

SUPPLEMENTARY HYDRAULIC ANALYSIS OF THE  
CHENANGO RIVER, BROOME COUNTY, NEW YORK,  
IN RELATION TO PLANNED HIGHWAY CONSTRUCTION

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Prepared in cooperation with  
New York State Department of Transportation

UNITED STATES  
DEPARTMENT OF THE INTERIOR  
GEOLOGICAL SURVEY

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## CONVERSION FACTORS AND ABBREVIATIONS

Multiply English units	By	To obtain metric units
miles (mi)	1.609	= kilometers (km)
feet (ft)	.3048	= meters (m)
feet per second (ft/s)	.3048	= meters per second (m/s)
cubic feet per second (ft <sup>3</sup> /s)	.02832	= cubic meters per second (m <sup>3</sup> /s)

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ABSTRACT

To supplement a previous study evaluating the hydraulic effects of alternative highway plans in the Towns of Fenton and Chenango, Broome County, N.Y., a combination of two of the alternatives was studied for its effect on 100-year flood levels in the 3.2-mile (5.1-kilometer) study reach. Two variations of the combination plan, which calls for crossings of the Chenango River, were analyzed: (1) with State Highway 12A bridge replaced by a new one of the same length, and (2) with it replaced by one 100 feet (30.5 meters) longer. Both variations call for an additional highway bridge 3.2 miles (5.1 kilometers) downstream. If the replacement bridge at State Highway 12A were the same length as the present one, the proposed plan would cause no significant increase in 100-year flood stage; if the longer bridge were used, the 100-year flood stage would be decreased slightly. The 100-year flood discharge was found to be 68,000 cubic feet per second (1,930 cubic meters per second), whereas that used in the original report was 58,000 cubic feet per second (1,640 cubic meters per second). Mean velocities and water-surface elevations at 20 cross sections in the reach are given for both variations of the combination plan and at the original and the revised discharges. Hundred-year flood velocities and water-surface elevations for present conditions are included for comparison.

## INTRODUCTION

An analysis of the possible effects of a proposed highway bridge plan on the 100-year flood in a 3.2-mi (5.1-km) reach of the Chenango River in the Towns of Fenton and Chenango, Broome County, N.Y., was done by the U.S. Geological Survey at the request of the New York State Department of Transportation to supplement an earlier study. The original report, titled "Hydraulic Analysis, Chenango River, Broome County, New York," by Bernard Dunn (1974), described results of an analysis of the hydraulic characteristics of the present channel and the channels that would be created by each of seven alternative highway plans.

This supplement summarizes the water-surface elevations and velocities that would result from the combination of alternatives 2 and 5 described in the original report. That combination calls for the construction of a 1,000-ft (304.8-m) span bridge with five piers near the downstream (west) end of the study reach. The hydraulic analysis for this combination plan gives water-surface elevations and mean velocities at 20 cross sections of the study reach (fig. 1-S) for the original and the revised 100-year flood discharge, for two proposed conditions:

1. With the 1,000-ft (304.8-m) bridge constructed at section 28A, and the present 490-ft (149.4-m) span State Highway 12A bridge 2.05 mi (3.30 km) upstream at section 38 replaced by a new one of the same length;
2. With the 1,000-ft (304.8-m) bridge constructed at section 28A but the present State Highway 12A bridge upstream replaced with a 600-ft (182.9-m) span bridge with three piers.

Both proposed conditions call for an overflow channel between the proposed highway and main channel of the river between sections 30 and 33A. Data for present conditions (with only the present Highway 12A bridge) are included for comparison.

Analyses of each of the above conditions include the present Erie-Lackawanna Railroad bridge at section 35.

Cross-sectional data were provided by the New York State Department of Transportation. All elevations are referred to mean sea level, datum of 1929.

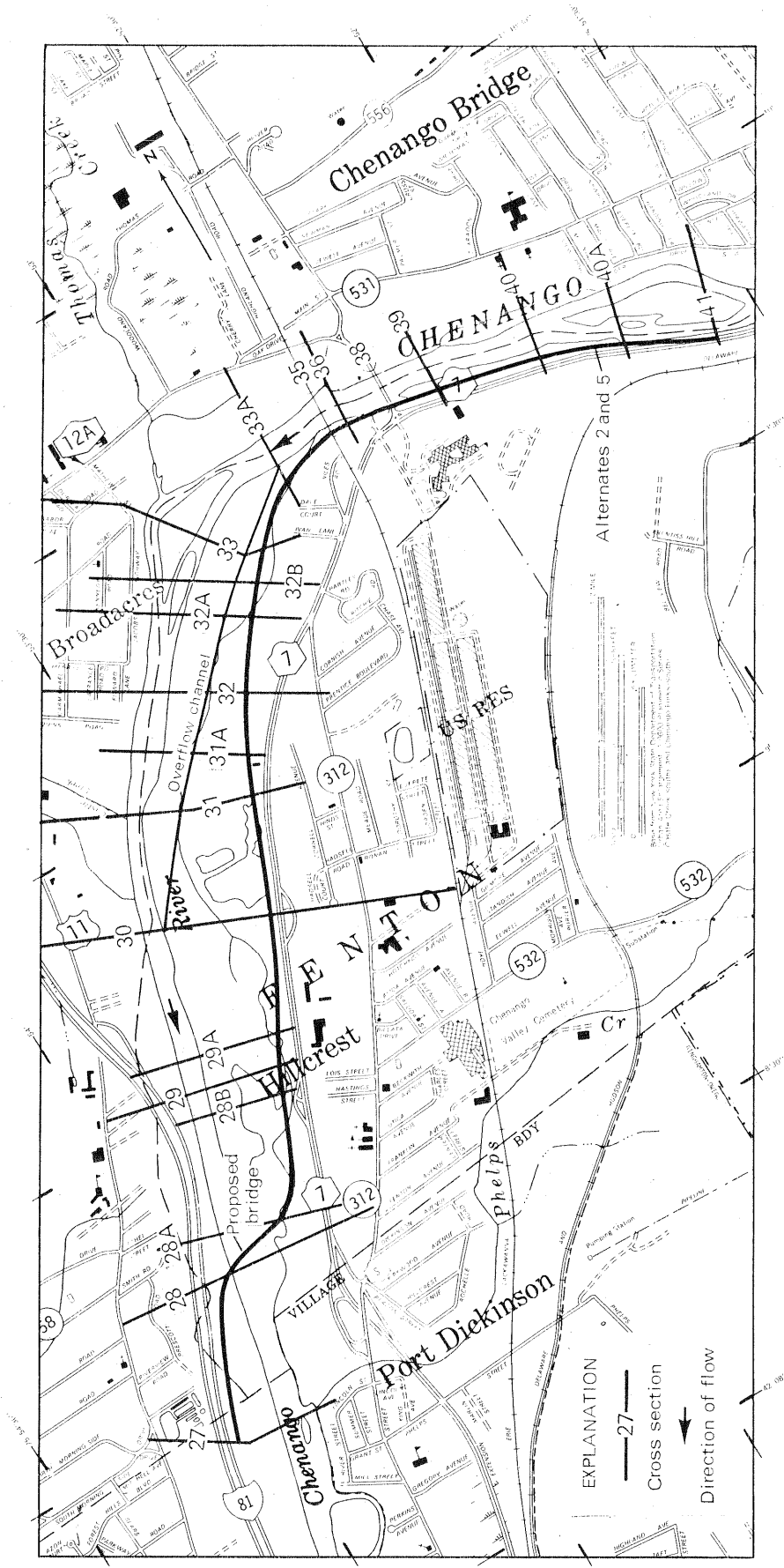


Figure 1-S.--Location of proposed highway (combination of alternatives 2 and 5 from original report) and State Highway 12A.

### Flood Frequency

The 100-year flood is the design flood for this project. When the original report (Dunn, 1974) was published, the 100-year discharge at the study site was estimated to be 58,000 ft<sup>3</sup>/s (1,640 m<sup>3</sup>/s). Since that time, additional data have become available and new techniques adopted for the estimation of flood frequencies, and it is now (1976) estimated that the 100-year flood discharge at the study site is 68,000 ft<sup>3</sup>/s (1,930 m<sup>3</sup>/s).

The term flood frequency (or recurrence interval) refers to the average interval of time within which a flood of given magnitude will be equaled or exceeded once. The flood-frequency data presented in this report are accurate within limitations of the method used to determine the flood frequency.

The U.S. Army Corps of Engineers determined that for a discharge of 68,000 ft<sup>3</sup>/s (1,930 m<sup>3</sup>/s), the water-surface elevation at the downstream end of this study reach would be 854.8 ft or 260.5 m (James Guerrini, oral commun., September 22, 1976).

### Analysis of Hydraulic Conditions

Water-surface profiles for the study reach were developed for flood discharges of 58,000 and 68,000 ft<sup>3</sup>/s (1,640 and 1,930 m<sup>3</sup>/s) for both variations of the proposed plan (combination of alternatives 2 and 5). Water-surface profiles were computed by the standard step-backwater method (Chow, 1959).

Results of the computations are presented in tables 1-S and 2-S, for 58,000 and 68,000 ft<sup>3</sup>/s, respectively. At a discharge of 68,000 ft<sup>3</sup>/s (1,930 m<sup>3</sup>/s), the proposed bridge at section 28A (fig. 1-S) together with a new 490-ft (149.4-m) State Highway 12A bridge at section 38 would cause a maximum increase in water-surface elevation of 0.3 ft (0.1 m) above present 100-year flood stage. If the present State Highway 12A bridge were replaced with a 600-ft (182.9-m) span bridge, the 100-year flood stage upstream from it between sections 39 and 41 would be decreased by 0.3 to 0.7 ft (0.1 to 0.2 m).

With the proposed bridge at section 28A and a new 490-ft (149.4-m) State Highway 12A bridge, mean velocities in the study reach at a discharge of 68,000 ft<sup>3</sup>/s (1,930 m<sup>3</sup>/s) would range from 3.0 to 5.3 ft/s (0.9 to 1.6 m/s), which is comparable to those under the present bridge condition, which range from 2.6 to 5.3 ft/s (0.6 to 1.6 m/s). If the present State Highway 12A bridge were replaced by a 600-ft (182.9-m) bridge, the resulting velocity range would be from 3.0 to 5.4 ft/s (0.9 to 1.6 m/s).

At sections 30 and 33A, the proposed overflow channel joins the main river channel, and there are no changes in the velocity distribution.

Table 1-S.--Water-surface elevations and mean velocities for discharge of 58,000 ft<sup>3</sup>/s,

combination of alternatives 2 and 5, Chenango River, Broome County, N.Y.

Section	Present condition		Proposed 1,000-ft bridge at section 28A and proposed 490-ft Highway 12A bridge		Proposed 1,000-ft bridge at section 28A and proposed 600-ft Highway 12A bridge		
	Water-surface elevation (ft)	Mean velocity (ft/s)	Water-surface elevation (ft)	Mean velocity (ft/s)	Water-surface elevation (ft)	Mean velocity (ft/s)	
27	854.0	2.8	854.0	2.8	854.0	2.8	
28	854.2	2.8	854.2	3.3	854.2	3.3	
28A	854.2	2.9	--	Proposed 1,000-ft Bridge	--	--	
28B	854.4	3.2	854.5	3.3	854.5	3.3	
29	854.6	2.4	854.6	3.2	854.6	3.2	
30	854.8	3.3	855.0	3.2	855.0	3.2	
31	855.3	3.1	855.5	3.0	855.5	3.0	
31A	855.6	4.1	855.6	3.8	855.6	3.8	
32	855.9	3.2	856.0	3.3	856.0	3.3	
32A	856.4	3.0	856.3	2.9	856.3	2.9	
32B	856.6	4.0	856.5	4.5	856.5	4.5	
33	857.2	3.6	857.1	3.7	857.1	3.7	
33A	858.2	4.3	858.1	4.3	858.1	4.3	
35	--	--	Erie-Lackawanna Railroad Bridge				--
36	859.2	4.8	859.2	4.8	859.2	4.8	
38	--	--	State Highway 12A Bridge				--
39	860.4	4.7	860.4	4.7	859.9	4.9	
40	861.4	4.9	861.4	4.9	861.0	5.1	
40A	862.0	4.9	862.0	4.9	861.7	5.0	
41	862.4	4.1	862.4	4.1	862.2	4.2	



Table 2-5. Water-surface elevations and velocity distribution for discharge of 68,000 ft<sup>3</sup>/s  
 combination of alternatives 2 and 5, Chenango River, Broome County, N.Y.

Section	Present condition										Proposed 1,000-ft bridge at Section 28A and Proposed 490-ft Highway I2A Bridge										Proposed 1,000-ft bridge at Section 28A and Proposed 600-ft Highway I2A Bridge									
	Water-surface elevation (ft)		Velocity (ft/s)		Water-surface elevation (ft)	Velocity (ft/s)		Water-surface elevation (ft)	Velocity (ft/s)		Extreme left side of section	Left bank	Overflow to main channel	Adjacent to main channel	Main channel	Right bank	Mean	Extreme left side of section	Left bank	Overflow to main channel	Adjacent to main channel	Main channel	Right bank	Mean						
	Left bank	Right bank	Left bank	Right bank		Left bank	Right bank		Left bank	Right bank															Left bank	Right bank	Left bank	Right bank	Left bank	Right bank
27	854.8	0.6	3.3	--	3.1	854.8	--	0.6	3.3	--	3.1	854.8	--	0.6	3.3	--	3.1	--	--	0.6	3.3	--	3.1							
28	855.0	1.2	3.8	2.0	3.1	855.0	--	1.4	4.0	--	3.7	855.0	--	1.4	4.0	--	3.7	--	--	1.4	4.0	--	3.7							
28A	855.0	1.3	4.1	1.9	3.2																									
28B	855.2	1.0	4.1	1.9	3.5	855.4	--	1.1	4.2	1.9	3.6	855.4	--	1.1	4.2	1.9	3.6	--	--	1.1	4.2	1.9	3.6							
29	855.4	1.1	3.2	1.6	2.6	855.5	--	1.0	3.9	1.9	3.5	855.5	--	1.0	3.9	1.9	3.5	--	--	1.0	3.9	1.9	3.5							
30	855.7	1.8	5.1	--	3.6	856.0	--	1.6	4.9	--	3.4	856.0	--	1.6	4.9	--	3.4	--	--	1.6	4.9	--	3.4							
31	856.2	2.3	5.0	2.0	3.4	856.5	1.4	3.8	4.3	1.7	3.2	856.5	1.4	3.8	4.3	1.7	3.2	1.4	3.8	3.0	4.3	1.7	3.2							
31A	856.4	1.2	5.1	1.3	4.3	856.7	1.3	4.2	4.4	5.0	3.9	856.7	1.3	4.2	4.4	5.0	3.9	1.3	4.2	4.4	5.0	3.9								
32	856.8	2.4	5.7	1.5	3.4	857.1	2.0	4.4	5.7	1.5	3.4	857.1	2.0	4.4	5.7	1.5	3.4	2.0	4.4	2.7	5.7	1.5	3.4							
32A	857.3	2.1	5.7	1.6	3.2	857.4	1.8	3.8	2.1	5.0	1.3	3.0	857.4	1.8	3.8	2.1	5.0	1.8	3.8	2.1	5.0	1.3	3.0							
32B	857.5	2.3	5.4	--	4.3	857.5	2.1	4.9	2.2	5.1	--	4.8	857.5	2.1	4.9	2.2	5.1	2.1	4.9	2.2	5.1	--	4.8							
33	858.1	2.4	4.6	--	3.8	858.2	2.4	5.1	2.5	4.1	2.0	3.8	858.2	2.4	5.1	2.5	4.1	2.4	5.1	2.5	4.1	2.0	3.8							
33A	859.1	2.6	7.0	2.4	4.6	859.2	--	2.8	7.3	2.4	4.5	859.2	--	2.8	7.3	2.4	4.5	--	--	2.8	7.3	2.4	4.5							
35																														
36	860.4	1.7	7.1	1.7	5.1	860.4	--	2.0	7.1	1.7	5.1	860.4	--	2.0	7.1	1.7	5.1	--	--	2.0	7.1	1.7	5.1							
38																														
39	861.8	4.9	6.8	2.9	4.8	861.8	--	4.9	6.8	2.9	4.8	861.1	--	4.9	6.8	2.9	4.8	--	--	4.9	6.8	2.9	4.8							
40	862.6	--	5.4	2.8	5.2	862.6	--	--	5.6	2.8	5.2	862.2	--	--	5.6	2.8	5.2	--	--	--	6.0	3.0	5.4							
40A	863.3	--	5.3	--	5.3	863.3	--	--	5.3	--	5.3	862.9	--	--	5.3	--	5.3	--	--	--	5.4	--	5.4							
41	863.7	--	4.5	--	4.5	863.7	--	--	4.5	--	4.5	863.4	--	--	4.5	--	4.5	--	--	--	4.6	--	4.6							

## SUMMARY

As a supplement to an earlier floodflow study (Dunn, 1974) the 100-year flood profiles for the Chenango River in the Towns of Fenton and Chenango, Broome County, N.Y., are given for a combination of two of the alternative bridge and highway plans described in that report (alternatives 2 and 5). Flood profiles are evaluated for two variations of this plan: (1) a new 490-ft (149.4-m) span State Highway 12A bridge to replace the present one, and (2) a 600-ft (182.9-m) span bridge to replace the present bridge. The present 100-year flood profile of the river is compared with those resulting from the two plan variations. The 100-year flood discharge in the study reach was found to be 68,000 ft<sup>3</sup>/s (1,930 m<sup>3</sup>/s); originally it had been estimated to be 58,000 ft<sup>3</sup>/s (1,640 m<sup>3</sup>/s). Major results of the study are summarized as follows:

1. Under present conditions, water-surface elevation at the downstream end of the study reach is 854.8 ft (260.5 m) at a discharge of 68,000 ft<sup>3</sup>/s (1,930 m<sup>3</sup>/s) and 854.0 ft (260.3 m) at a discharge of 58,000 ft<sup>3</sup>/s (1,640 m<sup>3</sup>/s).
2. If the present 490-ft (149.4-m) span State Highway 12A bridge is replaced by a new one of the same length, the proposed plan will cause no increase in 100-year flood stage over its present level between sections 39 and 41.
3. If the present State Highway 12A bridge is replaced with a proposed 600-ft (182.9-m) span bridge, the 100-year flood stage upstream between sections 39 and 41 will be decreased by from 0.3 to 0.7 ft (0.1 to 0.2 m) at a discharge of 68,000 ft<sup>3</sup>/s (1,930 m<sup>3</sup>/s), and from 0.2 to 0.5 ft (0.1 to 0.2 m) at a discharge of 58,000 ft<sup>3</sup>/s (1,640 m<sup>3</sup>/s).
4. Under present bridge conditions, mean velocities in the study reach range from 2.6 to 5.3 ft/s (0.6 to 1.6 m/s) at a discharge of 68,000 ft<sup>3</sup>/s (1,930 m<sup>3</sup>/s).
5. Under the plan variation in which the present State Highway 12A bridge is replaced by a new one of the same length, mean velocities in the study reach would range from 3.0 to 5.3 ft/s (0.9 to 1.6 m/s) at a discharge of 68,000 ft<sup>3</sup>/s.
6. Under the plan variation in which the present State Highway 12A bridge is replaced by a proposed 600-ft (182.9-m) bridge, velocities in the study reach would range from 3.0 to 5.4 ft/s (0.9 to 1.6 m/s) at a discharge of 68,000 ft<sup>3</sup>/s (1,930 m<sup>3</sup>/s).

#### SELECTED REFERENCES

- Chow, Ven Te, 1959, Open-channel hydraulics: New York, McGraw-Hill Book Co., Inc., 680 p.
- Dunn, Bernard, 1974, Hydraulic analysis, Chenango River, Broome County, New York: U.S. Geol. Survey open-file rept., 14 p.