The National Flood-Frequency Program—Methods for Estimating Flood Magnitude and Frequency in Rural Areas in Utah

Introduction

Estimates of the magnitude and frequency of flood-peak discharges and flood hydrographs are used for a variety of purposes, such as for the design of bridges, culverts, and flood-control structures; and for the management and regulation of flood plains. To provide simple methods of estimating flood-peak discharges, the U.S. Geological Survey (USGS) has developed and published equations for every State, the Commonwealth of Puerto Rico, American Samoa, and a number of metropolitan areas in the United States. In 1993, the USGS, in cooperation with the Federal Emergency Management Agency and the Federal Highway Administration, compiled all current USGS statewide and metropolitan area equations into a computer program, titled "The National Flood-Frequency (NFF) Program" (Jennings and others, 1994).

Since 1993, new or updated equations have been developed by the USGS for various areas of the Nation. These new equations have been incorporated into an updated version of the NFF Program.

Fact sheets that describe application of the updated NFF Program to various areas of the Nation are available. This fact sheet describes the application of the updated NFF Program to streams that drain rural areas in Utah.

Overview

The State of Utah is wholly located within a regional flood study area that encompasses the arid lands of the southwestern United States (Thomas and others, 1997). The study area is divided into 16 hydrologic flood regions, of which 7 include portions of Utah (fig. 1). These regions were delineated on the basis of regional flood sources (snowmelt, summer thunderstorms, or cyclonic rainfall), elevation, and analysis of flood yields and residuals of preliminary regional flood-
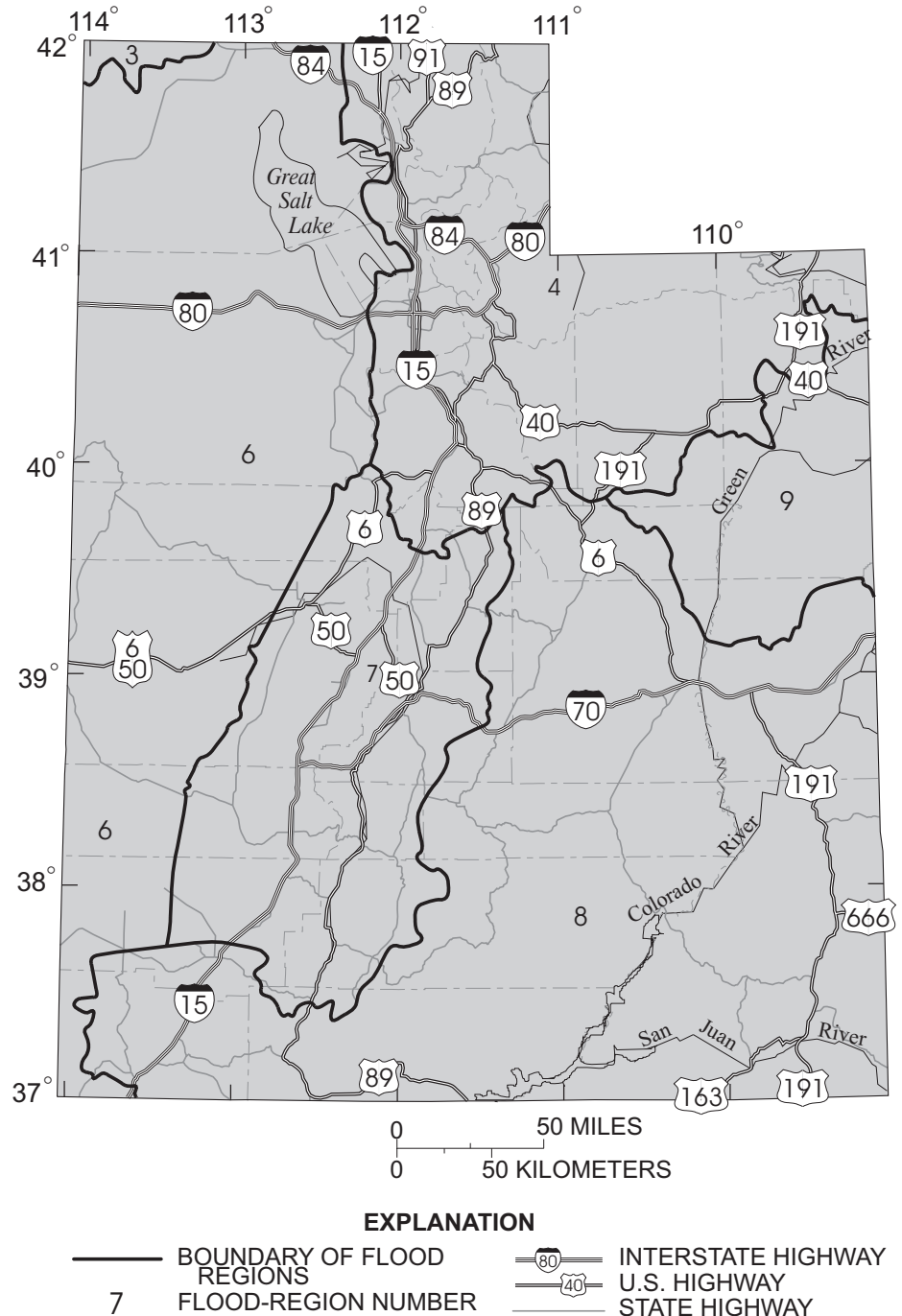

Figure 1. Hydrologic regions for Utah.

Table 1. Flood-peak discharge regression equations and associated statistics for regions 1, 3, 4, 7, 8, and 9 in Utah (modified from Thomas and others, 1997)

[Q_T, peak discharge for recurrence interval T, 2 to 100 years, in cubic feet per second; AREA, drainage area, in square miles; PREC, mean annual precipitation, in inches; ELEV, mean basin elevation, in feet above sea level (NGVD of 1929)]

Regression equation	Average standard error of prediction, in percent	Equivalent years of record
Region 1 (For sites located at elevations greater than elevation threshold from figure 2) -165 stations		
$Q_2 = 0.124 \text{AREA}^{0.845} \text{PREC}^{1.44}$	59	0.16
$Q_5 = 0.629 \text{AREA}^{0.807} \text{PREC}^{1.12}$	52	.62
$Q_{10} = 1.43 \text{AREA}^{0.786} \text{PREC}^{0.958}$	48	1.34
$Q_{25} = 3.08 \text{AREA}^{0.768} \text{PREC}^{0.811}$	46	2.50
$Q_{50} = 4.75 \text{AREA}^{0.758} \text{PREC}^{0.732}$	46	3.37
$Q_{100} = 6.78 \text{AREA}^{0.750} \text{PREC}^{0.668}$	46	4.19
Region 3 -35 stations		
$Q_2 = 0.444 \text{AREA}^{0.649} \text{PREC}^{1.15}$	86	0.29
$Q_5 = 1.21 \text{AREA}^{0.639} \text{PREC}^{0.995}$	83	.49
$Q_{10} = 1.99 \text{AREA}^{0.633} \text{PREC}^{0.924}$	80	.77
$Q_{25} = 3.37 \text{AREA}^{0.627} \text{PREC}^{0.849}$	78	1.23
$Q_{50} = 4.70 \text{AREA}^{0.625} \text{PREC}^{0.802}$	77	1.57
$Q_{100} = 6.42 \text{AREA}^{0.621} \text{PREC}^{0.757}$	78	1.92
Region 4 -108 stations		
$Q_2 = 0.0405 \text{AREA}^{0.701} (\text{ELEV}/1,000)^{2.91}$	64	0.39
$Q_5 = 0.408 \text{AREA}^{0.683} (\text{ELEV}/1,000)^{2.05}$	57	.95
$Q_{10} = 1.26 \text{AREA}^{0.674} (\text{ELEV}/1,000)^{1.64}$	53	1.76
$Q_{25} = 3.74 \text{AREA}^{0.667} (\text{ELEV}/1,000)^{1.24}$	51	3.02
$Q_{50} = 7.04 \text{AREA}^{0.664} (\text{ELEV}/1,000)^{1.02}$	52	3.89
$Q_{100} = 11.8 \text{AREA}^{0.662} (\text{ELEV}/1,000)^{0.835}$	53	4.65
Region 7 -28 stations		
$Q_2 = 0.0150 \text{AREA}^{0.697} (\text{ELEV}/1,000)^{3.16}$	56	0.25
$Q_5 = 0.306 \text{AREA}^{0.590} (\text{ELEV}/1,000)^{2.22}$	45	1.56
$Q_{10} = 1.25 \text{AREA}^{0.526} (\text{ELEV}/1,000)^{1.83}$	45	3.07
$Q_{25} = 122 \text{AREA}^{0.440}$	49	4.60
$Q_{50} = 183 \text{AREA}^{0.390}$	53	5.27
$Q_{100} = 264 \text{AREA}^{0.344}$	59	5.68
Region 8 -108 stations		
$Q_2 = 598 \text{AREA}^{0.501} (\text{ELEV}/1,000)^{-1.02}$	72	0.37
$Q_5 = 2,620 \text{AREA}^{0.449} (\text{ELEV}/1,000)^{-1.28}$	62	1.35
$Q_{10} = 5,310 \text{AREA}^{0.425} (\text{ELEV}/1,000)^{-1.40}$	57	2.88
$Q_{25} = 10,500 \text{AREA}^{0.403} (\text{ELEV}/1,000)^{-1.49}$	54	5.45
$Q_{50} = 16,000 \text{AREA}^{0.390} (\text{ELEV}/1,000)^{-1.54}$	53	7.45
$Q_{100} = 23,300 \text{AREA}^{0.377} (\text{ELEV}/1,000)^{-1.59}$	53	9.28
Region 9 -43 stations		
$Q_2 = 0.0204 \text{AREA}^{0.606} (\text{ELEV}/1,000)^{3.5}$	68	0.14
$Q_5 = 0.181 \text{AREA}^{0.515} (\text{ELEV}/1,000)^{2.9}$	55	.77
$Q_{10} = 1.18 \text{AREA}^{0.488} (\text{ELEV}/1,000)^{2.2}$	52	1.70
$Q_{25} = 18.2 \text{AREA}^{0.465} (\text{ELEV}/1,000)^{1.1}$	53	2.81
$Q_{50} = 248 \text{AREA}^{0.449}$	57	3.36
$Q_{100} = 292 \text{AREA}^{0.444}$	59	3.94

frequency relations. Within Utah, sites greater in elevation than a threshold that is based upon latitude (fig. 2) are considered to be in region 1. Sites located at elevations of 7,500 feet or less may belong to regions 3, 4, 6, 7, 8 or 9 on the basis of geographic location (fig. 1).

Thomas and others (1997) developed regression equations for estimating peak discharges (Q_T), in cubic feet per second, that have recurrence intervals that range from 2 to 100 years for ungaged, unregulated rural streams. The NFF Program provides estimates of the 500-year discharge on the basis of extrapolation. Although some sites with drainages greater than 200 square miles were used to develop the equations, applications are best limited to 200 square miles or less.

Procedure

The equations are based on the inch-pound system of units, but the NFF Program will accept and report either the inch-pound or metric system of units. The explanatory watershed variables used in the regression equations are as follows:

Drainage area (AREA), in square miles, is the total area that contributes runoff upstream of the location of the stream site of interest.

Mean annual precipitation (PREC), in inches, is the average mean annual precipitation for the basin as determined from isohyetal maps developed by the U.S. Weather Bureau (1963). The average is best determined by use of grid sampling techniques. Lines of equal precipitation from the Weather Bureau map are intersected with (drawn on to) a map of the drainage basin, a grid with equal-size cells is overlaid on the map, the mean annual precipitation is determined at each grid intersection, and the values are averaged.

Mean basin elevation (ELEV), in feet above sea level (National Vertical Datum of 1929), is also determined by grid sampling techniques. The elevations of a minimum of 20 equally spaced points are determined, and the average of the points is taken. As many as 100 points may be needed for large basins.

The regression equations, the average standard errors of prediction, and the equivalent years of record for regions 1, 3, 4, 7, 8, and 9 are given in table 1. The average standard errors of prediction are an average measure of the accuracy of the regression equations when estimating peak-

Table 2. Flood-peak discharge equations and associated statistics for region 6 in Utah (modified from Thomas and others, 1997)

[Q_T, peak discharge for recurrence interval T, 2 to 100 years, in cubic feet per second; AREA, drainage area, in square miles; ELEV, mean basin elevation, in feet above sea level (NGVD of 1929)]

Estimated average standard error of regression for these equations includes much of the within-station residual variance and therefore is not comparable to standard error of estimate from an ordinary-least-squares regression.

Regression equation	Estimated average standard error of regression, in log units	Equivalent years of record
Region 6 -80 stations		
$Q_2 = 0$	--	--
$Q_5 = 32 \text{AREA}^{0.80}(\text{ELEV}/1{,}000)^{-0.66}$	1.47	0.233
$Q_{10} = 590 \text{AREA}^{0.62}(\text{ELEV}/1{,}000)^{-1.6}$	1.12	.748
$Q_{25} = 3{,}200 \text{AREA}^{0.62}(\text{ELEV}/1{,}000)^{-2.1}$	.796	2.52
$Q_{50} = 5{,}300 \text{AREA}^{0.64}(\text{ELEV}/1{,}000)^{-2.1}$	1.10	1.75
$Q_{100} = 20{,}000 \text{AREA}^{0.51}(\text{ELEV}/1{,}000)^{-2.3}$	1.84	.794

Table 3. Range of explanatory variables for which regression equations are applicable

[--, not applicable.]

Hydrologic study region	Drainage area, in square miles[1]	Mean basin elevation, in feet above sea level[2]	Mean annual precipitation, in inches
Region 1	0.30–1,060[1]	--	10–45
Region 3	2.35-1,450[1]	--	10-46
Region 4	2.08-1,230[1]	5,740-10,700	--
Region 6	.20-1,670[1]	--	--
Region 7	5.58-340[1]	6,070-9,560	--
Region 8	.20-1,990[1]	4,300-10,200	--
Region 9	4.00-1,180[1]	6,550-10,060	--

[1]For best results, applications should be limited to basins of less than 200 square miles.
[2]NGVD of 1929.

discharge values for ungaged watersheds similar to those that were used to derive the regression equations. The equivalent years of record is the number of years of stream-flow record needed to achieve the same accuracy as the regression equation.

The regression equation for region 6 was developed using an iterative regression method (Hjalmarson and Thomas, 1992) and a modified form of the station year statistical analysis method (Fuller, 1914). The regression equation, the estimated average standard error of regression, and the equivalent years of record for region 6 are given in table 2. The average standard error of regression is an estimate of the predictive accuracy of these regression equations and is determined by a direct sampling method.

The approximate ranges of the explanatory watershed variables over which the equations are applicable are shown in table 3. Thomas and others (1997) presented the actual ranges of applicability as two-dimensional clusters or clouds of explanatory variables plotted against one another. The ranges shown in table 3 define a rectangular space that brackets the clouds and, therefore, include pairs of values of the explanatory variables near the corners of the rectangle that are outside of the clouds. Application of the equations for values of the variables near the extremes of a range should be done cautiously. The standard errors increase appreciably when any explanatory watershed variable is near or outside the quoted range.

Improving Estimates with Gaged Data

The U.S. Water Resources Council (1981, appendix 8) described weighting techniques to improve estimates of peak discharge at gaged locations by combining the estimates derived from analysis of gage records with estimates derived by other means, including regression equations.

The weights for these two estimates are based on the length of the stream gage record (in years) and the equivalent years of record of the applicable regression equation. The weighted estimate of peak discharge is computed as:

$$\log Q_T(W) = \frac{N \cdot \log Q_T(G) + EQ \cdot \log Q_T(R)}{N + EQ}$$

where

$Q_T(W)$ is the weighted estimate for recurrence interval T at the gaged site,
$Q_T(G)$ is the estimate of Q_T derived from analysis of the gage records,
$Q_T(R)$ is the estimate of Q_T derived from application of the regression equation,
N is the number of years of stream gage record, and
EQ is the equivalent years of record (table 1 and 2).

The accuracy of the weighted discharge estimate, in equivalent years of record, is equal to $N + EQ$. The NFF Program contains the appropriate algorithms for this computation, which differs slightly from that described by Thomas and others (1997).

Sites Near Gaged Sites on the Same Stream

Thomas and others (1997) showed how the weighted estimate of peak discharge can be used to improve estimates of peak discharge of an ungaged site on the same stream that has a drainage area that is between 50 and 150 percent of the drainage area of the gaged site. The weighted estimate is computed as:

$$Q_T(u) = Q_T(W) \cdot \left(\frac{Area_{ungaged}}{Area_{gaged}}\right)^b ,$$

where

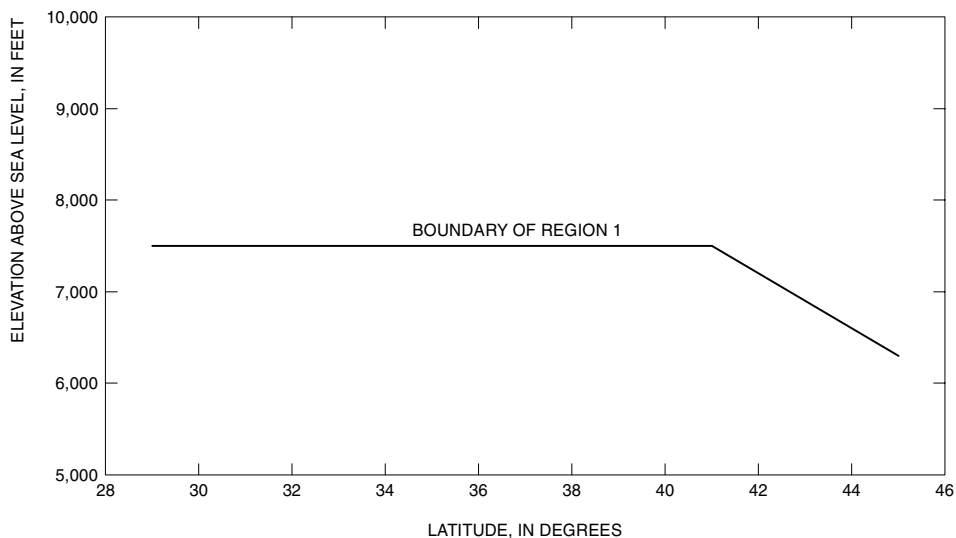$Q_T(u)$ is the weighted peak-discharge estimate for the recurrence interval T at the ungaged site,
$Q_T(W)$ is the weighted estimate of peak discharge at the gaged site,
$AREA_{ungaged}$ and $AREA_{gaged}$ are the drainage areas of the ungaged and gaged sites, respectively, and
b is an exponent for each region as follows:

Region	Exponent
1	0.8
3	.7
4	.7
6	.6
7	.5
8	.4
9	.5

Figure 2. Estimated elevation threshold for region 1 (from Thomas and others, 1997)

The adjustment to the weighted estimate of peak discharge at the gaged site can be used when the drainage area at the ungaged site is within 50 to 150 percent of the drainage area of the gaged site. Otherwise, the estimate at the ungaged site should be based on the appropriate regression equation only.

Sites in Transition Zones

When the drainage area of the site of interest is in more than one of the regions 3, 4, 6, 7, 8, or 9, a weighted estimate of the peak discharge should be computed. The equations for the appropriate regions should be applied independently by using basinwide estimates of the required explanatory variables. The weighted estimate is then computed by multiplying each regional estimate against the fraction of the drainage area in that region and summing the products. The NFF Program provides an algorithm for this computation.

When the elevation of the stream site of interest is between 6,800 and 7,500 feet, a weighted estimate of the peak discharge should be computed by using the equations for region 1 and the other regions in which the basin is located. The applicable equations are each applied by using basinwide estimates of the required explanatory variables, and the region estimates are weighted as a function of elevation as follows:

$$Q_T(W) = Q_T(u) \cdot \frac{7,500 - E}{700}$$

$$+ Q_T(Region 1) \cdot \left(1 - \frac{7,500 - E}{700}\right)$$

$Q_T(W)$ is the weighted of peak-discharge estimate for the recurrence interval T at the site of interest,

$Q_T(u)$ is the estimate of peak discharge using the equations for regions 3, 4, 6, 7, 8 or 9 as appropriate,

$Q_T(Region 1)$ is the estimate of the peak discharge using the equations for region 1, and

E is the elevation of the stream site of interest.

The NFF Program does not provide an algorithm for this weighting computation.

Thomas and others (1997) summarized the basin characteristics, the estimates of peak discharge, and the weighted estimates of peak discharge for most of the 1,323 sites used in the study, including 212 sites in Utah.

—*Prepared by Robert R. Mason, Jr., and Kernell G. Ries III, of the U.S. Geological Survey; and Jeffrey N. King and Wilbert O. Thomas, Jr., of Michael Baker, Jr., Inc.*

References

Fuller, W.E., 1914, Flood flows: American Society of Civil Engineers Transactions, v. 77, no. 1293, p. 564-617.

Hjalmarson, H.W., and Thomas, B.E., 1992, A new look at regional flood-frequency relations for arid lands: American Society of Civil Engineers, Journal of Hydraulic Engineering, v. 118, no. 6, p. 868-886.

Jennings, M.E., Thomas, W.O., Jr., and Riggs, H.C., comps., 1994, Nation-wide summary of U.S. Geological Survey regional regression equations for estimating magnitude and fre-quency of floods for ungaged sites, 1993: U.S. Geological Survey Water-Resources Investigations Report 94–4002, 196 p.

Thomas, B.E., Hjalmarson, H.W., and Waltemeyer, S.D., 1997, Methods for estimating magnitude and frequency of floods in the southwestern United States: U.S. Geological Survey Water-Supply Paper 2433, 195p.

U.S. Water Resources Council, 1981, Guidelines for determining flood flow frequency: U.S. Water Resources Council Bulletin 17B, 28p. 14 appen-dixes.

U.S. Weather Bureau, 1963, Normal annual precipitation (1931-60) for the States of Arizona, Colorado, New Mexico, Utah: U.S. Department of Commerce, Weather Bureau Map, scale 1:500,000.