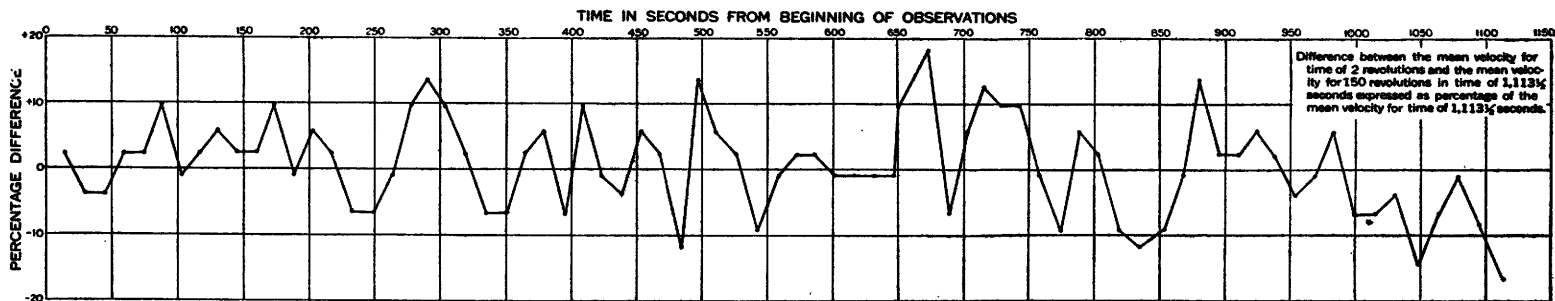
Difference between the mean velocity for time of 1 revolution and the mean velocity for 14 revolutions in time of 162 seconds expressed as percentage of the mean velocity for time of 162 seconds.

F



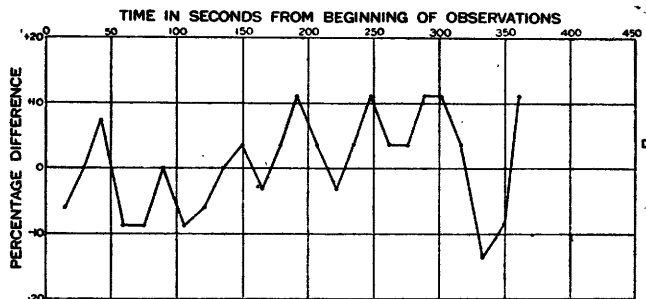
Difference between the mean velocity for time of 1 revolution and the mean velocity for 49 revolutions in time of 245½ seconds expressed as percentage of the mean velocity for time of 245½ seconds.

G



Difference between the mean velocity for time of 2 revolutions and the mean velocity for 150 revolutions in time of 1,113½ seconds expressed as percentage of the mean velocity for time of 1,113½ seconds.

H



Difference between the mean velocity for time of 2 revolutions and the mean velocity for 50 revolutions in time of 362 seconds expressed as percentage of the mean velocity for time of 362 seconds.

I

COMPARISONS OF PULSATIONS IN DIFFERENT PERIODS OF OBSERVATION.

Dear Mr. Smith,

I have received your letter of the 15th and am sorry to hear that you are having trouble with the machine. I will send you a replacement part as soon as possible.

Please let me know if you need any further assistance. I will be happy to help.

Sincerely,
John Doe

by noting the intervals of time between the revolutions of a current meter held in a fixed position in a stream. In addition to the short-period fluctuations, or pulsations that are characteristic of turbulent flow, such as is generally found in natural streams and in artificial channels where the depths and velocities are comparable with those of natural streams, water flowing in open channels nearly always exhibits longer-period variations in velocity, which are commonly called surges. Pulsations and surges in the flow of water in the 12-foot flume in the laboratory were indicative of turbulent flow. The results obtained by the investigation may be considered better applicable, therefore, to measurements of natural streams than they could have been if laminar flow had prevailed.

The condition of flow generally referred to as "turbulent," as differentiated from either "laminar" or "shooting" flow, is characterized by erratic movements of the individual particles of water wherein any particle may move in any direction with respect to any other particle.

As stated by O'Brien and Hickox:²

In most of the problems in the motion of air and water the flow is turbulent in the sense that there occur erratic variations in direction and velocity. Color injected into a turbulent flow quickly diffuses through it, and the motion of light particles is highly irregular. Direct measurements of these fluctuations show that they do not exhibit a regular period.

Owing, apparently, to the predominance of the dynamic characteristics over the viscous characteristics of turbulent flow as generally found in open channels, the velocity of water at a given point in an open channel is continually changing with respect to its component of motion in the direction of flow. For fully developed turbulence, the loss of head caused by frictional resistance is about proportional to the second power of the velocity, and the potential gradient will be governed by the mass acceleration.

The range in depth, velocity, and viscosity of the water in which current-meter measurements of natural streams are ordinarily made is generally within those limits in which the flow is turbulent in distinction from laminar or shooting flow, although the condition of turbulence may not always be complete, as judged by the usual criteria.³ According to Vogel,⁴

"Expressed mathematically, the criterion for turbulent flow is $Z = VD > 0.02$, D being expressed in feet and V in feet per second." As stated by Thompson,⁵ "Turbulence is a comparative phenomenon—that is, there are degrees of turbulence. Thus, in a regimen in which the value of Z is, say, 0.06, the effects of the

² O'Brien, M. P., and Hickox, G. H., Applied fluid mechanics, p. 25, 1937.

³ See Vogel, H. D., Practical river laboratory hydraulics, and discussions by P. W. Thompson and others: Am. Soc. Civil Eng. Trans., vol. 100, 1935.

⁴ Idem, p. 136.

⁵ Idem, p. 151.

forces of internal friction will be more nearly negligible than in a regimen in which Z has a value less than 0.06."

At the National Hydraulic Laboratory the flow in the 12-foot flume used in investigating current-meter performance in measurements of the velocity of water in shallow depths was turbulent, as shown by the Reynolds' number criterion V_r , which gave values between 2,000 and 186,000, and the criterion Z used by Vogel, which had a value of 0.04 or more. At the lower velocities and minimum depths at which measurements were made the Reynolds' number was between 2,000 and 3,000, and thus it is possible that the condition of turbulence may not have been fully developed. Special precautions were taken, by the use of surface floats, to eliminate surface disturbance and wave action.

The variations in the velocity of the water at a given point in the cross section of the 12-foot flume at the National Hydraulic Laboratory were investigated by observing the times of individual revolutions of the current meter, or the times of every 2, 5, or 10 revolutions, during periods of several minutes—sometimes the periods of observation were as long as 17 or 18 minutes. For each set of observations in the study of the effects of pulsations, the mean velocity indicated by the current meter during successive short periods of observation corresponding to 1, 2, 5, or 10 revolutions of the meter was compared with the average velocity corresponding to the total number of revolutions of the meter during the entire period, and the amount by which the mean velocity for each short period differed from the average for the entire period was expressed in percentage of the mean velocity for the entire period and plotted as the ordinate of a point, the abscissa of which was the total elapsed time from the beginning of the series of observations to the end of the individual observation for which the point was plotted. Diagrams showing these variations in velocity corresponding to several series of observations are shown in plates 5 and 6. Several other series of observations were obtained for different depths and velocities for both smooth bed and bed of $\frac{3}{4}$ -inch gravel. Diagrams A, B, C, D, and E in plate 5 show the manner in which pulsations in the same depth of water varied with different velocities and with different lengths of individual observations corresponding to 1, 5, or 10 revolutions of the current meter. Diagrams F, G, H, and I, in plate 6 show comparisons of pulsations for different conditions of smooth bed and $\frac{3}{4}$ -inch gravel and for different depths and velocities of water, as well as for differences in periods of observation corresponding to 1 and 2 revolutions of the current meter. Results obtained by use of the pygmy current meter are shown in plate 6, diagram G.

It is, of course, impossible to obtain an instantaneous observation of velocity by means of the current meter, for the principle of the instrument is such that the number of complete revolutions in a

measured interval of time corresponds to the average velocity of the water during that interval. The longer the period of time of an observation, therefore, the smaller will be the variation of the mean velocity during the observation period from the average velocity for a long-time period. A comparison of diagrams D and E in plate 5 shows that the variations in the mean velocity for periods corresponding to 5 revolutions of the current meter are much more pronounced and more erratic than the variations in the mean velocity for periods corresponding to 10 revolutions.

It is evident from these diagrams that a period of several minutes would have been required to eliminate entirely the effects of the pulsations in the flume that caused these variations of velocity. However, the comparatively large number of individual observations that make up a complete measurement of discharge in the 12-foot flume—the observation points being taken at 0.5-foot intervals across the 12-foot width, with additional observations at 0.25 foot from each side wall—gave a total period of observation of such length that the plus and minus differences in velocity caused by pulsations during periods of 1 minute or thereabouts were largely compensated.

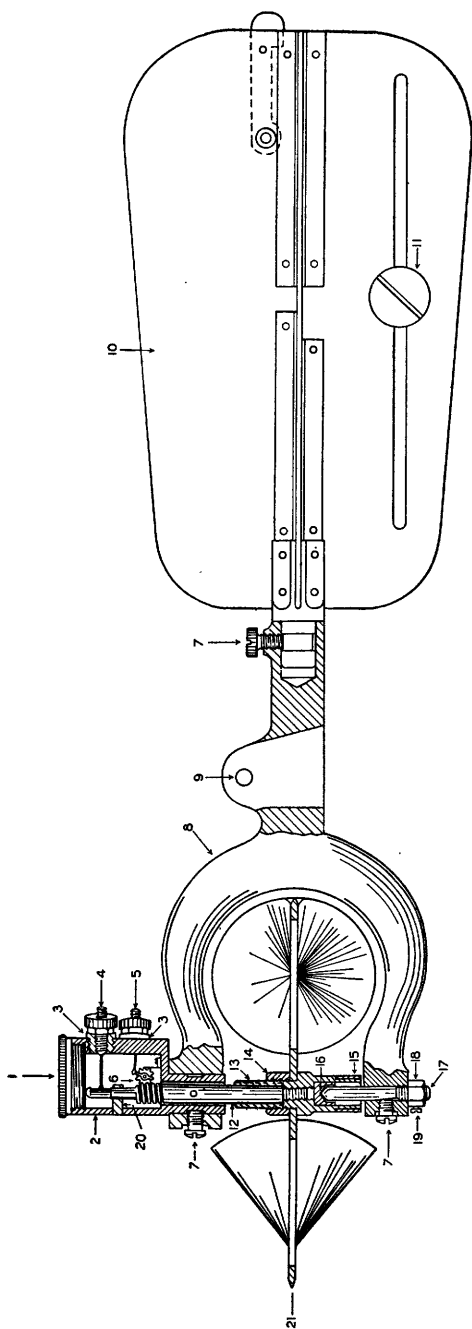
CURRENT METERS AND OTHER EQUIPMENT USED IN MEASURING DISCHARGE IN THE 12-FOOT FLUME

The investigation of the performance of current meters in measuring velocities in shallow depths necessarily was based on comparisons of discharge measurements by current meters and measurements of the same discharge by other methods. The current meters and other equipment that were used for measuring the discharge in the 12-foot flume are described in the following pages.

CURRENT METERS

The current meters that were used in this investigation consisted of seven Geological Survey standard-size small Price-type current meters, two Geological Survey cup-type pygmy current meters, one propeller-type pygmy meter, and one propeller-type Ott current meter.

The Geological Survey standard-size current meters were all of the same general type with respect to size and essential features of design (see pl. 1, A) and were selected as being representative of current meters generally used by engineers of the Geological Survey in measuring river discharge. Minor differences in design or construction of the current meters are mentioned in descriptions of the individual meters, but those minor differences had no distinguishable effects on the results of the measurements of the total discharge in the flume. The assembly of a Geological Survey type A small Price current meter, which is illustrative of the current meters referred to under that designation, is shown in figure 3.



EXPLANATION

- | | | | |
|----|---|----|------------------------------------|
| 1 | Cap for contact chamber | 11 | Balance weight |
| 2 | Cap screw | 12 | Hub nut |
| 3 | Insulating bushing for contact binding post | 13 | Bucket-wheel hub |
| 4 | Single-contact binding post (upper) | 14 | Bucket-wheel hub nut |
| 5 | Penta-contact binding post (lower) | 15 | Raising nut |
| 6 | Penta gear | 16 | Pivot bearing |
| 7 | Set screws | 17 | Pivot |
| 8 | Yoke | 18 | Pivot adjusting nut |
| 9 | Hole for hanger screw | 19 | Yoke screw for pivot adjusting nut |
| 10 | Tailpiece | 20 | Yoke screw for pivot adjusting nut |
| | | 21 | Bucket-wheel |

FIGURE 3.—Assembly of Geological Survey type A small Price current meter.

All the current meters were rated at the meter-rating station of the National Bureau of Standards. Diagrams showing the results of individual runs in the rating flume, plotted with revolutions per second as ordinates and velocity in feet per second as abscissas, together with the graphic presentation of the ratings for the range of velocities covered by the runs made in the rating flume, were furnished by that Bureau. The ratings were for velocities from 0.10 foot to 2.5 feet per second. Rating tables deduced from equations corresponding to the graphic presentation of the ratings were prepared by engineers of the Geological Survey. The current meters were rerated at frequent intervals, the intervals between ratings depending on the extent of use of the meters. The current meters used in this investigation were not used in other work during the period of investigation.

Current meter 1175, shown in plate 1, *A*, is a standard type A meter equipped with both single and penta contacts. Before its use in this investigation it had been used on routine stream-gaging work in the field and was one of the group of current meters first equipped with type A pivots and bearings. This meter was used for 160 discharge measurements in the 12-foot flume.

Current meter 1196 is similar to standard type A current meters in all respects except that it does not have the penta contact, and the point of the pivot is ground to a sharper angle. This meter was one of a group of meters intended for measuring low velocities in the field, and prior to its use in this investigation it had been in field service. This meter was used for 83 discharge measurements in the 12-foot flume.

Current meter 1217, a standard type A meter, had been used for experimental work at the meter-rating station of the National Bureau of Standards in 1932. This meter was used for 91 discharge measurements in the 12-foot flume.

Current meter 1310 is similar to standard type A meters except that it has the contact chamber built into the yoke about half an inch below the usual position. It was originally constructed as an experimental meter and as such was used in the field by several Survey engineers prior to this investigation. Because of its unusual construction with respect to the position of the contact chamber, meter 1310 was used only to a small extent. Only three of the discharge measurements in the 12-foot flume were made with this meter.

Current meter 1514 is a new type A meter that had not been used before this investigation. It was used in making 112 discharge measurements in the 12-foot flume.

Current meter 2C, a new meter, is of standard type A design except that the bucket wheel is reversed in position so that its direction of rotation is opposite to that of standard meters. This change in the

bucket wheel was made in connection with the study of the distribution of velocity near the side walls of the flume. (See pp. 10-12.) Meter 2C was used in making 17 discharge measurements in the 12-foot flume.

Current meter 730 is a "combination" or 623 type meter having interchangeable single-contact and penta heads. The single-contact head was used with this meter in making 14 discharge measurements in the 12-foot flume.

Pygmy meters A3 and A7, constructed by the Geological Survey, are vertical-axis cup-type meters similar to type A current meters but only two-fifths of their size. (See pl. 1, B.) The pygmy meters are provided with single contact only, and because of their greater number of revolutions for a foot of travel of the water, the intervals between the contacts at each revolution of the bucket wheel are much shorter than the intervals between the contacts made by the standard-size meters for the same velocities of the water. Although more sensitive to changes in velocity because of the lesser moment of inertia of the bucket wheel, the pygmy meters did not materially differ from the standard-size meters with respect to the precision of their ratings at the velocities for which they were used in this investigation.

Discharge measurements that were made by use of the pygmy meters are listed in the tables on pages 28-31 and include 173 measurements made with meter A3 and 22 measurements made with meter A7. Besides these measurements, the pygmy meters were used also in studying the distribution of velocities near flume walls and the variation of velocities in verticals.

Pygmy meter A2 is a horizontal-axis propeller-type meter having four blades of 20° pitch and a diameter of rotor of 0.75 inch. This meter was used for purposes of comparison with the vertical-axis cup-type current meters in studying the distribution of velocities in the flume and in making six discharge measurements listed on page 32.

The Ott Xb meter is a horizontal-axis propeller-type meter having three blades protected by a band 3 inches in diameter. Comparisons of measurements by this meter and by other meters and the pitot static tube are shown in the table on page 12. The results of 10 discharge measurements made with the Ott Xb meter are listed on page 32.

PITOT STATIC TUBE

The pitot static tube was used principally for investigating the distribution of velocities near the side walls and in verticals at various points across the width of the flume. Vertical velocity curves plotted from velocity measurements made with the pitot tube are shown in plate 4.

This instrument consists essentially of two concentric tubes: The inner tube, of $\frac{1}{8}$ -inch outside diameter, is open at the upstream end; the outer tube, of $\frac{1}{16}$ -inch outside diameter, is beveled at the upper end to the diameter of the inner tube. The space between the two tubes is filled by a jacket extending back $1\frac{1}{16}$ inches from the end. The length of the tube from the upstream end to the center of the two riser tubes is $5\frac{1}{2}$ inches. The static water pressure is communicated to the outer tube through 15 holes, each hole 1 mm. in diameter, arranged in three rows of five holes in each row. The first row of holes is $1\frac{1}{8}$ inches from the end of the tube; the other rows of holes are $\frac{3}{16}$ inch and $\frac{1}{8}$ inch, respectively, from the first row. The impulse tube and the static tube were connected to vertical riser tubes 2 feet in length. Flexible connections are carried from the end of each riser tube to inclined manometer gages. The riser tubes are supported by a traveler operating on a frame spanning the flume, and after the instrument is adjusted to the desired position in the water it is clamped in place so that the attention of the operator can be directed to readings of the manometer gages. These gages are inclined at a slope of 1 on 5. A calibration of the pitot static tube showed that the instrument constant was 1.000. The velocity of the water was computed from the formula $V = \sqrt{2g(h_1 - h_2)}$, in which h_1 and h_2 were obtained from readings of the manometer gages. Those gages were calibrated in the inclined position in which they were used, so that effects of possible differences in the slopes of the two gages or irregularities in their bore were eliminated.

Point measurements of velocities by means of the pitot static tube were obtained as close as 0.01 foot to the side walls and bottom of the flume and within 0.01 foot from the water surface. Variations in pressure caused by pulsations of the water made instantaneous readings somewhat uncertain; therefore the variations in h_1 and h_2 were averaged by inspection over an interval of several minutes before making determinations of height. Several measurements of discharge in the flume were made by means of the pitot static tube. Comparisons of three of those measurements with measurements of the same discharge by current meters are given in the table on page 12.

SHARP-CRESTED WEIR

The sharp-crested weir that was accepted as the standard of comparison for the current-meter measurements of discharge was at the lower end of the 12-foot flume, about 99 feet from the section of the flume where the measurements were made. The weir had a level crest 7.99 feet wide. The contractions at the ends of the weir were cut off by vertical approach walls that began 3 feet upstream from the weir and reached a width of 22 inches at a point 3 inches upstream from the crest, leaving a contraction of 2 inches at each end of the

weir. The top of the steel crest was 5 inches above the bottom of the flume, and it had a width of $\frac{1}{16}$ -inch with a downstream bevel of 45° . The thickness of the steel plate below the bevel was a quarter of an inch. The intake to the gage well led from the bottom of the flume, 5.62 feet from the weir crest. A hook gage was used in measuring the head on the weir. The weir was calibrated by means of an 8-inch venturi meter that had been calibrated previously by the weighing tanks. The weir was also calibrated volumetrically by observing heights of water in the tumble bay. The weir rating that was used in the computations of discharge was based on the laboratory calibrations by the above-described methods and was accepted as the standard of comparison for the current-meter measurements except for discharges less than 3.5 second-feet.

VENTURI METERS

The two venturi meters that were used in measurements of discharge below 3.5 second-feet were inserted in 8-inch and 4-inch supply pipes and had throat diameters of 4 inches and 2 inches, respectively. Both meters were calibrated by means of the weighing tanks, and the following equations of discharge were determined from plottings of the calibration curves: For the 8-inch venturi meter, $Q = 0.144\sqrt{h}$, and for the 4-inch venturi meter, $Q = 0.0234\sqrt{h}$, where h is the difference, in millimeters, of heads at the entrance and the throat of the meter.

The 8-inch venturi meter was used as the standard of measurement for discharges between 0.5 second-foot and 3.5 second-feet. The 4-inch venturi meter was used as the standard for measurements of 0.5 second-foot or less.

DISCHARGE MEASUREMENTS

Discharge measurements made with standard-size small Price-type current meters

Smooth concrete flume bed

Measure- ment No.	Meter No.	Depth of water (feet)	Method of measure- ment	Average velocity in flume (feet per second)	Total discharge (second-foot)		Coefficient for meter measure- ment
					Weir	Meter	
260	1514	0.20	0.35d	0.50	¹ 1.205	1.327	0.908
261	730	.20	.35d	.50	¹ 1.205	1.332	.905
262	1514	.20	.35d	1.00	¹ 2.400	2.026	1.185
263	1514	.20	.35d	1.25	¹ 3.014	2.257	1.335
264	1514	.20	.35d	1.52	3.653	2.454	1.489
266	1514	.20	.35d	.80	¹ 1.916	1.812	1.057
267	1514	.20	.35d	.40	1.966	1.087	.889
270	1514	.20	.35d	.20	.478	.516	.926
272	1514	.20	.35d	.30	1.734	.829	.885
297	1514	.20	.35d	.20	1.478	.552	.866

¹ By 8-inch venturi meter.

NOTE.—For the condition of 0.2-foot depth of water on the concrete bottom of the flume, the current-meter cups could not be placed exactly at middepth. For this condition the center of the meter cups was at a depth of 0.07 foot below the water surface when the bottom of the meter yoke rested on the concrete, and the top edges of the cups were out of the water. For descriptions of the individual current meters see pages 15-18.

Discharge measurements made with standard-size small Price-type current meters—
Continued

Smooth concrete flume bed—Continued

Measurement No.	Meter No.	Depth of water (feet)	Method of measurement	Average velocity in flume (feet per second)	Total discharge (second-feet)		Coefficient for meter measurement
					Weir	Meter	
244	1217	0.30	0.5d	1.51	5.55	4.41	1.236
245	1217	.30	.5d	1.26	4.54	4.09	1.110
246	1217	.30	.5d	1.02	3.66	3.58	1.022
247	1514	.30	.5d	.30	¹ 1.087	1.089	.998
250	1514	.30	.5d	.80	¹ 2.891	2.846	1.016
255	1514	.30	.5d	.50	¹ 1.799	1.787	1.007
256	730	.30	.5d	.50	¹ 1.799	1.763	1.020
269	1514	.30	.5d	.40	¹ 1.440	1,350	1.067
273	1514	.30	.5d	.20	¹ .734	.667	1.100
296	1514	.30	.5d	.40	¹ 1.440	1.391	1.035
298	1514	.30	.5d	.20	¹ .727	.680	1.069
299	1514	.30	.5d	.30	¹ 1.087	1.032	1.053
236	1217	.40	.5d	1.52	7.28	6.86	1.061
238	1217	.40	.5d	1.26	6.06	5.83	1.039
241	1514	.40	.5d	1.01	4.85	4.86	.998
243	1514	.40	.5d	.80	3.86	3.76	1.027
249	1514	.40	.5d	.50	¹ 2.405	2.340	1.028
265	1514	.40	.5d	.40	¹ 1,916	1.889	1.014
268	1514	.40	.5d	.30	¹ 1.440	1.410	1.021
271	1514	.40	.5d	.20	¹ .966	.925	1.044
236a	1217	.40	.6d	1.52	7.28	6.80	1.071
238a	1217	.40	.6d	1.26	6.06	5.65	1.073
241a	1514	.40	.6d	1.01	4.85	4.71	1.030
243a	1514	.40	.6d	.80	3.86	3.69	1.046
249a	1514	.40	.6d	.50	¹ 2.405	2.260	1.064
265a	1514	.40	.6d	.40	¹ 1,916	1.732	1.106
268a	1514	.40	.6d	.30	¹ 1.440	1.318	1.093
271a	1514	.40	.6d	.20	¹ .966	.874	1.105
224	1514	.52	.5d	1.20	7.56	7.80	.969
232	1217	.50	.5d	1.51	9.07	9.11	.996
237	1217	.50	.5d	1.00	6.03	6.16	.979
240	1514	.50	.5d	.81	4.85	4.95	.980
248	1514	.50	.5d	.40	¹ 2.405	2.449	.982
252	1514	.50	.5d	.50	¹ 3.014	3.049	.989
254	1514	.50	.5d	.30	¹ 1.799	1.719	1.047
259	1514	.50	.5d	.20	¹ 1.205	1.179	1.022
293	1514	.50	.5d	1.26	7.56	7.68	.984
294	1514	.50	.5d	.30	¹ 1.799	1.811	.993
224a	1514	.52	.6d	1.20	7.56	7.63	.991
232a	1217	.50	.6d	1.51	9.07	8.87	1.023
237a	1217	.50	.6d	1.00	6.03	6.02	1.002
240a	1514	.50	.6d	.81	4.85	4.85	1.000
248a	1514	.50	.6d	.40	¹ 2.405	2.328	1.033
252a	1514	.50	.6d	.50	¹ 3.014	2.898	1.040
254a	1514	.50	.6d	.30	¹ 1.799	1.637	1.099
259a	1514	.50	.6d	.20	¹ 1.205	1.128	1.068
293a	1514	.50	.6d	1.26	7.56	7.45	1.015
294a	1514	.50	.6d	.30	¹ 1.799	1.717	1.048
306	1514	.50	.6d	1.01	6.06	5.94	1.020
307	2C	.50	.6d	1.01	6.06	6.03	1.005
310	1514	.50	.6d	.50	¹ 3.000	2.928	1.025
311	2C	.50	.6d	.50	¹ 3.000	2.976	1.008
346	1514	.50	.6d	.50	¹ 3.000	2.947	1.018
347	2C	.50	.6d	.50	¹ 3.000	3.030	.990
350	2C	.50	.6d	1.00	6.01	6.02	.998
351	1514	.50	.6d	1.00	6.01	5.90	1.019
7	1196	.60	.5d	.19	1.376	1.431	.962
45	1196	.60	.5d	.10	1.720	.728	.992
65	730	.60	.5d	.20	¹ 1.440	1.447	.995
222	1217	.61	.5d	1.49	10.94	11.10	.988
233	1217	.60	.5d	1.26	9.07	9.20	.986

¹ By 8-inch venturi meter.

Discharge measurements made with standard-size small Price-type current meters—
Continued

Smooth concrete flume bed—Continued

Measure- ment No.	Meter No.	Depth of water (feet)	Method of measure- ment	Average velocity in flume (feet per second)	Total discharge (second-feet)		Coefficient for meter measure- ment
					Weir	Meter	
235	1217	0.60	0.5d	1.01	7.28	7.52	0.968
239	1175	.60	.5d	.81	5.80	6.04	.960
242	1514	.60	.5d	.50	3.62	3.71	.976
251	1514	.60	.5d	.40	1 2.891	3.010	.960
253	1514	.60	.5d	.30	1 2.156	2.171	.993
384	1514	.60	.5d	.30	1 2.160	2.176	.993
7a	1196	.60	.6d	.19	1.376	1.355	1.015
39	1196	.60	.6d	.09	.649	.603	1.076
40	1175	.60	.6d	.09	.649	.660	.983
45a	1196	.60	.6d	.10	1.720	.691	1.042
65a	730	.60	.6d	.20	1 1.440	1.392	1.034
222a	1217	.61	.6d	1.49	10.94	10.91	1.003
235a	1217	.60	.6d	1.26	9.07	8.91	1.018
234	1514	.60	.6d	1.01	7.28	7.24	1.006
239a	1175	.60	.6d	.81	5.80	5.81	.998
242a	1514	.60	.6d	.50	3.62	3.55	1.020
251a	1514	.60	.6d	.40	1 2.891	2.772	1.043
253a	1514	.60	.6d	.30	1 2.156	2.030	1.062
357	1175	.60	.6d	.20	1 1.436	1.375	1.044
384a	1514	.60	.6d	.30	1 2.160	1.990	1.085
3a	1175	.79	.5d	.50	4.70	5.00	.940
16a	1196	.80	.5d	.19	1.844	1.903	.969
26a	1196	.80	.5d	.40	3.80	3.93	.967
27a	1196	.80	.5d	.30	2.842	2.969	.957
42a	1175	.80	.5d	.10	1.966	1.017	.950
47a	1196	.80	.5d	.20	1 1.921	1.933	.994
56a	1196	.80	.5d	.20	1 1.916	1.895	1.011
61a	730	.80	.5d	.50	4.77	4.97	.960
213a	1217	.81	.5d	.99	9.68	10.14	.955
221a	1217	.80	.5d	1.50	14.47	14.88	.972
3b	1175	.79	.6d	.50	4.70	4.71	.998
16b	1196	.80	.6d	.19	1.844	1.823	1.012
26b	1196	.80	.6d	.40	3.80	3.69	1.030
27b	1196	.80	.6d	.30	2.842	2.858	.994
42b	1175	.80	.6d	.10	1.966	.947	1.020
47b	1196	.80	.6d	.20	1 1.921	1.902	1.010
50	1217	.80	.6d	.51	4.85	4.89	.992
56b	1196	.80	.6d	.20	1 1.916	1.825	1.050
58	1196	.80	.6d	.40	3.84	3.76	1.021
61b	730	.80	.6d	.50	4.77	4.76	1.002
213b	1217	.81	.6d	1.00	9.68	9.68	1.000
221b	1217	.80	.6d	1.50	14.47	14.33	1.010
358	1175	.80	.6d	.40	3.80	3.81	.997
57	1196	1.00	.5d	.20	2.405	2.300	1.046
214	1175	1.01	.5d	.50	6.11	6.42	.952
218	1217	1.00	.5d	1.50	18.10	18.39	.984
219	1217	1.00	.5d	1.00	12.09	12.57	.962
227	1514	1.01	.5d	.20	2.245	2.274	1.058
230	1175	1.00	.5d	.30	3.62	3.71	.976
1a	1175	1.00	.6d	.79	9.49	9.57	.992
4a	1175	1.00	.6d	.49	5.88	5.94	.990
6a	1196	1.00	.6d	.20	2.438	2.403	1.015
8a	1175	1.00	.6d	.61	7.28	7.44	.978
9a	1175	1.04	.6d	.96	12.03	12.31	.977
18a	1196	1.00	.6d	.30	3.55	3.64	.975
19a	1196	1.00	.6d	.40	4.75	4.84	.981
25	1196	.98	.6d	.10	1.159	1.204	.963
37	1196	1.00	.6d	.10	1.138	1.059	1.075
46a	1196	1.02	.6d	.09	1 1.106	1.047	1.056
48	1217	1.00	.6d	.80	9.61	9.80	.981
48a	1217	1.00	.6d	.80	9.61	9.69	.992
49a	1217	1.04	.6d	.96	11.96	12.17	.983
51a	1217	1.00	.6d	.49	5.93	5.94	.998
52a	1217	1.00	.6d	.60	7.20	7.36	.978

¹ By 8-inch venturi meter.

Discharge measurements made with standard-size small Price-type current meters—
Continued

Smooth concrete flume bed—Continued

Measure- ment No.	Meter No.	Depth of water (feet)	Method of measure- ment	Average velocity in flume (feet per second)	Total discharge (second-feet)		Coefficient for meter measure- ment
					Weir	Meter	
53a	1217	1.00	0.6d	0.40	4.80	4.90	0.980
54	1196	1.00	.6d	.10	¹ 1.205	1.068	1.128
57a	1196	1.00	.6d	.20	¹ 2.405	2.260	1.064
59a	730	1.04	.6d	.96	11.99	12.06	.994
60a	730	1.00	.6d	.50	6.01	6.02	.998
218a	1217	1.00	.6d	1.50	18.10	17.85	1.014
300a	1514	1.00	.6d	.20	¹ 2.401	2.329	1.031
302a	1217	1.00	.6d	1.52	18.18	18.12	1.003
305a	1514	1.00	.6d	.81	9.71	9.78	.993
360	1175	1.00	.6d	.50	5.99	5.94	1.008
1b	1175	1.00	.2d, .8d	.79	9.49	9.00	1.054
4d	1175	1.00	.2d, .8d	.49	5.88	5.63	1.044
6b	1196	1.00	.2d, .8d	.20	2.438	2.297	1.051
8b	1175	1.00	.2d, .8d	.61	7.28	6.97	1.044
9b	1175	1.04	.2d, .8d	.96	12.03	11.58	1.039
18b	1196	1.00	.2d, .8d	.30	3.55	3.41	1.041
19b	1196	1.00	.2d, .8d	.40	4.75	4.61	1.030
46b	1196	1.02	.2d, .8d	.09	¹ 1.106	1.023	1.081
48b	1217	1.00	.2d, .8d	.80	9.61	9.25	1.039
49b	1217	1.04	.2d, .8d	.96	11.96	11.58	1.033
51b	1217	1.00	.2d, .8d	.49	5.93	5.76	1.030
52b	1217	1.00	.2d, .8d	.60	7.20	7.24	.994
53b	1217	1.00	.2d, .8d	.40	4.80	4.70	1.021
55	1196	1.00	.2d, .8d	.10	¹ 1.205	1.101	1.094
57b	1196	1.00	.2d, .8d	.20	¹ 2.405	2.189	1.099
59b	730	1.04	.2d, .8d	.96	11.99	11.66	1.028
60b	730	1.00	.2d, .8d	.50	6.01	5.91	1.017
218b	1217	1.00	.2d, .8d	1.50	18.10	17.05	1.062
300b	1514	1.00	.2d, .8d	.20	¹ 2.401	2.303	1.043
302b	1514	1.00	.2d, .8d	1.52	18.18	17.36	1.047
305b	1514	1.00	.2d, .8d	.81	9.71	9.36	1.037
360	1175	1.00	.2d, .8d	.50	5.99	5.70	1.051
216	1217	1.26	.5d	1.49	22.66	23.10	.981
220	1217	1.25	.5d	1.00	15.11	15.50	.975
228a	1175	1.25	.5d	.50	7.56	7.90	.957
226a	1175	1.26	.5d	.30	4.54	4.77	.952
231	1514	1.26	.5d	.20	¹ 3.017	2.802	1.077
5a	1175	1.25	.6d	1.25	18.69	19.12	.978
11a	1196	1.25	.6d	.49	7.39	7.56	.978
12a	1175	1.24	.6d	1.01	15.04	15.30	.983
14a	1175	1.24	.6d	.81	12.08	12.52	.965
15a	1196	1.25	.6d	.20	2.950	2.938	1.004
22a	1196	1.25	.6d	.30	4.46	4.51	.989
24a	1196	1.25	.6d	.30	4.43	4.53	.978
28a	1217	1.24	.6d	.60	8.95	9.08	.986
29a	1175	1.24	.6d	.40	5.92	5.94	.997
43a	1175	1.24	.6d	.10	¹ 1.503	1.555	.967
62a	1217	1.24	.6d	.61	9.04	9.12	.991
216a	1217	1.26	.6d	1.50	22.66	22.70	.998
308a	1514	1.25	.6d	1.01	15.13	15.13	1.000
309a	2 C	1.25	.6d	1.01	15.13	15.56	.972
312a	1514	1.25	.6d	.51	7.59	7.75	.979
313a	2 C	1.25	.6d	.51	7.59	7.86	.966
314	1175	1.25	.6d	1.00	15.06	14.97	1.006
315	2 C	1.25	.6d	1.00	15.06	15.25	.988
330	1514	1.25	.6d	1.00	15.00	15.01	.999
331	2 C	1.25	.6d	1.00	15.00	15.33	.978
338	1514	1.25	.6d	.50	7.51	7.70	.975
339	2 C	1.25	.6d	.50	7.51	7.75	.969
361a	1175	1.25	.6d	.50	7.51	7.57	.992

¹ By 8-inch venturi meter.

Discharge measurements made with standard-size small Price-type current meters—
Continued

Smooth concrete flume bed—Continued

Measure- ment No.	Meter No.	Depth of water (feet)	Method of measure- ment	Average velocity in flume (feet per second)	Total discharge (second-foot)		Coefficient for meter measure- ment
					Weir	Meter	
5b	1175	1.25	0.2d, 0.8d	1.25	18.69	18.33	1.020
11b	1196	1.25	.2d, .8d	.49	7.39	7.24	1.021
12b	1175	1.24	.2d, .8d	1.01	15.04	14.84	1.013
14b	1175	1.24	.2d, .8d	.81	12.08	11.89	1.016
15b	1196	1.25	.2d, .8d	.20	2.950	2.893	1.020
22b	1196	1.25	.2d, .8d	.30	4.46	4.43	1.007
24b	1196	1.25	.2d, .8d	.30	4.43	4.34	1.021
28b	1217	1.24	.2d, .8d	.60	8.95	8.91	1.004
29b	1175	1.24	.2d, .8d	.40	5.92	5.85	1.012
43b	1175	1.24	.2d, .8d	.10	1.503	1.504	.999
62b	1175	1.24	.2d, .8d	.61	9.04	8.89	1.017
216b	1217	1.26	.2d, .8d	1.50	22.66	21.91	1.034
223b	1175	1.25	.2d, .8d	.50	7.56	7.45	1.015
226b	1175	1.26	.2d, .8d	.30	4.54	4.43	1.025
308b	1514	1.25	.2d, .8d	1.01	15.13	14.95	1.012
309b	2C	1.25	.2d, .8d	1.01	15.13	15.11	1.001
312b	1514	1.25	.2d, .8d	.51	7.59	7.67	.990
313b	2C	1.25	.2d, .8d	.51	7.59	7.62	.996
334	1514	1.25	.2d, .8d	1.00	15.02	14.78	1.016
335	2C	1.25	.2d, .8d	1.00	15.02	15.01	1.001
344	1514	1.25	.2d, .8d	.50	7.51	7.41	1.013
345	2C	1.25	.2d, .8d	.50	7.51	7.51	1.000
361b	1175	1.25	.2d, .8d	.50	7.51	7.32	1.026
212	1175	1.49	.5d	.50	9.07	9.49	.956
215	1217	1.52	.5d	1.49	27.33	27.87	.981
217	1217	1.49	.5d	1.01	18.14	18.63	.974
225	1514	1.53	.5d	.30	5.48	5.60	.979
229	1175	1.52	.5d	.20	3.62	3.59	1.008
2a	1175	1.50	.6d	.50	9.03	9.06	.997
13a	1175	1.50	.6d	1.00	18.01	18.39	.979
17a	1196	1.51	.6d	.20	3.55	3.50	1.014
20a	1175	1.50	.6d	1.25	22.51	22.54	.999
21a	1217	1.53	.6d	1.46	26.97	27.53	.980
23a	1217	1.50	.6d	1.25	22.58	22.61	.999
30a	1217	1.50	.6d	.80	14.42	14.69	.982
31a	1217	1.50	.6d	.60	10.75	10.76	.999
32a	1196	1.50	.6d	.40	7.17	6.87	1.044
33a	1196	1.51	.6d	.30	5.34	5.47	.976
34a	1196	1.50	.6d	.40	7.14	7.38	.967
41a	1175	1.50	.6d	.40	7.20	7.15	1.007
44a	1196	1.52	.6d	.10	1.799	1.697	1.060
63a	730	1.50	.6d	.50	8.98	9.15	.981
64a	730	1.50	.6d	1.00	18.03	18.00	1.002
225a	1514	1.53	.6d	.30	5.48	5.37	1.020
228	1514	1.52	.6d	.20	3.62	3.41	1.062
301a	1217	1.50	.6d	1.26	22.66	22.99	.986
303a	1514	1.50	.6d	.20	3.64	3.70	.984
304a	1514	1.50	.6d	.81	14.51	14.93	.972
362a	1175	1.50	.6d	.50	8.98	9.14	.982
2b	1175	1.50	.2d, .8d	.50	9.03	8.83	1.023
13b	1175	1.50	.2d, .8d	1.00	18.01	17.71	1.017
17b	1196	1.51	.2d, .8d	.20	3.55	3.45	1.029
20b	1175	1.50	.2d, .8d	1.25	22.51	22.36	1.007
21b	1217	1.53	.2d, .8d	1.46	26.97	26.68	1.011
23b	1217	1.50	.2d, .8d	1.25	22.58	22.34	1.011
30b	1217	1.50	.2d, .8d	.80	14.42	14.35	1.005
31b	1217	1.50	.2d, .8d	.60	10.75	10.60	1.014
32b	1196	1.50	.2d, .8d	.40	7.17	7.07	1.014
33b	1196	1.51	.2d, .8d	.30	5.34	5.28	1.011
34b	1196	1.51	.2d, .8d	.39	7.14	7.34	.973
41b	1175	1.50	.2d, .8d	.40	7.20	7.45	.966
44b	1196	1.52	.2d, .8d	.10	1.799	1.761	1.022
63b	730	1.50	.2d, .8d	.50	8.98	9.03	.994
64b	730	1.50	.2d, .8d	1.00	18.03	17.57	1.026
301b	1217	1.50	.2d, .8d	1.26	22.66	22.58	1.004
303b	1514	1.50	.2d, .8d	.20	3.64	3.57	1.020
304b	1514	1.50	.2d, .8d	.81	14.51	14.62	.992
362b	1175	1.50	.2d, .8d	.50	8.98	8.76	1.025

¹ By 8-inch venturi meter.

Discharge measurements made with standard-size small Price-type current meters—
Continued

Flume bed covered with 3/4-inch gravel

Measurement No.	Meter No.	Depth of water (feet)	Method of measurement	Average velocity in flume (feet per second)	Total discharge (second-feet)		Coefficient for meter measurement
					Weir	Meter	
108	1175	0.20	0.5d	0.30	1 0.720	0.706	1.020
109	1175	.21	.5d	.47	1 1.205	1.159	1.040
110	1175	.21	.5d	.94	1 2.400	1.867	1.285
113	1217	.21	.5d	1.18	1 3.021	2.145	1.408
114	1217	.21	.5d	1.18	1 3.021	2.155	1.402
117	1196	.20	.5d	.20	2 482	.475	1.015
129	1217	.23	.5d	1.28	3 554	2.633	1.350
168	1175	.21	.5d	.57	1 1.440	1.344	1.071
178	1175	.21	.5d	.76	1 1.916	1.645	1.165
181	1175	.20	.5d	.40	1 966	.935	1.033
185	1175	.21	.5d	.76	1 1.916	1.658	1.156
381	1514	.20	.5d	.50	1 1.207	1.280	.943
383	1514	.20	.5d	1.25	1 3.000	2.191	1.399
66	1175	.32	.5d	1.41	1 5.38	4.44	1.212
67	1175	.27	.43d	1.12	3 60	3.02	1.192
71	1196	.30	.5d	1.25	4 52	3.88	1.165
76	1196	.31	.5d	.77	1 2.887	2.713	1.064
80	1196	.31	.5d	.48	1 777	1.622	1.096
84	1175	.30	.5d	1.47	5 38	4.23	1.272
87	1310	.30	.5d	1.48	5 40	4.33	1.247
97	1196	.30	.5d	.30	1 1.087	1.063	1.023
107	1196	.30	.5d	.99	3 58	3.31	1.082
145	1175	.31	.5d	.48	1 1.799	1.725	1.043
157	1175	.30	.5d	1.25	4 52	3.76	1.202
166	1175	.32	.5d	.19	1 727	.628	1.158
167	1175	.29	.5d	.41	1 1.440	1.404	1.026
171	1175	.30	.5d	.80	1 2.891	2.560	1.129
177	1175	.30	.5d	.60	1 2.160	2.044	1.057
382	1514	.30	.5d	.50	1 1.799	1.619	1.111
68	1175	.41	.5d	1.46	7 23	6.85	1.055
70	1175	.39	.5d	1.03	4 80	4.69	1.023
72	1196	.40	.5d	1.25	6 03	6.01	1.003
81	1196	.40	.5d	.79	3 82	3.65	1.047
90	1175	.40	.5d	1.49	7 17	6.75	1.062
91	1310	.40	.5d	1.49	7 17	6.85	1.047
96	1217	.43	.5d	1.85	9 58	9.09	1.054
98	1196	.40	.5d	.30	1 1.440	1.462	.985
101	1175	.41	.5d	.49	1 2.405	2.487	.967
116	1196	.40	.5d	.10	2 480	.443	1.084
120	1175	.40	.5d	.20	1 966	.949	1.018
163	1217	.40	.5d	1.25	6 06	5.86	1.034
170	1175	.40	.5d	.60	1 2.891	2.831	1.021
176	1175	.40	.5d	.80	3 84	3.71	1.035
179	1175	.40	.5d	.40	1 1.916	1.927	.994
186	1175	.40	.5d	.50	1 2.405	2.433	.988
386	1514	.40	.5d	.50	1 2.401	2.375	1.011
68a	1175	.41	.6d	1.46	7 23	6.49	1.114
70a	1175	.39	.6d	1.03	4 80	4.47	1.074
72a	1196	.40	.6d	1.25	6 03	5.62	1.073
81a	1196	.40	.6d	.79	3 82	3.40	1.124
90a	1175	.40	.6d	1.49	7 17	6.45	1.112
91a	1310	.40	.6d	1.49	7 17	6.37	1.126
96a	1217	.43	.6d	1.85	9 58	8.99	1.066
98a	1196	.40	.6d	.30	1 1.440	1.329	1.084
101a	1175	.41	.6d	.49	1 2.405	2.346	1.025
116a	1196	.40	.6d	.10	2 480	.395	1.215
120a	1175	.40	.6d	.20	1 966	.885	1.092
180a	1175	.40	.6d	.50	1 2.405	2.226	1.080

¹ By 8-inch venturi meter.

² By 4-inch venturi meter.

NOTE.—For the condition of 0.2-foot depth of water on the bed covered with 3/4-inch gravel, the tailpiece was removed from the meter, and the bottom of the meter yoke was forced into the gravel bed until the tops of the meter cups were submerged. This position of the current meter permitted the observations to be made at middepth, as the diameter of the meter cups, which was 2 inches, was slightly less than the depth of water.

Discharge measurements made with standard-size small Price-type current meters—
Continued

Flume bed covered with 3/4-inch gravel—Continued

Meas- ure- ment No.	Meter No.	Depth of water (feet)	Method of meas- ure- ment	Average velocity in flume (feet per second)	Total discharge (second-feet)		Coefficient for meter meas- ure- ment
					Weir	Meter	
75	1196	.52	0.5d	1.44	9.01	9.24	0.975
77	1196	.50	.5d	1.00	6.03	6.19	.974
79	1175	.50	.5d	1.25	7.51	7.81	.962
83	1196	.47	.5d	.53	¹ 3.014	3.061	.985
99	1196	.50	.5d	.30	¹ 1.799	1.836	.980
153	1175	.51	.5d	.60	3.69	3.76	.981
155	1175	.51	.5d	.79	4.87	5.05	.964
182	1175	.50	.5d	.40	¹ 2.401	2.524	.951
183	1175	.52	.5d	.19	¹ 1.205	1.220	.988
187	1175	.50	.5d	.40	¹ 2.401	2.474	.970
75a	1196	.52	.6d	1.44	9.01	8.86	1.017
77a	1196	.50	.6d	1.00	6.03	5.86	1.029
79a	1175	.50	.6d	1.25	7.51	7.50	1.001
83a	1196	.47	.6d	.53	¹ 3.014	2.930	1.029
99a	1196	.50	.6d	.30	¹ 1.799	1.750	1.028
187a	1175	.50	.6d	.40	¹ 2.401	2.311	1.039
190	1175	.51	.6d	.79	4.87	5.01	.972
191	1175	.49	.6d	.20	¹ 1.205	1.138	1.059
200	1175	.51	.6d	.79	4.87	4.80	1.015
363	1514	.50	.6d	1.00	6.01	5.67	1.060
365	2 C	.50	.6d	.99	6.01	5.72	1.051
366	1514	.50	.6d	.50	¹ 3.003	2.853	1.053
368	2 C	.50	.6d	.50	¹ 3.003	2.939	1.022
69	1175	.59	.5d	1.53	10.80	11.44	.944
73	1196	.60	.5d	1.25	8.98	9.53	.942
78	1175	.60	.5d	1.05	7.51	7.90	.951
89	1175	.60	.5d	.97	6.98	7.47	.934
104	1175	.60	.5d	.29	2.116	2.229	.949
106	1196	.60	.5d	.50	3.58	3.75	.955
123	1196	.60	.5d	.20	1.440	1.481	.972
158	1175	.60	.5d	.61	4.40	4.72	.932
160	1217	.60	.5d	1.25	9.04	9.26	.976
169	1175	.60	.5d	.40	¹ 2.891	3.042	.950
173	1217	.61	.5d	.79	5.78	5.97	.968
184	1175	.60	.5d	.97	7.01	7.38	.950
384	1514	.60	.5d	.30	¹ 2.160	2.176	.993
69a	1175	.59	.6d	1.53	10.80	11.02	.980
73a	1196	.60	.6d	1.25	8.98	9.16	.980
78a	1175	.60	.6d	1.05	7.51	7.51	1.000
89a	1175	.60	.6d	.97	6.98	7.14	.978
104a	1175	.60	.6d	.29	2.116	2.106	1.005
106a	1196	.60	.6d	.50	3.58	3.62	.989
123a	1196	.60	.6d	.20	1.440	1.372	1.050
184a	1175	.60	.6d	.97	7.01	7.09	.989
188	1217	.61	.6d	.80	5.85	5.66	1.034
189	1175	.59	.6d	.62	4.40	4.35	1.011
192	1175	.60	.6d	.40	¹ 2.891	2.876	1.005
384a	1514	.60	.6d	.30	¹ 2.160	1.990	1.085
93	1175	.80	.5d	1.51	14.51	15.23	.953
102	1217	.80	.5d	1.00	9.58	10.06	.952
103	1175	.80	.5d	.30	2.87	3.03	.947
105	1196	.80	.5d	.50	4.80	5.05	.950
142	1217	.80	.5d	1.25	12.03	12.66	.950
172	1217	.80	.5d	.80	7.67	8.03	.955
174	1175	.81	.5d	.59	5.78	6.00	.963
175	1175	.79	.5d	.40	3.84	4.06	.946
180	1175	.81	.5d	.20	¹ 1.916	1.866	1.027
93a	1175	.80	.6d	1.51	14.51	14.48	1.002
102a	1217	.80	.6d	1.00	9.58	9.50	1.008
103a	1175	.80	.6d	.30	2.87	2.89	.993
105a	1196	.80	.6d	.50	4.80	4.83	.994

¹ By 8 inch venturi meter.

Discharge measurements made with standard-size small Price-type current meters—
Continued

Flume bed covered with 3/4-inch gravel—Continued

Measurement No.	Meter No.	Depth of water (feet)	Method of measurement	Average velocity in flume (feet per second)	Total discharge (second-feet)		Coefficient for meter for meter measurement
					Weir	Meter	
74	1175	0.99	0.5d	1.01	12.03	12.43	0.968
82	1175	1.00	.5d	1.50	18.06	19.22	.940
88	1175	.99	.5d	.50	5.98	6.25	.957
100	1175	1.00	.5d	.30	3.55	3.69	.962
115	1196	1.04	.5d	.19	2.410	2.434	.990
148	1217	.99	.5d	1.27	15.11	15.54	.972
154	1175	.99	.5d	.41	4.85	5.03	.964
161	1217	1.00	.5d	.81	9.68	10.27	.943
165	1175	1.01	.5d	.59	7.20	7.36	.978
74a	1175	.99	.6d	1.01	12.03	12.01	1.002
82a	1175	1.00	.6d	1.50	18.06	18.36	.984
88a	1175	.99	.6d	.50	5.98	6.00	.997
100a	1175	1.00	.6d	.30	3.55	3.47	1.023
115a	1196	1.04	.6d	.19	2.410	2.271	1.061
218a	1217	1.00	.6d	1.50	18.10	17.85	1.014
385	1514	1.00	.6d	.20	2.401	2.303	1.043
387	1514	1.00	.6d	.80	9.61	9.56	1.005
74b	1175	.99	.2d, .8d	1.01	12.03	11.33	1.062
82b	1175	1.00	.2d, .8d	1.50	18.06	17.02	1.061
88b	1175	.99	.2d, .8d	.50	5.98	5.71	1.047
100b	1175	1.00	.2d, .8d	.30	3.55	3.35	1.060
218b	1217	1.00	.2d, .8d	1.50	18.10	17.05	1.062
132	1175	1.25	.5d	.50	7.53	7.96	.946
143	1175	1.24	.5d	1.52	22.66	23.77	.953
144	1175	1.28	.5d	.99	15.20	16.21	.938
147	1217	1.25	.5d	1.25	18.88	19.94	.947
150	1217	1.25	.5d	.80	11.99	12.27	.977
156	1175	1.24	.5d	.20	3.017	3.077	.981
159	1175	1.25	.5d	.60	9.04	9.60	.942
162	1175	1.26	.5d	.40	6.01	6.34	.948
130a	1196	1.25	.6d	.30	4.49	4.46	1.007
131	1175	1.26	.6d	.30	4.49	4.44	1.011
132a	1175	1.25	.6d	.50	7.53	7.38	1.020
143a	1175	1.24	.6d	1.52	22.66	22.67	1.000
144a	1175	1.28	.6d	.99	15.20	15.36	.990
375	1514	1.25	.6d	.50	7.51	7.54	.996
376	2C	1.25	.6d	.50	7.51	7.56	.983
378	1514	1.25	.6d	.50	7.51	7.64	.983
388	1514	1.25	.6d	1.00	14.98	14.78	1.014
130b	1196	1.25	.2d, .8d	.30	4.49	4.36	1.030
132b	1175	1.25	.2d, .8d	.50	7.53	7.31	1.030
143b	1175	1.24	.2d, .8d	1.52	22.66	22.01	1.030
144b	1175	1.28	.2d, .8d	.99	15.20	15.22	.999
372	1514	1.25	.2d, .8d	.50	7.51	7.34	1.023
373	2C	1.25	.2d, .8d	.50	7.51	7.39	1.016
388	1514	1.25	.2d, .8d	1.00	14.98	14.62	1.025
133	1175	1.52	.5d	.50	9.07	9.50	.955
134	1175	1.50	.5d	1.50	26.97	28.60	.943
136	1175	1.52	.5d	.30	5.42	5.69	.953
138	1175	1.52	.5d	1.00	18.14	19.24	.943
146	1217	1.53	.5d	1.22	22.49	23.31	.965
149	1217	1.50	.5d	.80	14.47	15.21	.951
151	1217	1.51	.5d	.60	10.85	11.41	.951
152	1175	1.50	.5d	.21	3.82	3.99	.957
164	1175	1.50	.5d	.40	7.20	7.60	.947
133a	1175	1.52	.6d	.50	9.07	9.09	.998
135a	1217	1.50	.6d	1.50	26.97	27.06	.997
136a	1175	1.52	.6d	.30	5.42	5.34	1.015
139a	1217	1.52	.6d	1.00	18.14	17.94	1.011
133b	1175	1.52	.2d, .8d	.50	9.07	8.94	1.015
135b	1217	1.50	.2d, .8d	1.50	26.97	26.84	1.005
136b	1175	1.52	.2d, .8d	.30	5.42	5.43	.998
139b	1217	1.52	.2d, .8d	1.00	18.14	18.00	1.008

1 By 8-inch venturi meter.

Discharge measurements made with standard-size small Price-type current meters—
Continued

Flume bed covered with coarse gravel

Measurement No.	Meter No.	Depth of water (feet)	Method of measurement	Average velocity in flume (feet per second)	Total discharge (second-feet)		Coefficient for meter measurement
					Weir	Meter	
417	1175	0.40	0.5d	0.20	¹ 0.961	0.978	0.983
418	1175	.40	.5d	.40	¹ 1.919	1.989	.965
421	1175	.40	.5d	.60	¹ 2.880	2.833	1.017
425	1217	.40	.5d	1.49	7.17	6.52	1.100
440	1514	.40	.5d	.99	4.77	4.70	1.015
419	1514	.60	.5d	.20	¹ 1.440	1.474	.977
424	1217	.60	.5d	1.49	10.75	11.21	.959
426	1217	.60	.5d	1.00	7.17	7.71	.930
429	1175	.60	.5d	1.40	¹ 2.880	3.065	.940
439	1514	.60	.5d	.59	4.26	4.54	.938
419a	1514	.60	.6d	.20	¹ 1.440	1.385	1.010
424a	1217	.60	.6d	1.49	10.75	10.35	1.039
426a	1217	.60	.6d	1.00	7.17	6.98	1.027
429a	1175	.60	.6d	1.40	¹ 2.880	2.772	1.039
439a	1514	.60	.6d	.59	4.26	4.18	1.019
414	1514	1.00	.6d	1.50	17.95	17.76	1.011
420a	1175	1.00	.6d	.20	¹ 2.400	2.286	1.050
422a	1217	1.00	.6d	1.00	11.99	11.84	1.013
427a	1514	1.00	.6d	.60	7.17	7.17	1.000
441a	1514	1.00	.6d	.40	4.77	4.76	1.002
415	1514	1.00	.2d, .8d	1.50	17.95	16.76	1.071
420b	1175	1.00	.2d, .8d	.20	¹ 2.400	2.254	1.065
422b	1217	1.00	.2d, .8d	1.00	11.99	11.39	1.053
427b	1514	1.00	.2d, .8d	.60	7.17	6.84	1.048
441b	1514	1.00	.2d, .8d	.40	4.77	4.58	1.041
416a	1514	1.25	.6d	1.50	22.47	22.40	1.003
423a	1217	1.25	.6d	1.00	14.98	15.24	.983
428a	1175	1.25	.6d	.20	¹ 3.000	2.827	1.061
430a	1514	1.25	.6d	.60	8.95	9.13	.980
438a	1514	1.25	.6d	.40	5.95	5.99	.993
416b	1514	1.25	.2d, .8d	1.50	22.47	21.56	1.042
423b	1217	1.25	.2d, .8d	1.00	14.98	14.54	1.030
428b	1175	1.25	.2d, .8d	.20	¹ 3.000	2.811	1.067
430b	1514	1.25	.2d, .8d	.60	8.95	8.78	1.019
428b	1514	1.25	.2d, .8d	.40	5.95	5.83	1.021

¹ By 8-inch venturi meter.

Discharge measurements made with cup-type pygmy current meters

Smooth concrete flume bed

Measurement No.	Meter No.	Depth of water (feet)	Method of measurement	Average velocity in flume (feet per second)	Total discharge (second-feet)		Coefficient for meter measurement
					Weir	Meter	
275	A3	0.20	0.5d	0.20	10.478	0.438	1.091
276	A3	.20	.5d	1.52	3.64	3.36	1.083
284	A3	.20	.5d	.60	¹ 1.440	1.377	1.046
287	A3	.20	.5d	1.00	¹ 2.405	2.320	1.037
288	A3	.20	.5d	1.25	¹ 3.014	2.788	1.081
275a	A3	.20	.6d	.20	¹ 1.478	.410	1.166
276a	A3	.20	.6d	1.52	3.64	3.20	1.138
284a	A3	.20	.6d	.60	¹ 1.440	1.313	1.097
287a	A3	.20	.6d	1.00	¹ 2.405	2.201	1.093
288a	A3	.20	.6d	1.25	¹ 3.014	2.673	1.128
274	A3	.30	.5d	.20	¹ 1.720	.706	1.020
277	A3	.30	.5d	1.02	3.66	3.60	.992
281	A3	.30	.5d	1.51	5.45	5.26	1.036
285	A3	.30	.5d	.60	¹ 2.160	2.123	1.017
289	A3	.30	.5d	1.26	4.54	4.63	.981

¹ By 8-inch venturi meter.

Discharge measurements made with cup-type pygmy current meters—Continued

Smooth concrete flume bed—Continued

Measurement No.	Meter No.	Depth of water (feet)	Method of measurement	Average velocity in flume (feet per second)	Total discharge (second-feet)		Coefficient for meter measurement
					Weir	Meter	
274a	A3	0.30	0.6d	0.20	¹ 0.720	0.652	1.104
277a	A3	.30	.6d	1.02	3.66	3.51	1.043
281a	A3	.30	.6d	1.51	5.45	5.00	1.030
285a	A3	.30	.6d	.60	¹ 2.160	2.057	1.050
289a	A3	.30	.6d	1.26	4.54	4.36	1.041
279	A3	.40	.5d	1.52	7.28	7.39	.985
282	A3	.40	.5d	.20	¹ 1.966	1.949	1.018
286	A3	.40	.5d	.60	¹ 2.891	2.976	.971
290	A3	.40	.5d	1.01	4.87	4.92	.990
279a	A3	.40	.6d	1.52	7.28	7.06	1.031
282a	A3	.40	.6d	.20	¹ 1.966	1.911	1.060
286a	A3	.40	.6d	.60	¹ 2.891	2.843	1.017
290a	A3	.40	.6d	1.01	4.87	4.77	1.021
257	A3	.50	.5d	1.51	9.04	9.22	.980
402	A3	.50	.5d	.20	¹ 1.203	1.204	.999
404	A3	.50	.5d	1.50	8.98	9.50	.945
406	A3	.50	.5d	.60	3.58	3.80	.942
409	A3	.50	.5d	1.00	5.98	6.19	.966
257a	A3	.50	.6d	1.51	9.04	8.88	1.018
349	A3	.50	.6d	.50	¹ 3.000	2.972	1.009
352	A3	.50	.6d	1.00	6.01	5.99	1.003
402a	A3	.50	.6d	.20	¹ 1.203	1.150	1.046
404a	A3	.50	.6d	1.50	8.98	9.11	.986
406a	A3	.50	.6d	.60	3.58	3.64	.984
409a	A3	.50	.6d	1.00	5.98	5.93	1.008
258	A3	.60	.5d	.20	¹ 1.440	1.387	1.038
278	A3	.60	.5d	1.00	7.28	7.63	.954
280	A3	.60	.5d	1.50	10.91	11.11	.982
283	A3	.60	.5d	.20	¹ 1.440	1.379	1.044
291	A3	.60	.5d	.60	4.36	4.48	.973
292	A3	.60	.5d	.30	¹ 2.152	2.150	1.001
295	A3	.60	.5d	.40	¹ 2.891	2.894	.999
38	A3	.60	.6d	.09	.649	.593	1.094
278a	A3	.60	.6d	1.00	7.28	7.35	.990-
280a	A3	.60	.6d	1.50	10.91	10.77	1.013
283a	A3	.60	.6d	.20	¹ 1.440	1.357	1.061
291a	A3	.60	.6d	.60	4.36	4.30	1.014
292a	A3	.60	.6d	.30	¹ 2.152	2.060	1.045
295a	A3	.60	.6d	.40	¹ 2.891	2.807	1.030
403	A3	1.00	.5d	.20	¹ 2.397	2.304	1.040
405	A3	1.00	.5d	1.00	11.96	12.50	.957
408	A3	1.00	.5d	.60	7.14	7.36	.970
410	A3	1.00	.5d	1.50	17.95	18.92	.949
35a	A3	.99	.6d	.20	2.348	2.311	1.016
36	A3	1.00	.6d	.095	1.138	1.049	1.085
403a	A3	1.00	.6d	.20	¹ 2.397	2.234	1.073
405a	A3	1.00	.6d	1.00	11.96	12.38	.966
408a	A3	1.00	.6d	.60	7.14	7.26	.983
410a	A3	1.00	.6d	1.50	17.95	18.42	.974
35b	A3	.99	.2d, .8d	.20	2.348	2.299	1.021
405b	A3	1.00	.2d, .8d	.20	¹ 2.397	2.198	1.091
405b	A3	1.00	.2d, .8d	1.00	11.96	12.09	.989
408b	A3	1.00	.2d, .8d	.60	7.14	7.15	.999
410b	A3	1.00	.2d, .8d	1.50	17.95	18.02	.996
333	A3	1.25	.6d	1.00	15.00	15.03	.998
340	A3	1.25	.6d	.50	7.51	7.66	.980
337	A3	1.25	.2d, .8d	1.00	15.02	14.81	1.014
343	A3	1.25	.2d, .8d	.50	7.51	7.45	1.008
407a	A3	1.50	.6d	.20	3.58	3.60	.994
411a	A3	1.50	.6d	1.00	17.95	18.40	.976
412a	A3	1.50	.6d	.60	10.78	10.85	.994
413a	A3	1.50	.6d	1.50	26.97	27.86	.968
407b	A3	1.50	.2d, .8d	.20	3.58	3.42	1.047
411b	A3	1.50	.2d, .8d	1.00	17.95	18.09	.962
412b	A3	1.50	.2d, .8d	.60	10.78	10.71	1.007
413b	A3	1.50	.2d, .8d	1.50	26.97	27.19	.992

¹ By 8-inch venturi meter.

Discharge measurements made with cup-type pygmy current meters—Continued

Flume bed covered with 3/4-inch gravel

Measurement No.	Meter No.	Depth of water (feet)	Method of measurement	Average velocity in flume (feet per second)	Total discharge (second-feet)		Coefficient for meter measurement
					Weir	Meter	
111	A3	.21	.5d	0.94	1 2.401	2.466	0.974
112	A3	.21	.5d	1.18	1 3.015	3.015	1.000
118	A3	.20	.5d	.20	1 .480	.525	.914
128	A7	.22	.5d	1.35	3 3.554	3.650	.974
198	A3	.21	.5d	.57	1 1.440	1.404	1.026
391	A3	.20	.5d	.59	1 1.439	1.381	1.042
128a	A7	.22	.6d	1.35	3 3.554	3.380	1.051
391a	A3	.20	.6d	.59	1 1.439	1.254	1.148
85	A3	.30	.5d	1.47	5.38	5.93	.907
194	A3	.30	.5d	1.25	4.54	4.64	.978
195	A3	.30	.5d	1.01	3.64	3.84	.948
196	A3	.30	.5d	.60	1 2.160	2.226	.970
210	A7	.30	.5d	.20	1 .720	.743	.969
211	A3	.30	.5d	.20	1 .720	.702	1.026
392	A3	.30	.5d	.59	1 2.160	2.229	.969
393	A3	.30	.5d	1.48	5.35	5.67	.944
392a	A3	.30	.6d	.59	1 2.160	2.034	1.062
393a	A3	.30	.6d	1.48	5.35	5.19	1.031
92	A3	.40	.5d	1.49	7.17	7.58	.946
119	A3	.40	.5d	.20	1 .966	1.024	.943
125	A7	.40	.5d	.99	4.77	5.18	.921
193	A3	.40	.5d	.60	1 2.891	3.028	.955
92a	A3	.40	.6d	1.49	7.17	7.22	.993
119a	A3	.40	.6d	.20	1 .966	.962	1.004
125a	A7	.40	.6d	.99	4.77	4.89	.975
193a	A3	.40	.6d	.60	1 2.891	2.894	.999
94	A3	.50	.5d	1.49	8.98	9.41	.954
390	A3	.50	.5d	.20	1 1.203	1.207	.997
94a	A3	.50	.6d	1.49	8.98	9.26	.970
379	A3	.50	.6d	.99	6.01	6.01	1.000
380	A3	.50	.6d	.50	1 3.000	2.835	1.058
390a	A3	.50	.6d	.20	1 1.203	1.092	1.102
124	A7	.60	.5d	.20	1 1.440	1.529	.942
126	A7	.60	.5d	1.00	7.20	7.68	.938
127	A7	.60	.5d	1.50	10.78	11.40	.946
199	A3	.59	.5d	.20	1 1.440	1.257	1.146
201	A3	.60	.5d	.20	1 1.440	1.471	.979
202	A3	.60	.5d	.20	1 1.440	1.450	.993
203	A3	.59	.5d	.20	1 1.440	1.423	1.012
204	A7	.59	.5d	.20	1 1.440	1.489	.967
205	A3	.59	.5d	.20	1 1.440	1.475	.976
206	A7	.59	.5d	.20	1 1.440	1.505	.957
207	A3	.59	.5d	.20	1 1.440	1.480	.973
208	A7	.59	.5d	.20	1 1.440	1.496	.963
209	A3	.59	.5d	.20	1 1.440	1.475	.976
394	A3	.60	.5d	.50	3.60	3.86	.933
122	A3	.60	.6d	.20	1 1.440	1.360	1.059
124a	A7	.60	.6d	.20	1 1.440	1.501	.959
126a	A7	.60	.6d	1.00	7.20	7.44	.968
127a	A7	.60	.6d	1.50	10.78	11.03	.977
394a	A3	.60	.6d	.50	3.60	3.66	.984
121	A3	1.00	.5d	.20	1 2.405	2.448	.982
140	A7	1.00	.5d	1.51	18.14	18.73	.968
141	A7	1.00	.5d	1.00	12.03	12.51	.962
389	A3	1.00	.5d	.50	5.98	6.26	.955
121a	A3	1.00	.6d	.20	1 2.405	2.360	1.019
140a	A7	1.00	.6d	1.51	18.14	18.31	.991
141a	A7	1.00	.6d	1.00	12.03	12.11	.993
389a	A3	1.00	.6d	.50	5.98	5.96	1.003
121b	A3	1.00	.2d, .8d	.20	1 2.405	2.277	1.056
140b	A7	1.00	.2d, .8d	1.51	18.14	17.96	1.010
141b	A7	1.00	.2d, .8d	1.00	12.03	11.87	1.013
389b	A3	1.00	.2d, .8d	.50	5.98	5.85	1.022

1By 8-inch venturi meter.

Discharge measurements made with cup-type pygmy current meters—Continued

Flume bed covered with 3/4-inch gravel—Continued

Measure- ment No.	Meter No.	Depth of water (feet)	Method of measure- ment	Average velocity in flume (feet per second)	Total discharge (second-feet)		Coefficient for meter measure- ment
					Weir	Meter	
378a	A3	1.25	0.6d	0.50	7.51	7.64	0.983
398a	A3	1.25	.6d	1.00	14.96	15.30	.978
401a	A3	1.25	.6d	1.50	22.45	23.36	.961
378b	A3	1.25	.2d, .8d	.50	7.51	7.50	1.001
398b	A3	1.25	.2d, .8d	1.00	14.96	15.01	.997
401b	A3	1.25	.2d, .8d	1.50	22.45	22.30	1.007
137	A7	1.52	.6d	.30	5.42	5.51	.984
137a	A7	1.52	.6d	.30	5.42	5.42	1.000
395a	A3	1.50	.6d	1.50	26.92	27.79	.969
397a	A3	1.50	.6d	1.00	17.95	18.27	.982
399a	A3	1.50	.6d	.50	8.98	9.03	.994
400a	A3	1.50	.6d	1.50	26.97	27.89	.967
395b	A3	1.50	.2d, .8d	1.50	26.92	27.27	.987
397b	A3	1.50	.2d, .8d	1.00	17.95	17.81	1.008
399b	A3	1.50	.2d, .8d	.50	8.98	8.93	1.006
400b	A3	1.50	.2d, .8d	1.50	26.97	27.16	.993

Flume bed covered with coarse gravel

431	A3	0.40	0.5d	0.60	12.880	3.188	0.903
433	A3	.40	.5d	.40	11.918	2.027	.928
434	A3	.41	.5d	.20	1.961	.985	.976
443	A3	.40	.5d	.99	4.75	5.11	.930
444	A3	.40	.5d	1.49	7.14	7.61	.938
432	A3	.60	.5d	.40	12.880	3.082	.934
436	A3	.60	.5d	.20	11.440	1.502	.959
446	A3	.60	.5d	.99	7.14	7.68	.930
447	A3	.60	.5d	.59	4.28	4.72	.907
450	A3	.60	.5d	1.50	10.78	11.63	.927
432a	A3	.60	.6d	.40	12.880	2.852	1.010
436a	A3	.60	.6d	.20	11.440	1.368	1.053
446a	A3	.60	.6d	.99	7.14	7.32	.975
447a	A3	.60	.6d	.59	4.28	4.38	.977
450a	A3	.60	.6d	1.50	10.78	10.99	.981
435a	A3	1.00	.6d	.20	12.399	2.282	1.051
442a	A3	1.00	.6d	.40	4.77	4.82	.990
445a	A3	1.00	.6d	.60	7.14	7.26	.983
451a	A3	1.00	.6d	1.49	17.91	18.22	.983
452a	A3	1.00	.6d	1.00	11.94	12.10	.987
435b	A3	1.00	.2d, .8d	.20	12.399	2.252	1.065
442b	A3	1.00	.2d, .8d	.40	4.77	4.63	1.019
445b	A3	1.00	.2d, .8d	.60	7.14	7.24	.986
451b	A3	1.00	.2d, .8d	1.49	17.91	17.90	1.001
452b	A3	1.00	.2d, .8d	1.00	11.94	12.05	.991
437a	A3	1.25	.6d	.20	13.000	2.854	1.051
448a	A3	1.25	.6d	.40	5.98	5.99	.998
449a	A3	1.25	.6d	.59	8.92	9.19	.971
453a	A3	1.25	.6d	1.00	14.94	15.34	.974
454a	A3	1.25	.6d	1.50	22.45	23.14	.970
437b	A3	1.25	.2d, .8d	.20	3.000	2.782	1.078
448b	A3	1.25	.2d, .8d	.40	5.98	5.84	1.024
449b	A3	1.25	.2d, .8d	.59	8.92	9.05	.986
453b	A3	1.25	.2d, .8d	1.00	14.94	15.09	.990
454b	A3	1.25	.2d, .8d	1.50	22.45	22.76	.986

¹ By 8-inch venturi meter.

Discharge measurements made with Ott propeller-type current meter

Measurement No.	Depth of water (feet)	Method of measurement	Average velocity in flume (feet per second)	Total discharge (second-feet)		Coefficient for meter measurement	Flume bed
				Weir	Meter		
348	0.50	.6d	0.50	13.000	3.132	.958	Smooth concrete. Do. ¾-inch gravel. Do.
353	.50	.6d	1.00	6.01	.612	.982	
364	.50	.6d	1.00	6.01	.608	.988	
367	.50	.6d	.50	13.005	3.148	.955	
332	1.25	.6d	1.00	15.00	15.35	.977	Smooth concrete. Do. ¾-inch gravel.
341	1.25	.6d	.50	7.51	7.89	.952	
377	1.25	.6d	.50	7.51	7.81	.962	
336	1.25	.2d, .8d	1.00	15.02	15.19	.989	Smooth concrete. Do. ¾-inch gravel.
342	1.25	.2d, .8d	.50	7.51	7.65	.982	
374	1.25	.2d, .8d	.50	7.51	7.85	.957	

¹ By 8-inch venturi meter.

Discharge measurements made with propeller-type pygmy current meter

Measurement No.	Depth of water (feet)	Method of measurement	Average velocity in flume (feet per second)	Total discharge (second-feet)		Coefficient for meter measurement	Flume bed
				Weir	Meter		
86	0.30	.5d	1.48	5.40	6.08	.888	¾-inch gravel. Do.
95	.50	.5d	1.49	8.95	10.08	.888	
10	1.04	.6d	.96	12.0	12.2	.984	Smooth concrete. Do.
23a	1.50	.6d	1.26	22.6	23.2	.974	
95a	.50	.6d	1.49	8.95	9.70	.923	¾-inch gravel.
10a	1.04	.2d, .8d	.96	12.0	11.6	1.034	Smooth concrete.

RESULTS OF THE INVESTIGATION

The principal results of the investigation of the performance of current meters in water of shallow depth at the National Hydraulic Laboratory consist of a determination of coefficients to be applied as correction factors to current-meter measurements of velocities within the range of depths and velocities included in the scope of the investigation. Some conclusions also may be reached in regard to methods most suitable for use in current-meter measurements of velocity in shallow depths. Separate analyses have been made of data for the standard-size current meters and for the pygmy meters. The coefficients for use with standard-size current meters are shown by the diagrams in plates 7 to 10; those for pygmy meters, by the diagrams in plates 11 to 13.

STANDARD-SIZE CURRENT METERS

The coefficients for use with the standard-size small Price-type current meters that are shown in plates 7 to 10 apply to the methods commonly used in measurements of velocities in shallow depths where the meter is placed at middepth, at 0.6-depth, or at 0.2- and 0.8-

depth. Other methods that were investigated did not appear to give better or more consistent results than those.

As mentioned elsewhere in this report, there were some variations in the coefficients for individual measurements and some scattering of points when the coefficients were plotted. Two sets of curves were prepared preliminary to each diagram shown in plates 7 to 13. Velocities and coefficients were used as coordinates for one set of curves, and a separate curve was prepared for each depth at which measurements were made. The plotting of the points corresponding to the coefficients for the individual measurements under the different conditions of bed and by the different methods of measurement when plotted to show variations in coefficients with change in velocity are shown in plates 14, 16, 18, and 20. Depths and coefficients were used as coordinates for the other set of curves, and a separate curve was prepared for each velocity at which measurements were made. These curves plotted to show variations in coefficients with change in depth are shown in plates 15, 17, 19, and 21. The curves were drawn to average the results of the individual measurements so far as possible, and the two sets of curves necessarily were made consistent with each other. These curves were used as the bases of the diagrams shown in plates 7 to 13, one set of curves corresponding to horizontal sections through the diagram and the other set corresponding to vertical sections.

It may be seen from the curves in plates 14 and 16 that for the 0.5-depth method in depths of 0.2 and 0.3 foot, the coefficients for a bed covered with $\frac{3}{4}$ -inch gravel are considerably larger than the coefficients for a smooth concrete bed for the same velocities. This difference in coefficients for the 0.2-foot depth may possibly be due to a difference in the position of the current meter, as explained on page 7. Because of some variations in the coefficients obtained from different measurements made at the same depths and velocities and with the same conditions of bed, it is uncertain to what extent the different conditions of bed may have contributed to the small differences in coefficients, and to what extent the results may have been affected by minor differences in the position of the current meter, especially for those small depths where the current-meter cups were not completely submerged. Analyses of the data obtained from measurements made with standard-size current meters by the 0.5-depth method indicate, however, that when the 0.5-depth method is used the coefficients for use with smooth bed and with gravel bed should be different. The two sets of coefficients for the different conditions of smooth bed and $\frac{3}{4}$ -inch gravel bed are shown in plates 7 and 8.

For measurements made by the 0.6-depth and the 0.2- and 0.8-depth methods, there was no consistent difference in the results ob-

tained with the different conditions of flume bed for which the measurements were made, the variations in coefficients for measurements made under the same conditions being as much as or more than the variations in the results obtained for the different beds. No consistent differences were found in the coefficients for the two conditions of gravel bed that were investigated.

Although some inconsistencies exist in the results of individual measurements,⁶ the methods used in the analyses and interpretation of the data were such that the results of the investigation presented in the accompanying diagrams of coefficients should be representative of average relations between the actual discharge and the discharge as measured by current meters by the methods indicated. It is probable that the conditions of flow in the 12-foot flume in the laboratory were more favorable for current-meter measurements than the conditions that may be found in most natural channels of similar depths and comparable velocities.

PYGMY CURRENT METERS

The coefficients for measurements made with pygmy current meters by the 0.5-depth method shown in plate 11 are average values for the three conditions of bed, as also are those for the 0.6-depth and the 0.2- and 0.8-depth methods shown in plates 12 and 13. It is possible that a larger number of measurements with the pygmy meters might indicate some differences in the coefficients for the 0.5-depth method with the different conditions of bed so that separate curves might be developed that would agree with the observational data somewhat more closely than the average values that have been used. The plotting of the points corresponding to the coefficients for the individual measurements made with the pygmy current meters under the different conditions of bed and by the different methods of measurement are shown in plates 22 to 27.

Measurements made with the pygmy current meters were much fewer than those made with the standard-size current meters, for the pygmy meters were included within the scope of this investigation in order to obtain general information regarding their operation under the adverse conditions of very shallow water.

APPLICATION OF COEFFICIENTS

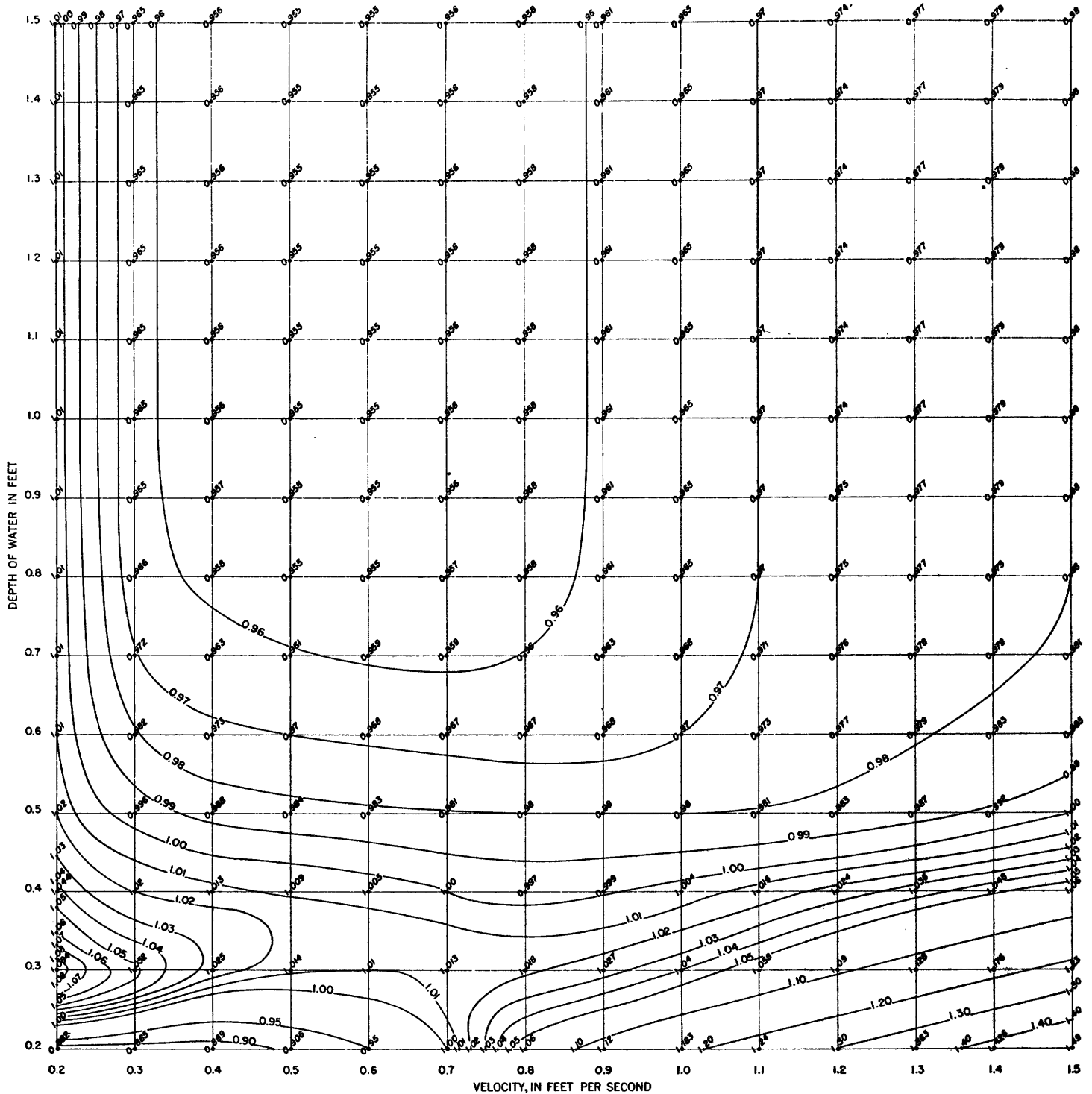
The coefficients shown in the diagrams, plates 7 to 13, apply to those methods commonly used in current-meter measurements of velocity of water in shallow depths. It is generally considered desirable to select a method of measurement in which the current meter

⁶ Subsequent to the completion of this investigation a water-stage recorder with a time scale of 3.6 inches = 1 hour and a height scale of 1 inch = 0.2 foot was temporarily installed on the flume. The records obtained by this instrument indicated considerable variation in the flow of water at the measuring section.

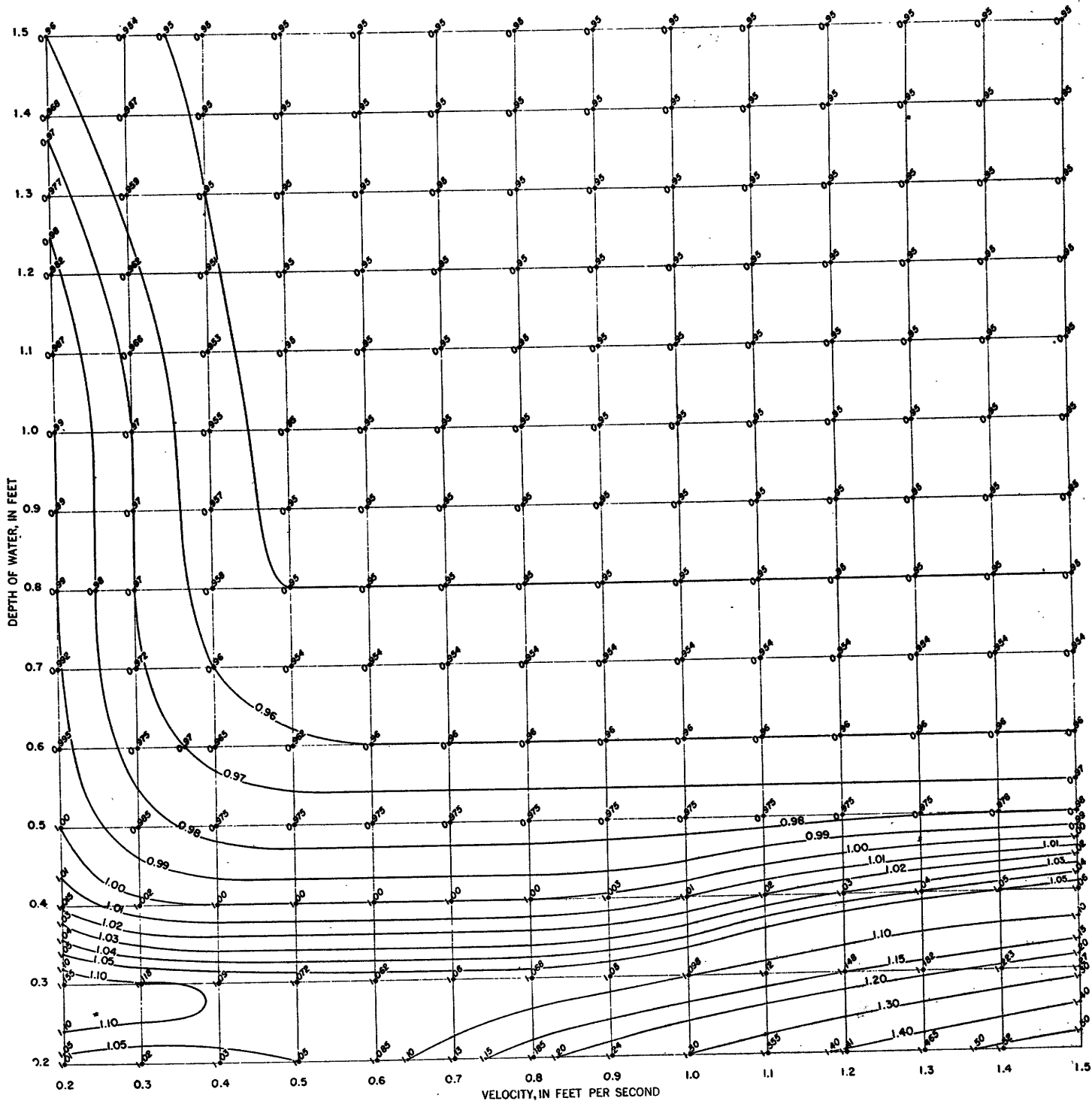
is placed in a position or positions in the vertical where the velocity indicated by the meter in the method used for the measurement is the same as the mean velocity in the vertical, or as nearly the same as may be obtained. In other words, it is desirable to keep the coefficient as near unity as possible by selection of the most suitable method, if that method is equally reliable in other respects.

The diagrams indicate that for depths between 0.5 foot and 1.5 feet the coefficients are nearer unity for the 0.6-depth method than for either of the other methods, and that for depths less than 0.5 foot the coefficients are nearer unity for the 0.5-depth method. The greater reliability of the 0.2- and 0.8-depth method in natural streams where the conditions of bed and the distribution of velocities are more variable than those found in the laboratory, however, might warrant the use of that method for depths between 1.0 foot and 1.5 feet, as well as for greater depths, even though the coefficients to be used with the 0.2- and 0.8-depth method are not so near unity as those for the 0.6-depth method.

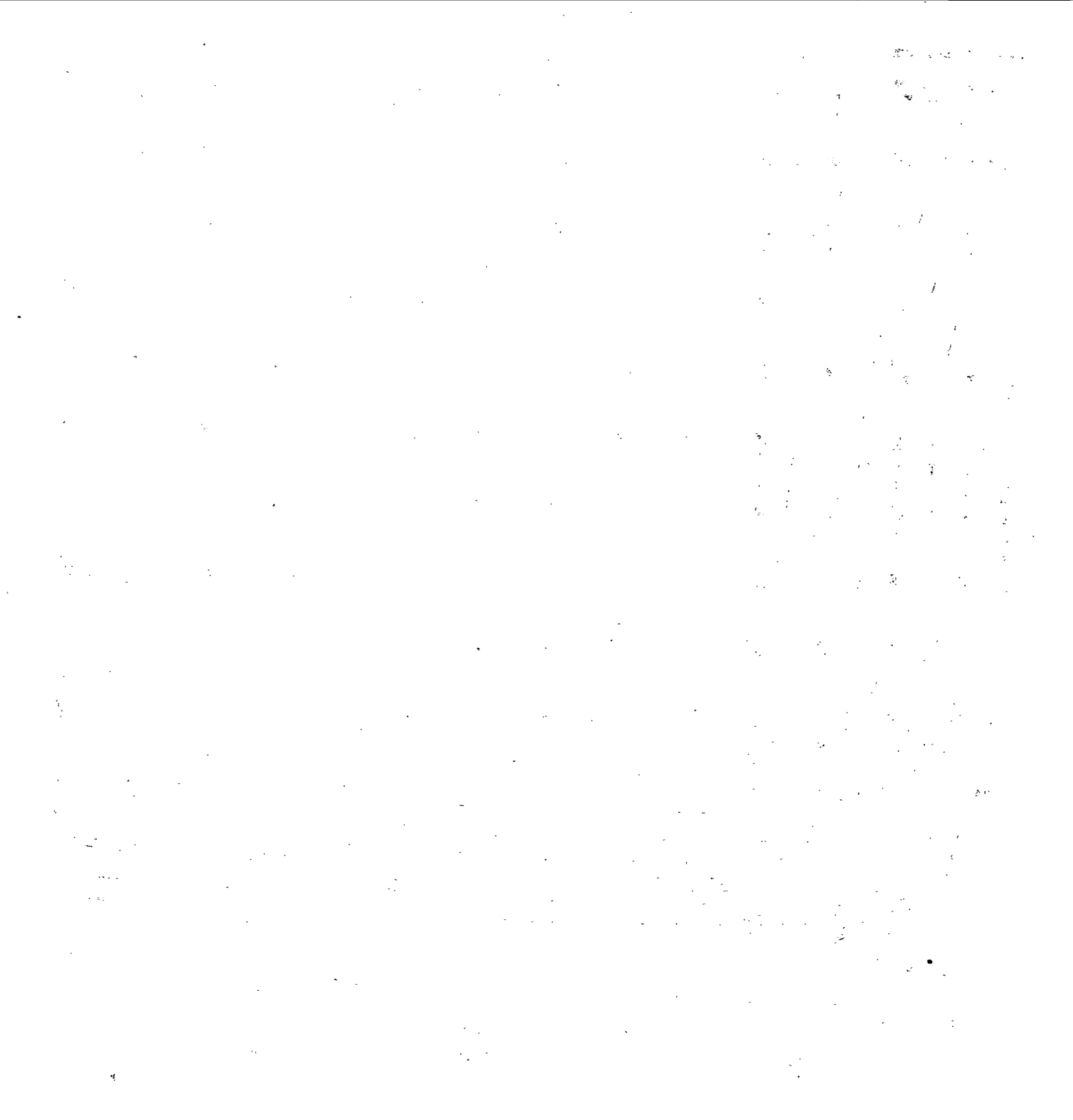
It is desirable, so far as possible, to select measuring sections where the velocity is not less than 0.4 foot per second. If the variations in the depth at a measuring station are not too great, the use of the same method for the entire section may be preferable. The coefficient to be applied to the velocity observation at each measuring point should be selected in accordance with the method of measurement and the depth and velocity at the point of observation.

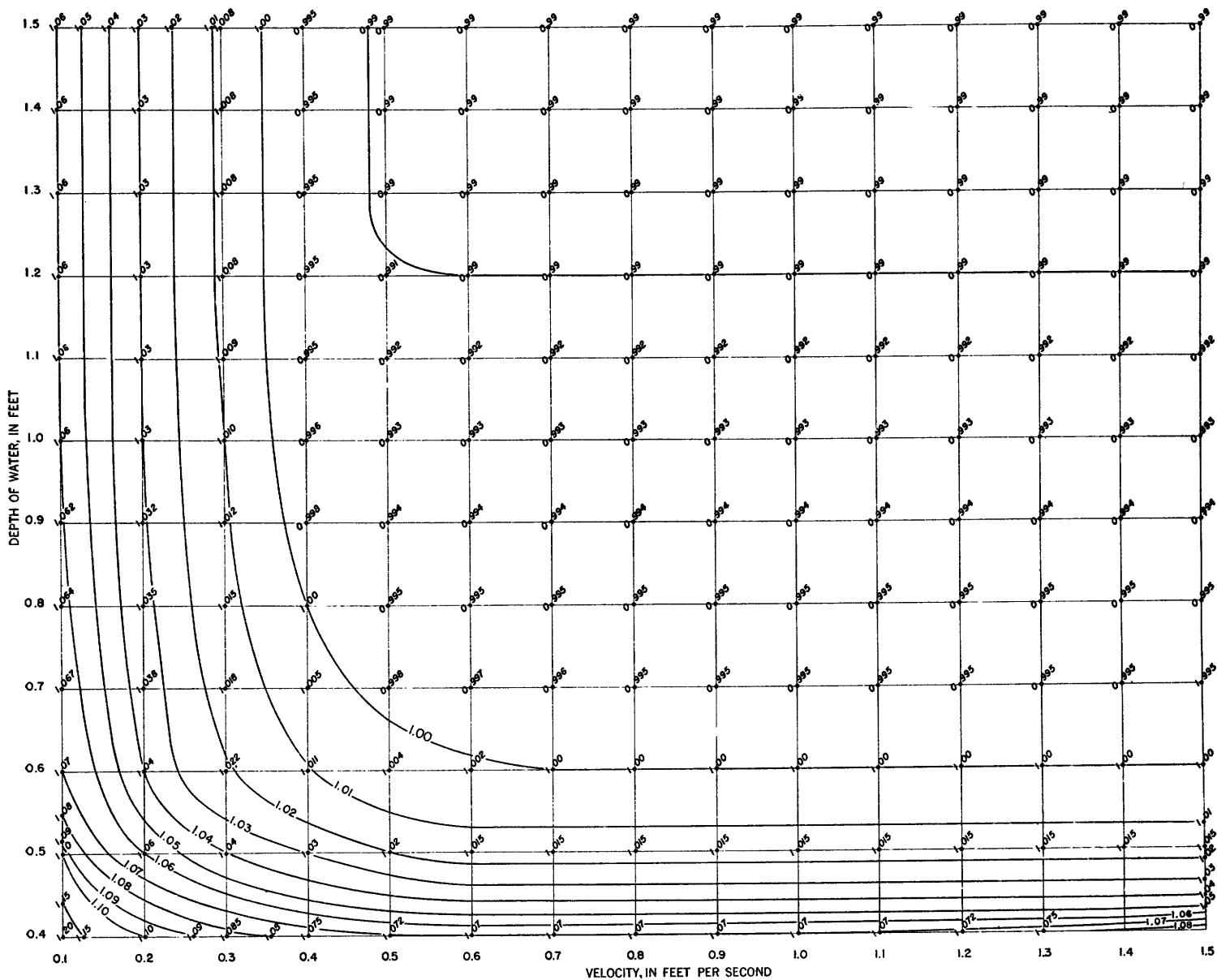


COEFFICIENTS FOR USE WITH 0.5-DEPTH METHOD, STANDARD-SIZE CURRENT METERS, SMOOTH BED.

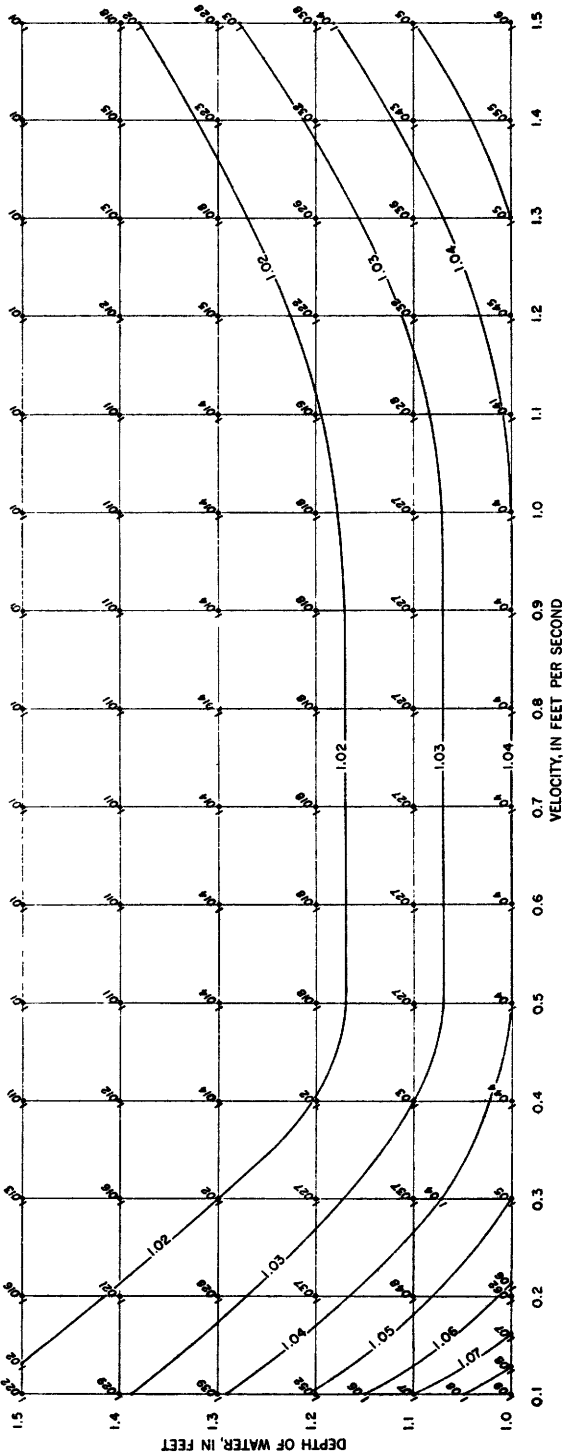


COEFFICIENTS FOR USE WITH 0.5-DEPTH METHOD, STANDARD-SIZE CURRENT METERS, BED OF 3/4-INCH GRAVEL.

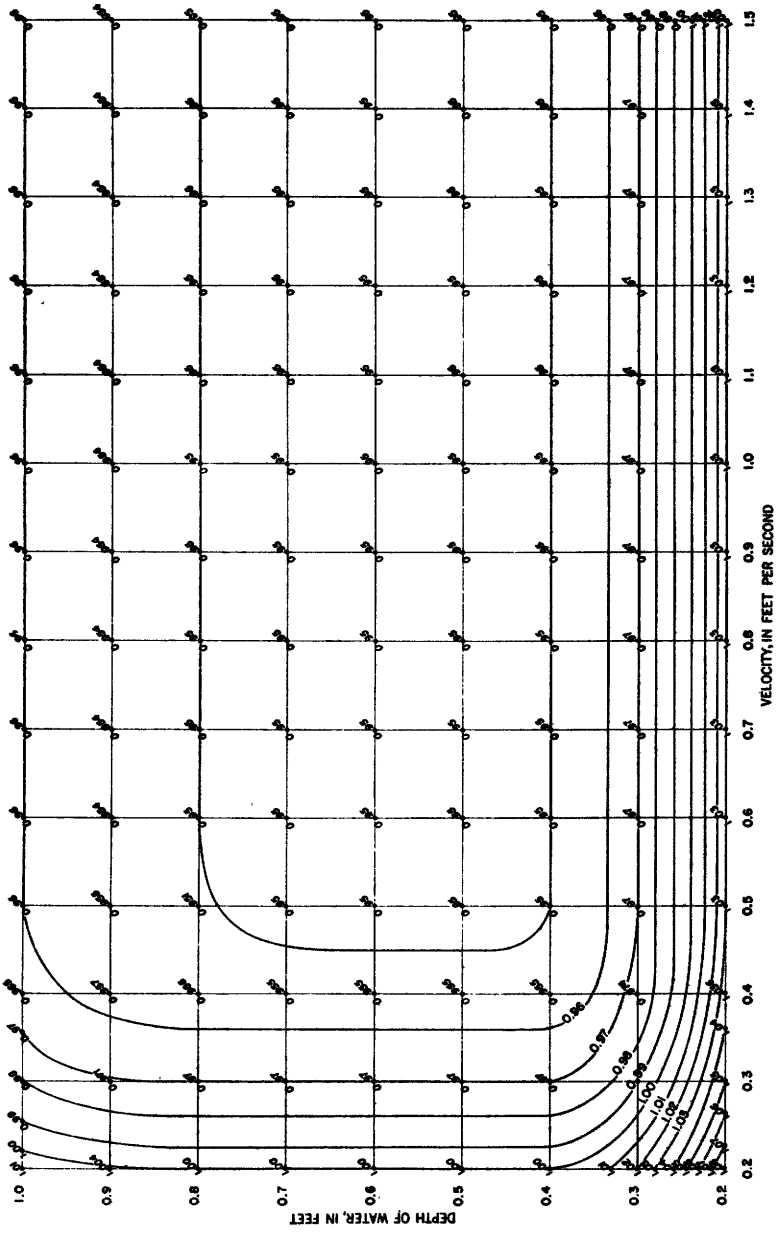




COEFFICIENTS FOR USE WITH 0-6-DEPTH METHOD, STANDARD-SIZE CURRENT METERS.

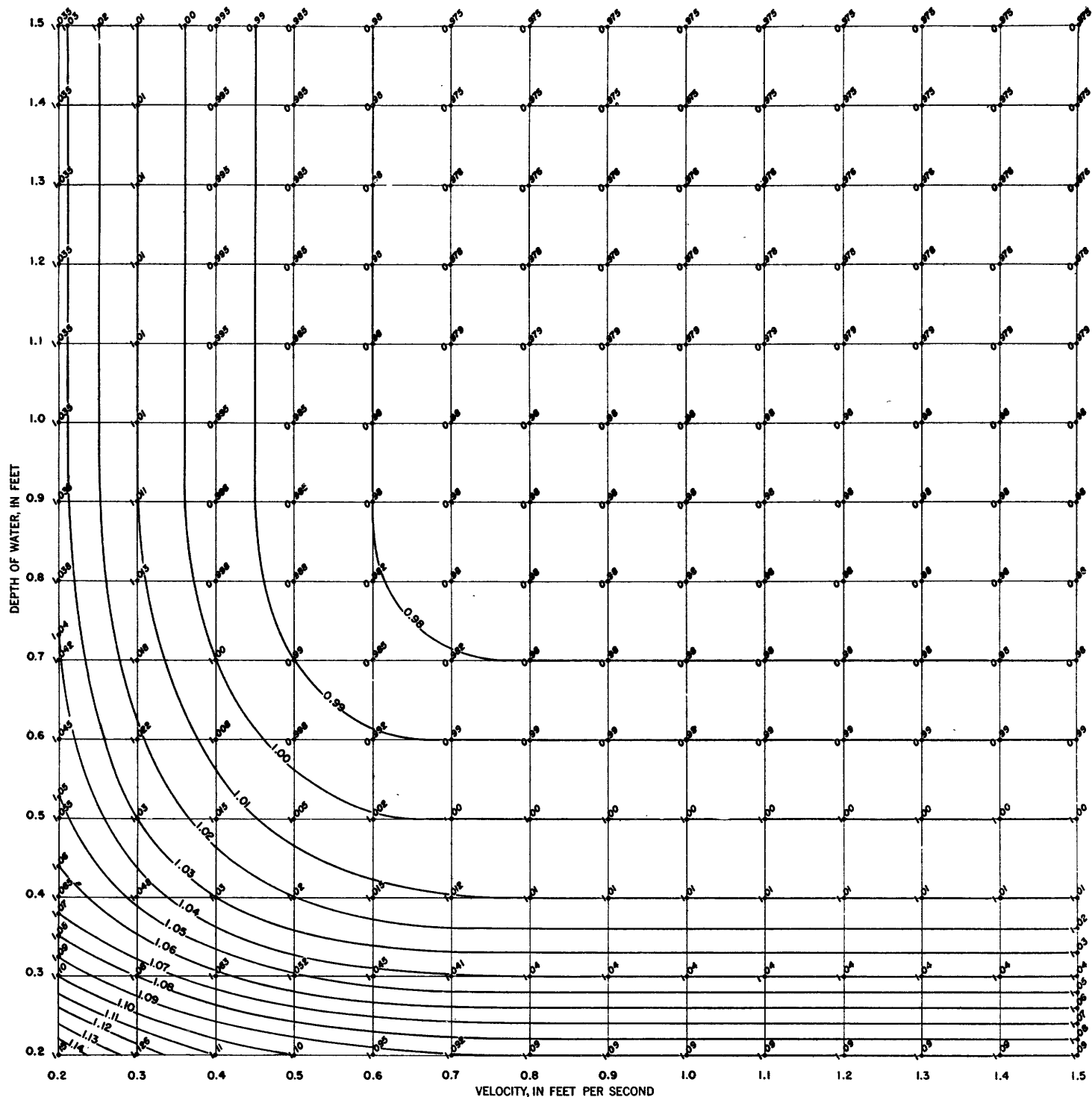


COEFFICIENTS FOR USE WITH 0.2- AND 0.8-DEPTH METHOD, STANDARD-SIZE CURRENT METERS.

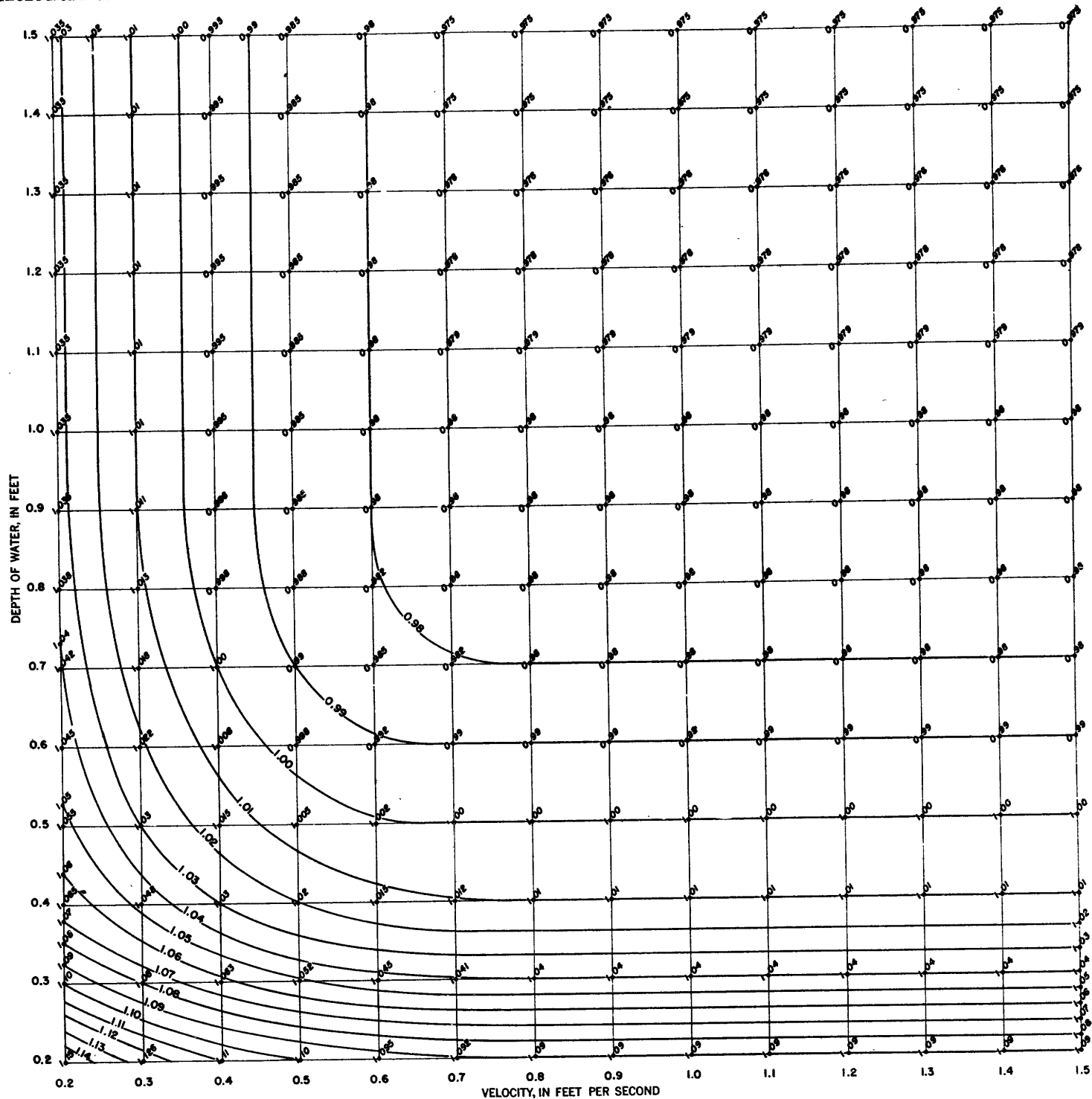


COEFFICIENTS FOR USE WITH 0.5-DEPTH METHOD, PYGMY CURRENT METERS.

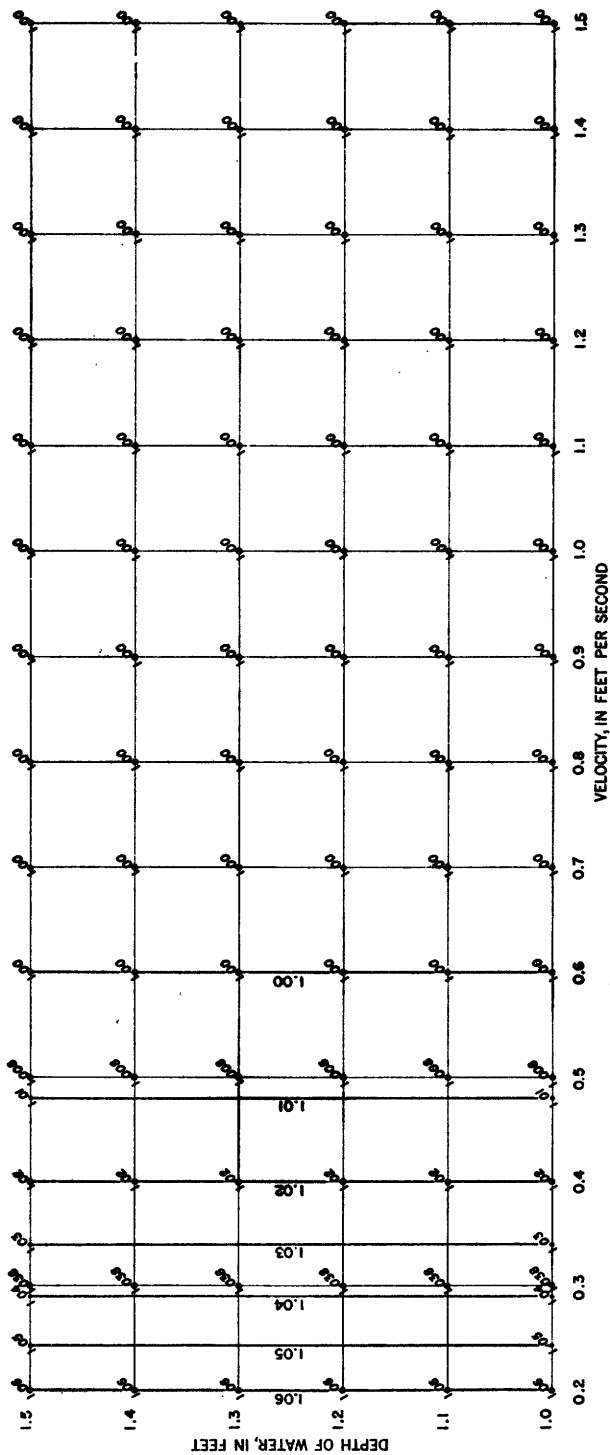




COEFFICIENTS FOR USE WITH 0.6-DEPTH METHOD, PYGMY CURRENT METERS.

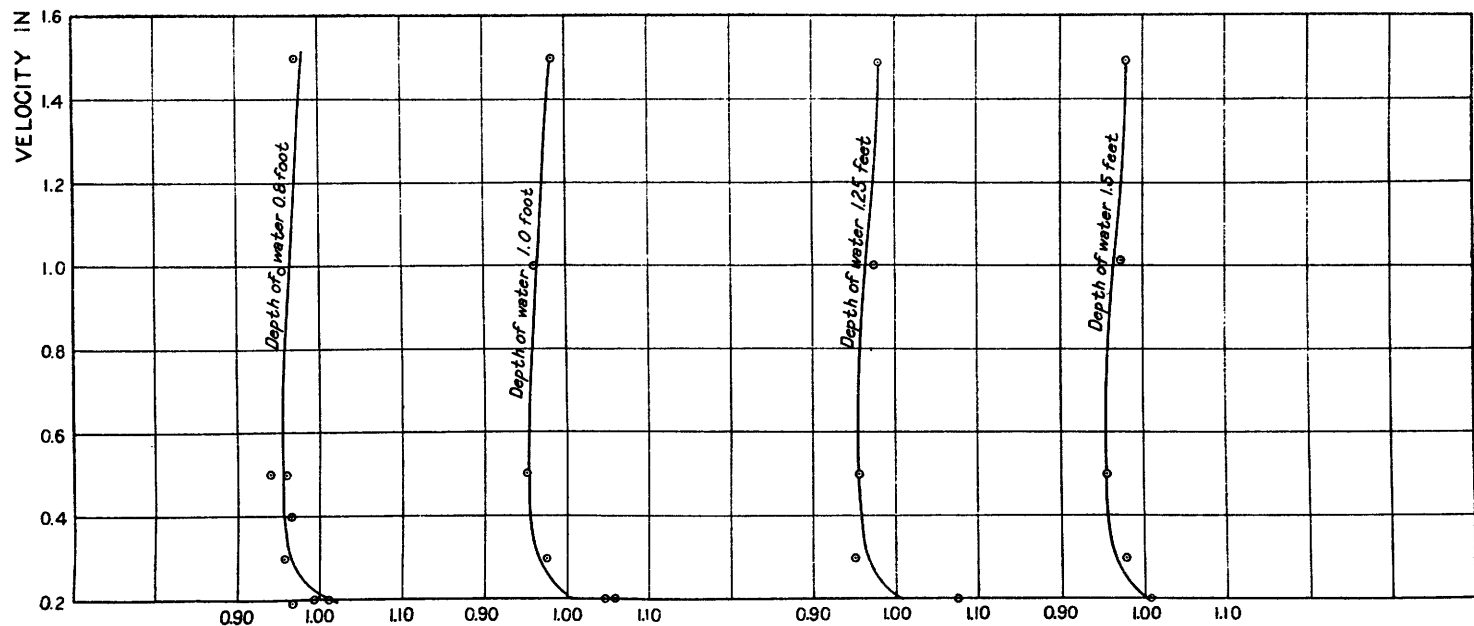
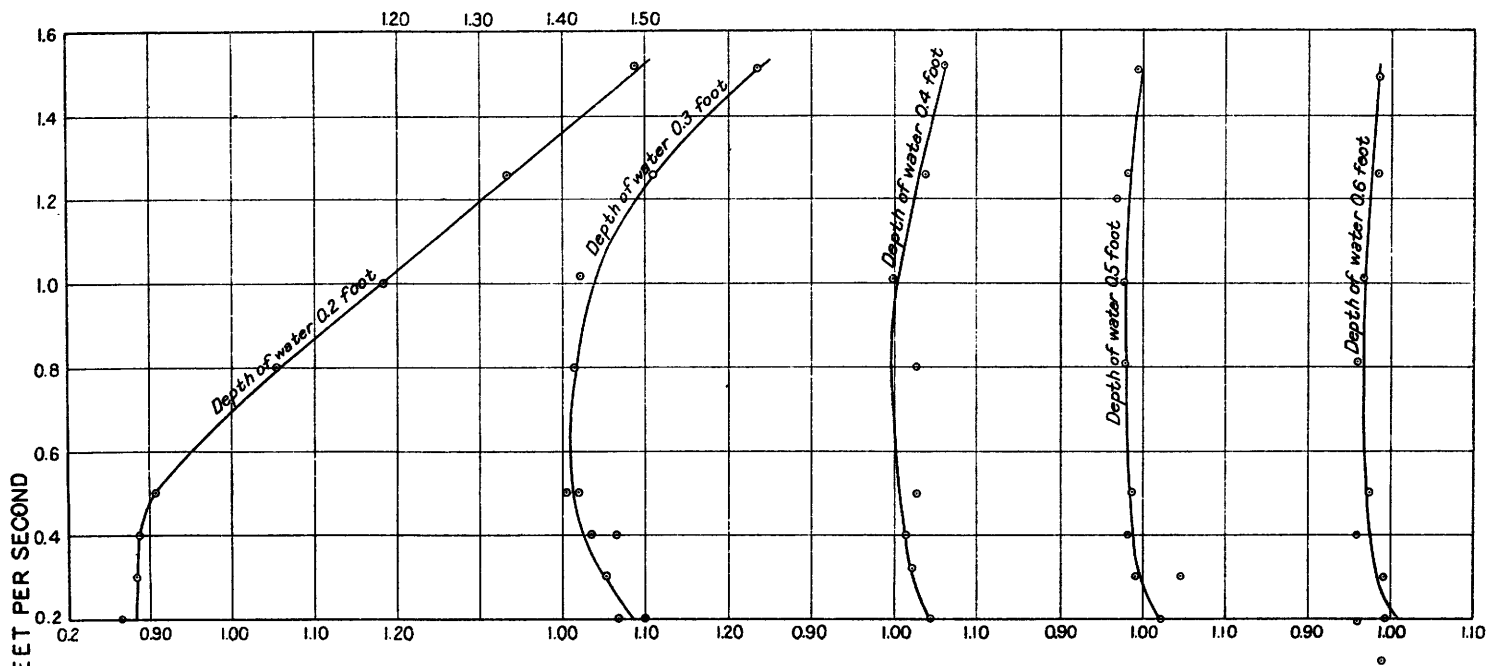


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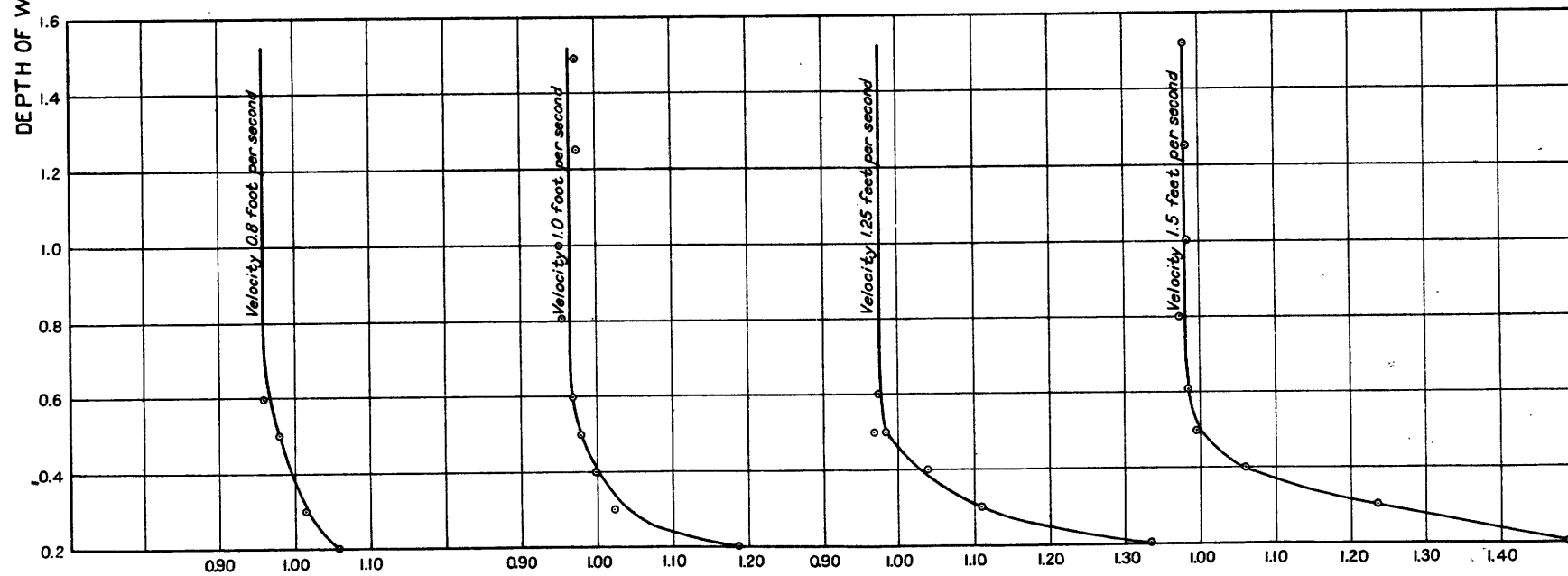
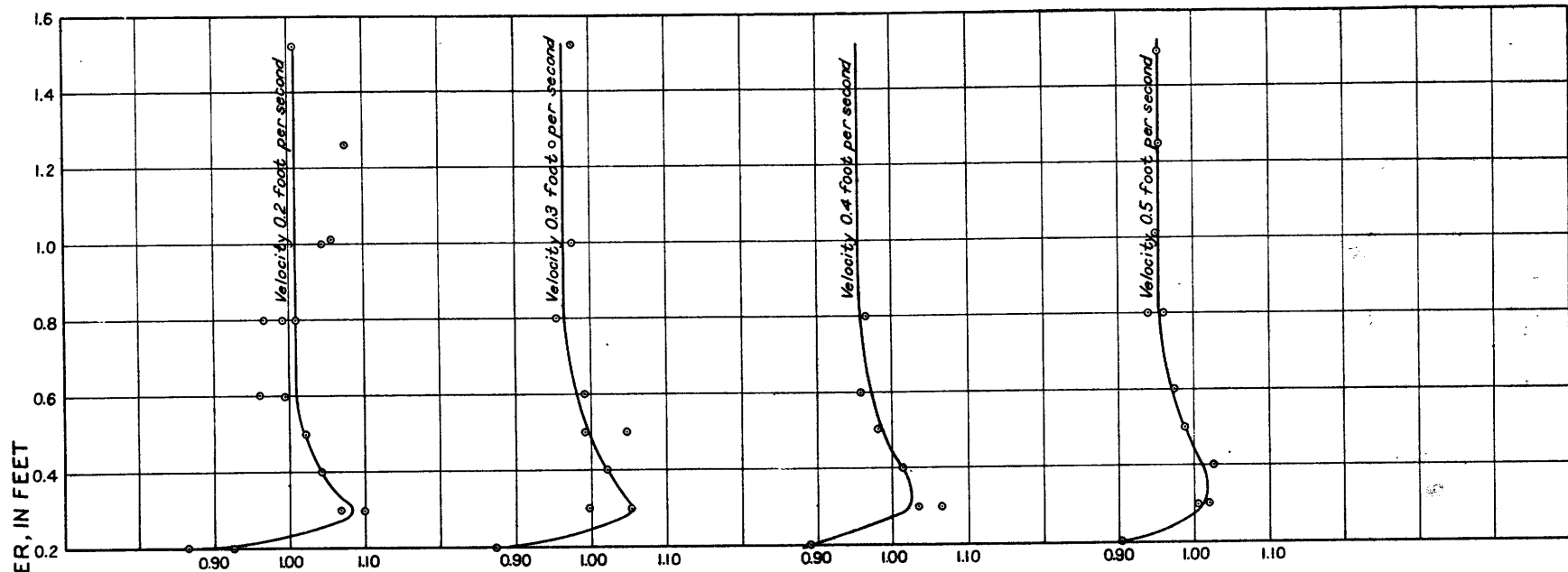


COEFFICIENTS FOR USE WITH 0.2- AND 0.8-DEPTH METHOD, PYGMY CURRENT METERS.





VARIATION OF COEFFICIENTS WITH CHANGE IN VELOCITY, SMOOTH BED, 0.5-DEPTH METHOD, SMALL PRICE-TYPE CURRENT METERS.



VARIATION OF COEFFICIENTS WITH CHANGE IN DEPTH, SMOOTH BED, 0.5-DEPTH METHOD, SMALL PRICE-TYPE CURRENT METERS.

THE HISTORY OF THE UNITED STATES

CHAPTER I

SECTION I

ARTICLE I

SECTION I

ARTICLE I

SECTION I

ARTICLE I

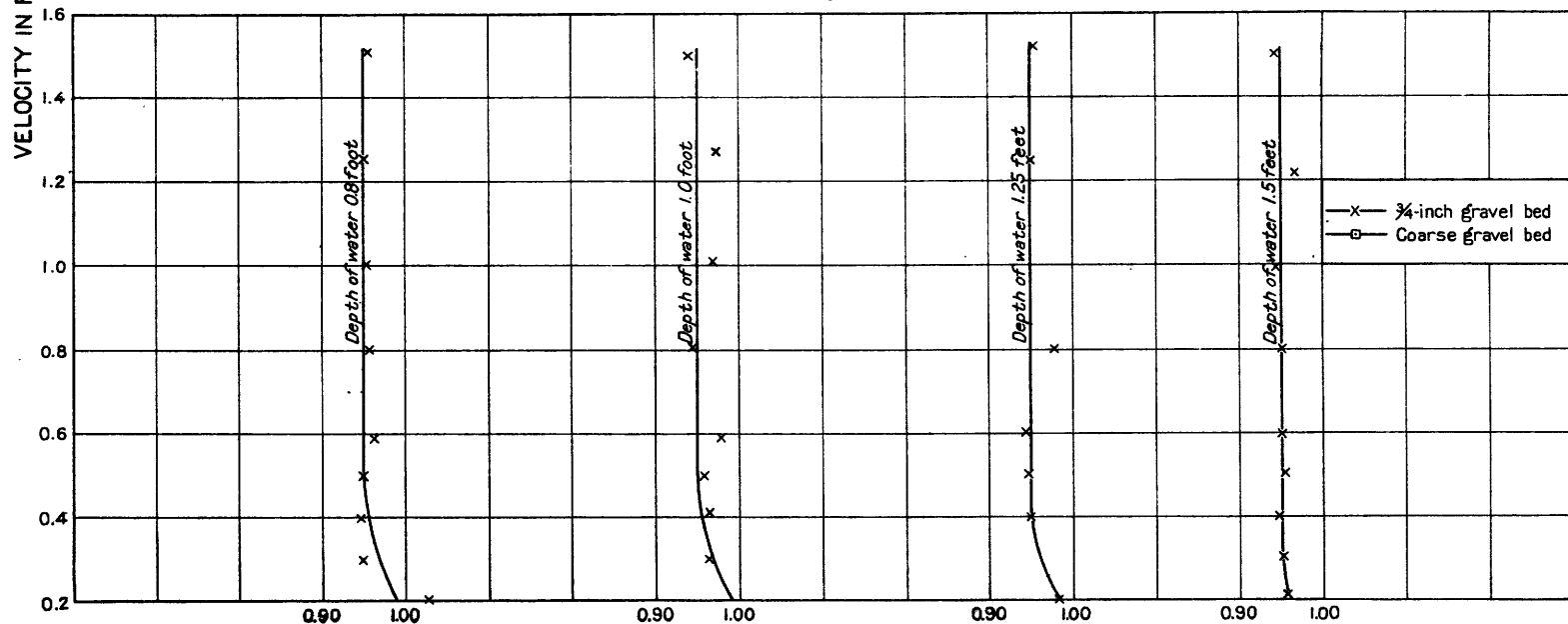
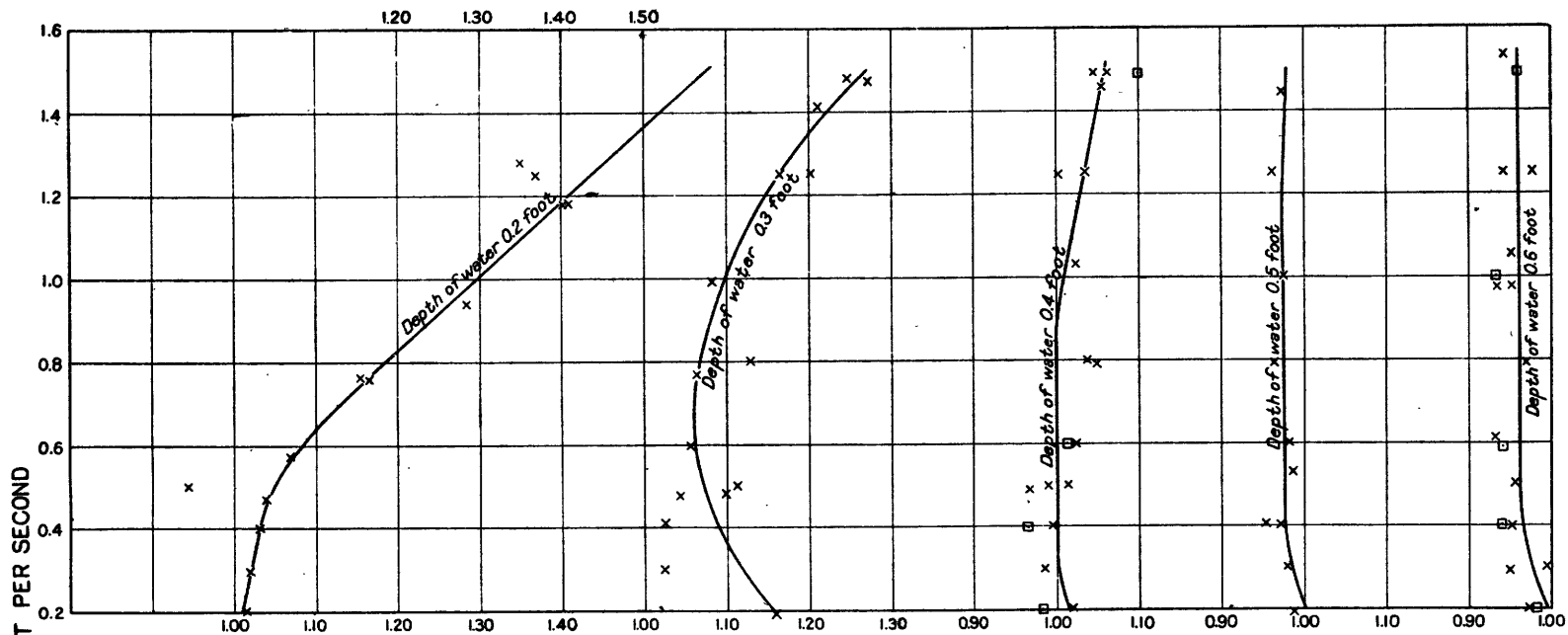
SECTION I

ARTICLE I

SECTION I

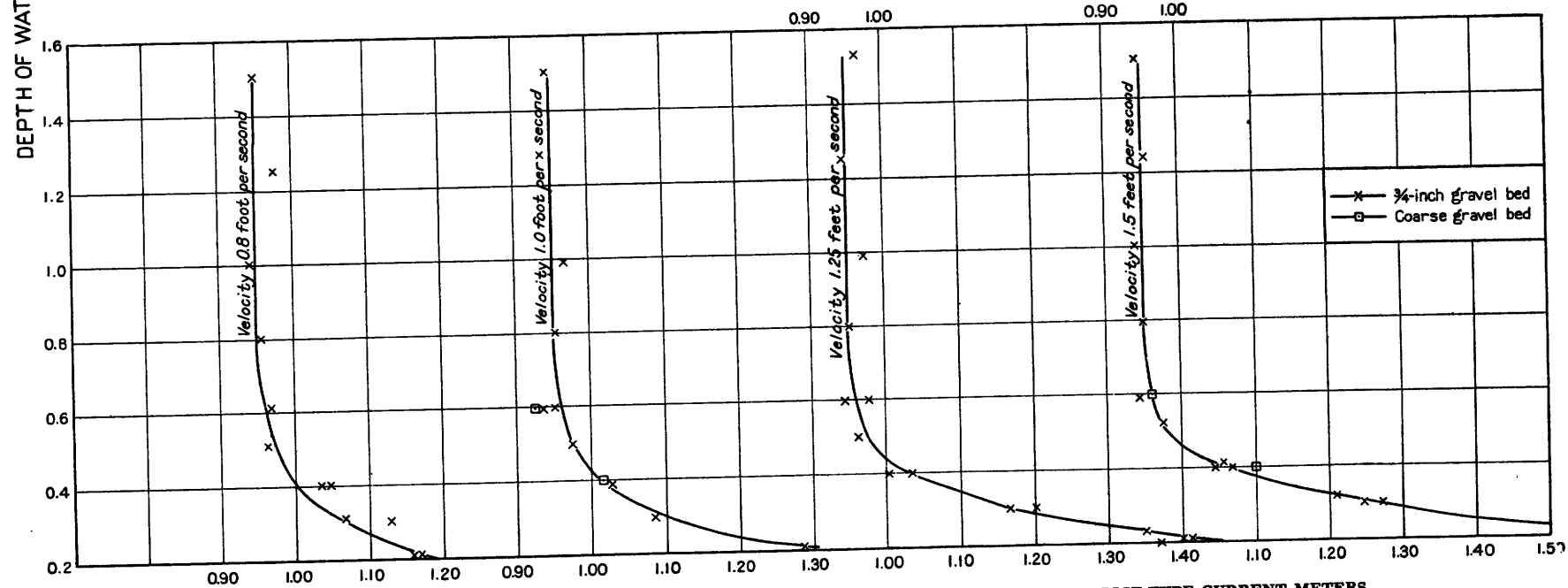
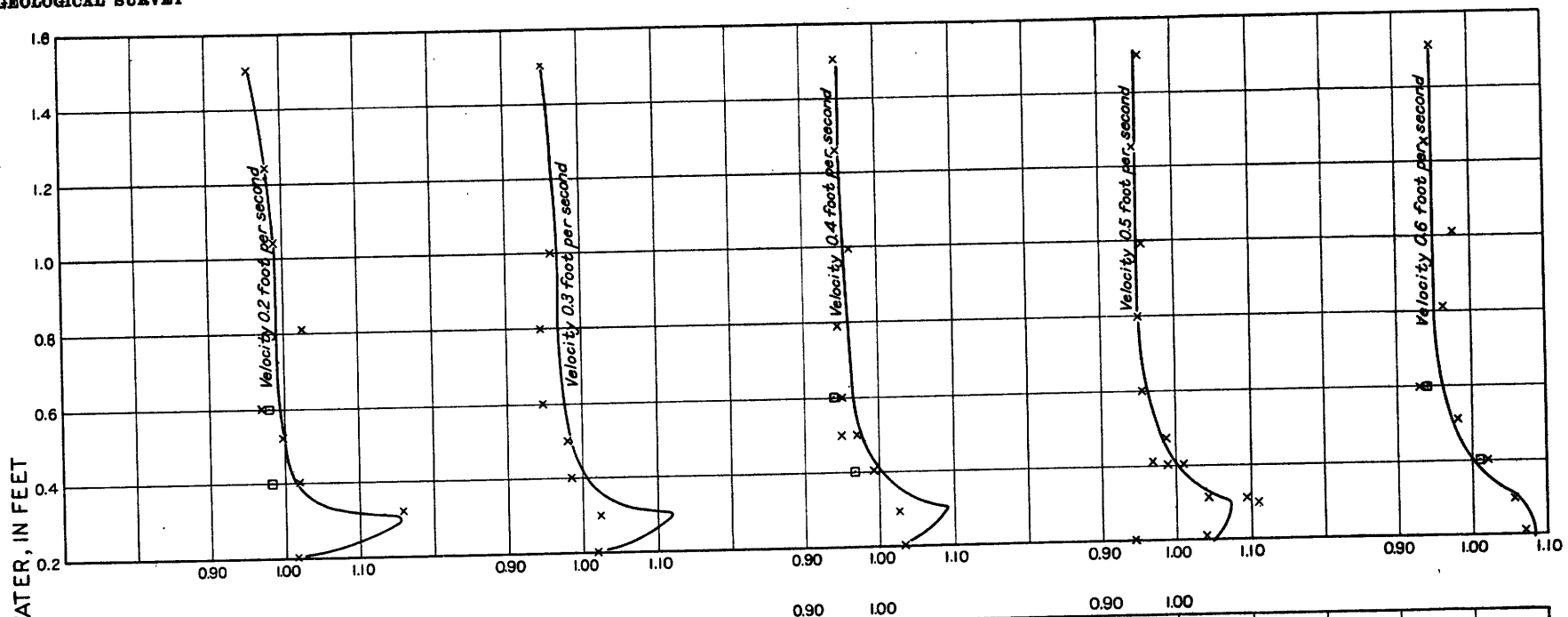
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SECTION I

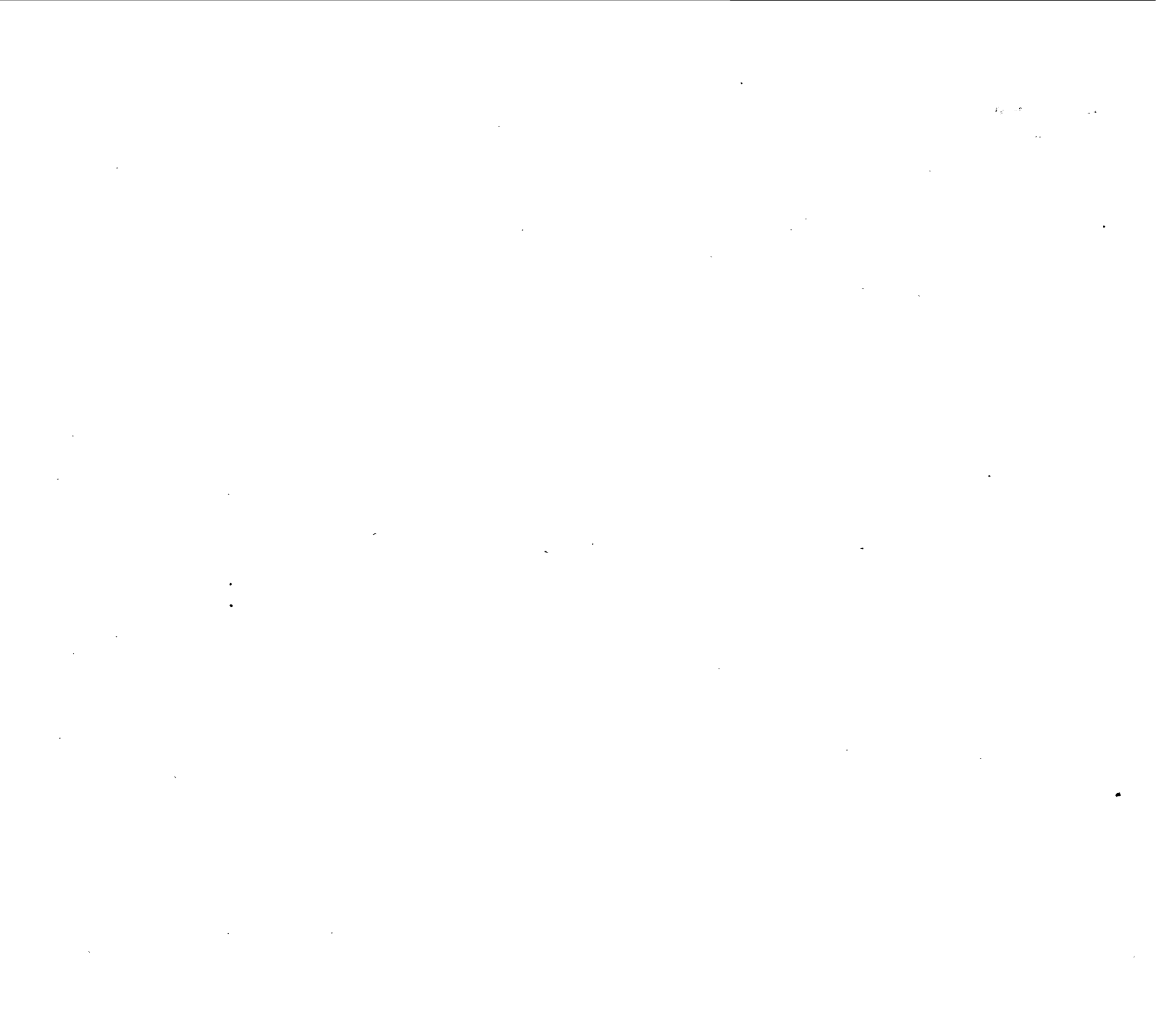


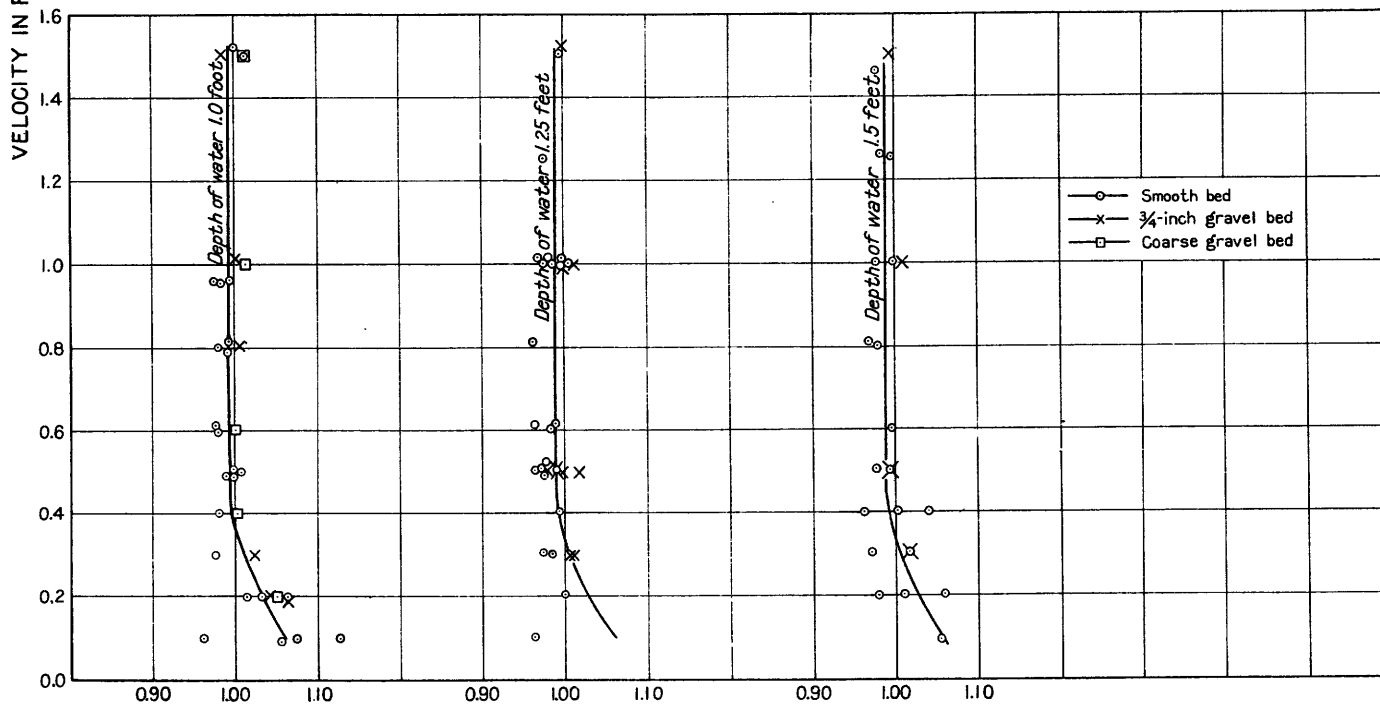
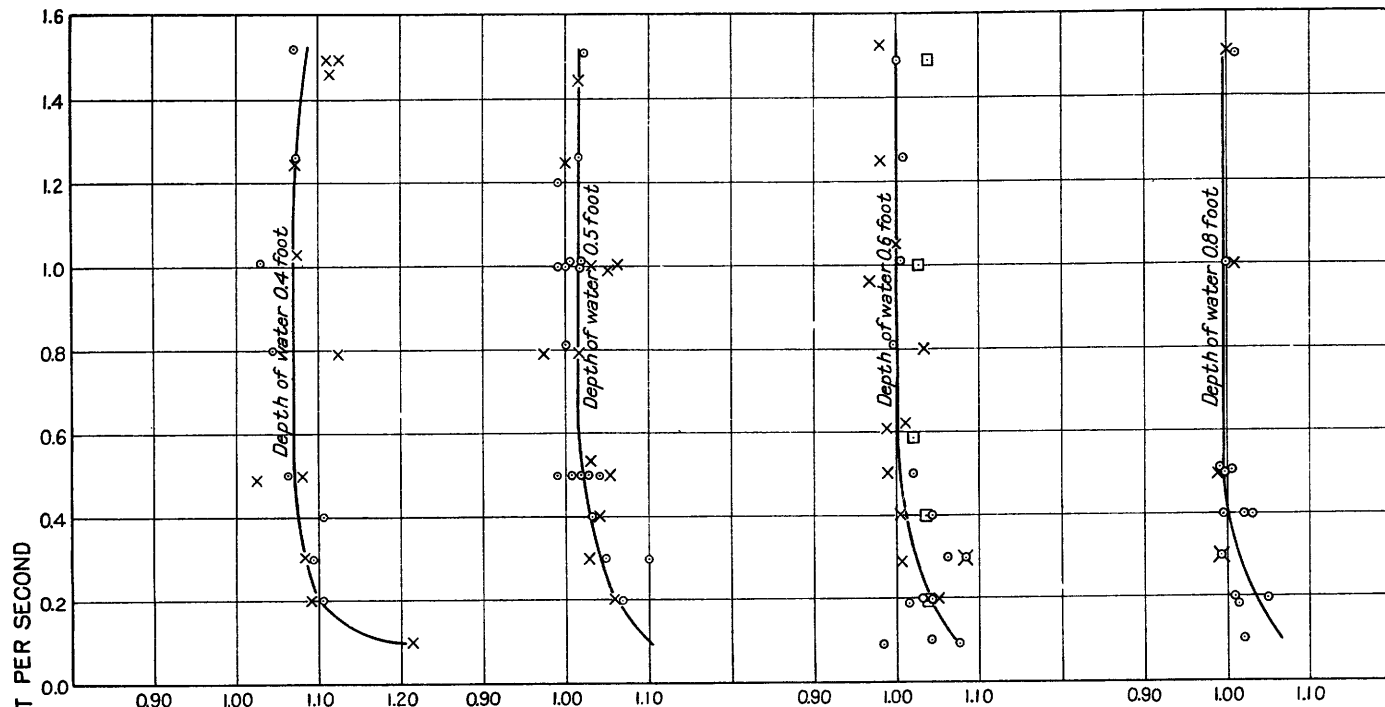
VARIATION OF COEFFICIENTS WITH CHANGE IN VELOCITY, GRAVEL BED, 0.5-DEPTH METHOD, SMALL PRICE-TYPE CURRENT METERS.

GEOLOGICAL SURVEY

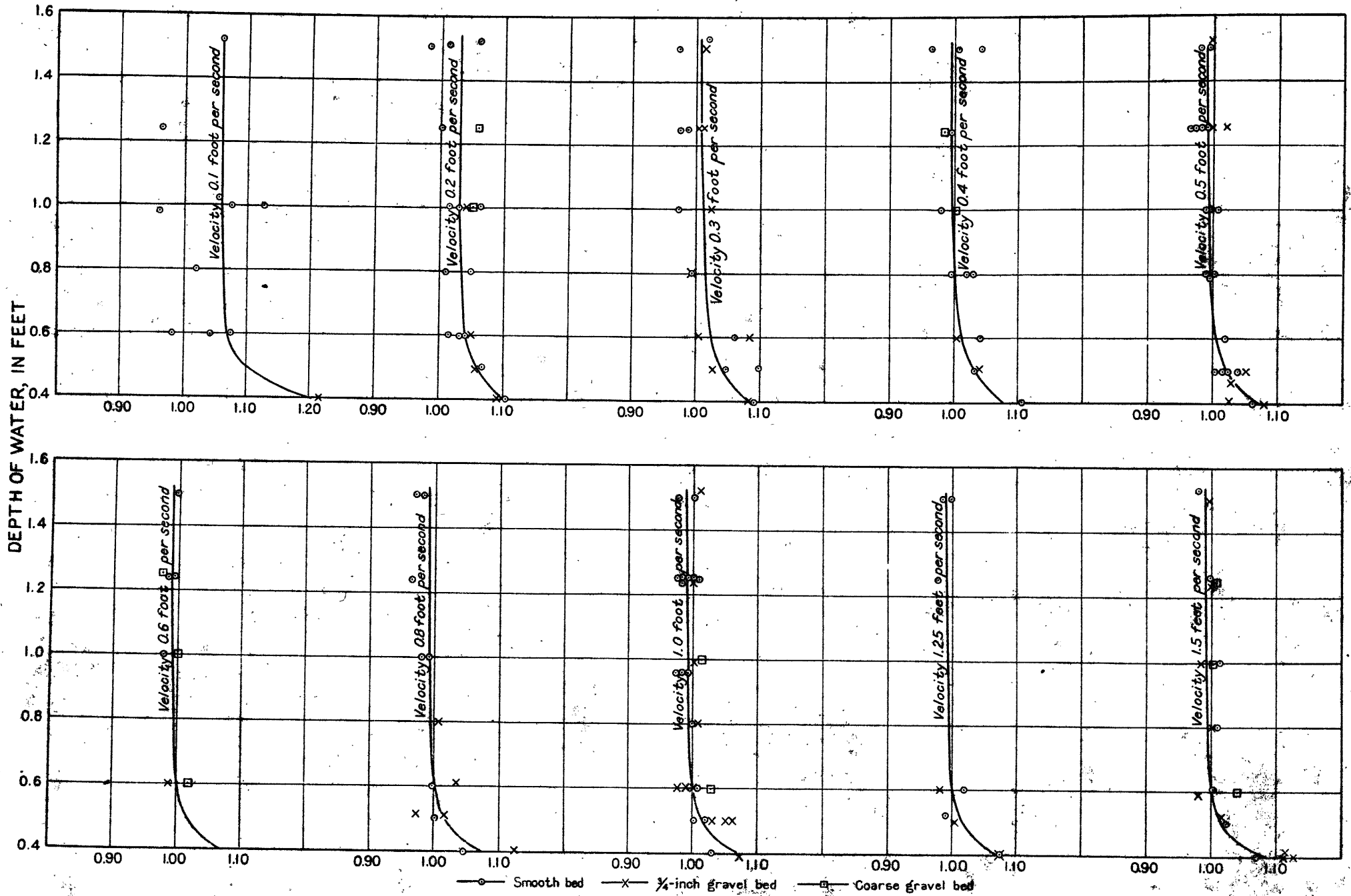


VARIATION OF COEFFICIENTS WITH CHANGE IN DEPTH, GRAVEL BED, 0.5-DEPTH METHOD, SMALL PRICE-TYPE CURRENT METERS.

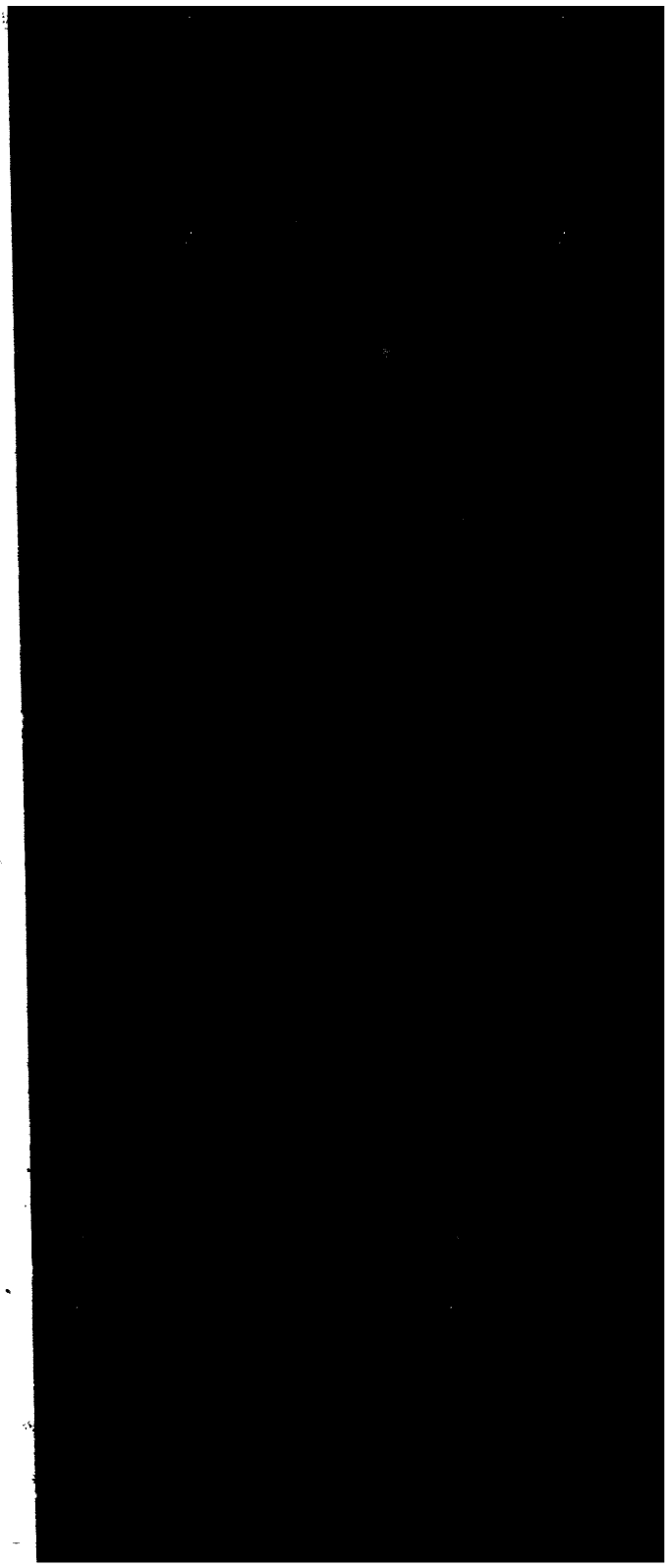
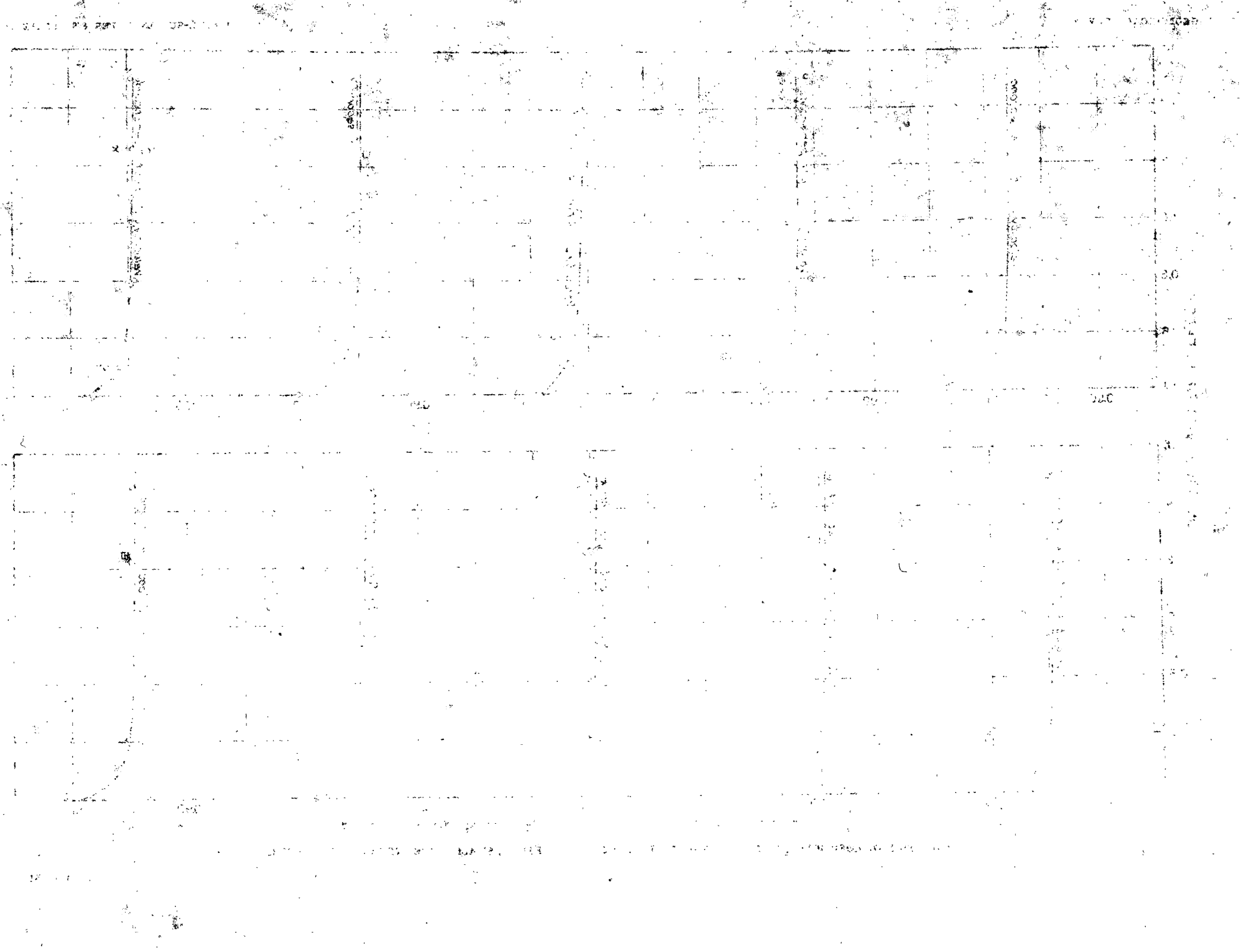


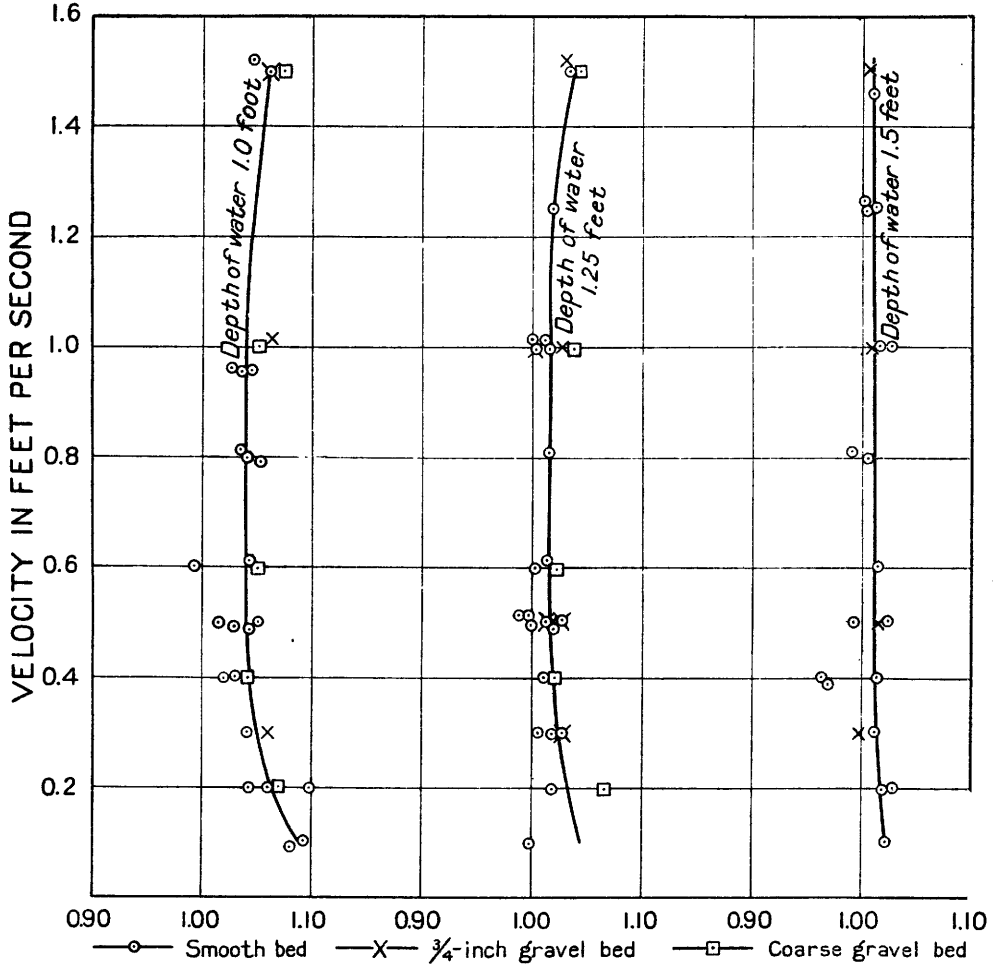


VARIATION OF COEFFICIENTS WITH CHANGE IN VELOCITY, 0.6-DEPTH METHOD, SMALL PRICE-TYPE CURRENT METERS.

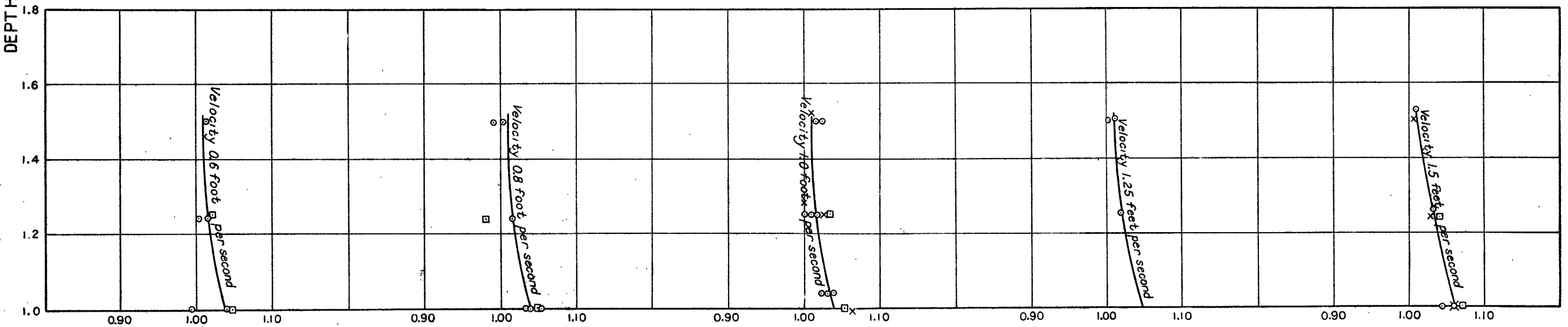
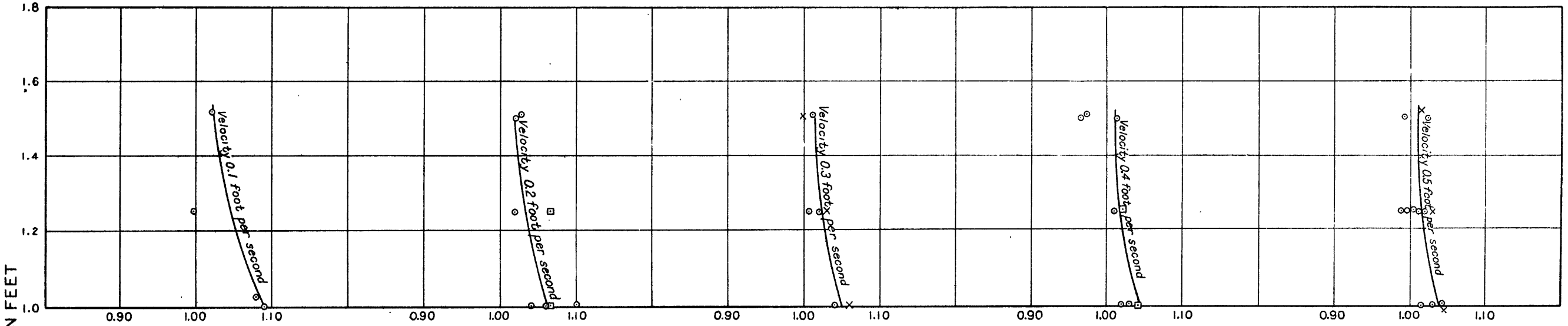


VARIATION OF COEFFICIENTS WITH CHANGE IN DEPTH, 0.4-DEPTH METHOD, SMALL PRICE-TYPE CURRENT METERS.





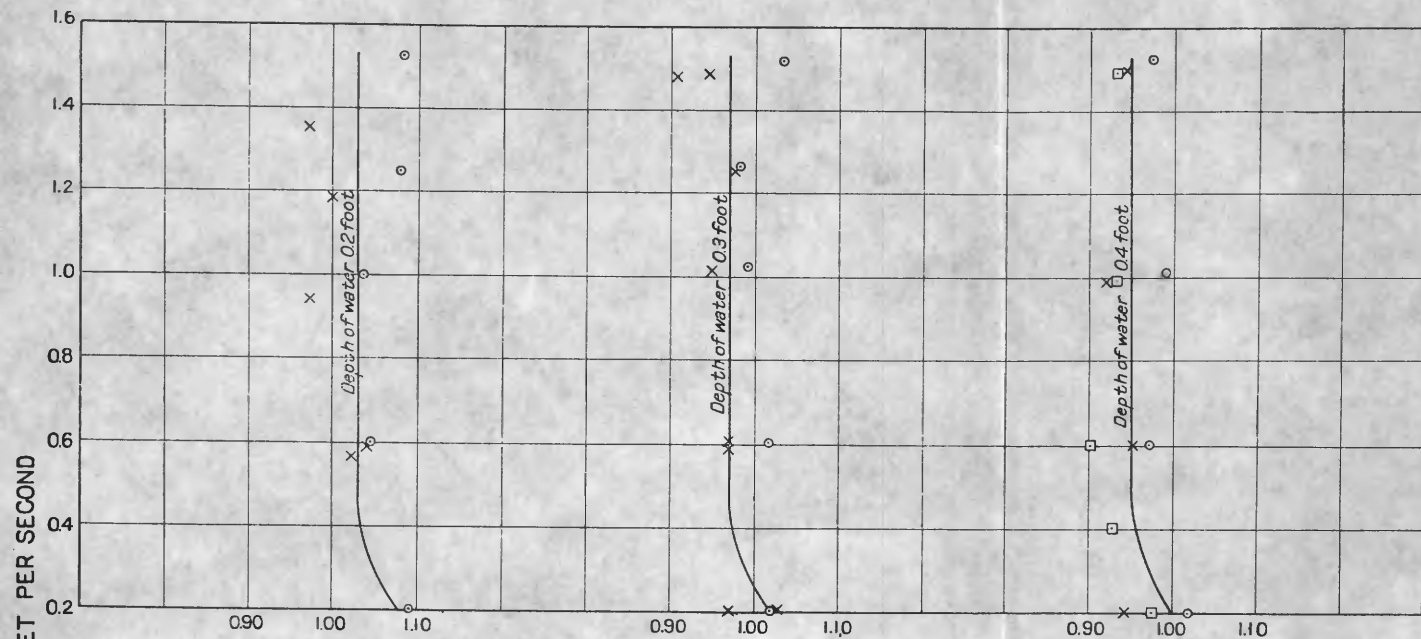
VARIAION OF COEFFICIENTS WITH CHANGE IN VELOCITY, 0.2- AND 0.8-DEPTH METHOD, SMALL PRICE-TYPE CURRENT METERS.



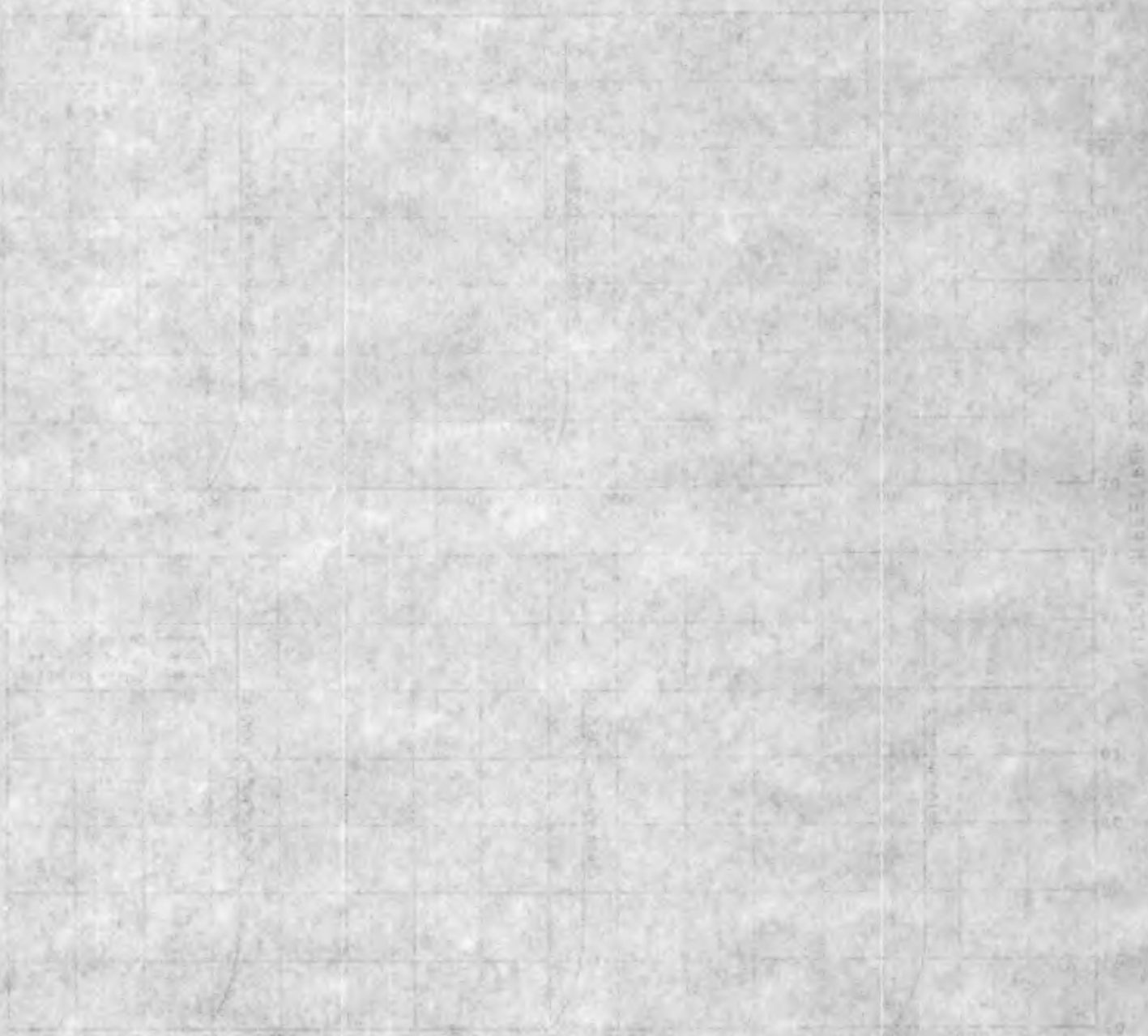
○ Smooth bed x 3/4-inch gravel bed □ Coarse gravel bed

VARIATION OF COEFFICIENTS WITH CHANGE IN DEPTH, 0.2- AND 0.8-DEPTH METHOD, SMALL PRICE-TYPE CURRENT METERS.

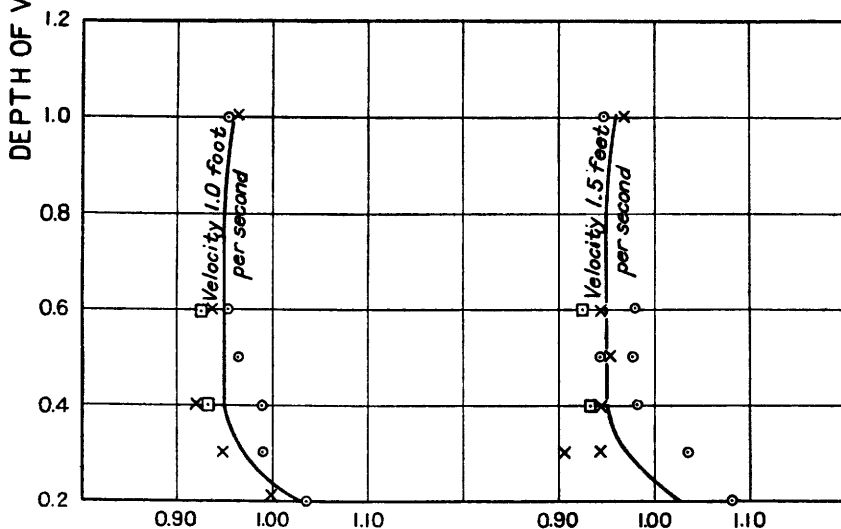
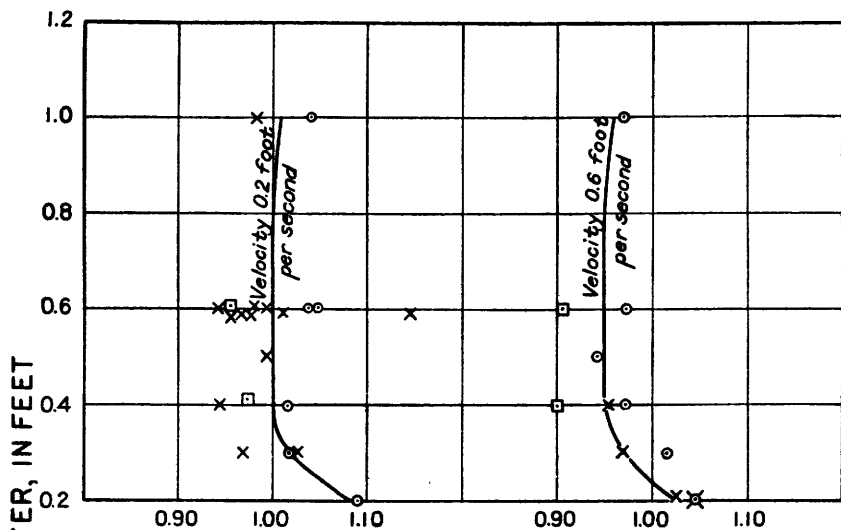
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VARIATION OF COEFFICIENTS WITH CHANGE IN VELOCITY, 0.5-DEPTH METHOD, PYGMY CURRENT METERS.

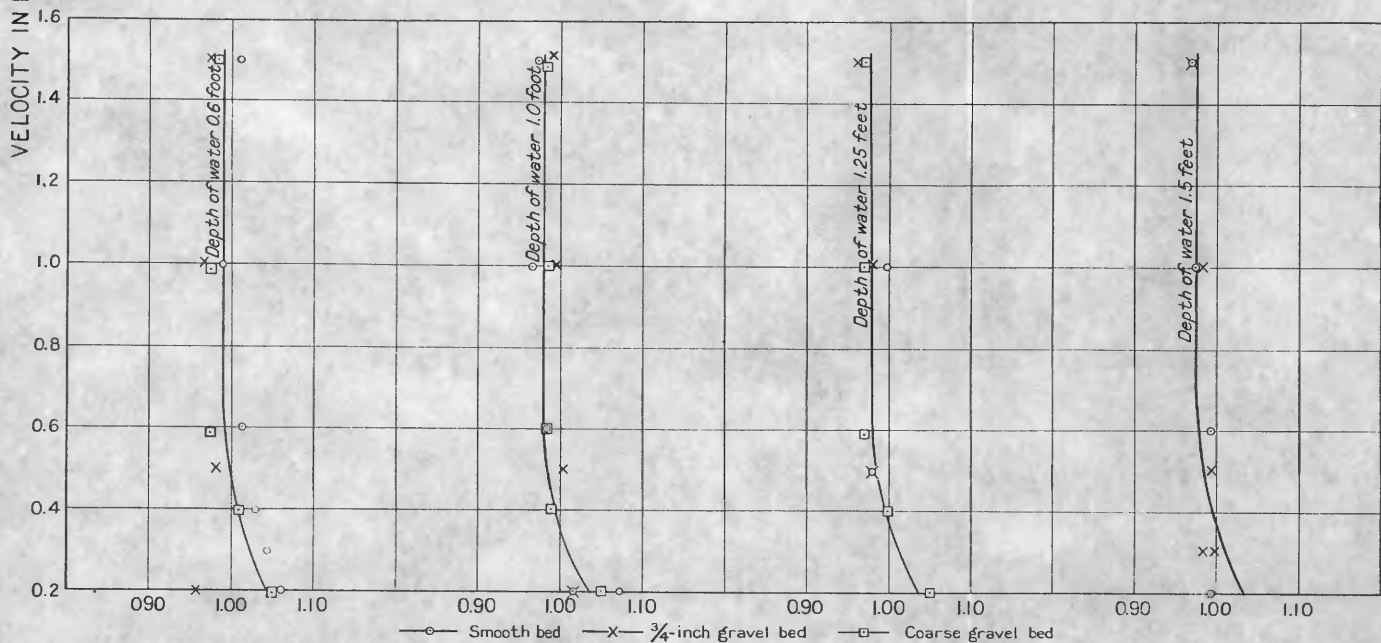
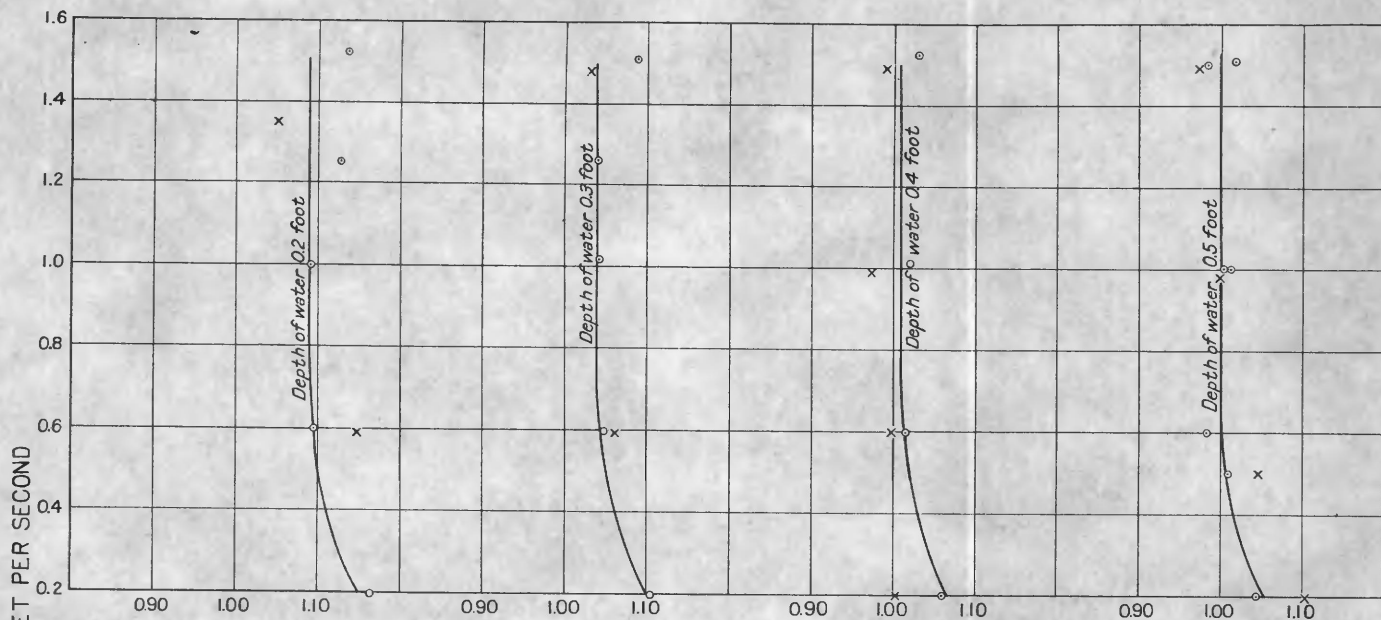


STATE OF NEW YORK

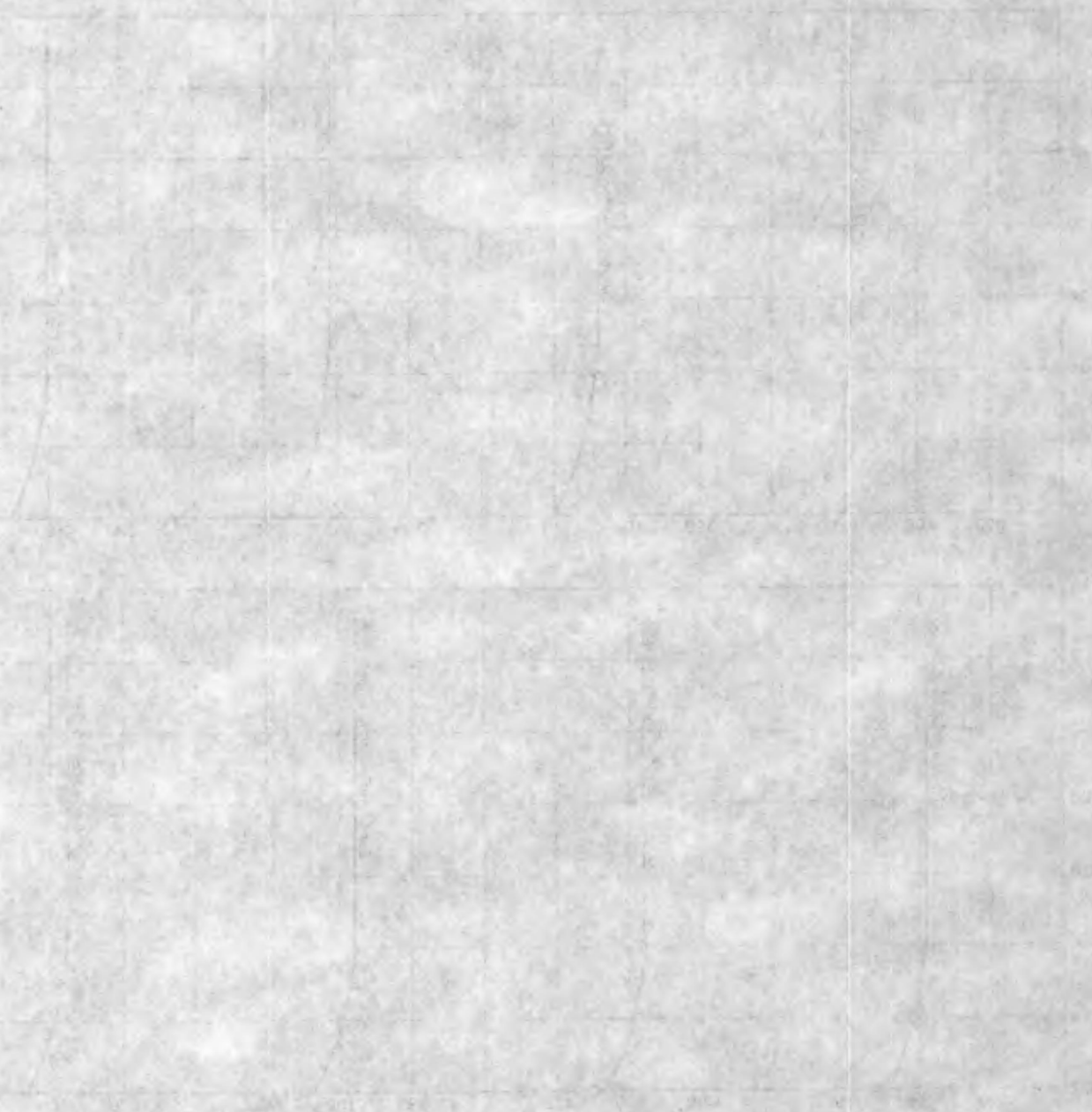


—○— Smooth bed —X— 3/4-inch gravel bed —□— Coarse gravel bed

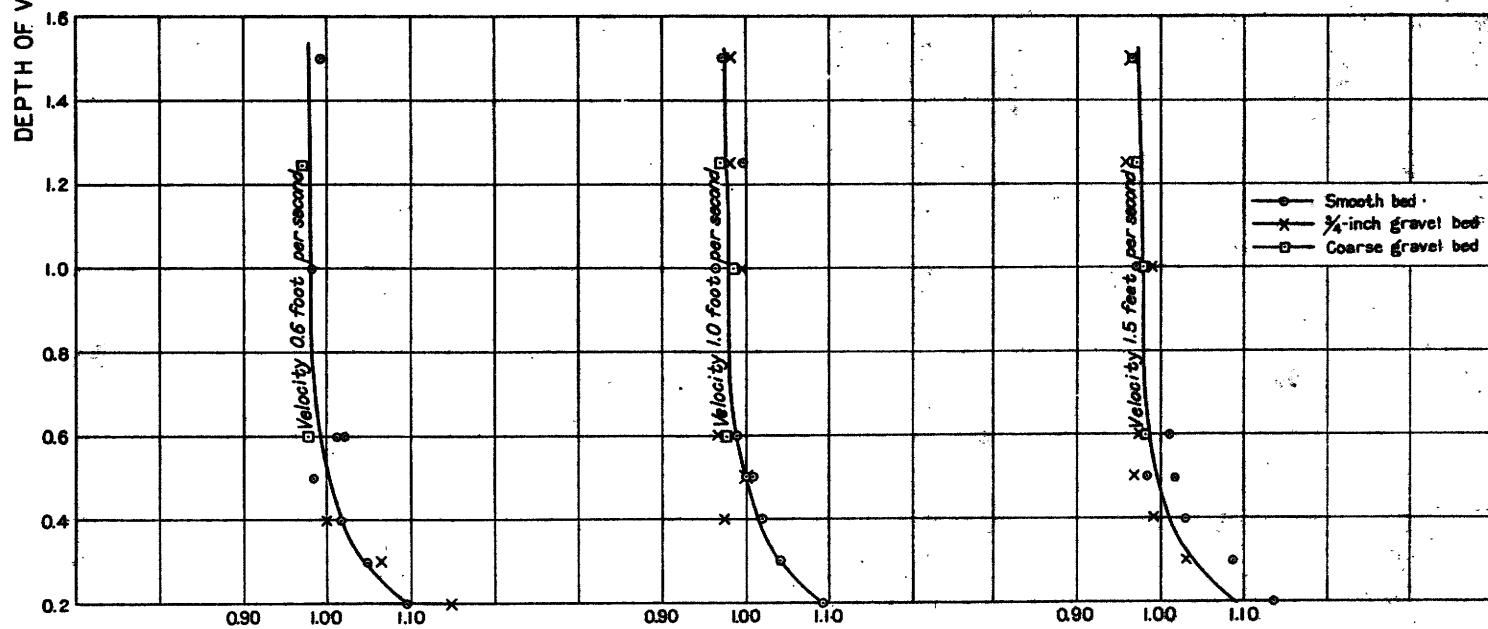
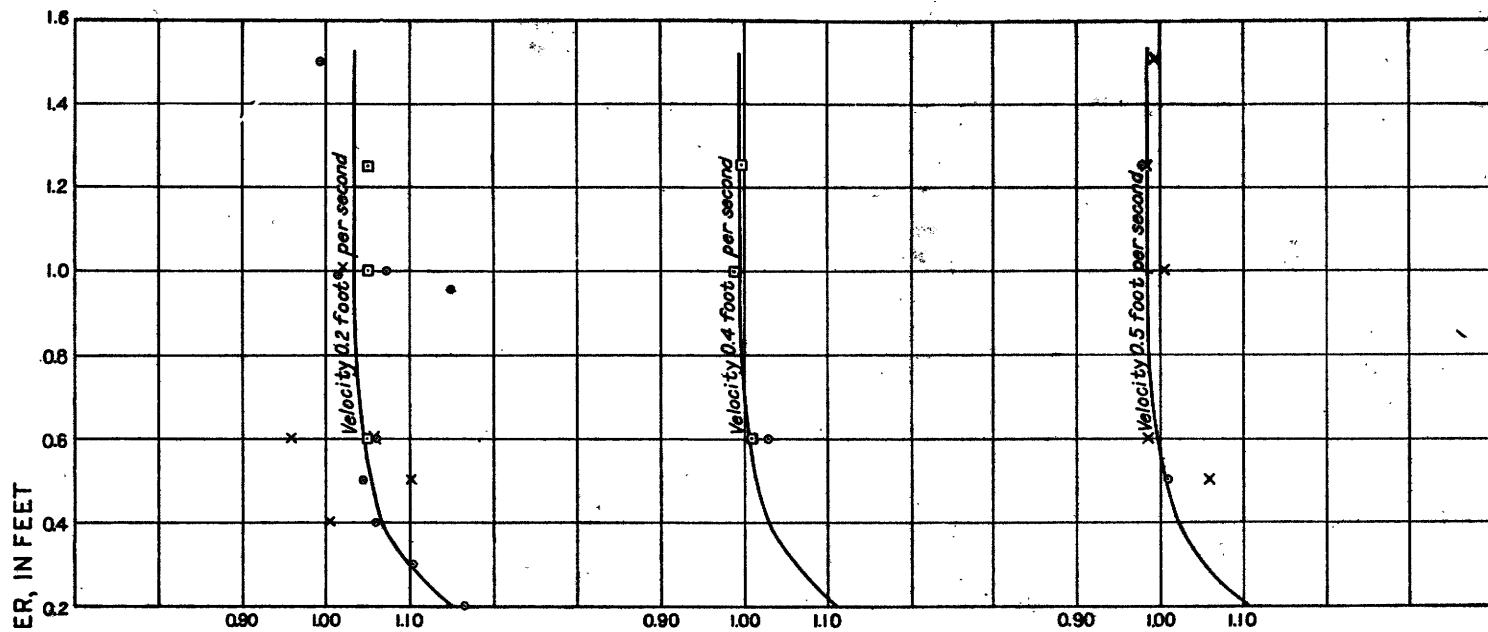
VARIATION OF COEFFICIENTS WITH CHANGE IN DEPTH, 0.5-DEPTH METHOD, PYGMY CURRENT METERS.



VARIATION OF COEFFICIENTS WITH CHANGE IN VELOCITY, 0.6-DEPTH METHOD, PYGMY CURRENT METERS.



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VARIATION OF COEFFICIENTS WITH CHANGE IN DEPTH, 0.6-DEPTH METHOD, PYGMY CURRENT METERS.

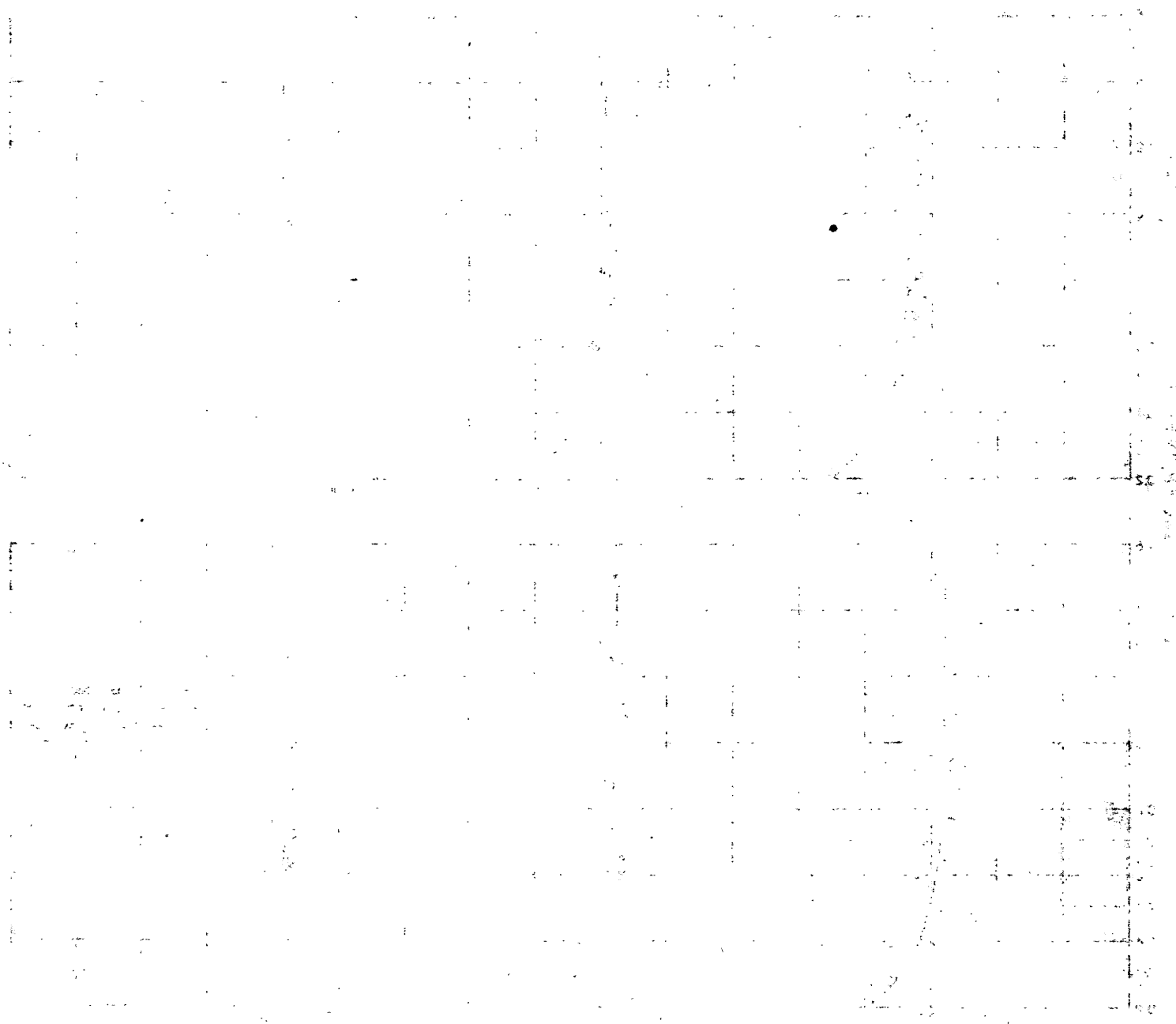
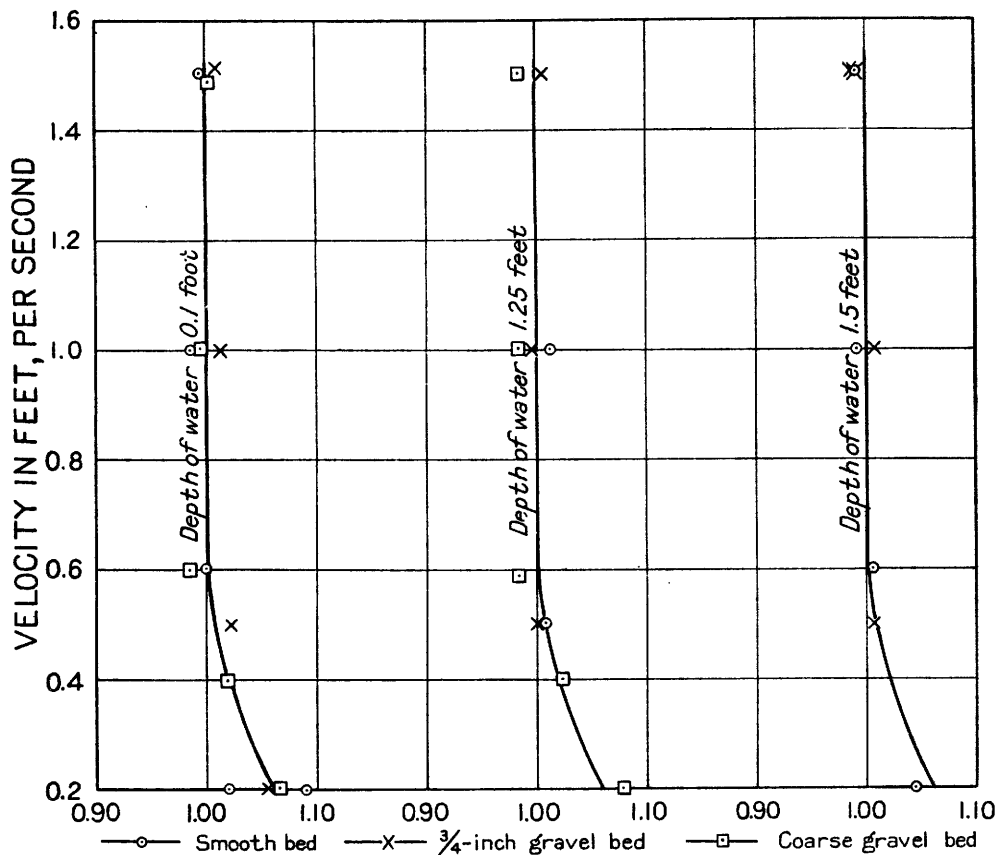
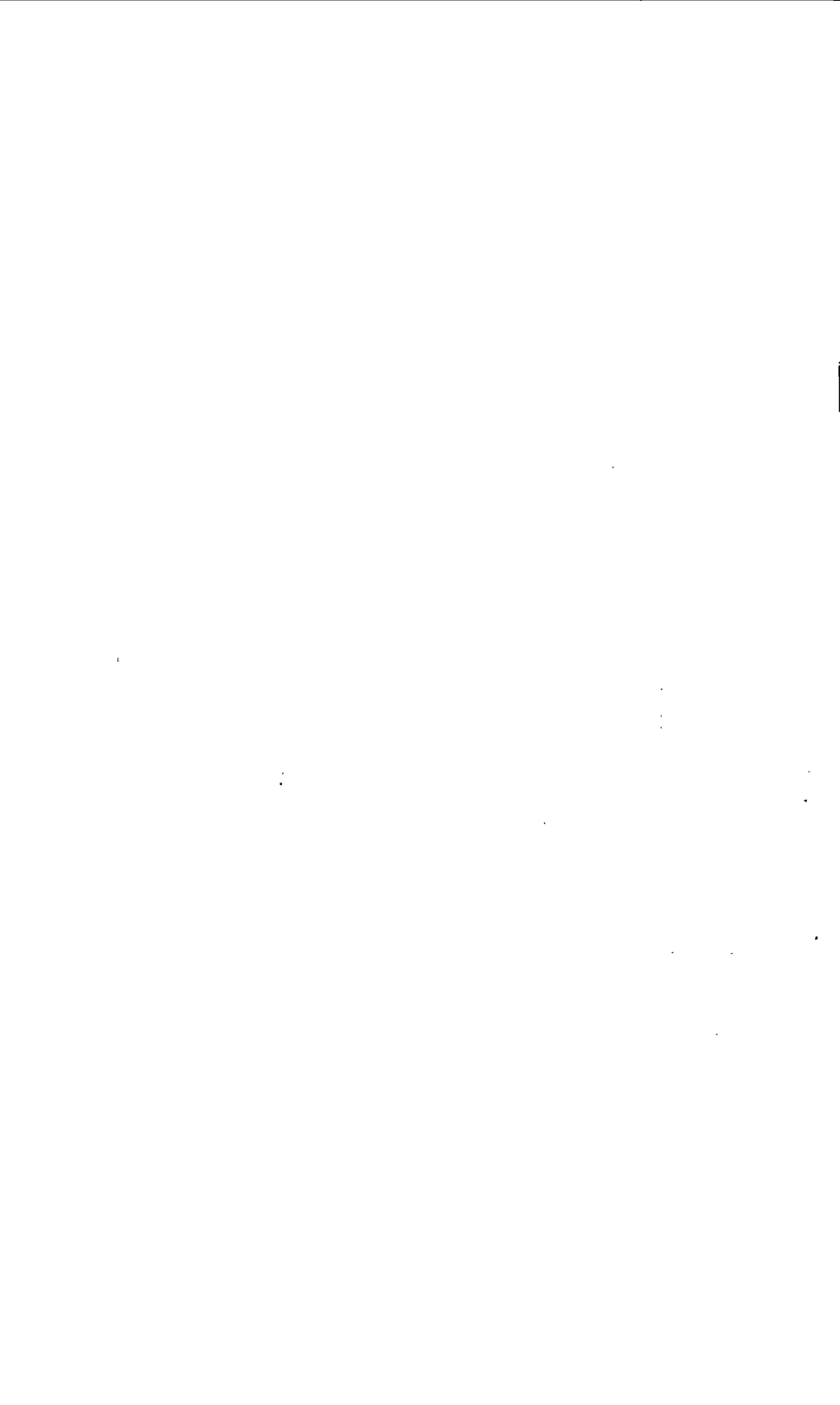
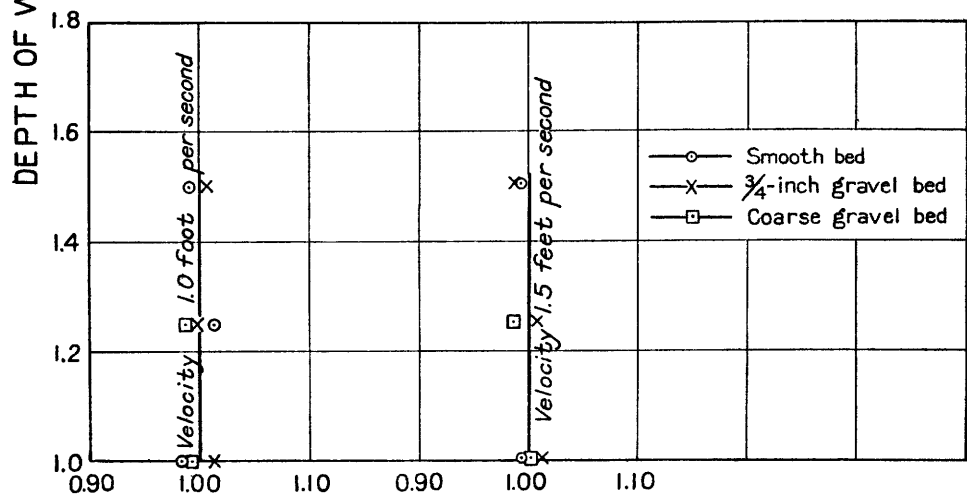
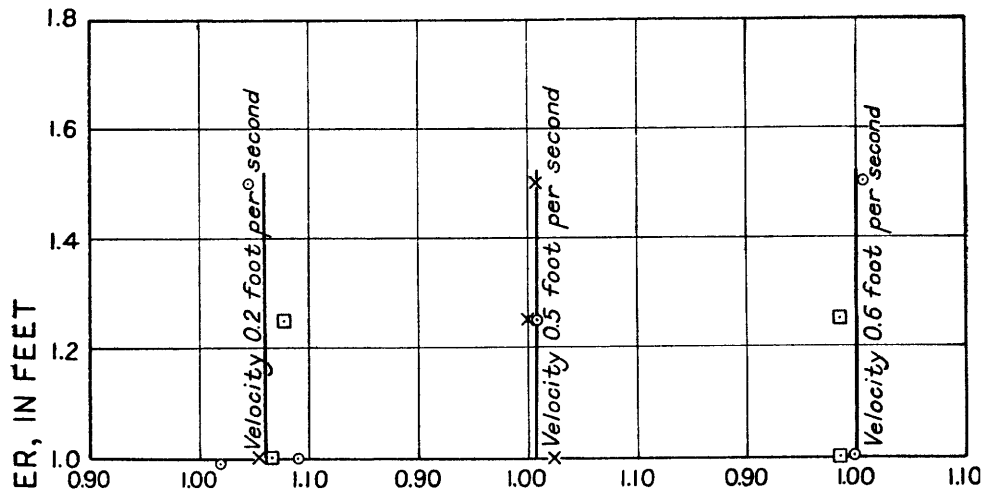


FIG. 1. A. B. C. D. E. F. G. H. I. J. K. L. M. N. O. P. Q. R. S. T. U. V. W. X. Y. Z.



—○— Smooth bed —×— 3/4-inch gravel bed —□— Coarse gravel bed
 VARIATION OF COEFFICIENTS WITH CHANGE IN VELOCITY, 0.2- AND 0.8-DEPTH METHOD, PYGMY CURRENT METERS.





VARIATION OF COEFFICIENTS WITH CHANGE IN DEPTH, 0.2- AND 0.8-DEPTH METHOD, PYGMY CURRENT METERS.

