

TABLE 1.—Discharge, peak velocity, and recurrence intervals of peak water flow for 10-, 25-, and 50-year floods on Little Buffalo Creek, West Jefferson, North Carolina.

Station name	Discharge at peak water flow (cfs)			Velocity at peak water flow (feet per second)		
	10-year	25-year	50-year	10-year	25-year	50-year
Little Buffalo Creek at State Highway 88	100	150	200	1.11	1.31	1.43
Little Buffalo Creek near West Jefferson	100	150	200	1.29	1.51	1.63
Little Buffalo Creek at State Highway 88	600	900	1200	0.87	1.08	1.19
Little Buffalo Creek at State Highway 88	600	900	1200	1.44	1.74	1.85

TABLE 2.—Average water flow, Little Buffalo Creek, West Jefferson, North Carolina.

Station name	Average water flow (cfs)		
	10-year	25-year	50-year
Little Buffalo Creek at State Highway 88	100	150	200
Little Buffalo Creek near West Jefferson	100	150	200
Little Buffalo Creek at State Highway 88	600	900	1200
Little Buffalo Creek at State Highway 88	600	900	1200



EXPLANATION

Area flooded, hypothetical flood
 Boundary of hypothetical 10-year flood
 Boundary of hypothetical 25-year flood
 Boundary of hypothetical 50-year flood
 Benchmark mark
 Feet upstream from State Highway 88
 X 2000
 Direction of flow

FLOODS ON LITTLE BUFFALO CREEK AT WEST JEFFERSON, NORTH CAROLINA

This report was prepared by the U.S. Geological Survey to further the objectives of the Appalachian Regional Commission. It provides technical guidance for those who plan the economic uses of flood plain lands. Hydrologic data, extent, depth, and frequency of flooding expected on Little Buffalo Creek in the vicinity of West Jefferson, North Carolina. The approximate boundaries of inundation by floods having average recurrence intervals of 10, 25, and 50 years are outlined on a photomosaic map. West Jefferson is in the Kananah River basin of Ashe County, North Carolina. The 17-mile study begins in West Jefferson at U.S. Highway 221, extends in a northerly direction along the eastern edge of the city and ends at State Highway 88. The drainage area is 6,043 square miles at the lower end. The fall of about 67 feet per mile in the reach. The steep slope causes excessive water velocities during large discharges. The general procedure used in defining flood boundaries for Little Buffalo Creek were as follows:

1. Recurrence intervals of 10, 25, and 50 years.
 2. Utilizing the step-backwater method, complete profiles for these floods with consideration of the effects of obstructions.
 3. Delineate the horizontal extent of flooding on a photomosaic map.
- The determination of flood discharges and profiles was based on the following assumptions:
1. Field conditions were used as a basis for the determination of the flood.
 2. Bridges and culverts would not be clogged with debris nor otherwise be responsible for floods.
 3. The discharge of floods would be of equal magnitude throughout the basin.

Flood History.—Newspaper reports and records of nearby streams indicate that the flood of August 14, 1940, is the highest known at West Jefferson, exceeding the large floods of both 1916 and 1978. In addition to causing major property damage, the flood of 1940 resulted in the destruction of several persons in Ashe County. Although some destruction resulted from flood in 1978, the high velocity of flow was the major cause of property damage. The flood of 1940 was the result of a major rain storm in the southeastern States. Flow data for this flood in the West Jefferson area are not available but the recurrence interval has been estimated on a regional basis to exceed 100 years. Eight inches of rain fell in the West Jefferson area, resulting in flow washed in debris that blocked many culverts and bridges. Since then road and flood plain elevations have been raised and many culverts have been enlarged.

Determination of discharge.—Little Buffalo Creek is an ungaged stream and no records of discharge are available. Flood discharge methods defined by Hinson (1965). Drainage areas and the magnitude and velocity of floods having selected recurrence intervals were determined at four selected sites along the stream. These data are listed in table 1.

Recurrence Interval.—Recurrence interval is defined as the average interval of time within which a given magnitude of flood is expected to occur. The state of occurrence of floods of a given magnitude cannot be predicted, but the probable number of such floods during a long period of time can be estimated with reasonable accuracy. For example, a 2-year flood can be expected to occur in a 100-year period, and 4 floods of at least the magnitude of a 25-year flood can be expected to occur in a 100-year period. Floods of at least the magnitude of a 50-year flood can be expected to occur in any one year.

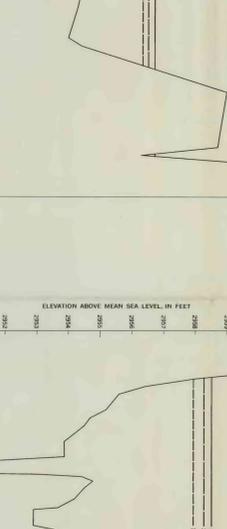
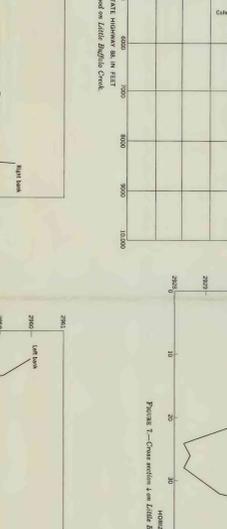
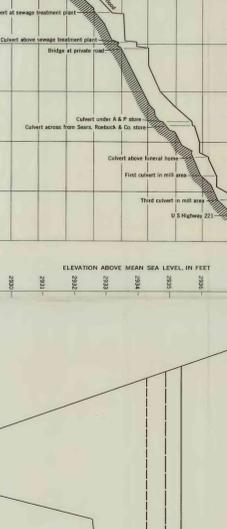
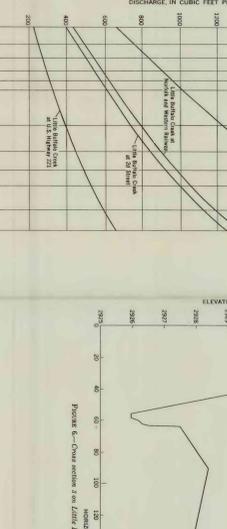
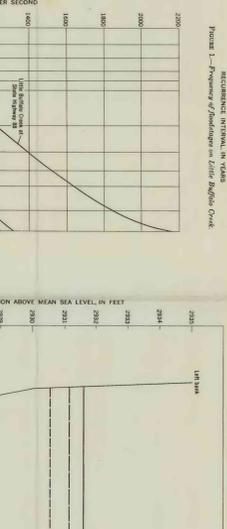
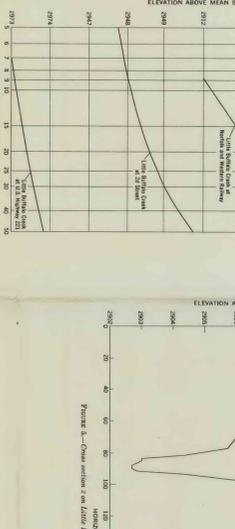
Velocity.—Velocities of floodflow in both the main channel and on the floodplain vary considerably. Generally the velocities on the flood plain are less than those in the main channel. Average figures were obtained by dividing the instantaneous peak discharge by the cross-sectional area of the selected sites. The high stream slopes and high flows, the high velocities may cause damage or even destroy buildings, embankments, and pavements and thus be more damaging than inundation alone.

Additional Information.—Supporting data and computations pertaining to this report are in the files of the U.S. Geological Survey, Raleigh, N.C. Areas can be obtained from the following published reports:

Hinson, H. G., 1965. Floods on small streams in North Carolina, probable magnitude and recurrence intervals. U.S. Geological Survey, Water Resources Branch, 1949, Floods of August 1940 in the southeastern States: U.S. Geol. Survey Water-Supply Paper 1066, 554 p.

Extent and depth of flooding.—Areas adjacent to streams become inundated when the flow of water exceeds the capacity of the stream to convey water. The elevation of the flood depends upon the amount of floodwater and up on the capacity of the channel. Roadfills, landfills, bridges, culverts, buildings, and other structures are damaged or destroyed. Flood computations were made assuming all bridge and culvert openings to be unobstructed. Should an opening become blocked during a flood, the water upstream would back up and the flood would be more extensive.

Depth of flooding at any point can be estimated for floods of each recurrence interval from combined use of the map, flood profile, and the typical cross sections, figures 4, 5, 6, and 7. From any point in question one of the reference marks described in table 2. The depth of flooding shown on the illustrations and the flood areas determined on the map reflect present conditions. Future development along the stream, such as new highways and bridges, by relocation or excavation of stream channels, the main channel and on the floodplain vary considerably. Generally the velocities on the flood plain are less than those in the main channel. Average figures were obtained by dividing the instantaneous peak discharge by the cross-sectional area of the selected sites. The high stream slopes and high flows, the high velocities may cause damage or even destroy buildings, embankments, and pavements and thus be more damaging than inundation alone.



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Unannotated photomosaic size reduced from original size. Flood boundaries by North Carolina Department of Transportation.

