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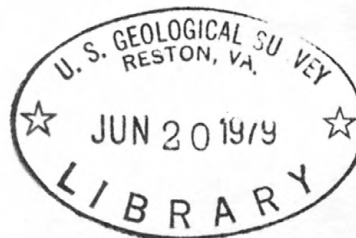
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GEOLOGICAL SURVEY

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SEISMIC ENGINEERING DATA REPORT

1974-75 RECORDS

STRONG-MOTION EARTHQUAKE ACCELEROGRAMS
DIGITIZATION AND ANALYSIS



OPEN-FILE REPORT 79-929

Prepared on behalf of the
National Science Foundation
Grant CA-114

This report is preliminary and has not been edited or reviewed for conformity
with Geological Survey standards and nomenclature

Menlo Park, California

May 1979

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U.S. GEOLOGICAL SURVEY

SEISMIC ENGINEERING DATA REPORT

1974-75 RECORDS

[Reports - Open file series]

STRONG-MOTION EARTHQUAKE ACCELEROGRAMS

DIGITIZATION AND ANALYSIS

by

A. G. Brady and V. Perez

OPEN FILE REPORT

No. 79-929

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PREFACE

This is the fourth of a series of reports planned to include the results of digitization and routine analyses of strong-motion earthquake accelerograms published by the U.S. Geological Survey. Serving as a model for this effort is the collection of data reports published by the Earthquake Engineering Research Laboratory of the California Institute of Technology during the years 1969 - 1975 and covering the significant records of the period from 1933 up to the San Fernando earthquake of February 9, 1971. Earlier reports in the present series have covered records from 1971, Peru, and 1972 (USGS Open File Report Nos. 76-609, 77-587, and 78-941, respectively). The present report includes a selection of 1974 and 1975 records.

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INTRODUCTION TO THE DATA MANAGEMENT PROJECT

A five year project for the continued operation and development of the national program in strong-motion instrumentation and data management was initiated in 1974 through an interagency agreement between the National Science Foundation and the U.S. Geological Survey. The present operation has overall direction and funding provided by the National Science Foundation while day-to-day management is provided by the U.S. Geological Survey.

Data management is concerned with archiving the original records, processing the significant records, and disseminating the resulting data and other information to the user community. One objective of data management is to develop a complete and unified processing system for the strong-motion data recorded by the networks of instruments. This report contains the results of processing the following twelve records obtained from earthquakes in 1974 and 1975:

- | | |
|---|------------------|
| 1. Gavilan College, Gilroy, Calif | 28 November 1974 |
| 2. Hollister City Hall, Calif. | 28 November 1974 |
| 3. 24 Polk St., San Juan Bautista, Calif. | 28 November 1974 |
| 4. Petrolia, Calif.: Cape Mendocino | 11 January 1975 |
| 5. Petrolia, Calif, Calif: General Store | 11 January 1975 |
| 6. Shelter Cove, Calif.: No. 1 | 6 May 1975 |
| 7. Shelter Cove, Calif.: No. 2 | 6 May 1975 |
| 8. Ferndale City Hall, Calif. | 7 June 1975 |
| 9. Petrolia: Cape Mendocino | 7 June 1975 |
| 10. Petrolia: General Store | 7 June 1975 |
| 11. Shelter Cove, Calif.: No. 1 | 7 June 1975 |
| 12. Shelter Cove, Calif.: No. 2 | 7 June 1975 |

AVAILABILITY OF DIGITAL DATA

This report contains only plots of the digital data and the analyses performed on the records. Additional copies are available from:

USGS Open File Services Section
Branch of Distribution, Box 25425
Federal Center
Denver, CO 80225

The digital data, including stage 1 uncorrected accelerations, stage 2 corrected acceleration, velocity, and displacement, and stage 3 response and Fourier spectra are available on magnetic tape, distributed by the Environmental Data and Information Service, whose address is:

NGSDC, Code D62
EDIS/NOAA
Boulder, CO 80302

Cards are also available from the Environmental Data and Information Service.

Each tape contains all the digital data that corresponds to each of the published data reports, as in the following list:

| Report No. | Tape No. | Description |
|--------------------------|----------|---------------------------------------|
| Open file report 76-609 | 71 | 1971 records |
| Open file report 77-587 | PP | Records from Lima, Peru: 1951 to 1974 |
| Open file report 78-941 | 72 | 1972 records |
| Open file report 79-929 | 74 | 1974-75 records (this report) |
| Open file report _____ | 67 | 1967-75 records |
| Open file report 78-1022 | Romania | Romanian and Greek records |

DATA PREPARATION AND DIGITIZING

The criteria and procedures used in the strong-motion data management project for the selection, preparation, and digitization of records is as follows:

Significant records. An arbitrarily selected peak amplitude of 10% g has been chosen as the main criterion for a record to be considered significant, if it is a ground level record, and 15% g if an upper story structural record. This may be altered for future reports if the number of records designated "significant" by the 10% criterion becomes greater (or less) than the current staff and program can expeditiously handle.

Film preparation. One aspect of the archiving system for the significant records is the preparation of full-size contact film negatives from the original records which were recorded in the field on photographic paper or film. This reproduction process is carried out under the supervision of the staff of the Seismic Engineering Branch by commercial photographic concerns in the San Francisco Bay Area, using the original records. From these film negatives, contact prints are made on Mylar-based film, either translucent frosted film for subsequent hand digitizing, (e.g., 6 in. or 12 in. records), or clear film for automatic digitizing procedures (e.g., 70 mm records). Measurements have shown that these prints differ in size from the original film or paper records by less than 0.1%. The Mylar film is mechanically strong, dimensionally stable, and affords excellent optical contrast. Experiments on photographic development techniques have resulted in standard methods for producing an optimum balance between contrast and trace width.

For 6 in. and 12 in. photographic paper records a choice is made, depending on the convenience of the digitizer, whether to digitize from the original or a contact print. For 70 mm or 35 mm film records, digitizing by hand requires enlargement of the original. The enlargement is 2X or 3X for 70 mm film, and 3X for 35 mm film, these being a compromise between the resultant effective sensitivity of the

acceleration traces, the original length, and the size of the vacuum table carrying the copying film. Without compromise is the requirement that the enlargement be performed in one step, using a lens of absolute minimum distortion, and viewing the entire original length in one frame.

Digitizing options. The 12 in. records and the enlarged 70 mm records in this report have been digitized at Dynamics Graphics, in Berkeley, California, on a Calma digitizing system. This includes a hand-held restrained cursor moving on a 60"-long table with resolution of 1000 points per inch and an RMS error of perhaps 0.003". Digitized output, selected at approximately 50 points per second of record time, is recorded on magnetic tape compatible with the U.C. Berkeley 6600 computer.

For 70 mm film, another digitizing facility is currently used. IOM-TOWILL, in Santa Clara, California, has a trace-following, laser scanning system capable of handling a record in 12-cm sections, with a resolution of 1 micron (10^{-6} meter) and an RMS error of the order of 10 microns. Digitized output at approximately 500 points per cm (corresponding to 500 points per sec) is recorded on computer compatible magnetic tape. The density of 500 points per cm was chosen to lessen the chance that peak values would be seriously decreased in the digitization.

CURRENT DATA PROCESSING FOR ACCELEROGRAMS

Many slight variations in standard processing continue to be made as more experience is gained. A description follows of the data processing as it existed for the analyses producing the plots in this report.

All processing of the raw digital tapes is done at the U.C. Berkeley computer center and Lawrence Berkeley Laboratory facilities. The programs developed at Caltech during the years 1968 to 1972 (Trifunac and Lee, 1973) form the basis for the processing described here, but changes have been made as the procedures were upgraded and obvious changes were made in preparing the programs for use with the CDC 6000 and 7000 computer systems. The total programming package includes the following phases:

Phase 1: processing to obtain "uncorrected" acceleration data.

Phase 2: introduction of instrument correction and baseline correction to obtain corrected acceleration, velocity, and displacement.

Phase 3: calculation of response spectra and Fourier amplitude spectra at the same values of period.

Phase 4: note that the results of the fast Fourier spectra have not been adapted for inclusion in this report.

Phase 5: calculation of time-dependent spectra

Phase 1 - Uncorrected Accelerations. This first phase of processing presents the digitized accelerograms of strong earthquake ground motions as processed from records obtained from the strong-motion accelerograph network maintained by the U.S. Geological Survey. No baseline or instrumental corrections or adjustments have been made at this stage - the data may thus be regarded as "uncorrected" in the sense that no modifications have been introduced which involve any hypotheses as to the character of the ground motions or instruments involved. This digitized data is thus believed to be as close a representation of the original, raw information as it is feasible to achieve with digital processing.

The records have been digitized on an unequal time basis, which gives, if the points are well chosen, a good definition of the trace for a given number of data points. All visible local peaks and changes of slope have been picked, along with as many intermediate points as are needed to maintain an average number of points per second of record of at least 50 throughout the accelerograms. If equispaced data is needed for a particular computer program, it is a relatively simple matter to set up an interpolation program as the first step in the computation.

Considerable thought has been given to the length of record to be digitized. Although the actual strong-motion portion of the record is the most important part for engineering purposes, a sufficient length of the later lower amplitude accelerations have been included to permit studies of long-period characteristics. Such studies are, of course, limited by other factors of instrument characteristics and accuracy. For certain investigations for which the longer-period components are not important, only a fraction of the whole digitized record might be used.

Most records contain several traces produced by "fixed" mirrors rigidly attached to the accelerograph frame. In some cases, these fixed traces depart measurably from straight lines, usually involving long-period components to be ascribed to paper distortion, motions of the paper in the drive mechanism, etc. For all records on which fixed traces are present, the fixed traces are digitized at intervals of the order of one half-second, smoothed by weighted averaging over every three consecutive points, and subtracted from the accelerometer traces as a first step in the data processing.

Timing marks on the record were also digitized and smoothed by a $1/4$, $1/2$, $1/4$ running average to form the basic time coordinate.

To fix the particular values of the digitized ordinates, some more-or-less arbitrary decision has been made as to the position of a straight reference line. When the record is placed on the table of the digitizing machine, it is lined

up with the horizontal axis of the machine as closely as can be judged by eye. For this purpose the fixed traces serve as useful guides, as do the zero trace sections at the beginning of the record before the triggering of the instrument. It must be realized, however, that imperceptible shifts of the horizontal axis in translation or rotation lead to large deviations in double integrated displacement curves, so some technique which assures a uniform result is needed. For this purpose, the following procedures have been adopted. The film record is first placed on the digitizing table with the horizontal axis lined up by eye parallel to an estimated zero axis. If the record trace and the fixed mirror traces have been digitized without moving the record on the table of the digitizing machine, then the subtraction of the two traces will correct for any slight rotation of the record on the table, so that only translation of the horizontal axis is required. This axis is therefore translated to a position which reduces the integral of the digitized acceleration curve to zero, when the integral is taken over the length of the record. This is, in principle, the same as making the mean acceleration value zero, or making the sum of the squares of the deviations from the horizontal axis a minimum. Although this means physically that the change in ground velocity from beginning to end of the record is zero, which is actually not the case for records triggered by the excitation itself, this method for selecting the horizontal axis position would seem to be the most logical choice for a standard procedure.

For those few records for which fixed traces are not available, or for which the record has been moved on the table between the digitizing of the record trace and the fixed trace, the horizontal axis is not only first translated to make the mean zero as above, but then a very small rotation is introduced to make the sum of the squares of the deviations from the zero line a minimum. This removes the effects of any slight rotational misalignment without interfering with the basic data.

It is believed that the above data processing techniques represent a minimum adjustment of the data, which consequently may be referred to as the basic

"uncorrected" data.

Computer plots of the uncorrected data are included in this report. The three components of each record are shown, except for those records where fewer than three components were digitized. They appear in the same order as on the original record, so that it is not necessary that the vertical component be the third, as has previously been the case. The scale for the time axis is standard throughout, wherever possible, so that a quick glance shows immediately whether or not there were particularly high frequency or long period ground motions present. With the high amplitudes being recorded and plotted, it is no longer possible to retain the standard scale for acceleration amplitudes.

The components mentioned, e.g. S44W, N46W, Up, indicate the direction of the transducer pendulum motion when moved by hand, for the trace to be deflected upwards on the record as it is viewed in the normal way with time increasing from left to right and the emulsion side up. This has been the standard practice since the strong-motion program began in the early thirties under the name of the Seismological Field Survey. A true ground acceleration, during earthquake motion, will be positive (or upwards) on these plots when in the opposite direction to these component labels. Records from 1978 on will have their components identified by the true ground acceleration direction for an upwards trace on the record.

Phase 2 - Corrected accelerations. The processing necessary to correct accelerograms includes the corrections for instrumental response and adjusted baseline. Preliminary smoothing is carried out first. From the uncorrected accelerograms, digitized at unequally spaced points, equally spaced data with 100 points per second are interpolated. This is low-pass filtered using an Ormsby filter (Ormsby, 1961), having a cutoff frequency of 25 cps and a roll-off termination frequency of 27 cps. Decimation follows, selecting every second point, resulting in smoothed data with 50 points per second, corresponding to a Nyquist frequency of 25 cps.

The instrument correction affects particularly those frequency components higher than about $1/2$ of the natural frequency of the instrument transducer, and is particularly important towards the upper limit of the frequency band retained, i.e., 25 cps. The correction is performed using the standard second-order differential equation governing oscillator motion together with the natural frequency and fraction of critical damping obtained from calibration tests of each accelerograph component. Differentiation of the instrumental response that is required for this correction is carried out by the simplest central difference method.

The baseline correction removes from the accelerogram all Fourier components with periods longer than an upper period limit chosen on the basis of careful tests. The upper limit for long (greater than 60 sec) hand-digitized paper records has a cut-off period of 14 sec, and roll-off termination at 20 sec - the corresponding frequencies are 0.07 and 0.05 cps. This correction procedure, developed in the Caltech standard data processing project, replaces the earlier systems of baseline correction that consisted of the least squares fitting of a constant, a straight line, or a parabola, etc.

Recent experience with records shorter than approximately 60 seconds has shown that the upper limit for long periods must be selected with due consideration given to the record length. It is not advisable to assume that a satisfactory measure of the Fourier components with periods longer than $1/4$ or $1/3$ of the record length can be obtained from any record. For records shorter than 60 seconds revised cut-off and roll-off termination periods are used. These restrict the periods retained in the corrected data to 15 seconds, or $1/4$ the record length, whichever is smaller. The actual cut-off and roll-off termination frequencies, at both sides of the pass band, are indicated in the title of the Phase 2 results.

The details of the baseline correction may be found in the reports from the Caltech data processing project (see References, Phase 2) but in general terms

the procedure is made up of the following steps:

1. Least square fit a straight line to the uncorrected acceleration as an initial baseline and initial correction to the acceleration.
2. Compute the velocity, assuming a zero initial value, and least square fit a straight line to the velocity.
3. Add the slope of this fitted line to the acceleration obtained in step 1.
4. Low-pass filter the acceleration of step 3 with a running mean filter, decimate it, and low-pass again with an Ormsby filter using the cut-off and roll-off termination frequencies given above (for 60 second records, the values are .07 and .05 cps). The remaining very long period components form the new baseline, which is subtracted from the acceleration of step 3. This complete step consequently results in a high-pass filtered acceleration.
5. Again compute the velocity, assuming zero initial values, and least square fit a straight line to the velocity.
6. Incorporate the slope of this fitted line into the acceleration of step 4.
7. Apply the complete step 4 to the velocity of step 5, resulting in a high-pass filtered velocity and a particular initial value of the velocity.
8. Compute the displacement from the velocity of step 7, assuming a zero initial value, and apply the high-pass filter of step 4 to this displacement, which also results in a particular initial value of the displacement.

The instrument correction and baseline correction outlined above result in a corrected accelerogram which represents the acceleration of the instrument support in the frequency band between a particular low level frequency and 25 cps. The accuracy of the long period Fourier components of displacement has been exten-

sively investigated (Hanks, 1973) using results obtained from the San Fernando earthquake. The accuracy is estimated to be somewhat better than 1 cm in the period range of 5-8 seconds, approximately 2 cm at periods near 10 seconds, and several (2-4) cm in the 10-15 second period range. It is quite likely here also in some of the displacement plots of phase 2 processing that some low amplitude long period noise may still be present. There are no components remaining with periods longer than those determined by the cut-off and roll-off termination periods, but it is evident that for some records this cut-off period is in fact too long if displacement errors are to be kept smaller than the estimates indicated above. The cause for this lies in the presence of noise arising primarily from random digitization noise, independent of the acceleration amplitudes. The very low amplitude of long period accelerations throughout the record therefore gives a low signal-to-noise ratio at these long periods. With this in mind, the displacement plots indicate that a cut-off period lower than that used could perhaps remove these components from the acceleration data. In the future, routine processing of subsequent records will include the ability to make a preliminary selection of a cut-off period optimal for a particular record by viewing displacements during computing runs and phase 2 processing will incorporate this selection.

Computer printout of the corrected data of phase 2 can be obtained from a companion tape to this report (See the chapter on Availability of Digital Data.). The equal time interval is 0.02 seconds. Identification labels, instrument characteristics, peak values, and initial values are included in the headings for each component. Plots of acceleration, velocity, and displacement are shown in this report with axes and scales selected suitably to show the entire record. Peak values are indicated. The definition of component direction has been described previously for the phase 1 plots. For the phase 2 plots, the same component name is retained. But for the corrected data, in order for the component name to indicate the direction of positive ground acceleration, velocity,

and displacement, the signs on the vertical axes have been changed. All ground motions, both portrayed in the plots and listed in tables, are now associated with a positive sign when in the direction of the named component direction, and such a motion is plotted downwards. As examples of this convention, if the vertical component is labelled "Down", then a phase 2 plot will present an actual downward ground displacement (or velocity, or acceleration) with a curve that is down below the axis. If a horizontal component is labelled "East", a phase 2 plot will present an eastward ground displacement (or velocity, or acceleration) with a curve that is down below the axis.

The high frequencies present in some of these records have shown rather clearly in detailed plots that 50 points per sec is not a dense enough data rate for accurate representation of high frequency content. The effect is most visible when the peak values of the uncorrected stage 1 data are compared with the corresponding values of the same points after stage 2 analysis. The relevant procedures include (a), interpolation at 0.01 sec which does not retain most peaks; (b), filtering out data above 25 Hz; (c), decimation to 0.02 sec which further cuts the high frequency peak values; and (d), the instrument correction, which in general will amplify any high frequency content. The overall effect on actual peak values is governed by (c), with the result that peak values can be severely reduced by the stage 2 processing, as initially pointed out by Hudson in the Index Volume to the Caltech Strong Motion Earthquake Accelerograms reports. The effect on the frequency content for these records, as indicated by the stage 3 spectral processing, is obviously dependent on the severity of the interpolation and decimation effects.

Recent data reduction of short duration accelerograms - as experienced in aftershocks, or accelerograms with high frequency content as indicated above - strongly suggests that digitizing density must be increased (a difficult procedure when manual digitizing is involved) and that filtering techniques must be revised to improve the representation of corrected acceleration. Unfortunately new techniques require time to develop and would cause delays in the dissemination of

data. The intent of this report is to furnish data using present available techniques so as not to delay unduly the accessibility of data to all interested parties.

Phase 3 - Response spectra. An introduction to response spectrum techniques as they are commonly applied in the earthquake engineering field may be found in the references, particularly the Caltech report, Volume IIIA. The following remarks apply primarily to the influence of the data processing techniques on the computation of these spectra.

The component directions have been described in the earlier sections of phase 1 and 2. The spectral calculations of phase 3 are concerned with absolute values of response and the particular component sense is thus immaterial.

The earthquake response spectrum was originally introduced as a means of characterizing the response of structures to the exciting ground motion, and hence such spectra are calculated from ground motions. There are many instances, however, in which a structural motion is the excitation, for example, of mechanical equipment or other objects attached at upper story locations in buildings. In such cases it may be useful to calculate a response spectrum based on an acceleration-time function which is in itself a response to the earthquake ground motion. Such response spectra of the motion of a particular structure will ordinarily contain predominant peaks corresponding to structural frequencies and will consequently appear very different from ground spectra. The reader is cautioned to keep this point in mind and to avoid such building spectra when studying the characteristics of the ground motion itself.

Response spectra calculated from the corrected accelerograms of phase 2 are presented in the data section of the report. They represent the best combination of accuracy and frequency range that it is feasible to achieve with the currently available instrumentation and data processing. As future investigations improve these data processing techniques, and as more refined instruments make it possible

to attain higher levels of accuracy and a wider frequency range, such calculations can no doubt be extended to take advantage of such improvements.

The long period noise described in the section on phase 2, and evident in some of the displacement plots appearing in this report, have an effect on the appearance of the phase 3 plots. Only experience can help in determining if the high spectral amplitudes at long periods correspond to the evident long period oscillations in some of the displacement plots. The extent to which long period noise plays a part in this behavior also becomes clear with experience and is discussed in the references under phase 2.

The phase 3 plots and printouts are presented later in this report, and on the companion tape, respectively. For each component there are two figures showing response spectra, described in detail in the following paragraphs.

The first plot is that of the true relative velocity response spectrum, RV, with an identifying descriptive title. The five continuous line plots correspond to damping values of 0, 2, 5, 10 and 20 percent of critical and these curves will usually be easily distinguishable, with the zero damped curves having the greatest ordinates.

The dashed curve on this plot is the unsmoothed Fourier amplitude spectrum, FAS, calculated at the same periods as the relative velocity response spectra. The ordinates for the spectra are in units of cm/sec, and the scale is chosen to fill the available space. The periods extend to 15 seconds, close to the maximum long period cut-off point in the corrected data of phase 2, but the axis is divided into two separated linear portions. From 0 to 3 seconds takes up three-quarters of the axis, and from 3 to 15 seconds takes up the remainder.

The second plot is that of the pseudo velocity response spectrum, PSRV, together with the relative displacement spectrum, RD, and the pseudo acceleration spectrum, PSAA, in the tripartite logarithmic plot versus period. This convenient plot is made possible by the relationships between PSRV, RD, and PSAA:

$$PSRV = (2\pi/T) RD$$

$$PSAA = (2\pi/T)^2 RD$$

The units used are cm/sec, cm, and g.

The corresponding tape contains values of the ordinates for the previous plots. First is listed the Fourier amplitude spectrum (FAS), and then five sets of four arrays containing RD, RV, PSRV, and absolute acceleration spectra (AA) for all of the five damping values.

Phase 4 - Fourier spectra. As indicated earlier in this section, the Fourier spectra calculated by fast Fourier transform subroutines are not included in this report. They will be incorporated in a subsequent report devoted solely to Fourier spectra.

Phase 5 - Time-dependent Spectral Analysis. The response spectrum was first introduced into earthquake engineering by Benioff (1934) and refined by Biot (1941). With improvement and refinement by others (Alford et al., 1951; Housner et al., 1953; Hudson, 1956), this technique has become an important tool in the design of earthquake resistant structures when dealing with buildings of simple design and special structures such as elevated water tanks. The response of each mode of multi-degree-of-freedom systems such as tall buildings, chimneys, or towers can be calculated utilizing the same equation of motion used to obtain response spectra. Each modal response can then be superposed to obtain the total response of the system (Merchant and Hudson, 1962). When the approximate design method utilizing response spectra is not sufficiently accurate and the more involved and costly technique of time-history dynamic response is needed, a response spectrum can be used for the preliminary design. Considering the importance of the response spectrum in seismic engineering, a maximum amount of information should be extracted from it. One method involves the study of response as a function of time (Perez, 1973a; Trifunac, 1971; Hays et al., 1973; Blume and Associates, 1973).

The response spectrum is a plot of the maximum response of a single-degree-of-

freedom oscillator, subjected to the particular earthquake, for a given damping factor and for a spectrum of frequencies. However, this focusing on the maximum value ultimately ignores any relationship that exists between the time history of the response and the ground motion. In particular, no information is retained on the difference in time between the strongest part of the ground motion and the strongest part of the resulting response. This type of relationship can be qualitatively investigated using time-dependent spectral analysis, which helps in understanding the effects of high levels of ground acceleration on the response spectrum (Perez, 1973b). The understanding of high levels of ground acceleration is critical, as values have been recorded as high as 1.25 g for Pacoima Dam in 1971 (Trifunac and Hudson, 1971), and 0.7 g for Melendy Ranch in 1972 (Morrill et al., 1974). It can be expected that higher peak accelerations will continue to be recorded.

Important structural engineering information can be obtained by studying in detail the length of time that the general level of velocity response, as indicated by its envelope, is greater than particular predetermined levels. Although certain levels of shaking may do minimal damage to structures at the onset of an earthquake, prolonged shaking at those levels could cause extensive damage due to progressive failure. At the present time, the correlation of building damage versus levels of response and their respective time duration has not been developed. However, attempts in these two areas are being made by several investigators. For example, Matthiesen and Rojahn (1972) have made estimates of threshold structural damage levels for various classes of buildings. The structural response duration at different levels and its relation to structural damage by low-cycle fatigue has been studied by Kasiraj and Yao (1968); Suidan and Eubanks (1973) have studied the cumulative fatigue damage in seismic structures; Popov and Bertero (1973) have studied cyclic loading of steel beams and connections.

The Velocity Response Envelope Spectrum (VRES). This section describes the construction of the contour plots of the velocity levels attained by the velocity

response envelopes. The data represented by the plots are used in the Time Duration Spectra of the next section. The original response spectrum is based on the response of the single-degree-of-freedom, viscously-damped, linear oscillator subjected to earthquake ground motion. Such an oscillator acts as a narrow-band filter which amplifies the input frequencies centered around the natural frequency of the oscillator (Trifunac, 1971). To study the response as it varies with time, the envelope of the response is used instead of the actual response. The envelope contains all of the important information required to calculate the maximum relative velocity as normally defined, while maintaining the history of the response as it varies in time. For any particular oscillator those times at which the envelope of the oscillator response rises above or falls below predetermined velocity levels are noted and the results can be plotted in the VRES two-dimensional contour plots.

The following method was used to calculate the VRES as a two-dimensional function of time and natural period. The oscillator response was computed for 41 natural periods. The periods selected were: 0.05, 0.075 seconds; from 0.1 to 1.0 seconds at 0.05 second intervals; from 1.0 to 2.0 seconds at 0.1 second intervals; and from 2.0 to 4.0 seconds at 0.2 second intervals. This scheme was chosen to obtain an appropriate density distribution at the higher frequency end of the spectrum. For each period the response envelope was approximated by connecting the absolute value of the local peaks of the response curve. The envelope curve was then interpolated at 160 equal intervals regardless of length of time (i.e. if length of time was 16 sec, then interpolation interval was 1/10 sec). Levels were chosen according to the maximum response, with no more than six levels, regardless of amplitude.

These 41 periods, with their respective VRES calculated at equal time intervals, represent a rectangular grid of spectral values. Contours of equal amplitude were produced by plotting interpolated values from the grid, giving a

contour map of the VRES amplitude values as a function of time and period. The maximum relative velocity response spectrum is plotted to the right of the contour map. The contour map shows the peaks and valleys of the VRES as a function of time and period, while the maximum relative velocity response spectrum shows the silhouette of the peaks. The input acceleration is plotted below the contour map to show the relationship between acceleration and the VRES as they vary in time.

In most instances, connecting the local peaks of the relative response is a good approximation to the response envelope (Perez, 1974). In some instances, the high frequency content in the relative response gives an envelope of these high frequencies and not of the period being analyzed. A better approximation to the envelope of the response would be obtained by filtering out frequencies appreciably higher than the ones being studied, without altering either the phase or the magnitude of the response of a given oscillator. Since filtering would eliminate the high frequency information that exists in the relative response of longer period oscillators, and since the VRES plots can easily be inspected when these high frequencies are present, no smoothing of the response is contemplated at the present time.

Displacement and Pseudo-absolute Acceleration Response Envelope Spectrum

(DRES AND ARES) Time-dependent spectral analysis may be expressed not only as relative velocity (VRES), but also as relative displacement (DRES) and absolute acceleration (ARES). Relative displacement is important because the shear force exerted by the columns of a structure on the ground are directly proportional to the relative displacement. The absolute acceleration is a measure of the seismic forces acting on the mass of a structure.

A partial solution to obtain VRES, DRES, and ARES is to approximate these quantities through a calculation commonly used by structural engineers. If the response is assumed to be approximately sinusoidal, then the displacement response can be approximated by dividing the velocity response by $\omega_0 = 2\pi/T_0$ (T_0 is

the natural period of the oscillator) and the absolute acceleration response can be approximated by multiplying the velocity by ω_0 .

The damping of structures undergoing small amplitude oscillations is generally found to be in the range of 1 to 5 percent critical damping; for structures behaving in the nonlinear and plastic range, the damping can be 10 percent or more (Trifunac, 1971). In this analysis, 5 percent critical damping was used in calculating the VRES.

Since the period range for the VRES is between 0.05 and 4.0 seconds, harmonic theory indicates that the response delay to any significant pulse should be no more than 0.0125 to 1 second (i.e., period/4). However, an examination of the VRES and the acceleration record incorporated alongside shows that the maximum acceleration and the maximum velocity response need not be separated by this delay time interval (Perez, 1974).

Time Duration Spectrum of the Response Envelope. From an engineering point of view, it is important to study not only the peak response and time of occurrence, but also the time duration above a given level of response (Perez, 1973a). The time duration spectrum is defined as the cumulative total time that the VRES equalled or exceeded a given level during the entire acceleration record. In the time duration spectra plots, the levels of response chosen are identical to those chosen for the computation of the VRES plots. The total time duration of different amplitude levels of the VRES can also be expressed in terms of the number of cycles that occurred at or above a particular level. Due to the filtering properties of a simple harmonic oscillator, the dominant period of the velocity response is approximately equal to the natural period of the oscillator. Therefore, by dividing the duration by the period of the oscillator, a family of straight lines indicating the number of cycles for a given velocity response level can be generated.

BIBLIOGRAPHY

General

- Trifunac, M. D. and Lee, V., 1973, Routine Computer Processing of Strong Motion Accelerograms, Report No. EERL 73-03, Earthquake Engineering Research Laboratory, Caltech, Pasadena.
- USGS, 1976, "Seismic Engineering Data Report: 1971 Records" USGS Open file report No 76-609.
- USGS, 1977, "Seismic Engineering Data Report: Records from Lima, Peru, 1951 to 1974," USGS Open file report No. 77-587.
- Brady, A. G., and Perez, V., 1978, "Seismic Engineering Data Report: 1972 Records", USGS Open file report No. 78-941.

Phase I

- Strong Motion Earthquake Accelerograms, Vol. I, Part A, Rept. No. EERL 70-20, EERL, Caltech, Pasadena, 1969.
- Strong-Motion Earthquake Accelerograms, Vol. I, Part G, Rept. No. EERL 72-20, EERL, Caltech, Pasadena, 1972.
- Strong-Motion Earthquake Accelerograms, Vol. I, Part P, Rept. No. EERL 73-21, EERL, Caltech, Pasadena, 1973.

Phase 2

- Strong-Motion Earthquake Accelerograms, Vol. II, Part A, Rept. No. EERL 71-50, EERL, Caltech, Pasadena, 1971.
- Strong-Motion Earthquake Accelerograms, Vol. II, Part B, Rept. No. EERL 72-50, EERL, Caltech, Pasadena, 1972.
- Trifunac, M. D., 1971, Zero Baseline Correction of Strong Motion Accelerograms, Bull. Seis. Soc. Am., v. 61, no. 5, p. 1201.
- _____, 1972, A Note on Correction of Strong-Motion Accelerograms for Instrument Response, Bull. Seis. Soc. Am., v. 62, no. 1, p. 401.
- Hanks, T. C., 1975, Strong Ground Motion of the San Fernando, California Earthquake: Ground Displacements, Bull. Seis. Soc. Am., v. 65, no. 1, p. 193.

_____ 1973, Current Assessment of Long Period Errors, in Strong Motion Earthquake Accelerograms - Vol. IIG, Earthquake Engineering Research Laboratory, EERL 73-52, Caltech, Pasadena.

Ormsby, J.F.A., 1961, Design of Numerical Filters with Application to Missile Data Processing: Association for Computing Machinery Journal, v. 8, p. 440-446.

Phase 3

Strong Motion Earthquake Accelerograms, Vol. III, Part A, Rept. No. EERL 72-80, EERL, Caltech, Pasadena, 1972.

Housner, G. W., 1941, Calculating the Response of an Oscillator to Arbitrary Ground Motion: Bull. Seis. Soc. Am., v. 31, p. 143-149.

Hudson, D. E., 1962, Some Problems in the Application of Spectrum Techniques to Strong-Motion Earthquake Analysis: Bull. Seis. Soc. Am., v. 52, p. 417-430.

Nigam, N. C., and Jennings, P. C., 1969, Calculation of Response Spectra from Strong Motion Earthquake Records: Bull. Seis. Soc. Am., v. 59, p. 909-922.

Phase 5

Alford, J. L., Housner, G. W., and Martel, R. R., 1951, Spectrum Analyses of Strong Motion Earthquakes: Earthquake Eng. Res. Lab., Pasadena, CA, (Revised, 1964).

Benioff, H., 1934, The Physical Evaluation of Seismic Destructiveness: Bull. Seis. Soc. Am., v. 24, p. 398-403.

Biot, M. A., 1941, A Mechanical Analyzer for the Prediction of Earthquake Stress: Bull. Seis. Soc. Am., v. 31, p. 151-171.

Blume and Associates, 1973, Recommendations for Shape of Earthquake Response Spectra: Report for the Directorate of Licensing, U.S. Atomic Ener. Comm., Washington, DC.

Hays, C. F., Bennet, T. J., and Brumley, M. D., 1973, Time Dependent Spectral Analysis of Ground Motion: Environmental Research Corp., Las Vegas, Nevada.

- Housner, G. W., Martel, R. R., and Alford, J. L., 1953, Spectrum Analysis of Strong-Motion Earthquakes: Bull. Seis. Soc. Am., v. 43, p. 97-119.
- Hudson, D. E., 1956, Response Spectrum Techniques in Engineering Seismology: Proc. World Conf. Earthquake Engineering. Univ. of California, Berkeley, Calif.
- Kasiraj, I. and Yao, J.T.P., 1968, Low-cycle Fatigue Failure of Seismic Structures: Technical report CE-11(68) NSF 065, University of New Mexico.
- Matthiesen, R. B. and Rojahn, C. 1972, Techniques for Seismic Zoning-Structural Considerations: Proc. of the International Conf. on Microzonation for Safer Construction Research and Applications, Seattle, Wash., v. II, p. 929-942.
- Merchant, H. E. and Hudson, D. E., 1962, Mode Superposition in Multi-Degree of Freedom Systems Using Earthquake Response Spectra: Bull. Seis. Soc. Am., v. 52, p. 405-416.
- Morrill, B. J., Perez, V., and Maley, R. P., 1974, Acceleration Records from Bear Valley, Calif., Earthquake of September 4, 1972, Abstract: Seis. Soc. Am., Annual meeting, Abstract with Program, Las Vegas, Nevada.
- Perez, V., 1973a, Velocity Response Envelope Spectrum as a Function of Time, for Pacoima Dam, San Fernando Earthquake, February 9, 1971: Bull. Seis. Soc. Am., v. 63, p. 299-313.
- _____ 1973b, Peak Ground Acceleration and Their Effect on the Velocity Response Envelope Spectrum as a Function of Time, San Fernando Earthquake, February 9, 1971: Proc. Fifth World Conf. on Earthquake Engineering, Rome, Italy.
- _____ 1974, Time Dependent Spectral Analysis of Thirty-One Strong-Motion Earthquake Records: U.S. Geological Survey Open File Rept. 74-48, 144 p.
- Popov, E. P. and Bertero, V. V., 1973, Cyclic Loading of Steel Beams and Connections: Journal of the Structural Div., ASCE, v. 99, p. 1189-1204.
- Suidan, M. T. and Eubanks, R. A., 1973, Cumulative Fatigue Damage in Seismic Structures. Journal of the Structural Div., ASCE, v. 99, p. 923-943.

Trifunac, M. D., 1971, Response Envelope Spectrum and Interpretation of Strong Earthquake Ground Motion: Bull. Seis. Soc. Am., v. 61, p. 343-356.

Trifunac, M. D., and Hudson, D. E., 1971, Analysis of the Pacoima Dam Accelerogram: Bull. Seis. Soc. Am., v. 61, p. 1393-1411.

SUMMARY OF RECORDS INCLUDED IN THIS REPORT

This report contains plots of the data reduction and spectral analysis of twelve selected strong-motion earthquake accelerograms recorded at eight different stations, generated by four earthquakes that occurred in the U.S. during 1974 and 1975. Information on the earthquakes, stations, and records are summarized in Tables 1, 2, and 3, respectively, which include the required cross-referencing data. Plots of uncorrected accelerograms; corrected acceleration, velocity, and displacement; spectra for velocity response, pseudo-velocity response, and Fourier amplitude; velocity response envelope spectrum; and duration of velocity response envelope, for each of the components of the records studied are presented in the following figures.

Note: 1. The title for Shelter Cove, Calif., No. 2, for the earthquake of June 7, 1975, on the response spectrum plots, should read S20E, not S20W.

2. The plots are reproduced from glossy prints which in turn have been made from 35 mm microfilm output from the LBL computing center.

3. The vertical component's trace for the instrument at Shelter Cove, Calif., No. 2 was partially obscured by the reference trace for the events of May 6 and June 7, 1975, and was consequently not digitized. Shelter Cove No. 1 vertical trace was not digitized for the event of June 7, 1975.

References

Maley, R. P., Porcella, R. L., and Perez, V., 1978, Strong-motion data data from the Humboldt County, Calif. Earthquake of June 7, 1975, Earthquake Notes, 49, 1, Jan.-March 1978.

Table 1
EARTHQUAKE DATA

| Eq. No. | Location | Date | Origin Time | Epicenter | Depth (km) | Mag. | Max MMI | Felt Area (km ²) | Records |
|---------|-----------------|-----------|-------------|---------------------|------------|------|---------|------------------------------|------------------|
| 1 | Hollister area | 28 Nov 74 | 2301 UTC | 36.9 N 121.5 W | 9 | - | VI | 18,000 | 1,2,3 |
| 2 | Northern Calif. | 11 Jan 75 | 1737 PST | 40.22N 124.26W | 2 | 4.7 | VI | 7,000 | 4,5 |
| 3 | Northern Calif. | 6 May 75 | 1835 PST | 40.28N 124.67W | - | 4.0 | - | - | 6,7 |
| 4 | Northern Calif. | 7 June 75 | 0846 UTC | 40.57 N 124.14 W | 21 | 5.7 | VII | 25,000 | 8,9,10, 11,12 |

Table 2
STATION DATA

| Station No. | Identification Name | Coords. | Type, owner-S/N | Structure type/size | Instr. location | Data source | Records |
|-------------|---|---------------------|-------------------|---------------------|-----------------|-------------|---------|
| 1023 | Ferndale, Calif City Hall | 40.58 N 124.26 W | S-M FS-23 | 2-story bldg. | Ground level | USGS | 8 |
| 1028 | Hollister, Calif. City Hall | 36.85 N 121.40 W | S-M FS-24 | 1-story bldg. | Basement | USGS | 2 |
| 1249 | Petrolia, Calif. Cape Mendocino | 40.35 N 124.35 W | SMA-1 MG-1601 | Inst. shltr. | Ground level | USGS | 4,9 |
| 1250 | Gilroy, Calif. Gavilan College | 36.97 N 121.57 W | SMA-1 MG-760 | 1-story bldg. | Ground level | USGS | 1 |
| 1277 | Shelter Cove, Calif. Shelter Cove No. 1 | 40.04 N 124.06 W | RFT-250 MG-487 | Inst. shltr. | Ground level | USGS | 6,11 |
| 1278 | Shelter Cove, Calif. Shelter Cove No. 2 | 40.03 N 124.06 W | RFT-250 MG-485 | Inst. shltr. | Ground level | USGS | 7,12 |
| 1377 | San Juan Bautista, Calif. 24 Polk St. | 36.86 N 121.54 W | SMA-1 MG-1678 | 1-story bldg. | Ground level | USGS | 3 |
| 1398 | Petrolia, Calif. General store | 40.32 N 124.29 W | SMA-1 MG-1684 | Inst. shltr. | Ground level | USGS | 5, 10 |

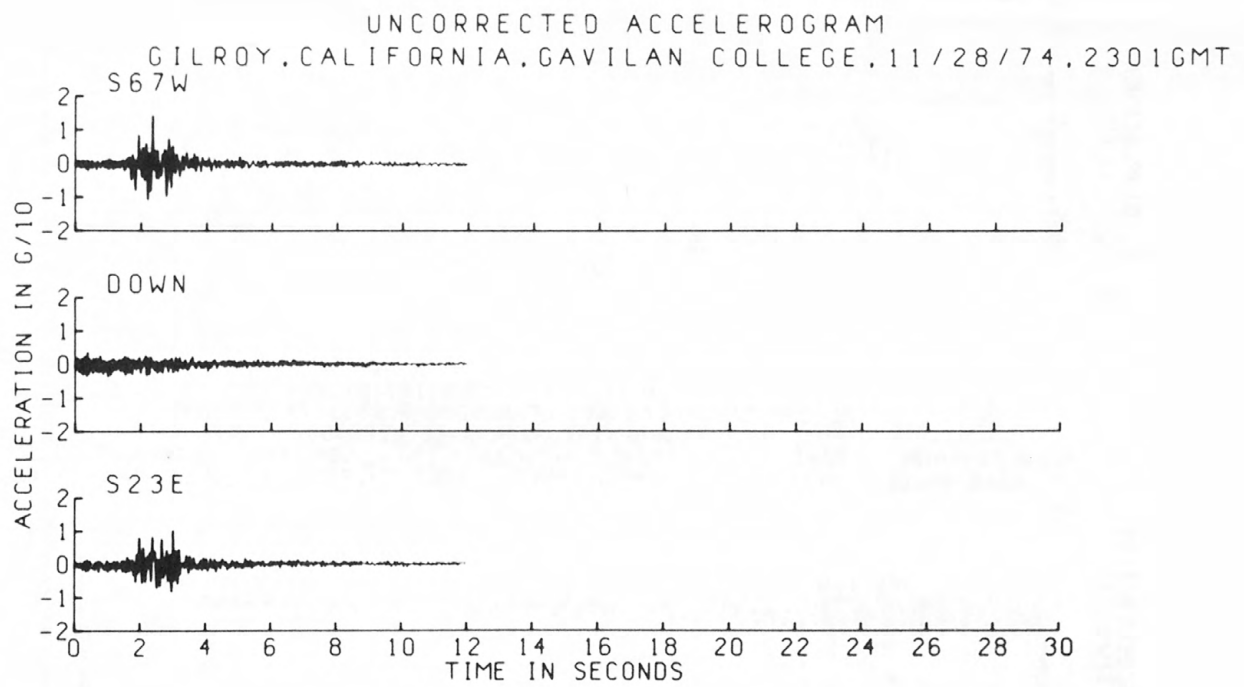
MG: Calif. Division of Mines and Geology
FS: U.S. Geological Survey

Table 3
RECORD DATA

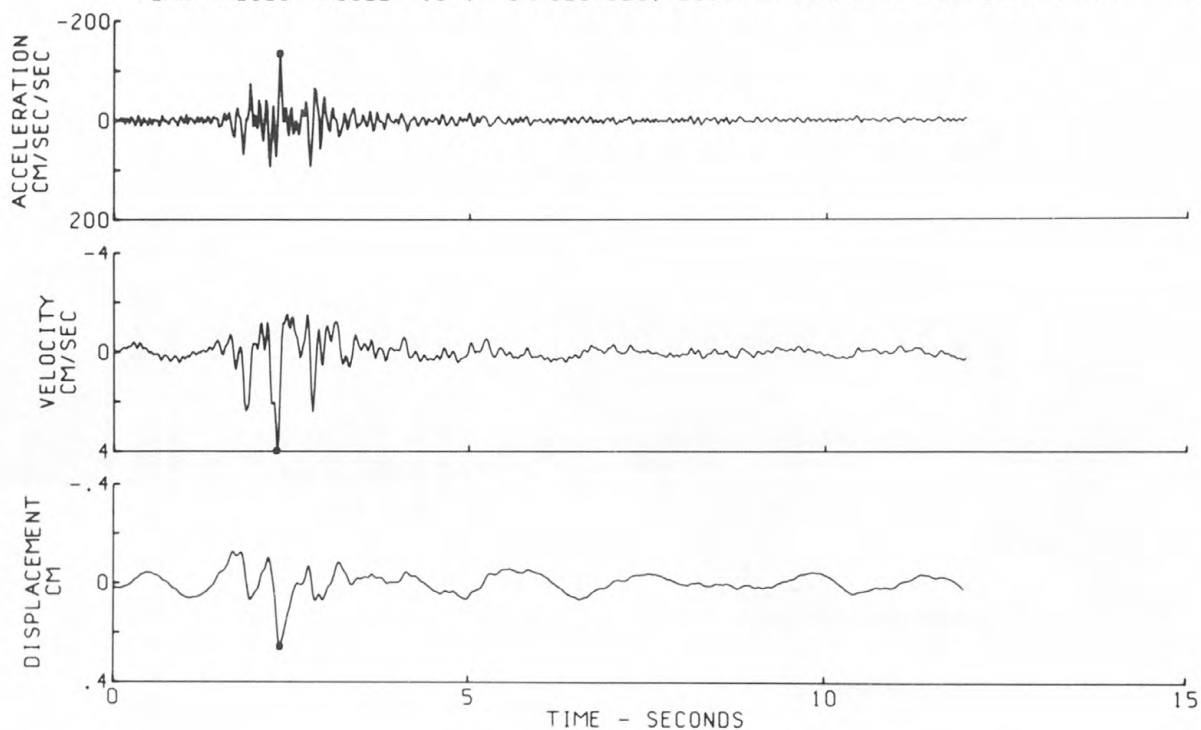
| No. | Date | Eq. No. | Station | Stn. No. | Δ (km) | MMI | Comp. | Sens. cm/g | Period (sec) | Damp fraction | Dig'd length (sec) |
|-----|-----------|------------|-------------------------------------|-------------|---------------|-----|----------------------|----------------------|----------------------|-------------------|-----------------------|
| 1 | 28 Nov 74 | 1 | Gilroy Gavilan College | 1250 | 13 | V | S67W Down S23E | 1.80 1.80 1.85 | .038 .039 .038 | .59 .59 .59 | 12 |
| 2 | 28 Nov 74 | 1 | Hollister City Hall | 1028 | 11 | VI | Up S01W N89W | 13.8 12.9 12.7 | .067 .065 .065 | .58 .62 .50 | 33 |
| 3 | 28 Nov 74 | 1 | San Juan Bautista 24 Polk St. | 1377 | 8 | V | S57E Down N33E | 1.80 1.78 1.86 | .039 .039 .039 | .59 .59 .59 | 20 |
| 4 | 11 Jan 75 | 2 | Petrolia Cape Mendocino | 1249 | 27 | VI | S60E Down N30E | 1.91 1.90 1.89 | .040 .040 .039 | .59 .59 .59 | 8 |
| 5 | 11 Jan 75 | 2 | Petrolia General Store | 1398 | 11 | VI | N75E Down N15W | 1.81 1.79 1.94 | .038 .038 .040 | .59 .59 .59 | 11 |
| 6 | 6 May 75 | 3 | Shelter Cove Shelter Cove #1 | 1277 | 34 | - | N70W Down S20W | 2.00 1.89 1.90 | .048 .048 .047 | .59 .59 .59 | 12 |

Table 3 (Continued)
RECORD DATA

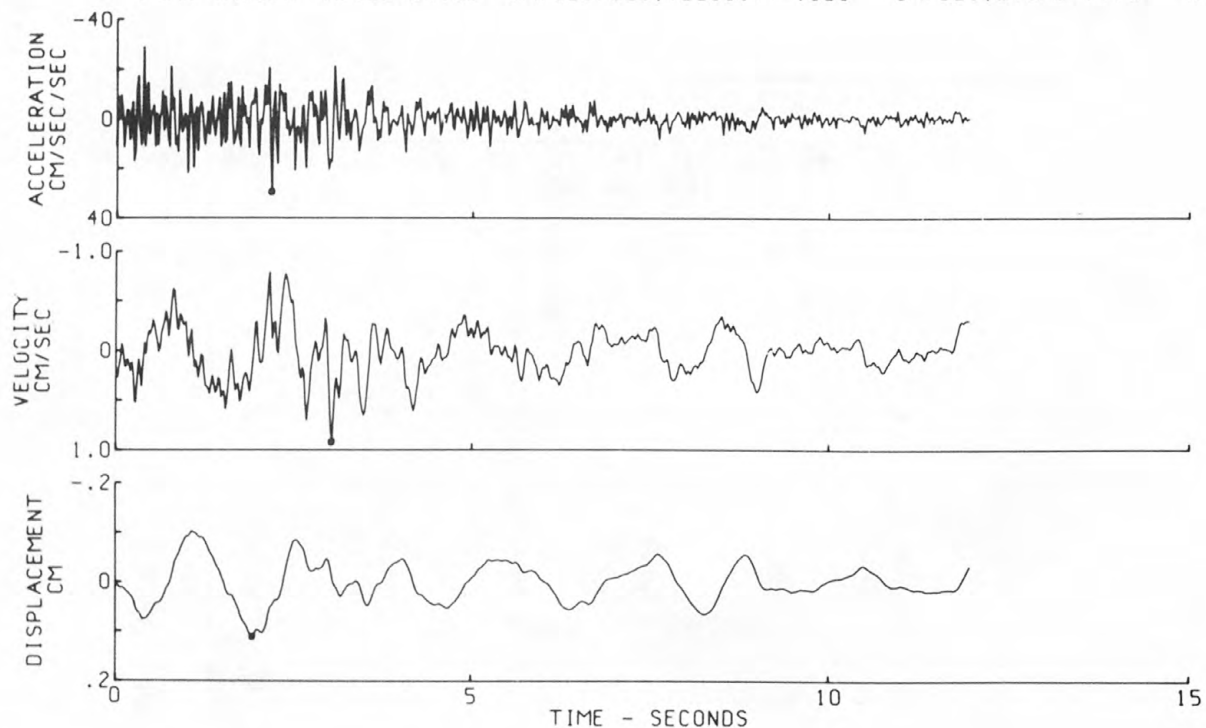
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|-----|-----------|------------|---------------------------------|-------------|---------------|-----|----------------------|----------------------|----------------------|-------------------|-----------------------|
| 7 | 6 May 75 | 3 | Shelter Cove Shelter Cove #2 | 1278 | 34 | - | S20E Down N70E | 1.87 1.95 1.90 | .045 .046 .047 | .59 .59 .59 | 14 0 14 |
| 8 | 7 June 75 | 4 | Ferndale City Hall | 1023 | 5 | VII | Up S44W N46W | 13.8 13.8 12.3 | .068 .067 .065 | .57 .57 .59 | 72 |
| 9 | 7 June 75 | 4 | Petrolia Cape Mendocino | 1249 | 22 | VI | S60E Down N30E | 1.91 1.90 1.89 | .040 .040 .039 | .59 .59 .59 | 14 |
| 10 | 7 June 75 | 4 | Petrolia General Store | 1398 | 24 | VII | N75E Down N15W | 1.81 1.79 1.94 | .038 .038 .040 | .59 .59 .59 | 17 |
| 11 | 7 June 75 | 4 | Shelter Cove Shelter Cove #1 | 1277 | 59 | - | N70W Down S20W | 2.00 1.89 1.90 | .048 .047 .047 | .59 .59 .59 | 11 0 11 |
| 12 | 7 June 75 | 4 | Shelter Cove Shelter Cove #2 | 1278 | 60 | - | S20E Down N70E | 1.87 1.95 1.90 | .045 .045 .047 | .59 .59 .59 | 15 0 15 |



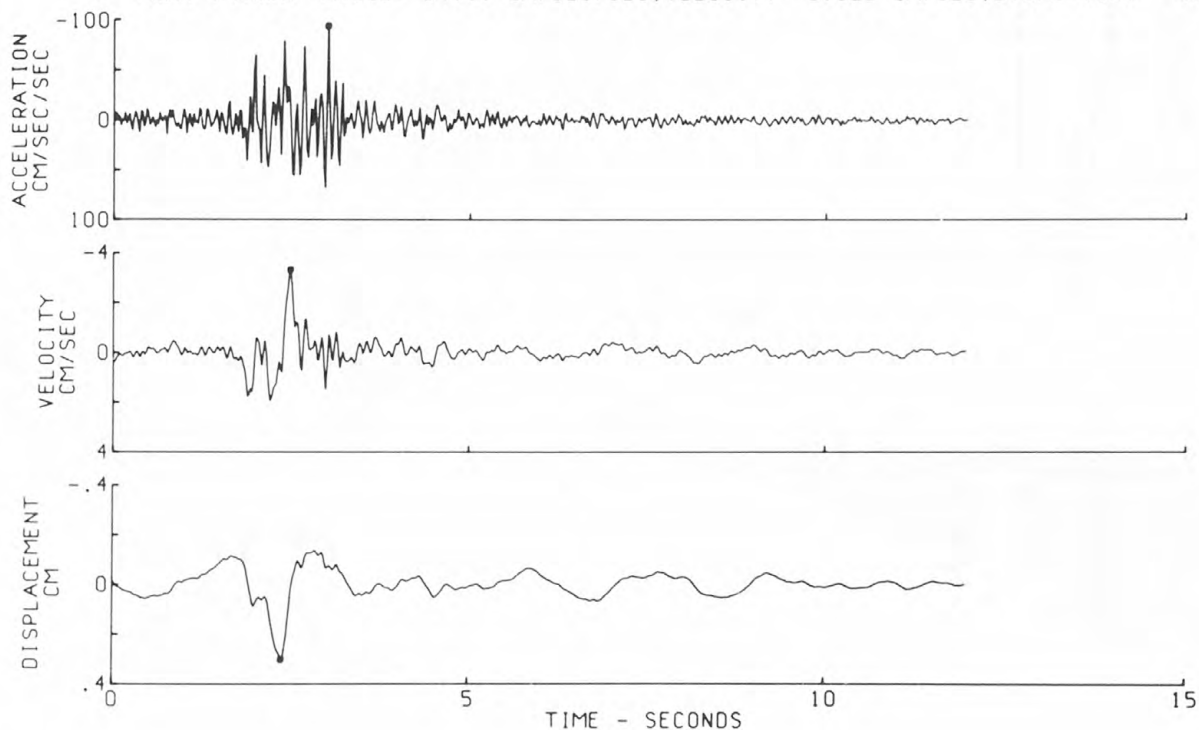
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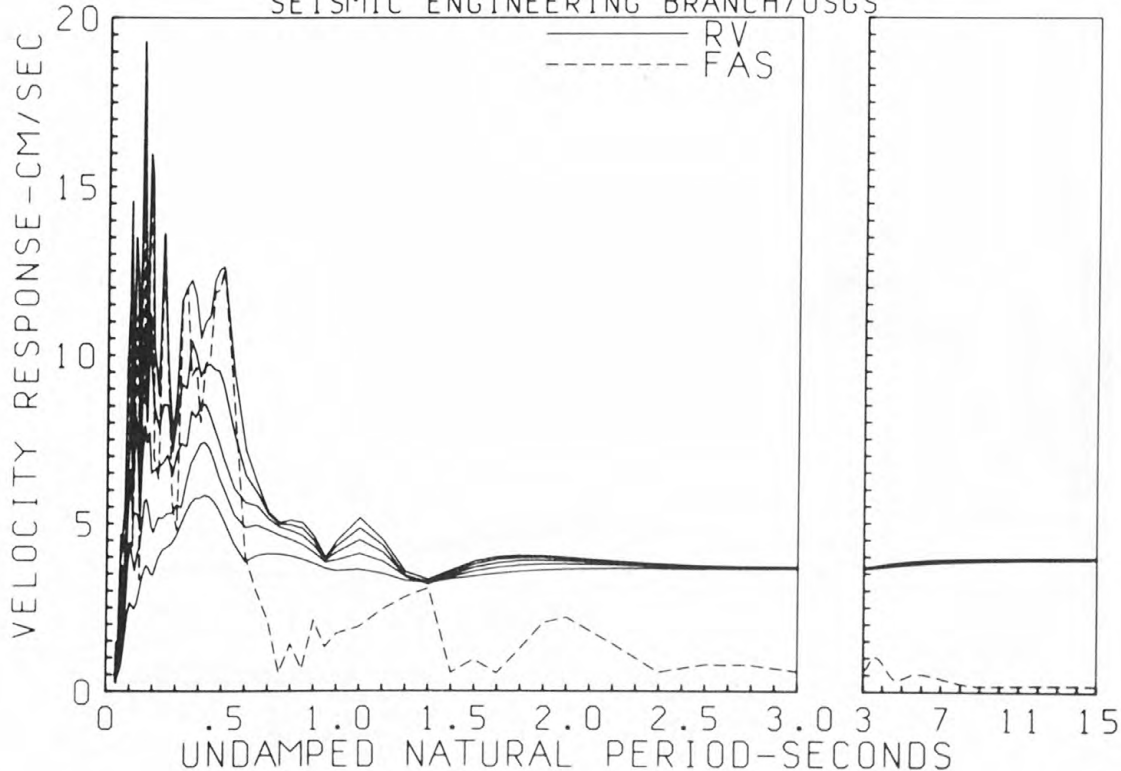
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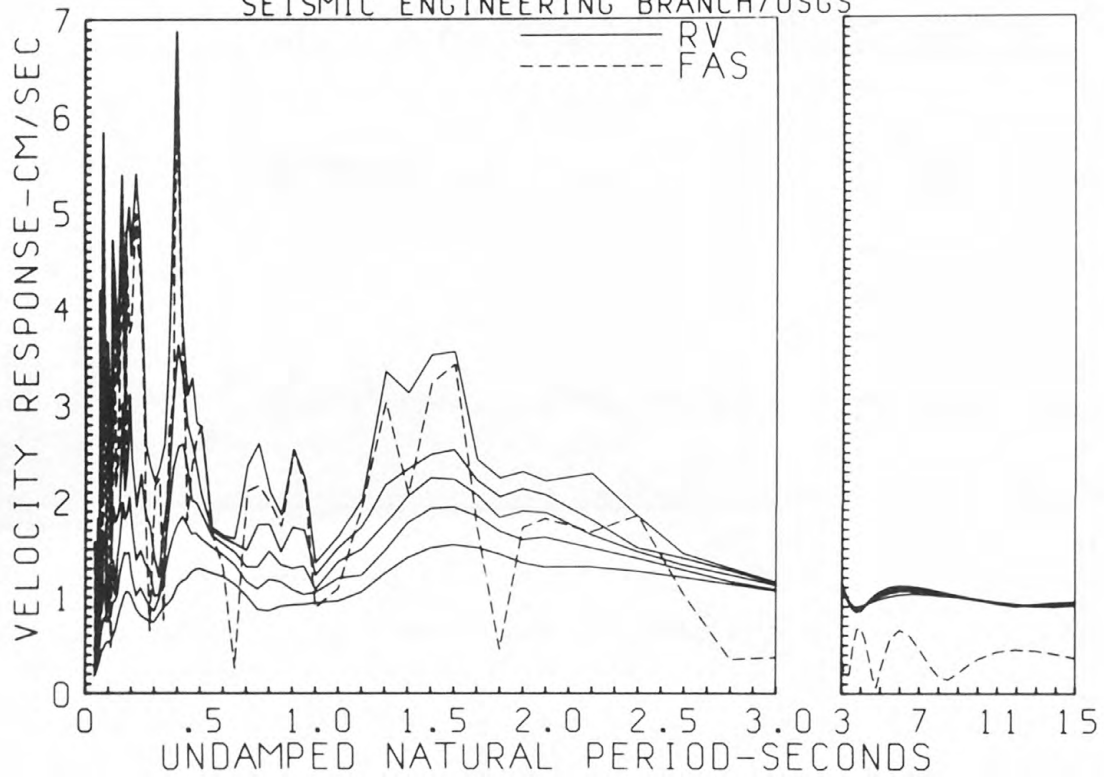
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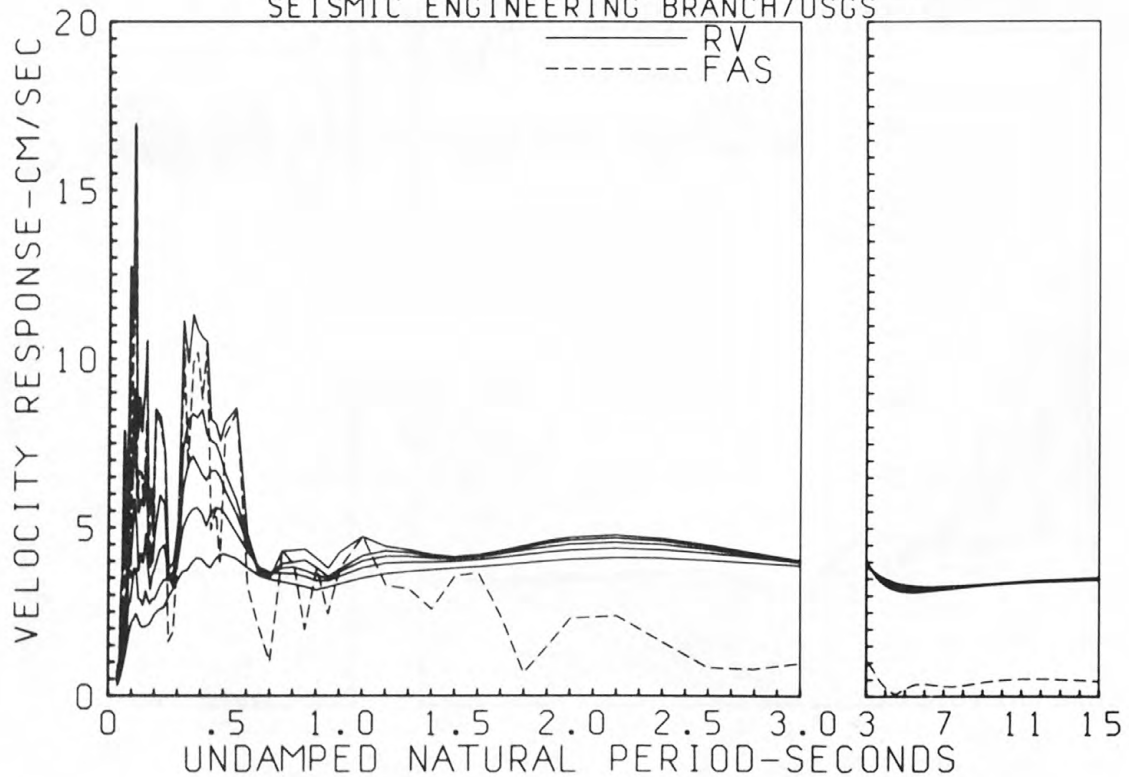
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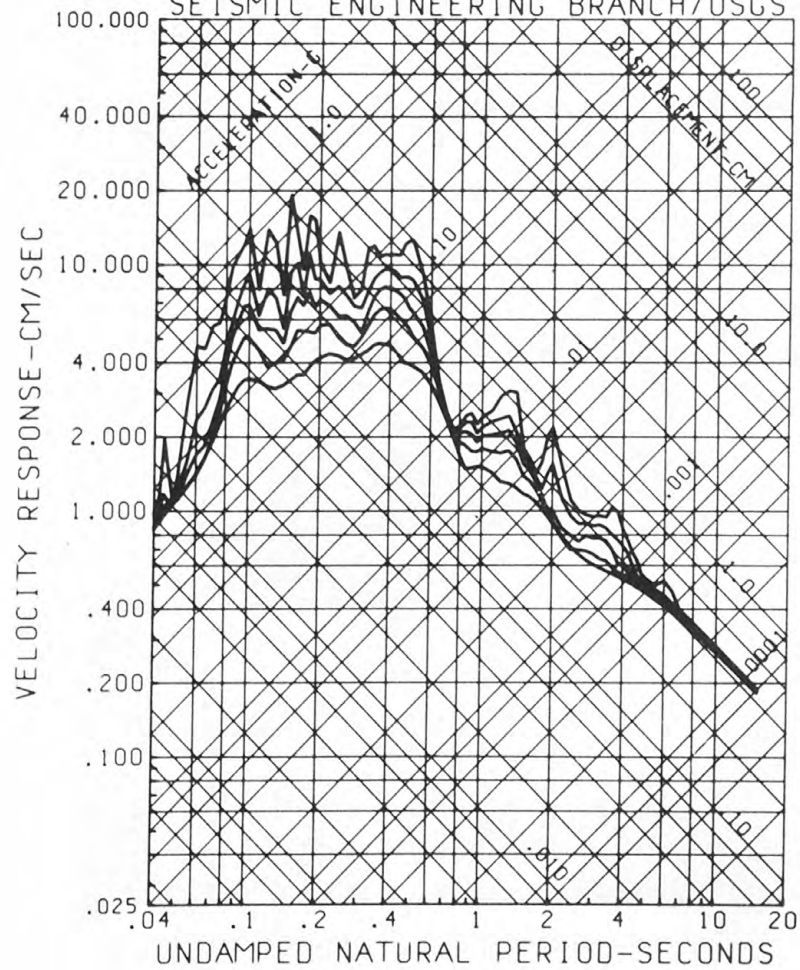
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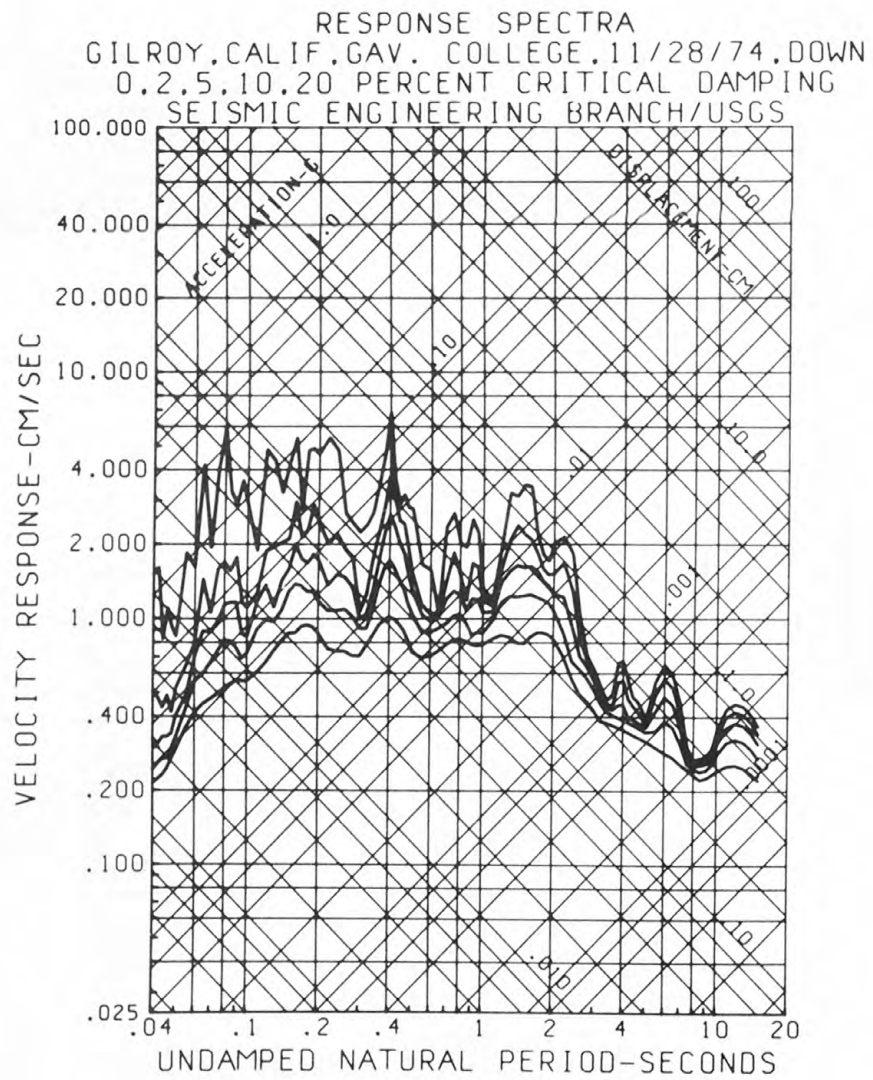


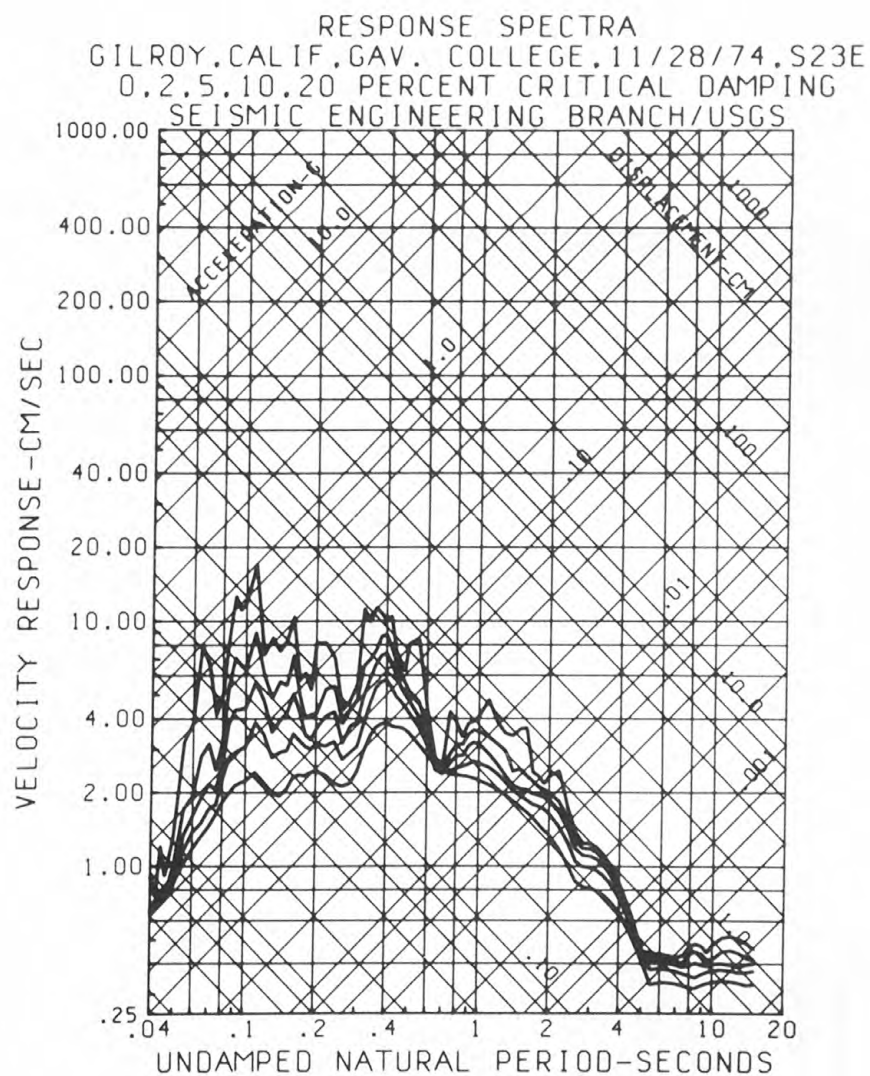
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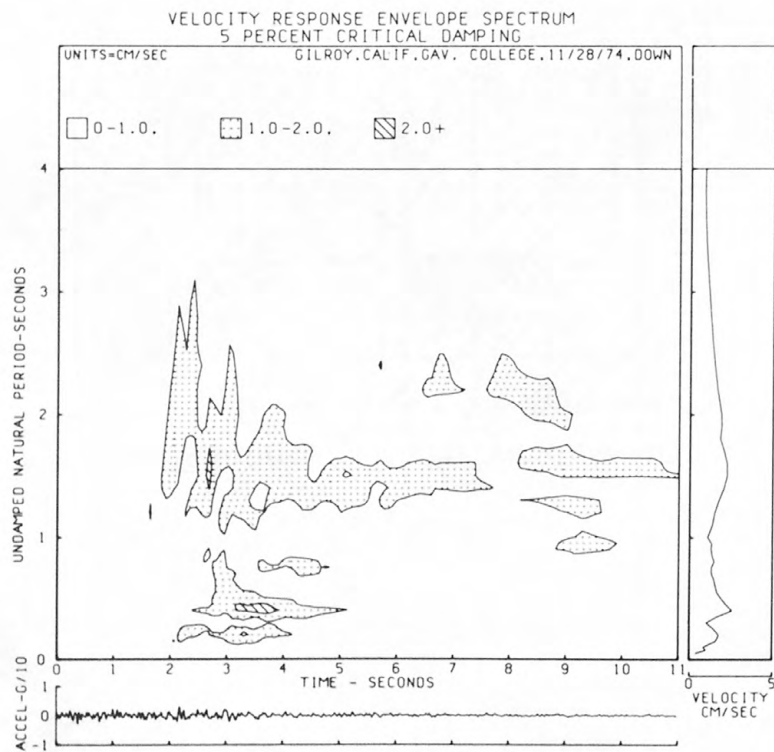
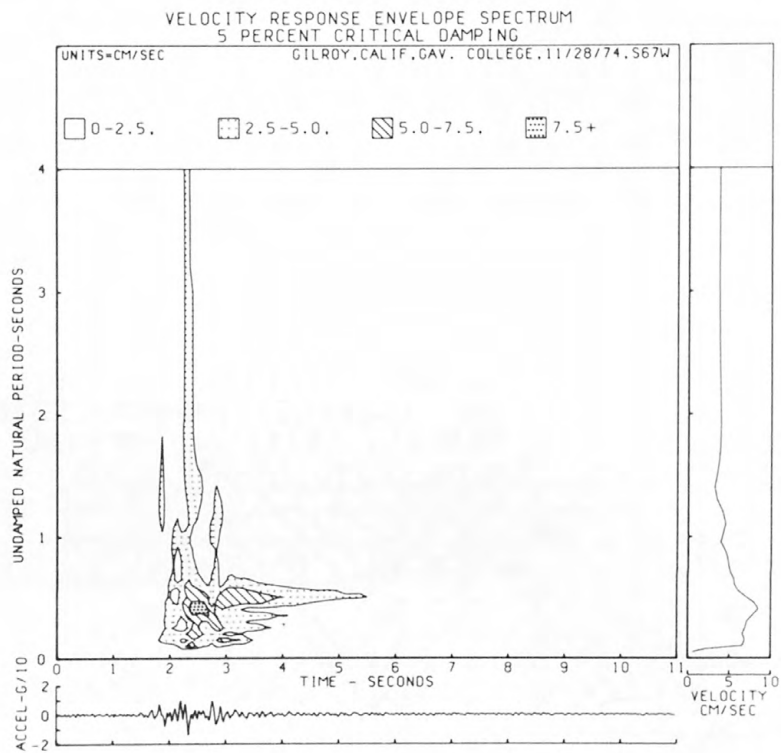


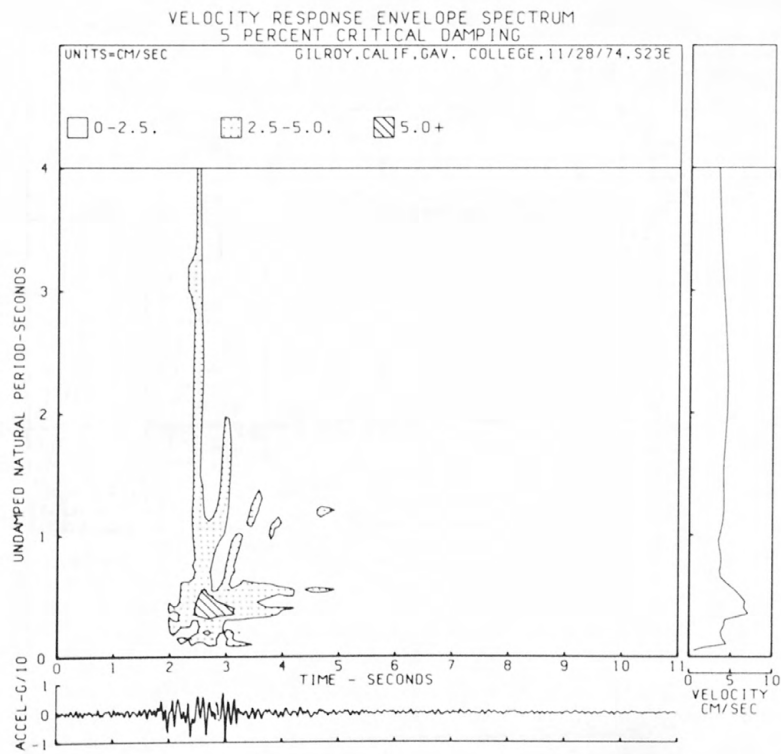
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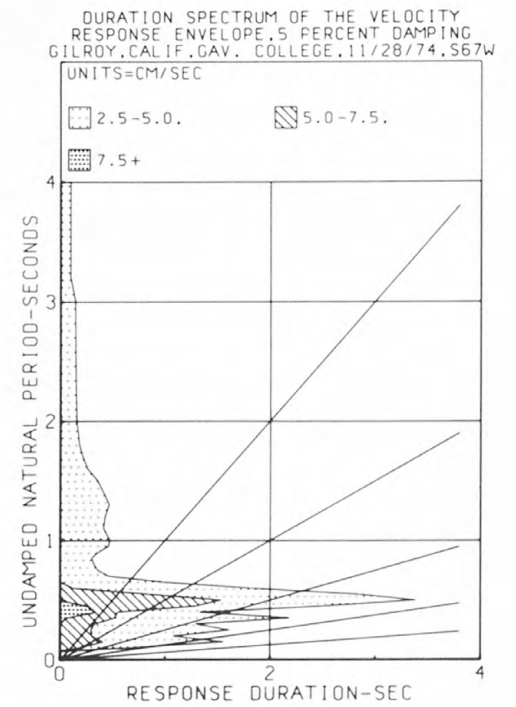
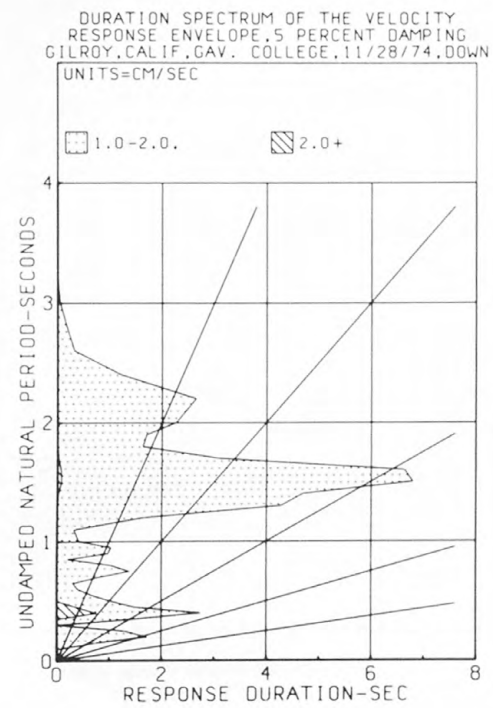
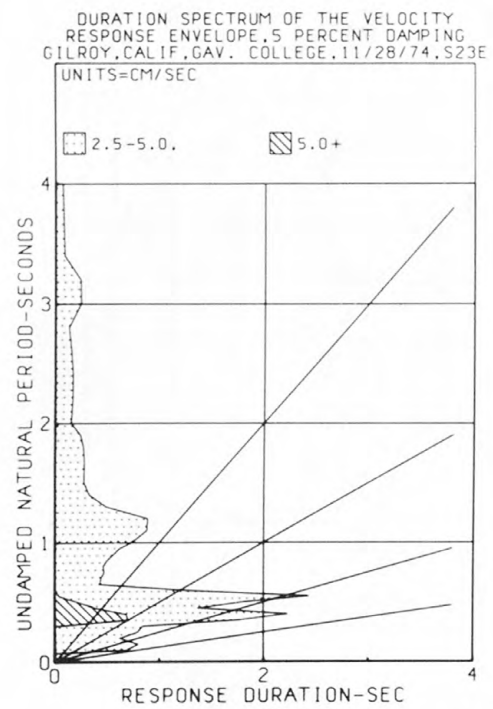


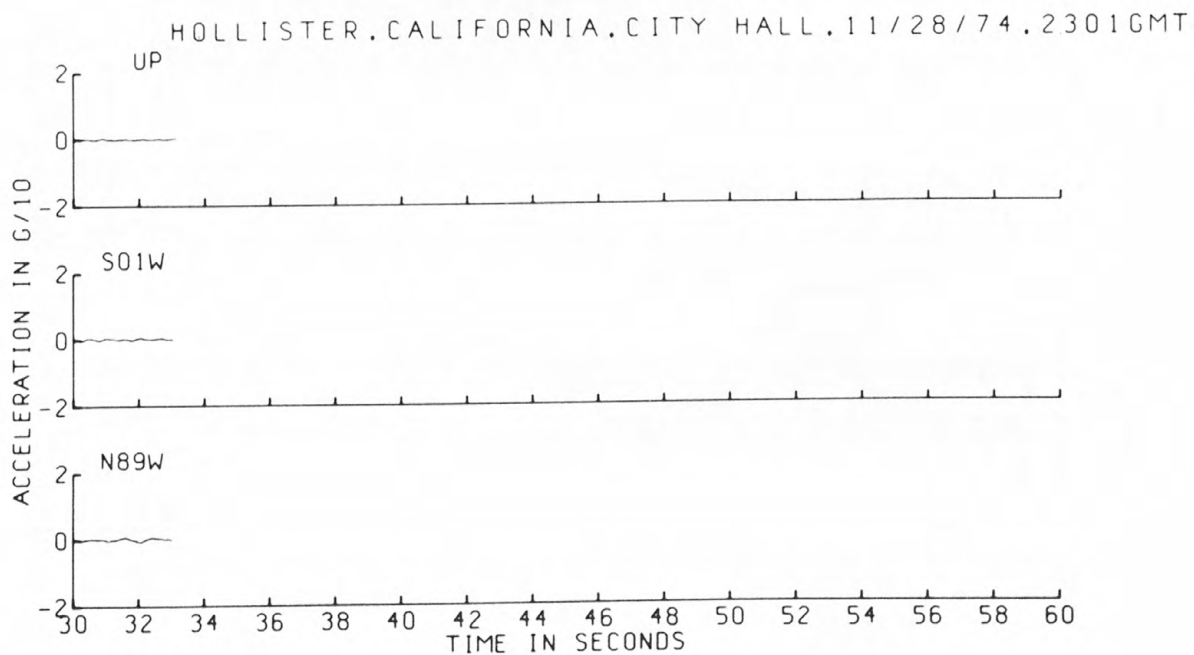
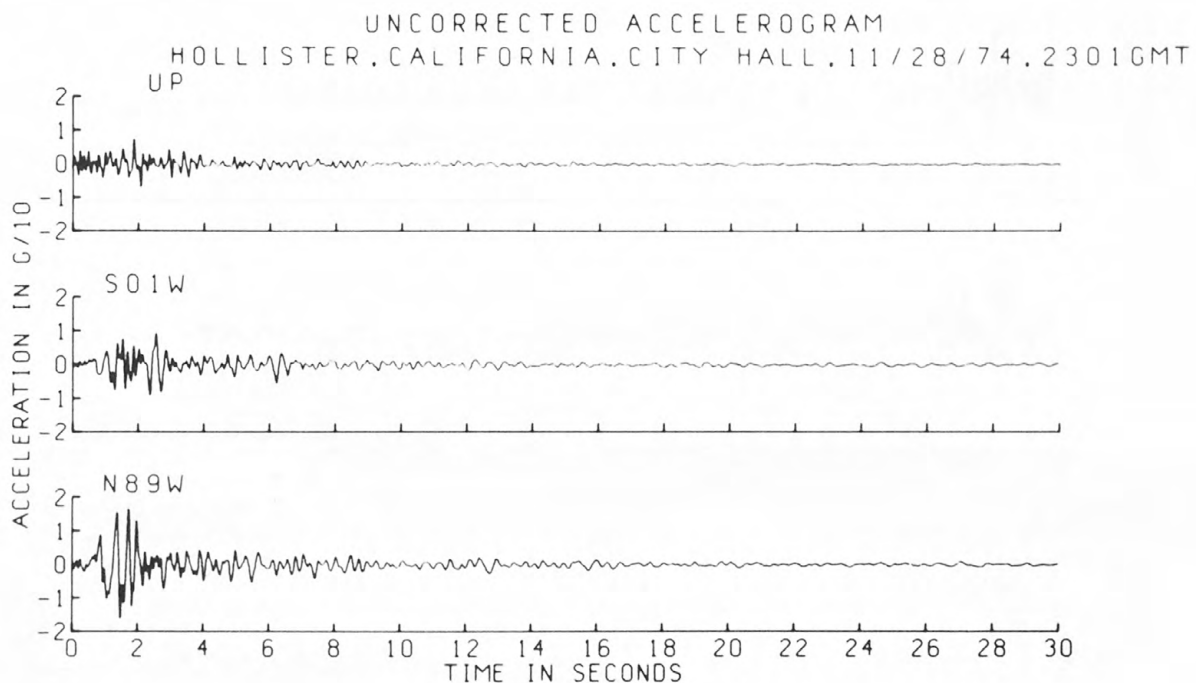




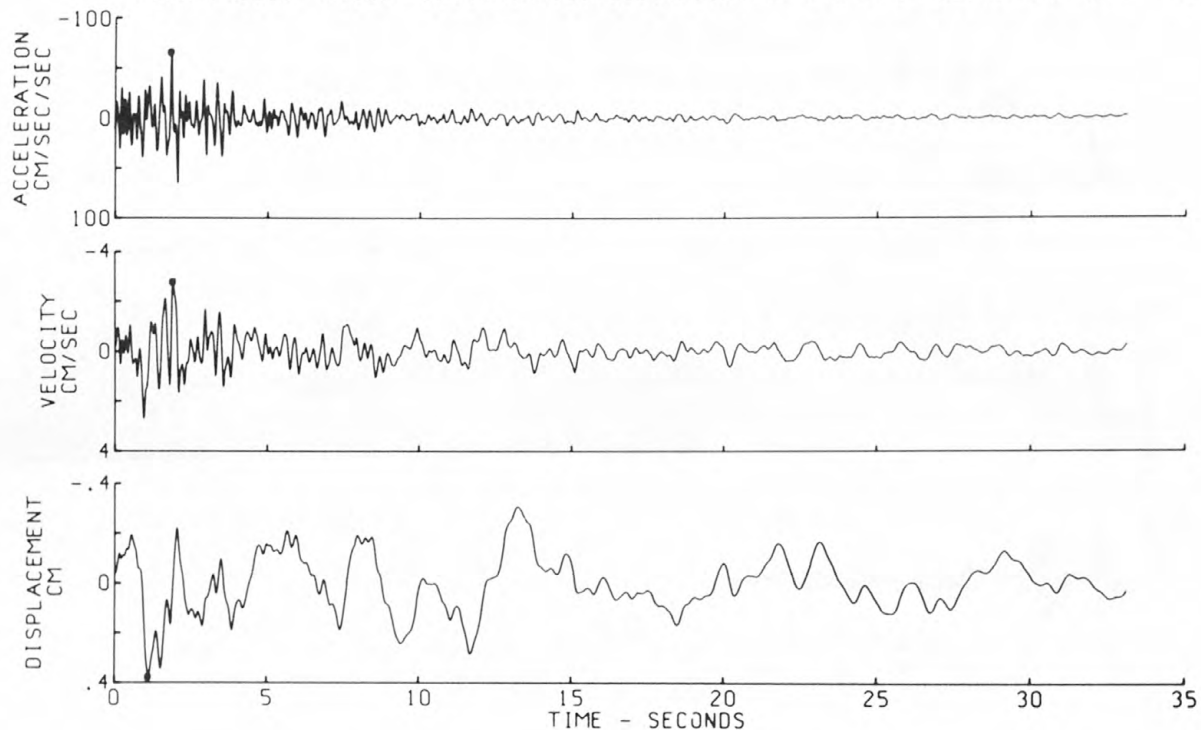




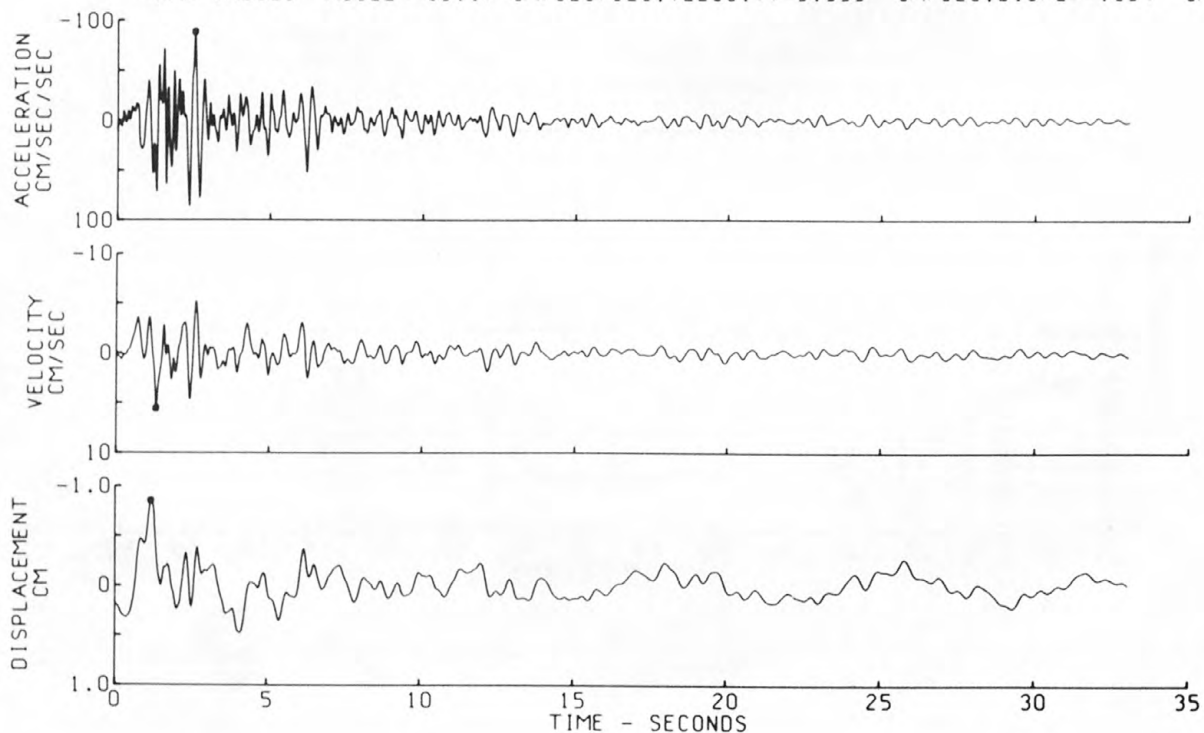




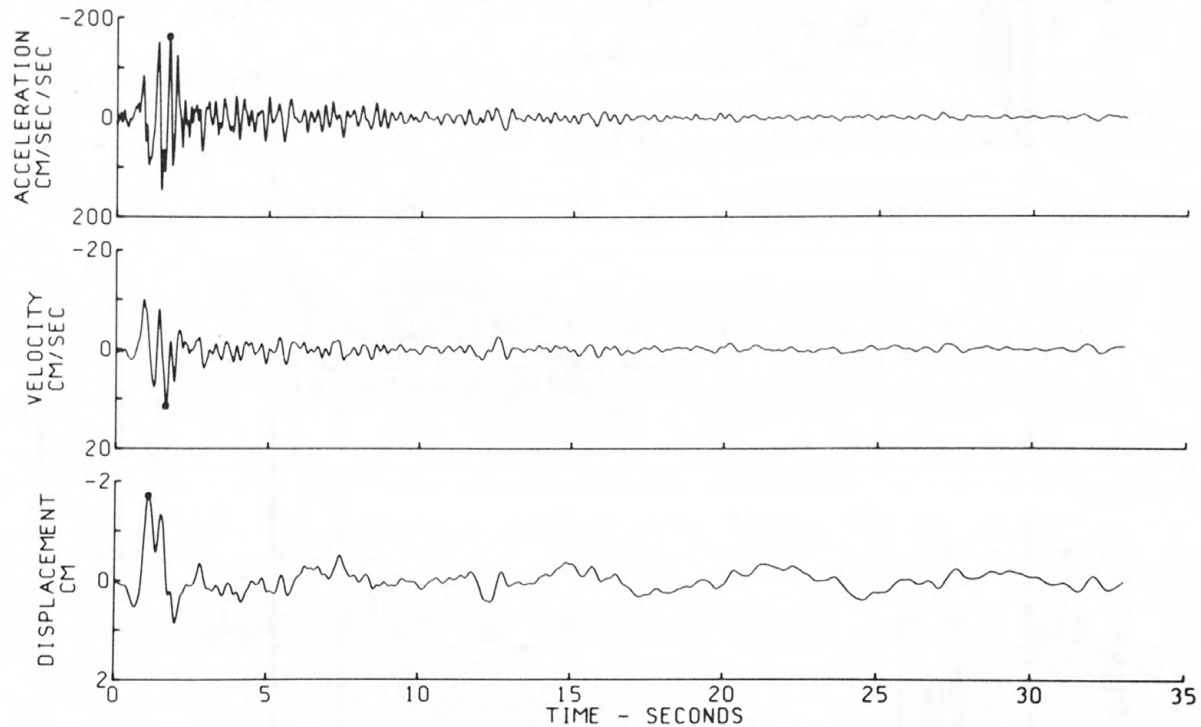
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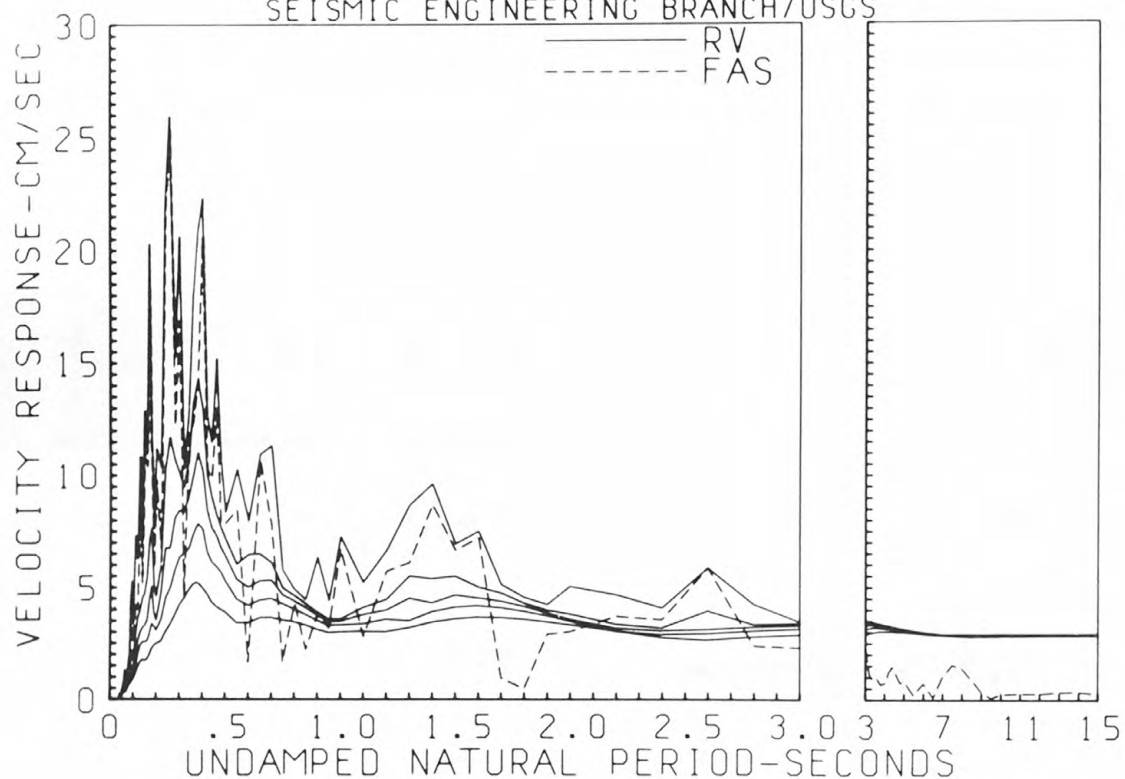
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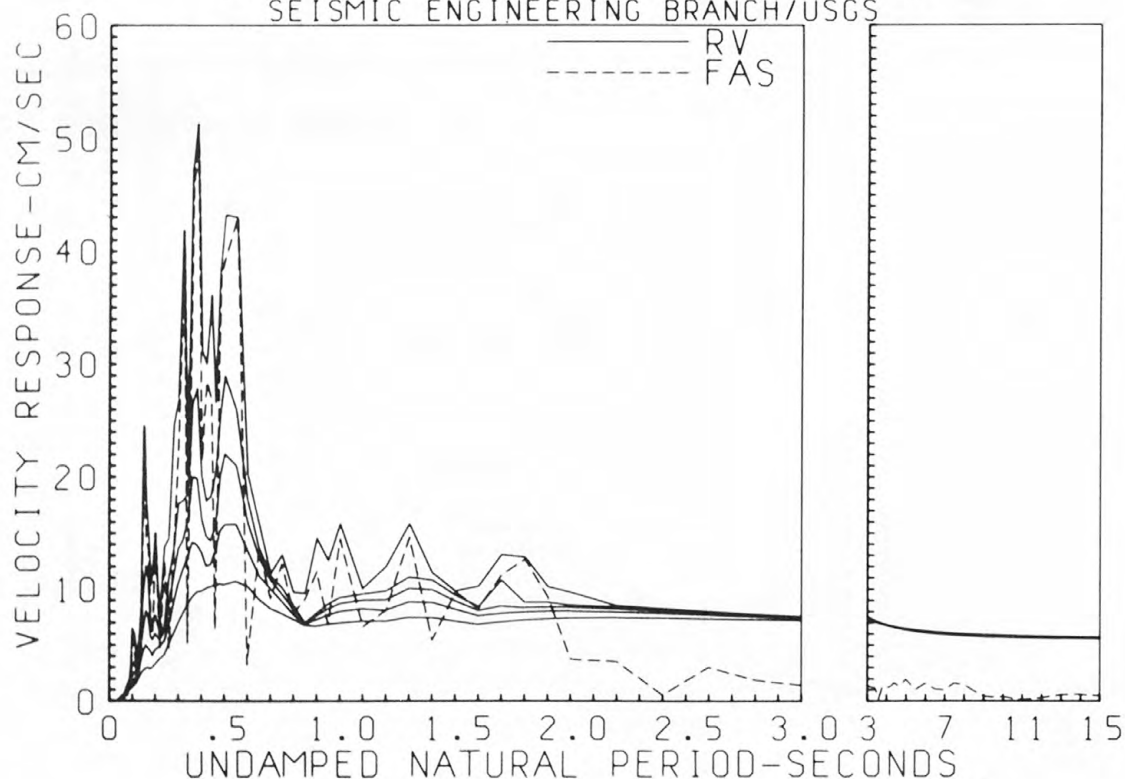
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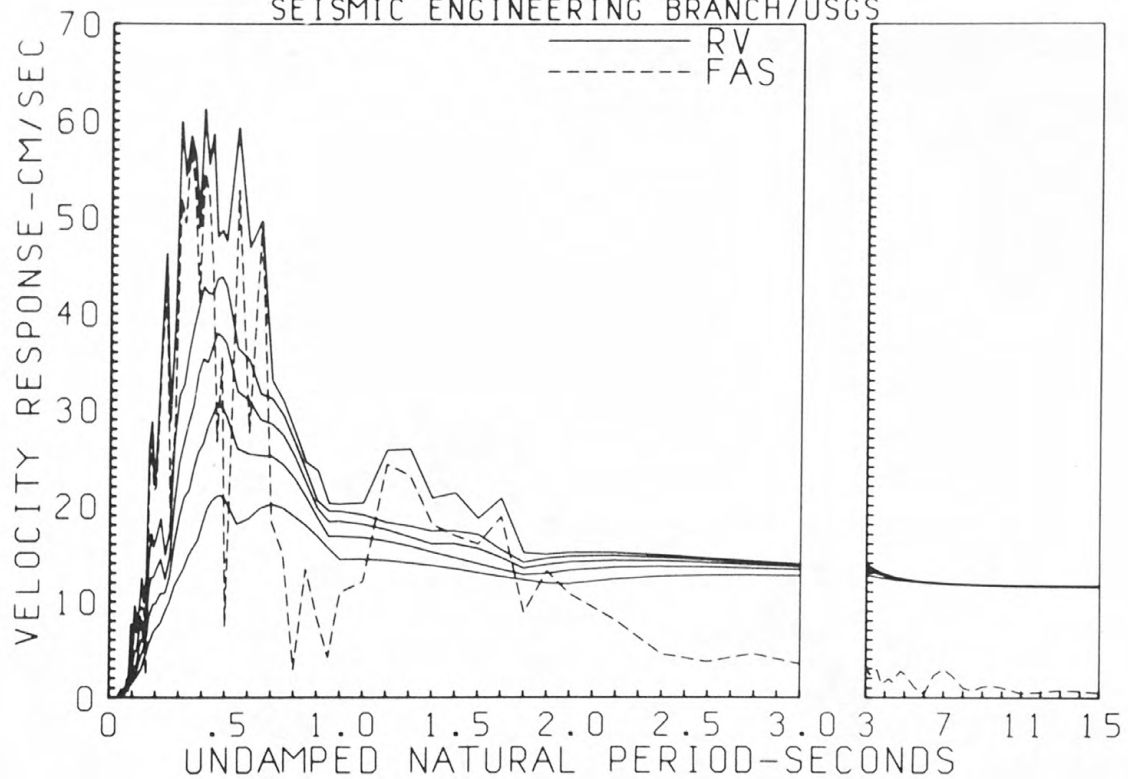
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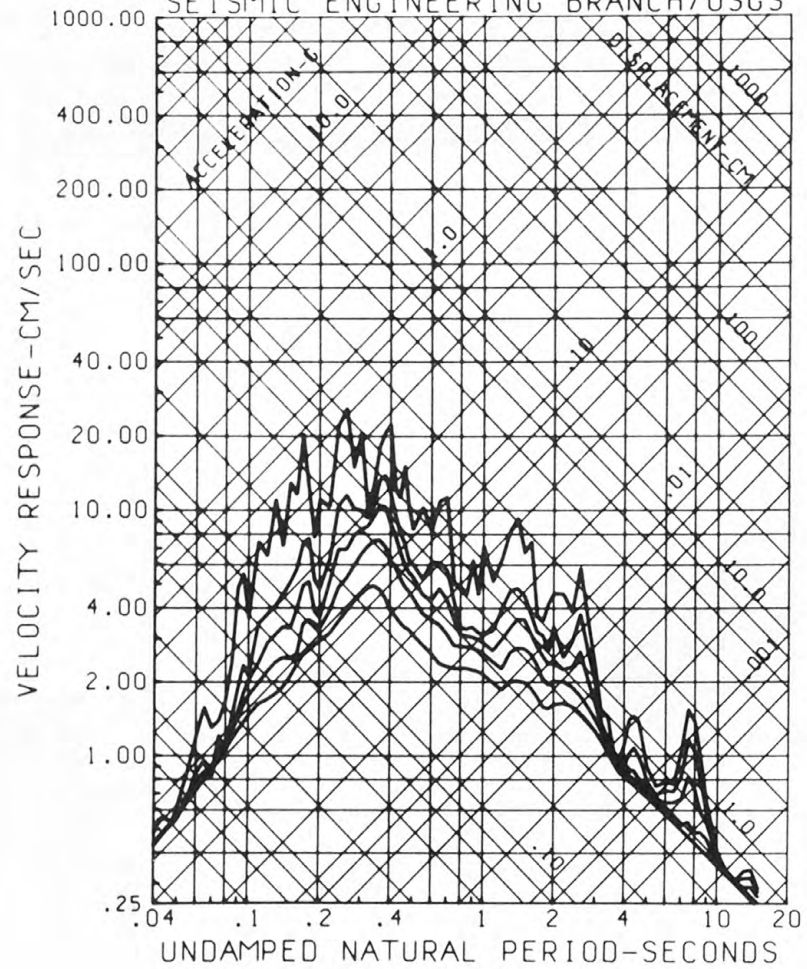
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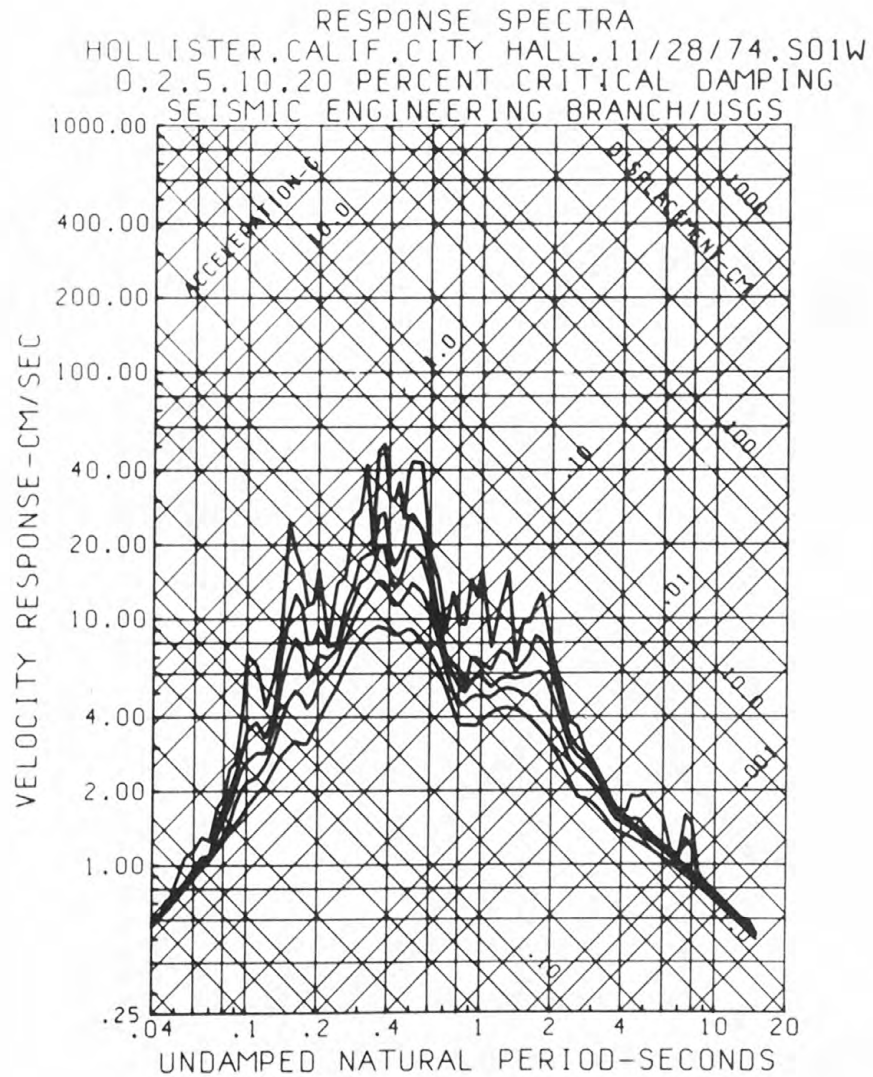


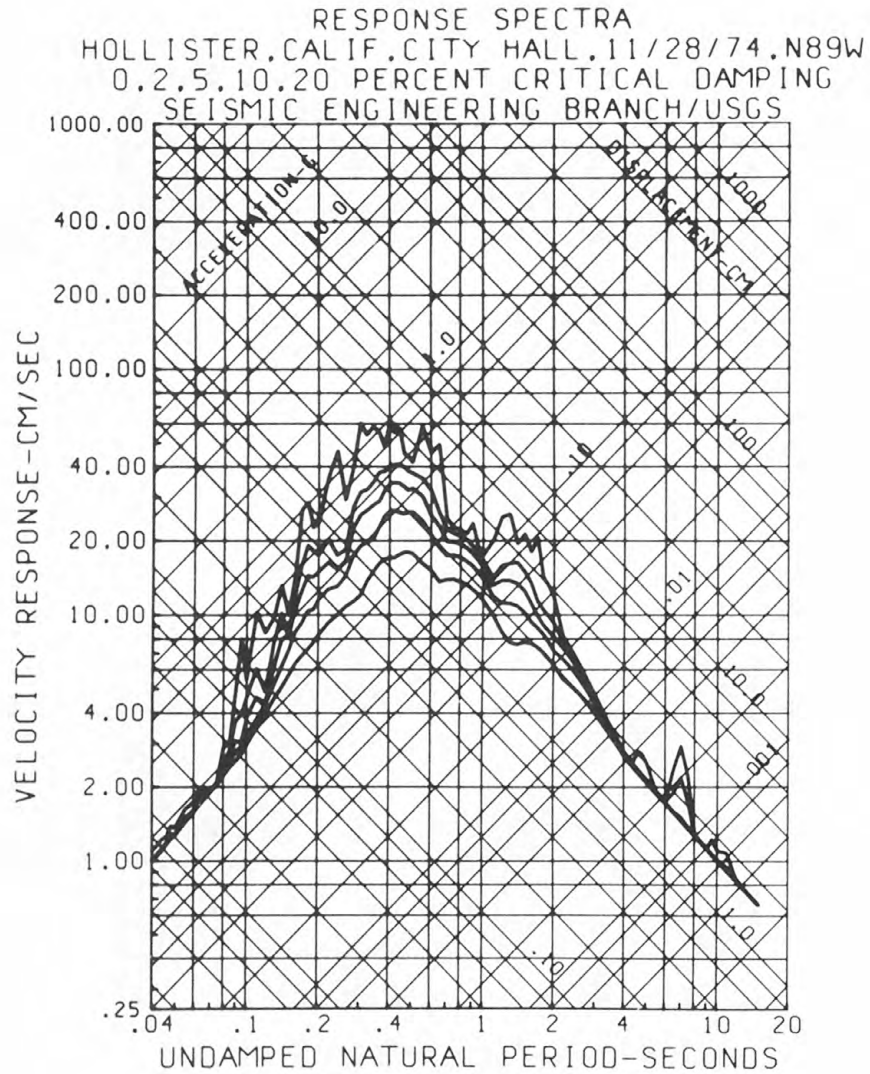
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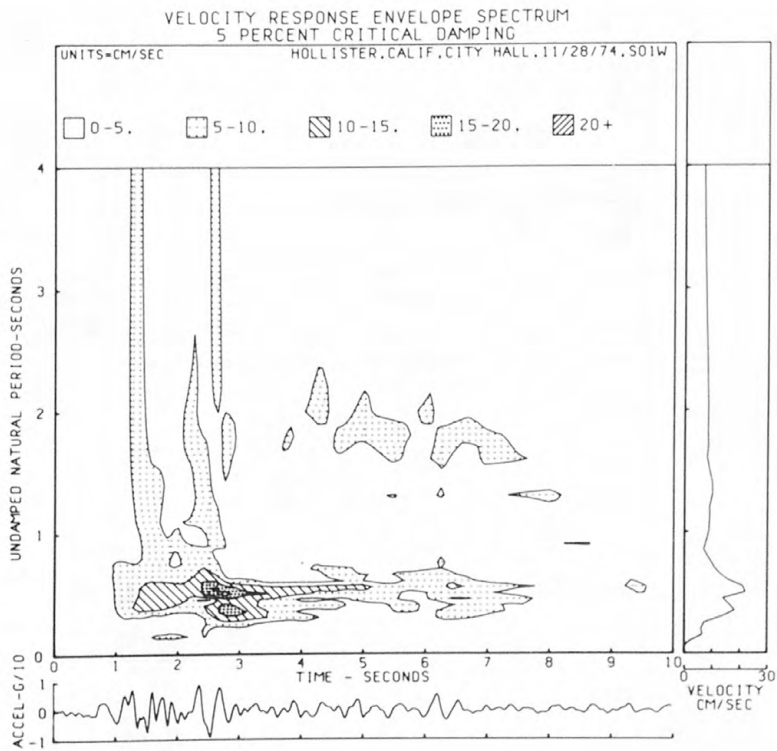
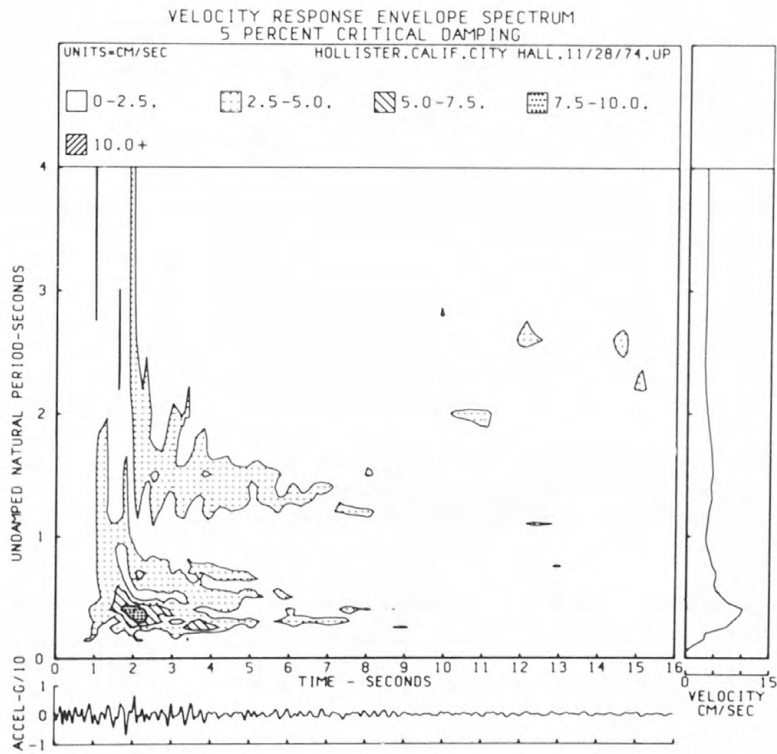


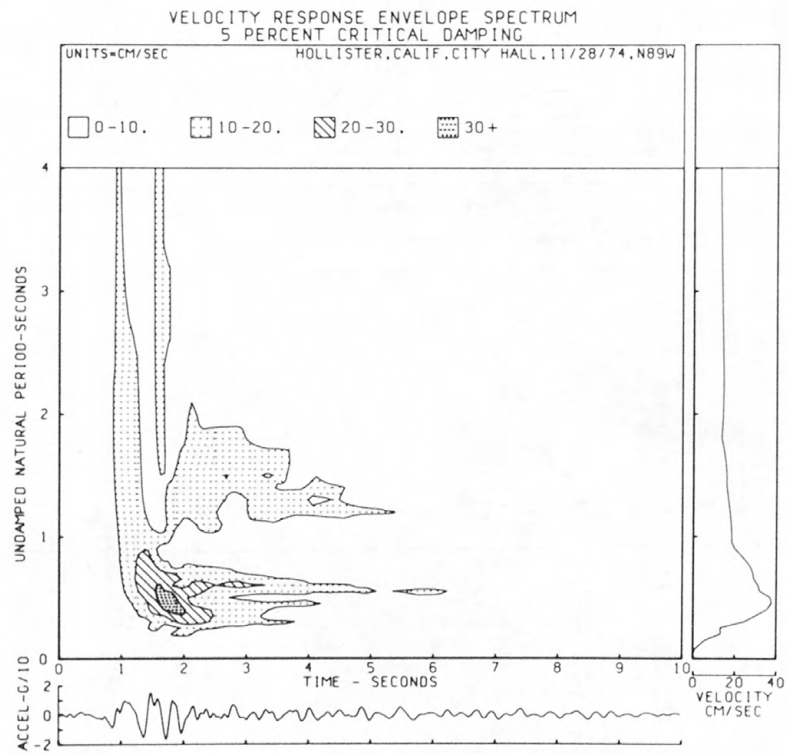
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 HOLLISTER, CALIF. CITY HALL, 11/28/74, UP
 0.2, 5, 10, 20 PERCENT CRITICAL DAMPING
 SEISMIC ENGINEERING BRANCH/USGS



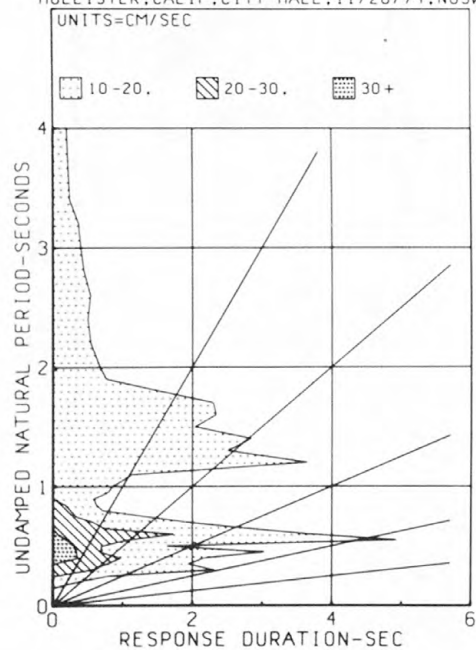




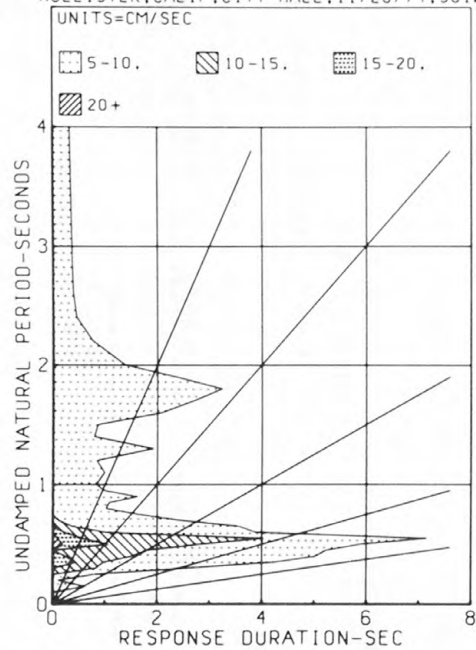




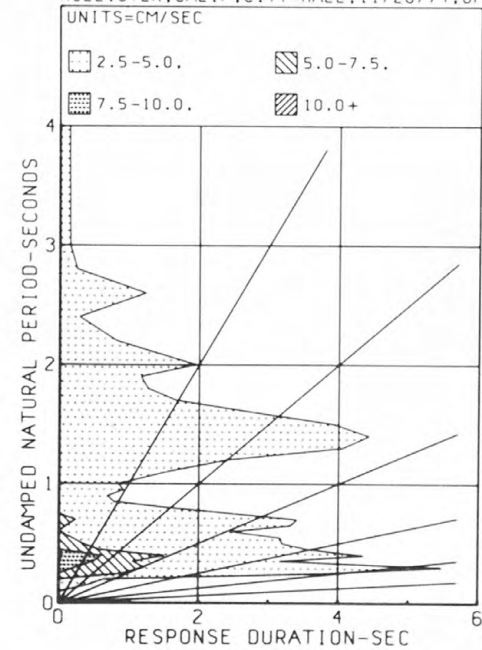
DURATION SPECTRUM OF THE VELOCITY
RESPONSE ENVELOPE, 5 PERCENT DAMPING
HOLLISTER, CALIF. CITY HALL, 11/28/74, N89W

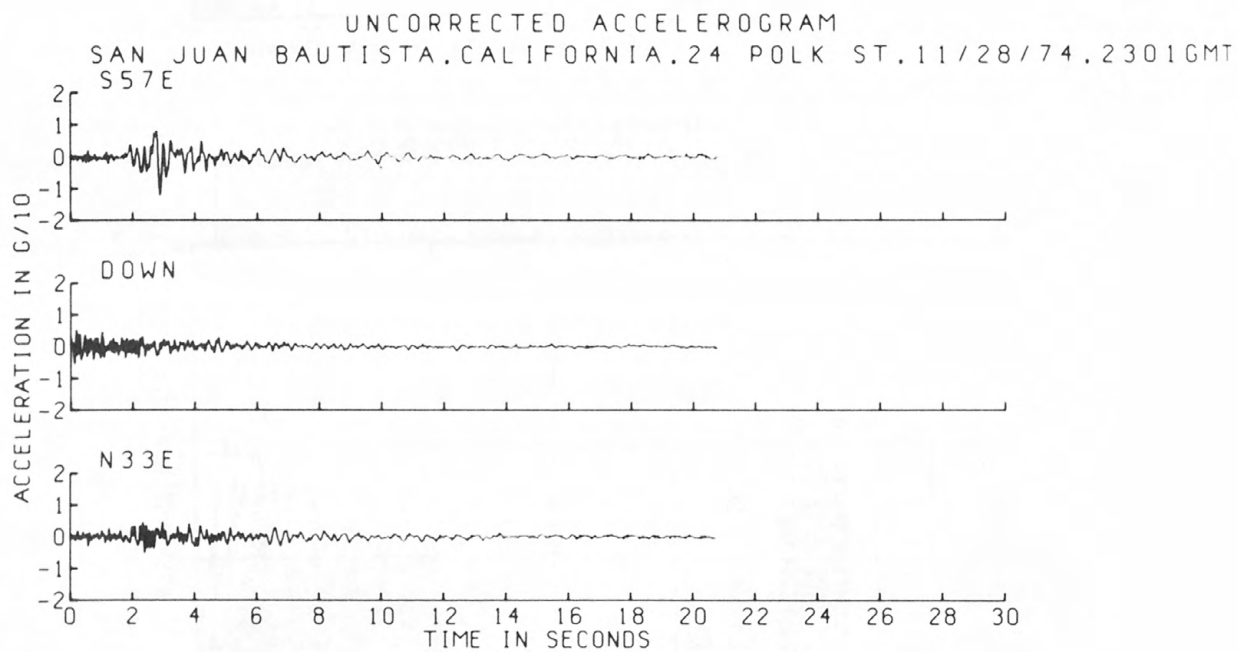


DURATION SPECTRUM OF THE VELOCITY
RESPONSE ENVELOPE, 5 PERCENT DAMPING
HOLLISTER, CALIF. CITY HALL, 11/28/74, S01W

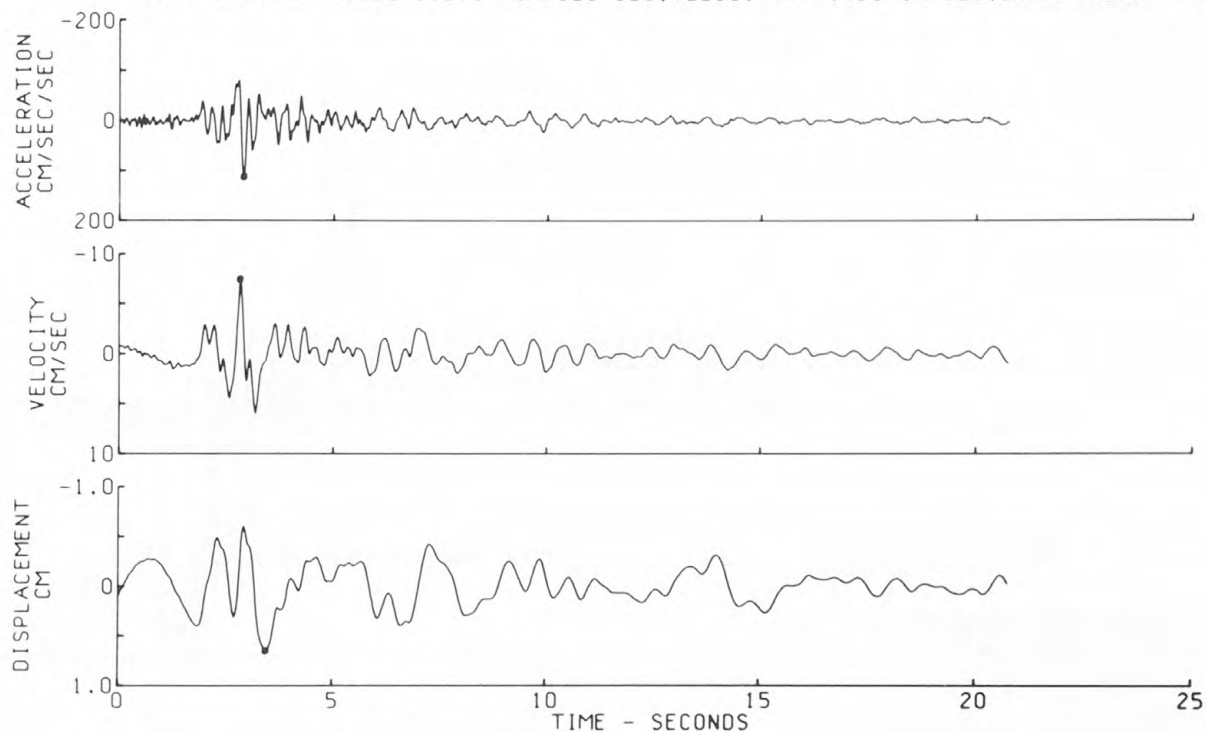


DURATION SPECTRUM OF THE VELOCITY
RESPONSE ENVELOPE, 5 PERCENT DAMPING
HOLLISTER, CALIF. CITY HALL, 11/28/74, UP

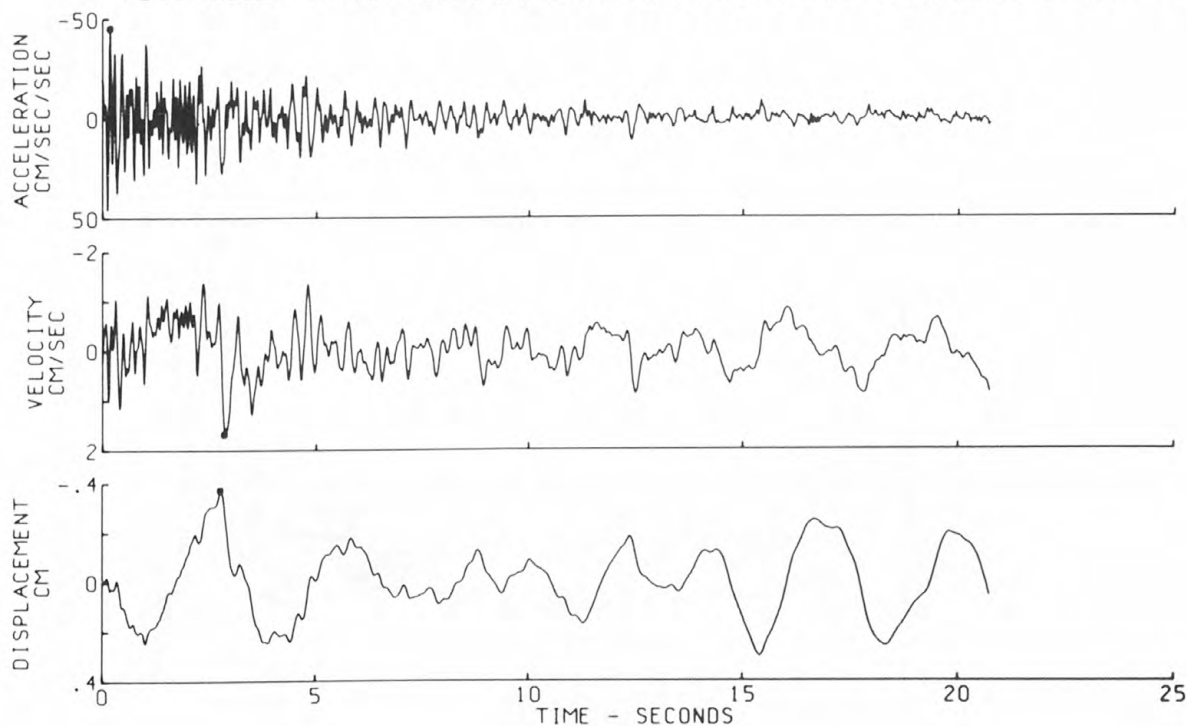




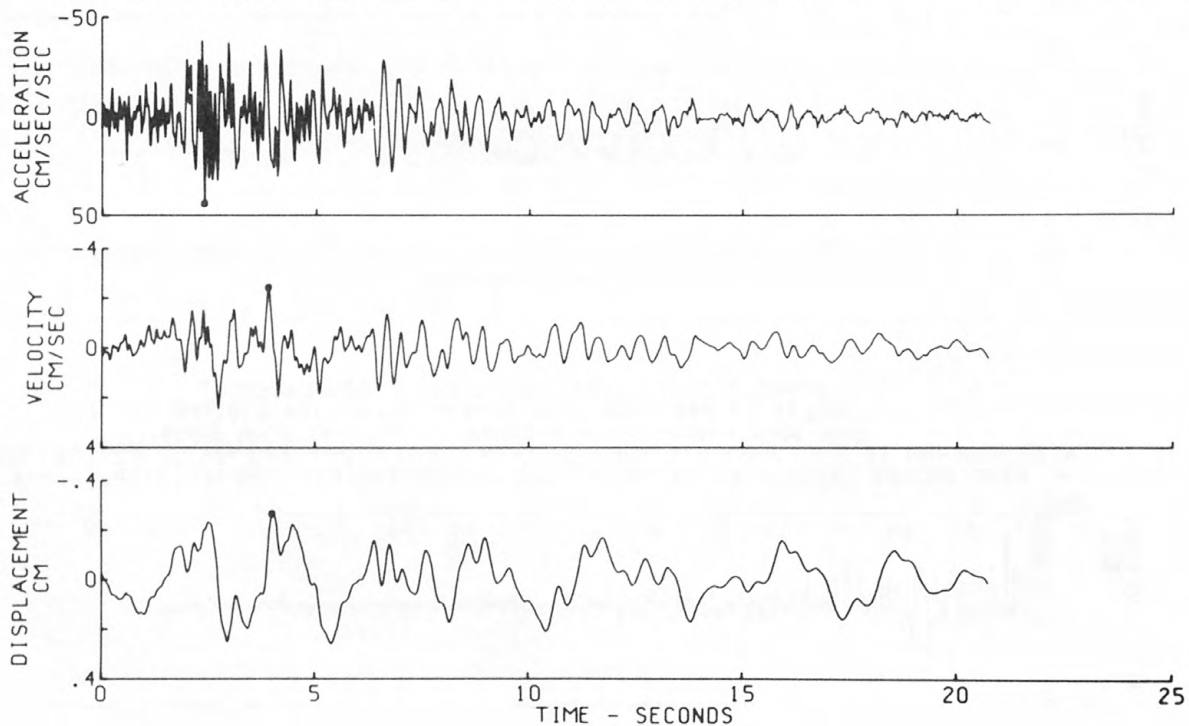
CORRECTED ACCELERATION, VELOCITY, DISPLACEMENT
 HOLLISTER EARTHQUAKE OF NOVEMBER 28, 1974, 2301GMT
 SAN JUAN BAUTISTA, CALIFORNIA, 24 POLK ST. S57E COMP
 ACCELEROGRAM IS BAND PASS FILTERED BETWEEN .170 - .250 AND 25.00 - 27.00 CYC/SEC
 • PEAK VALUES ACCEL=112.1 CM/SEC/SEC, VELOCITY=-7.453 CM/SEC, DISPL=.650 CM



CORRECTED ACCELERATION, VELOCITY, DISPLACEMENT
 HOLLISTER EARTHQUAKE OF NOVEMBER 28, 1974, 2301GMT
 SAN JUAN BAUTISTA, CALIFORNIA, 24 POLK ST. DOWN COMP
 ACCELEROGRAM IS BAND PASS FILTERED BETWEEN .170 - .250 AND 25.00 - 27.00 CYC/SEC
 • PEAK VALUES ACCEL=-45.59 CM/SEC/SEC, VELOCITY=1.672 CM/SEC, DISPL=-.374 CM



CORRECTED ACCELERATION, VELOCITY, DISPLACEMENT
 HOLLISTER EARTHQUAKE OF NOVEMBER 28, 1974, 2301GMT
 SAN JUAN BAUTISTA, CALIFORNIA, 24 POLK ST. N33E COMP
 ACCELEROGRAM IS BAND PASS FILTERED BETWEEN .170 - .250 AND 25.00 - 27.00 CYC/SEC
 • PEAK VALUES ACCEL=43.94 CM/SEC/SEC, VELOCITY=-2.434 CM/SEC, DISPL=-.267 CM

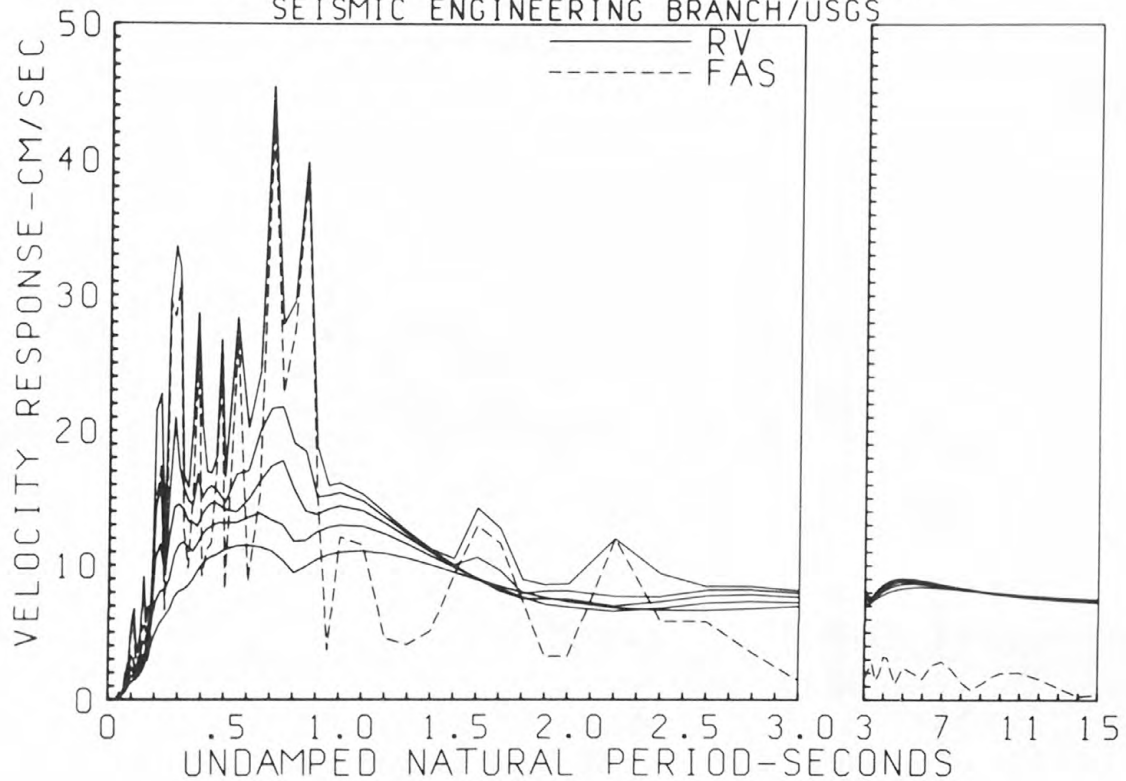


RELATIVE VELOCITY RESPONSE SPECTRUM

SJ BAUTISTA,CALIF,POLK ST,11/28/74,S57E

0.2,5,10,20 PERCENT CRITICAL DAMPING

SEISMIC ENGINEERING BRANCH/USGS

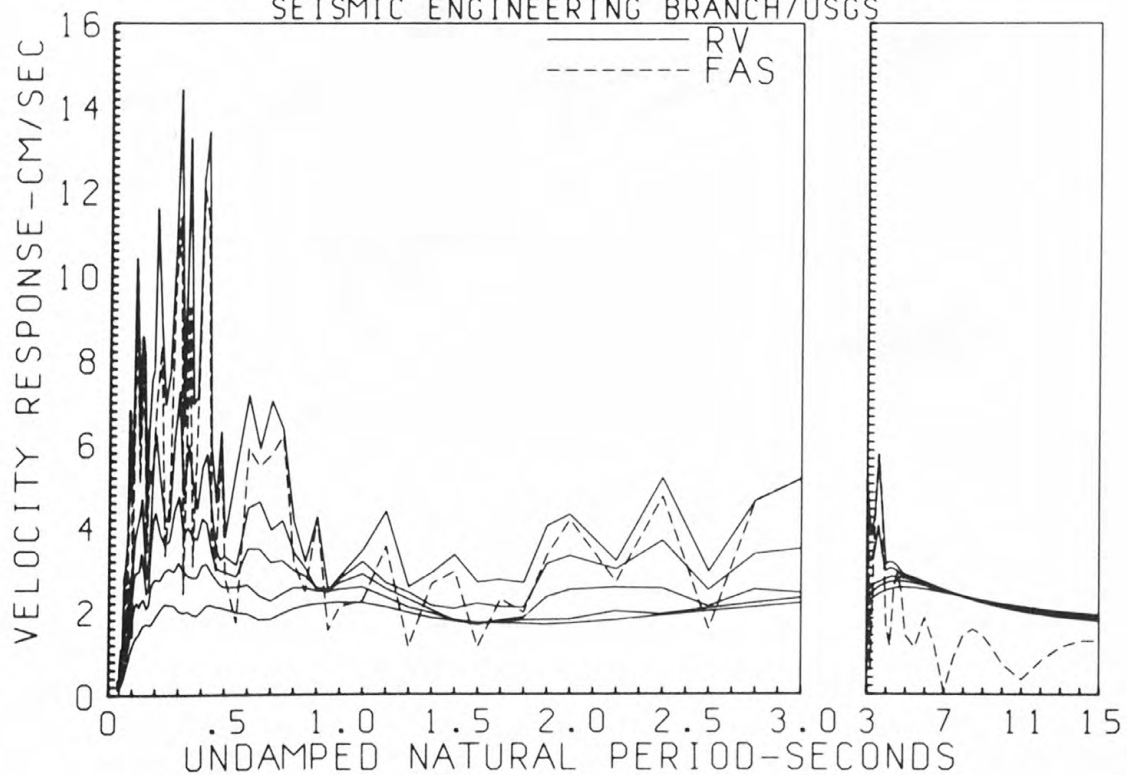


RELATIVE VELOCITY RESPONSE SPECTRUM

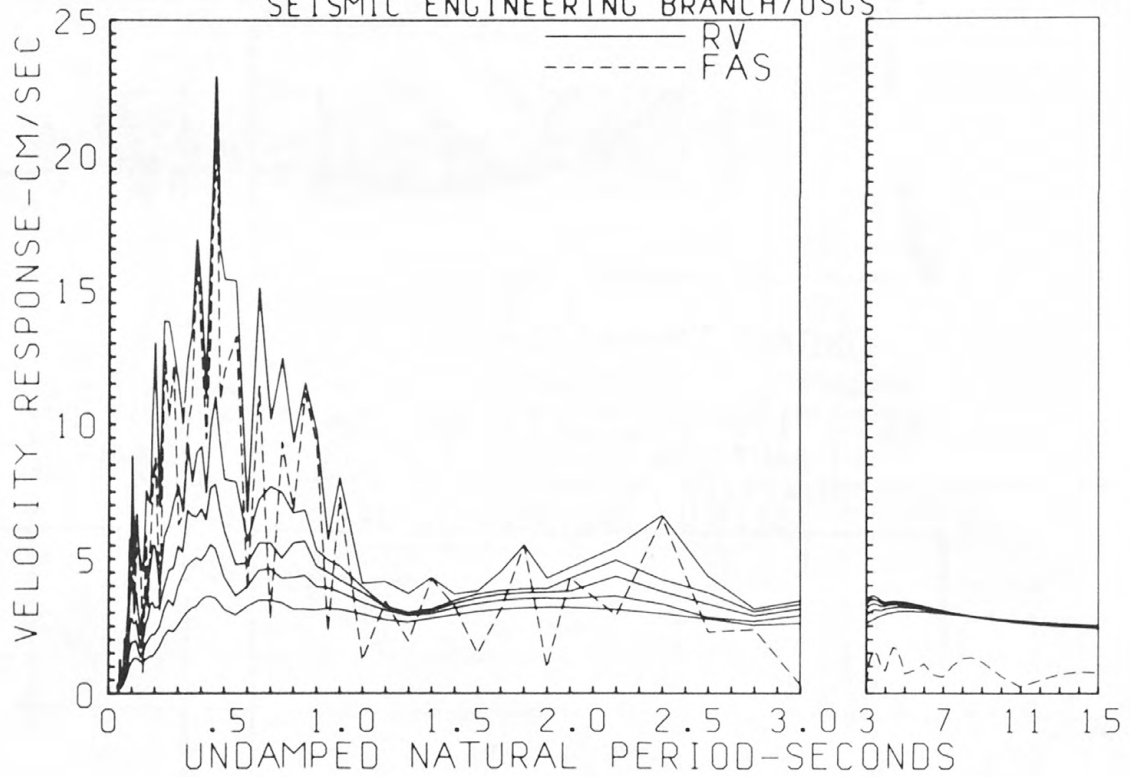
SJ BAUTISTA,CALIF,POLK ST,11/28/74,DOWN

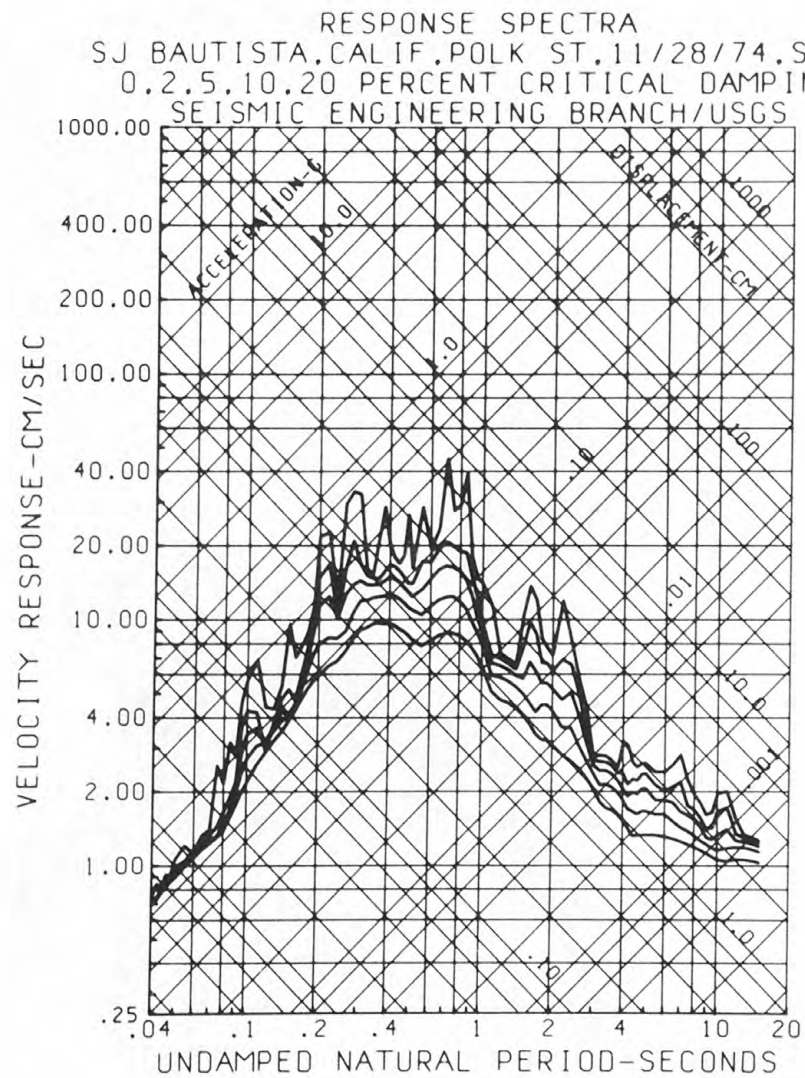
0.2,5,10,20 PERCENT CRITICAL DAMPING

SEISMIC ENGINEERING BRANCH/USGS

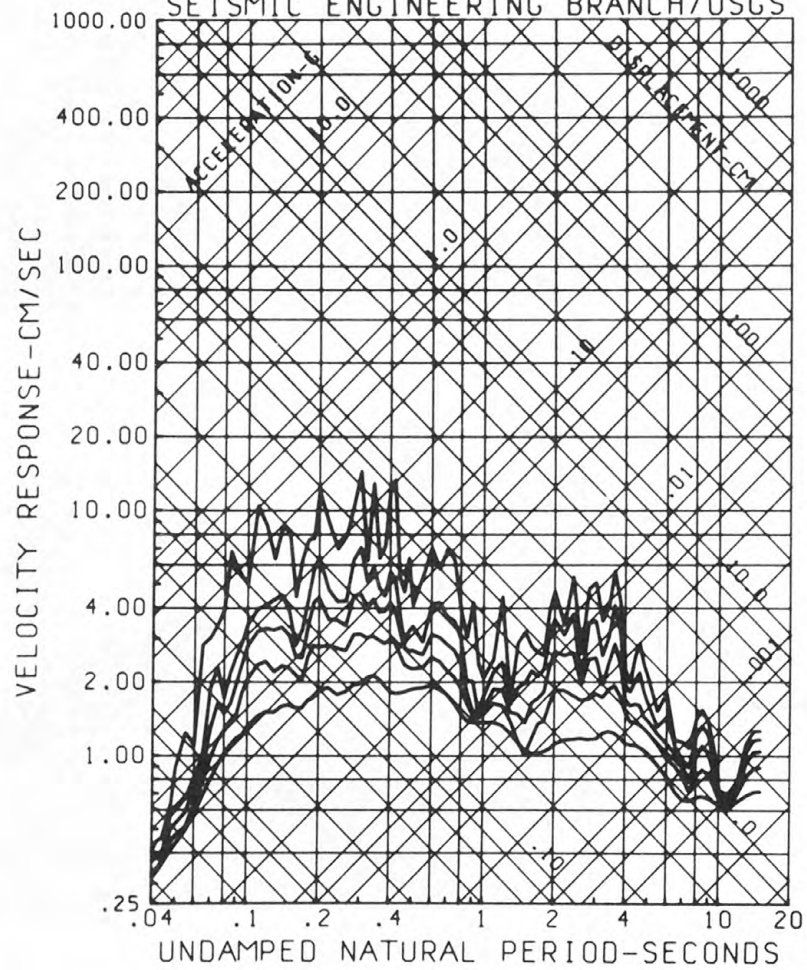


RELATIVE VELOCITY RESPONSE SPECTRUM
SJ BAUTISTA,CALIF.POLK ST.11/28/74.N33E
0.2,5,10,20 PERCENT CRITICAL DAMPING
SEISMIC ENGINEERING BRANCH/USGS

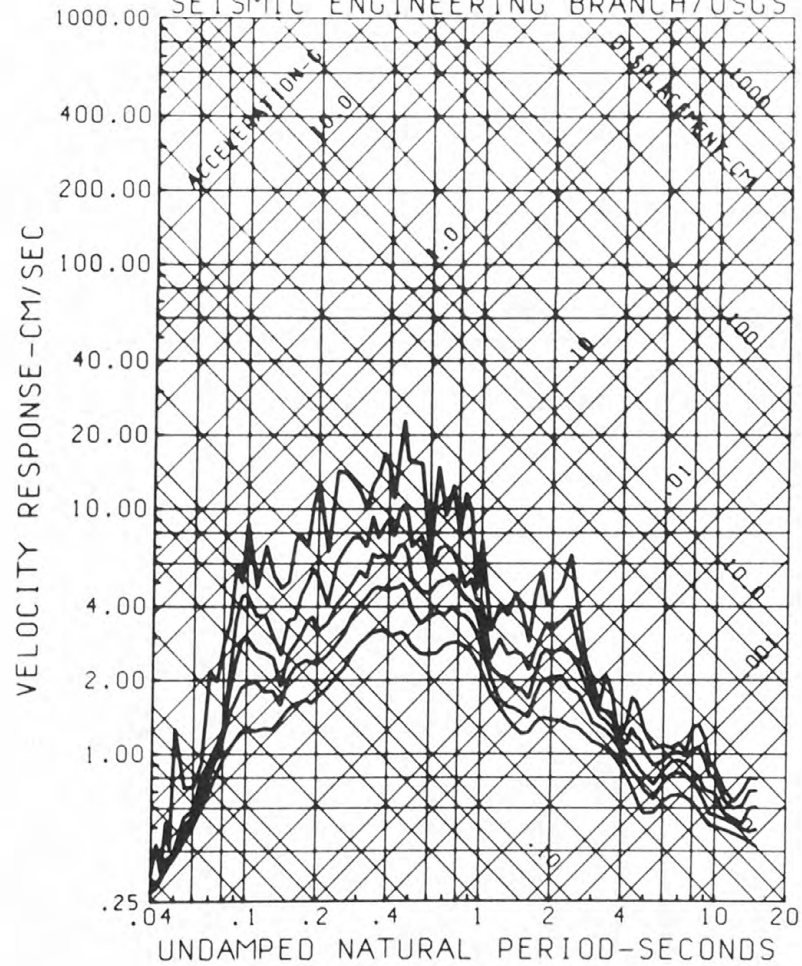


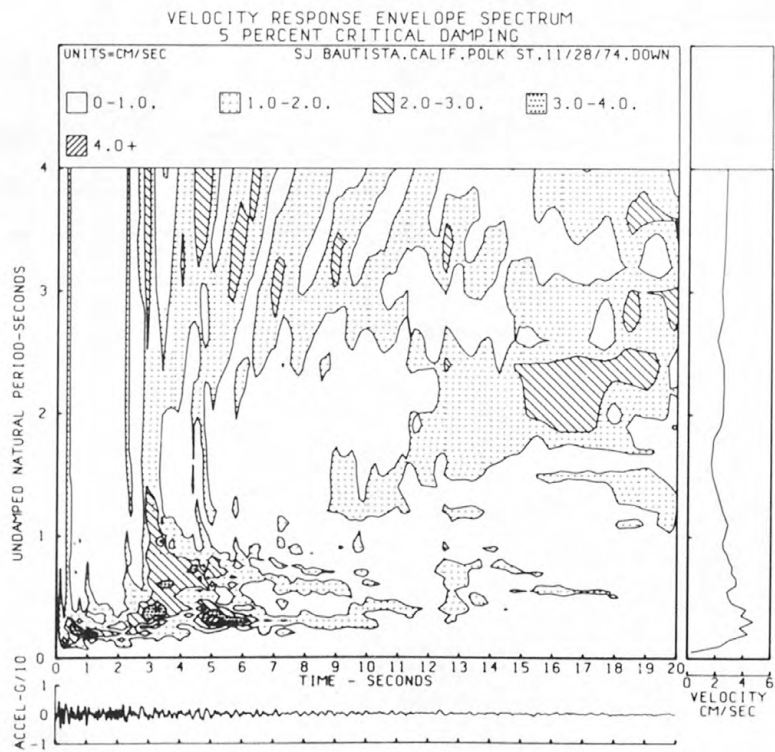
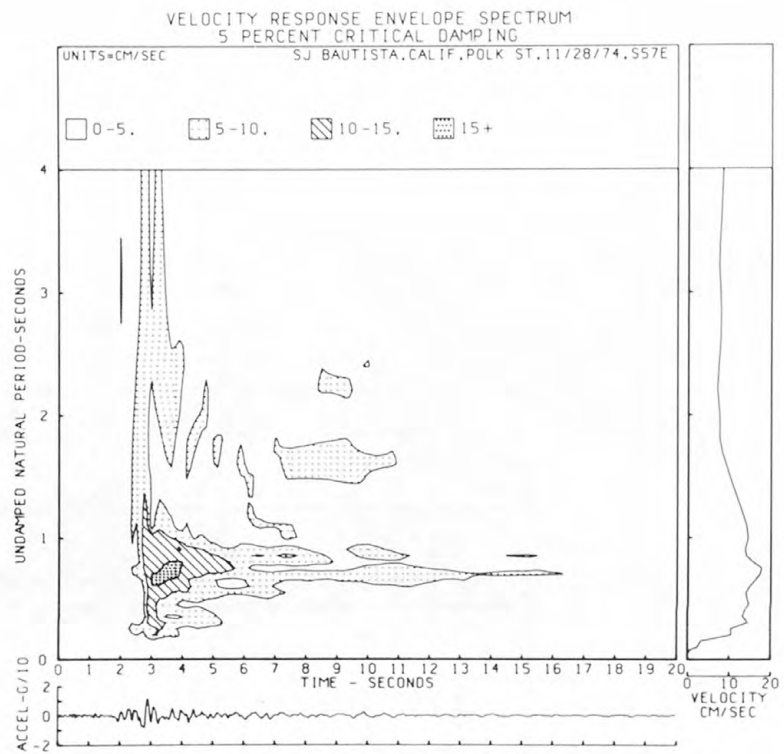


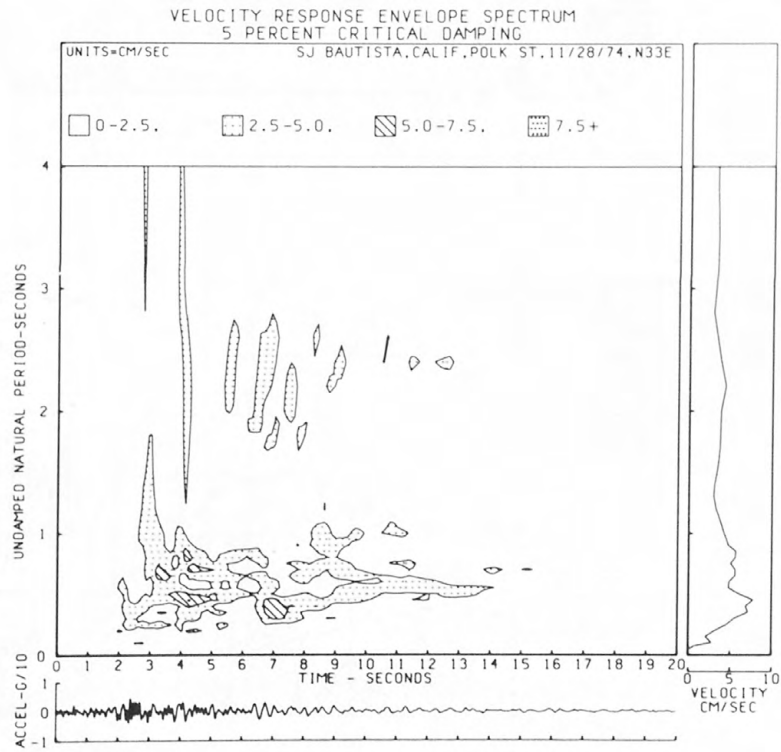
RESPONSE SPECTRA
 SJ BAUTISTA,CALIF,POLK ST,11/28/74,DOWN
 0.2,5,10,20 PERCENT CRITICAL DAMPING
 SEISMIC ENGINEERING BRANCH/USGS

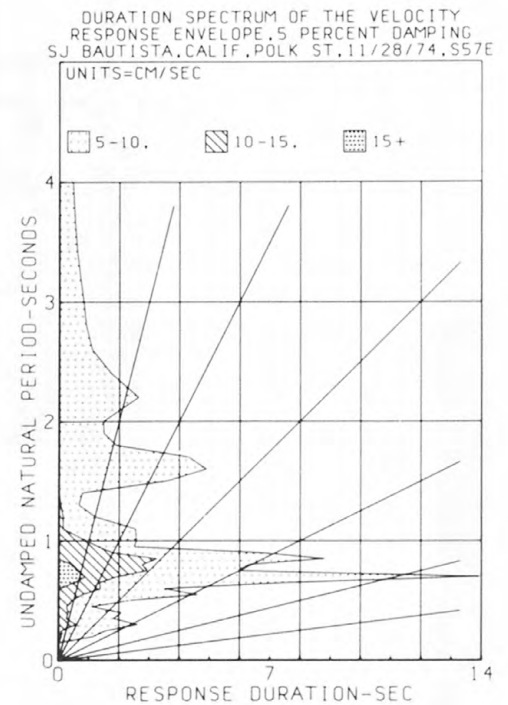
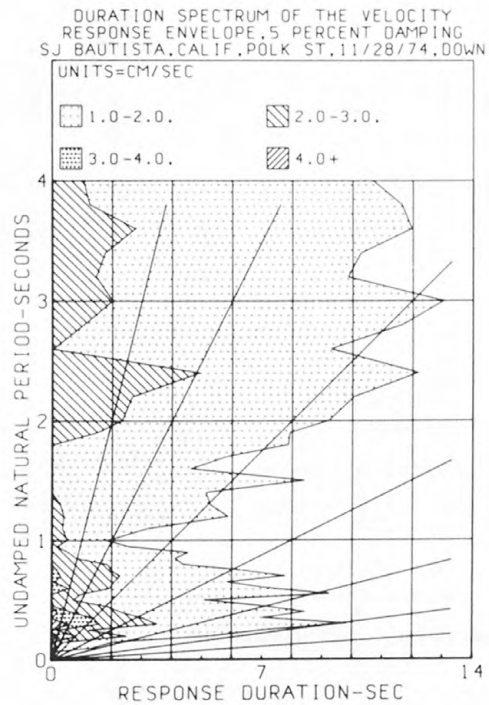
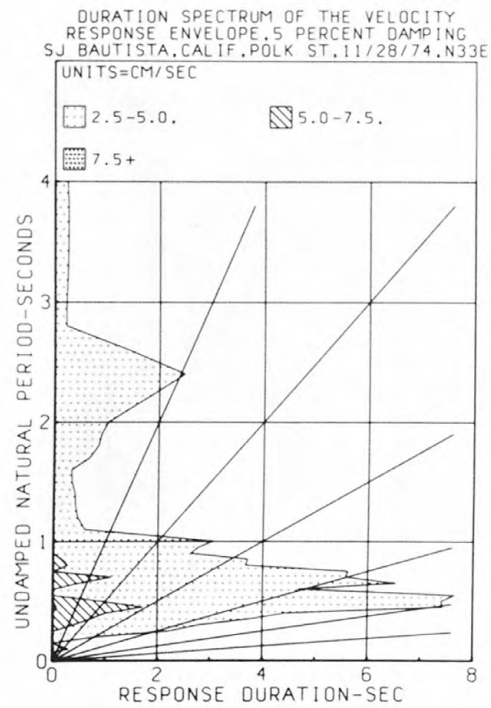


RESPONSE SPECTRA
 SJ BAUTISTA, CALIF. POLK ST. 11/28/74. N33E
 0.2.5.10.20 PERCENT CRITICAL DAMPING
 SEISMIC ENGINEERING BRANCH/USGS





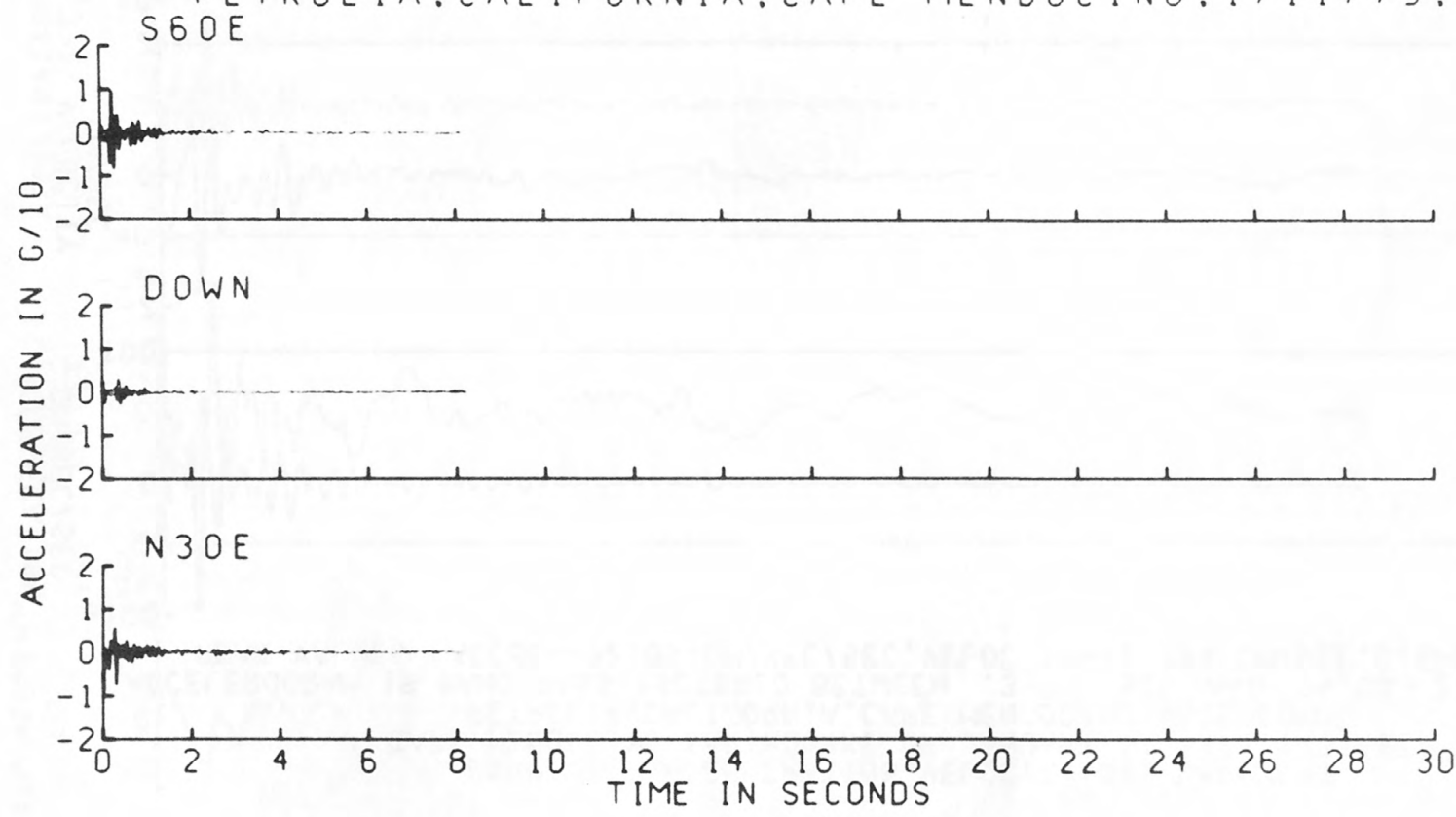


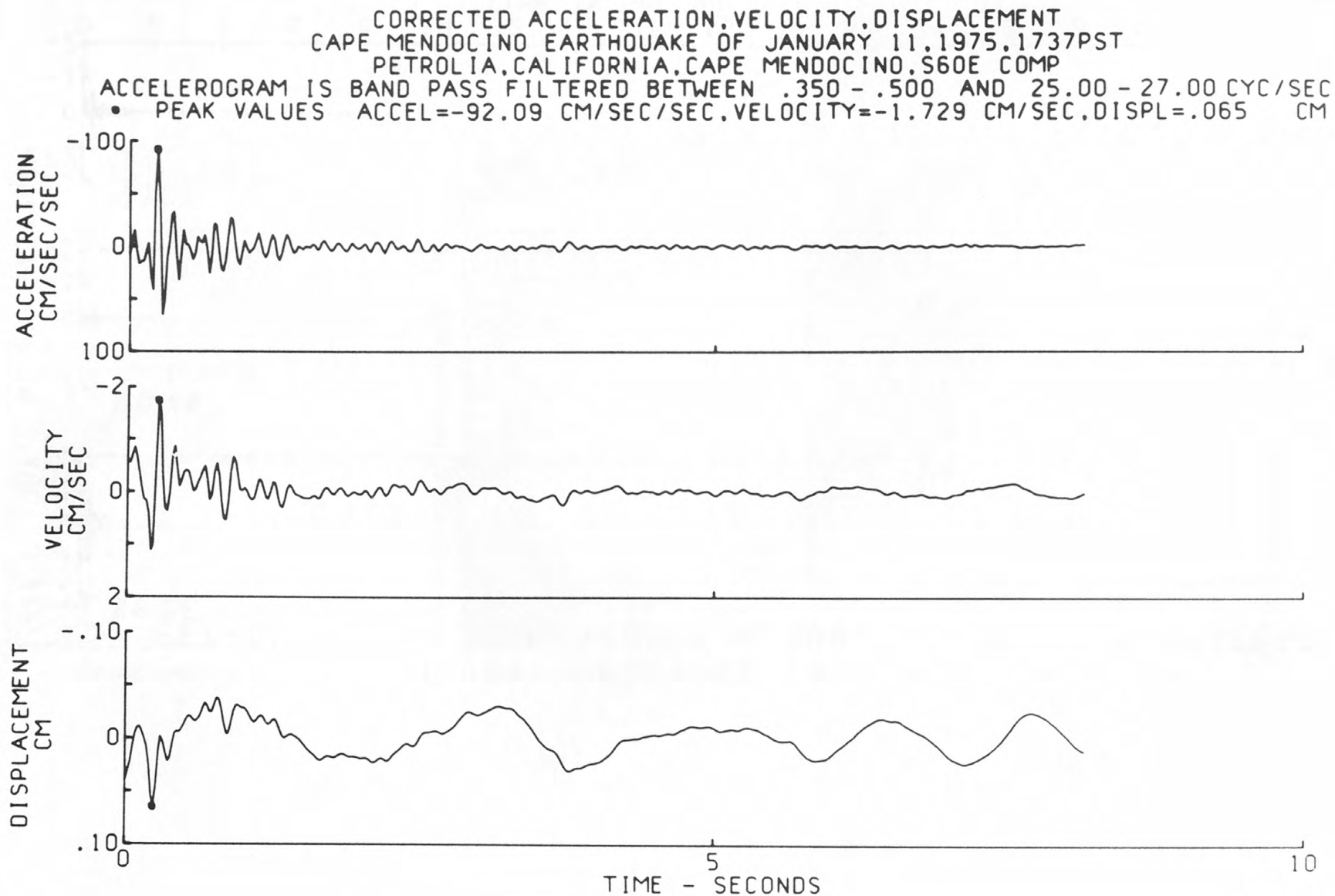


1/11/75 1737 PST

CORRECTED ACCELEROGRAM
CAPE MENDOCINO
PETROLIA, CALIF.

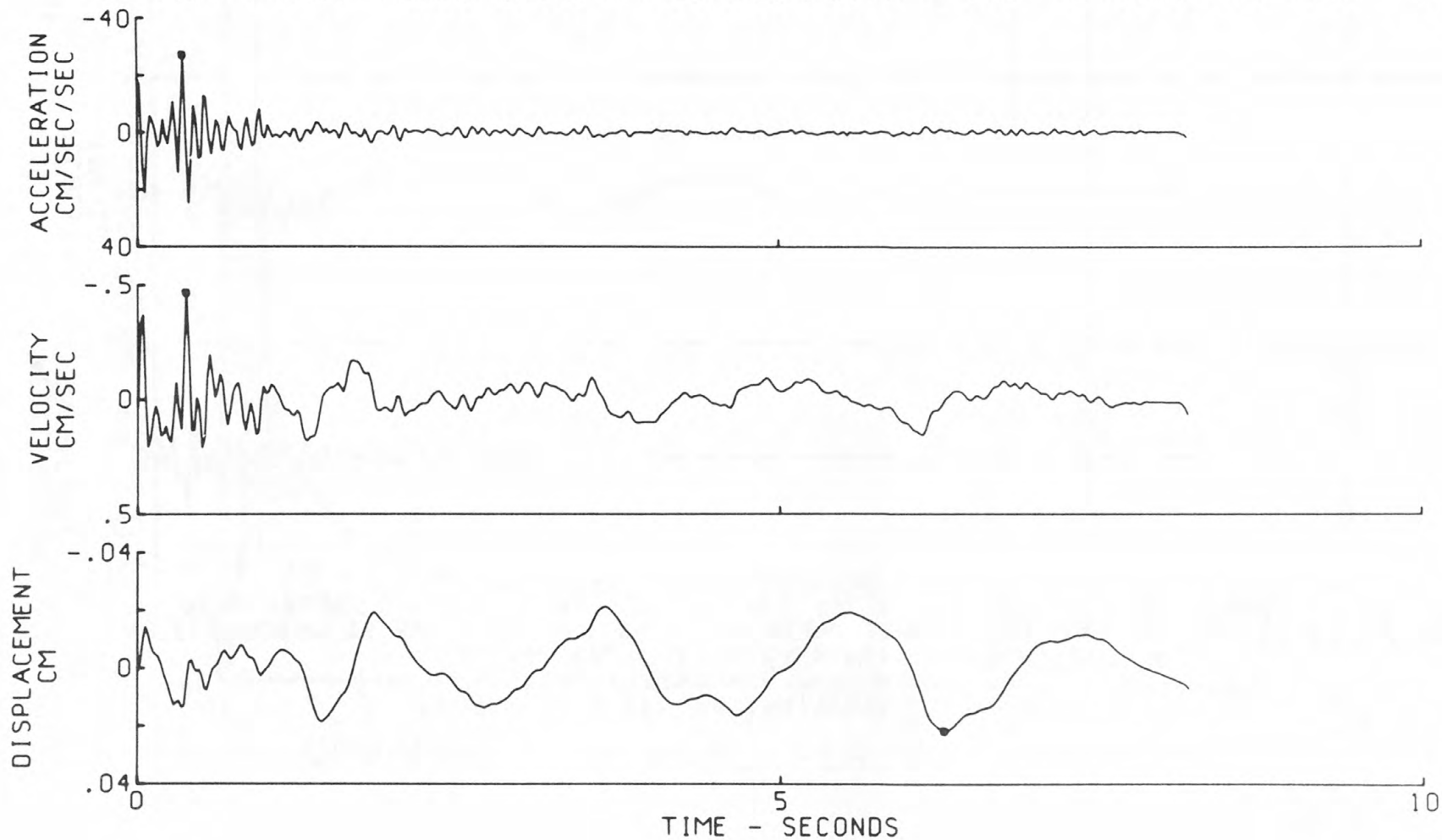
UNCORRECTED ACCELEROGRAM
PETROLIA, CALIFORNIA, CAPE MENDOCINO, 1/11/75, 1737 PST





CORRECTED ACCELERATION, VELOCITY, DISPLACEMENT
CAPE MENDOCINO EARTHQUAKE OF JANUARY 11, 1975, 1737PST
PETROLIA, CALIFORNIA, CAPE MENDOCINO, DOWN COMP

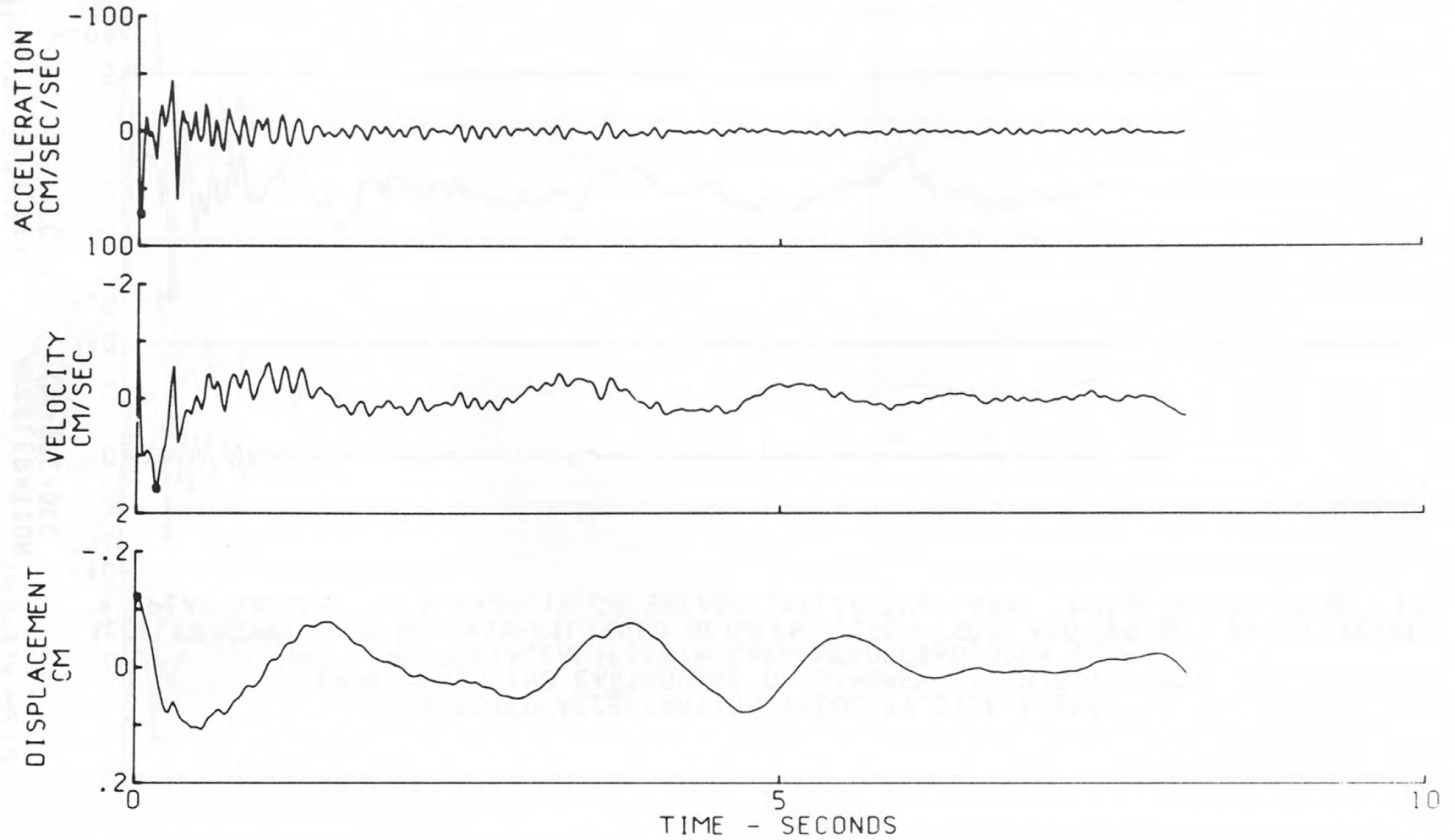
ACCELEROGRAM IS BAND PASS FILTERED BETWEEN .350 - .500 AND 25.00 - 27.00 CYC/SEC
• PEAK VALUES ACCEL=-27.19 CM/SEC/SEC, VELOCITY=-.464 CM/SEC, DISPL=.022 CM



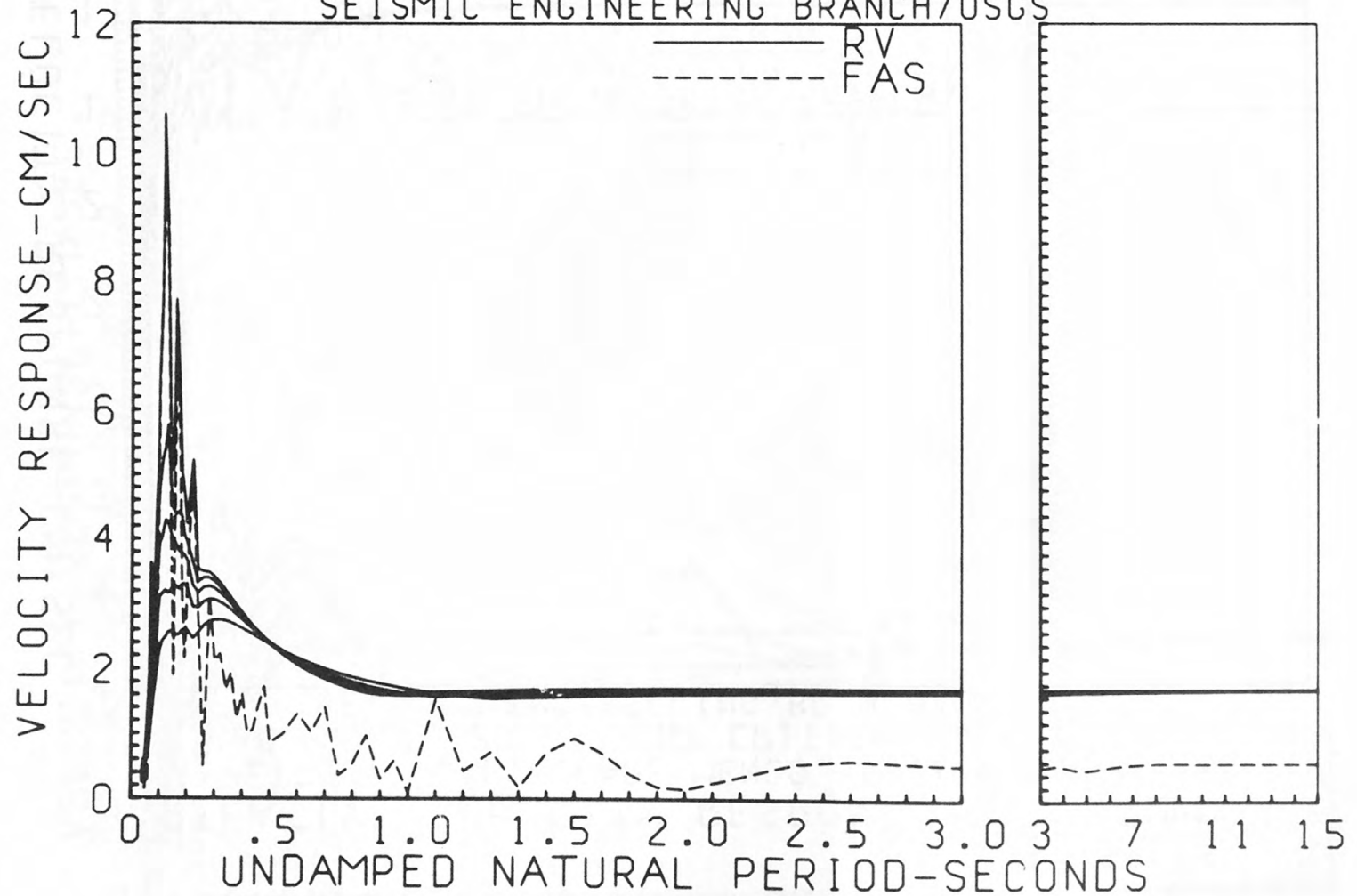
CORRECTED ACCELERATION, VELOCITY, DISPLACEMENT
CAPE MENDOCINO EARTHQUAKE OF JANUARY 11, 1975, 1737PST
PETROLIA, CALIFORNIA, CAPE MENDOCINO, N30E COMP

ACCELEROGRAM IS BAND PASS FILTERED BETWEEN .350 - .500 AND 25.00 - 27.00 CYC/SEC

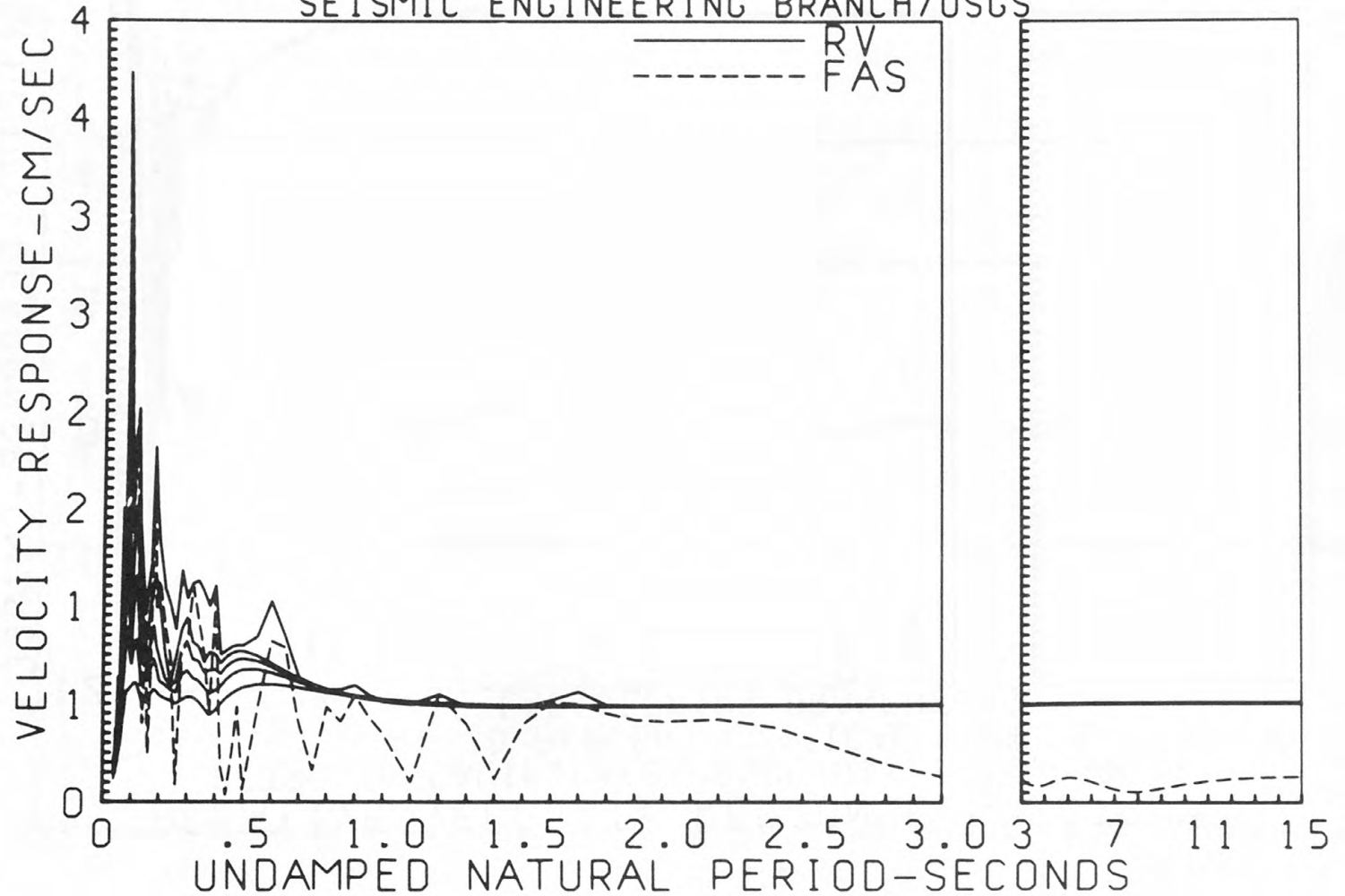
• PEAK VALUES ACCEL=72.42 CM/SEC/SEC, VELOCITY=1.580 CM/SEC, DISPL=-.122 CM



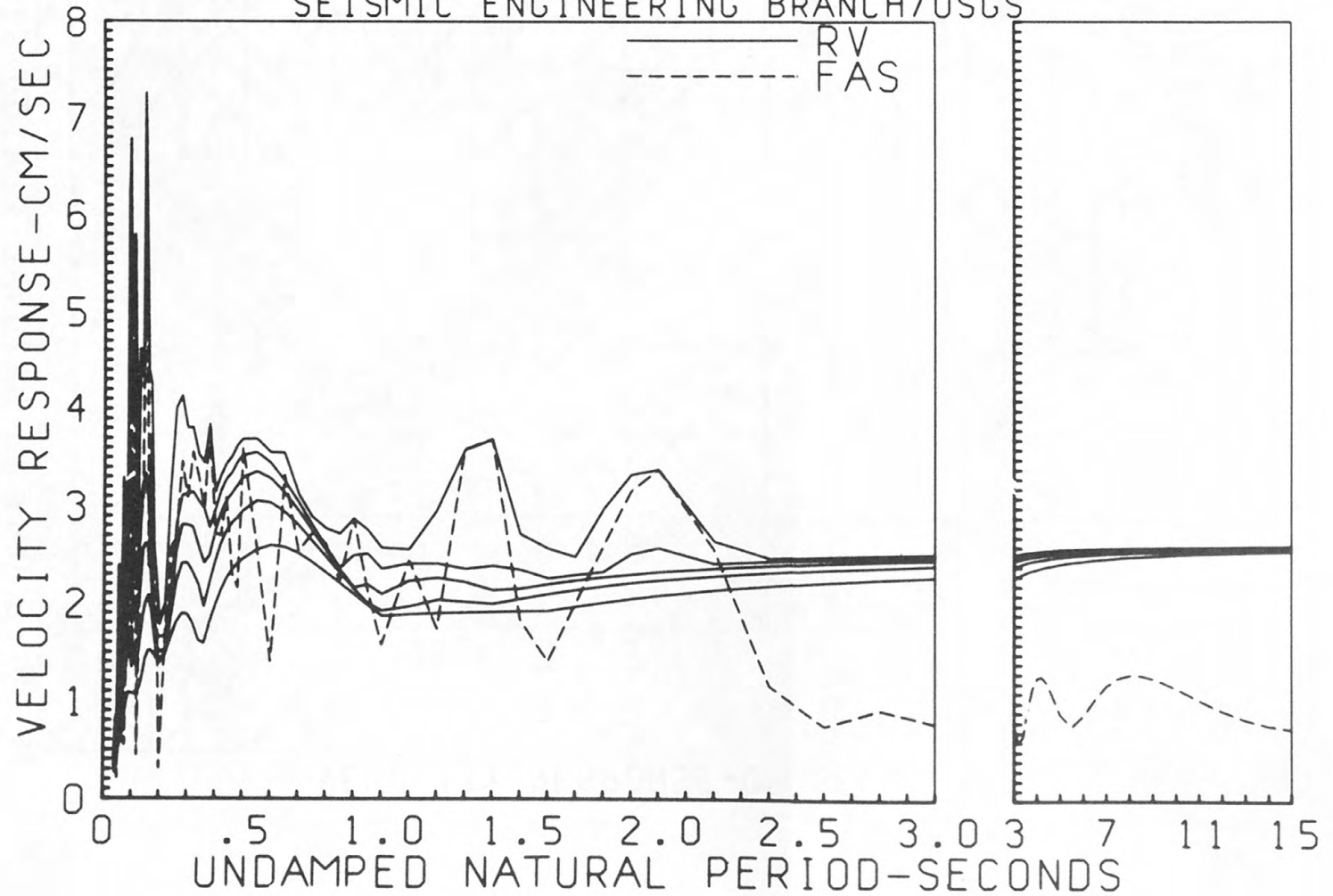
RELATIVE VELOCITY RESPONSE SPECTRUM
 PETROLIA, CALIF, CAPE MENDO, 01/11/75, S60E
 0.2, 5, 10, 20 PERCENT CRITICAL DAMPING
 SEISMIC ENGINEERING BRANCH/USGS



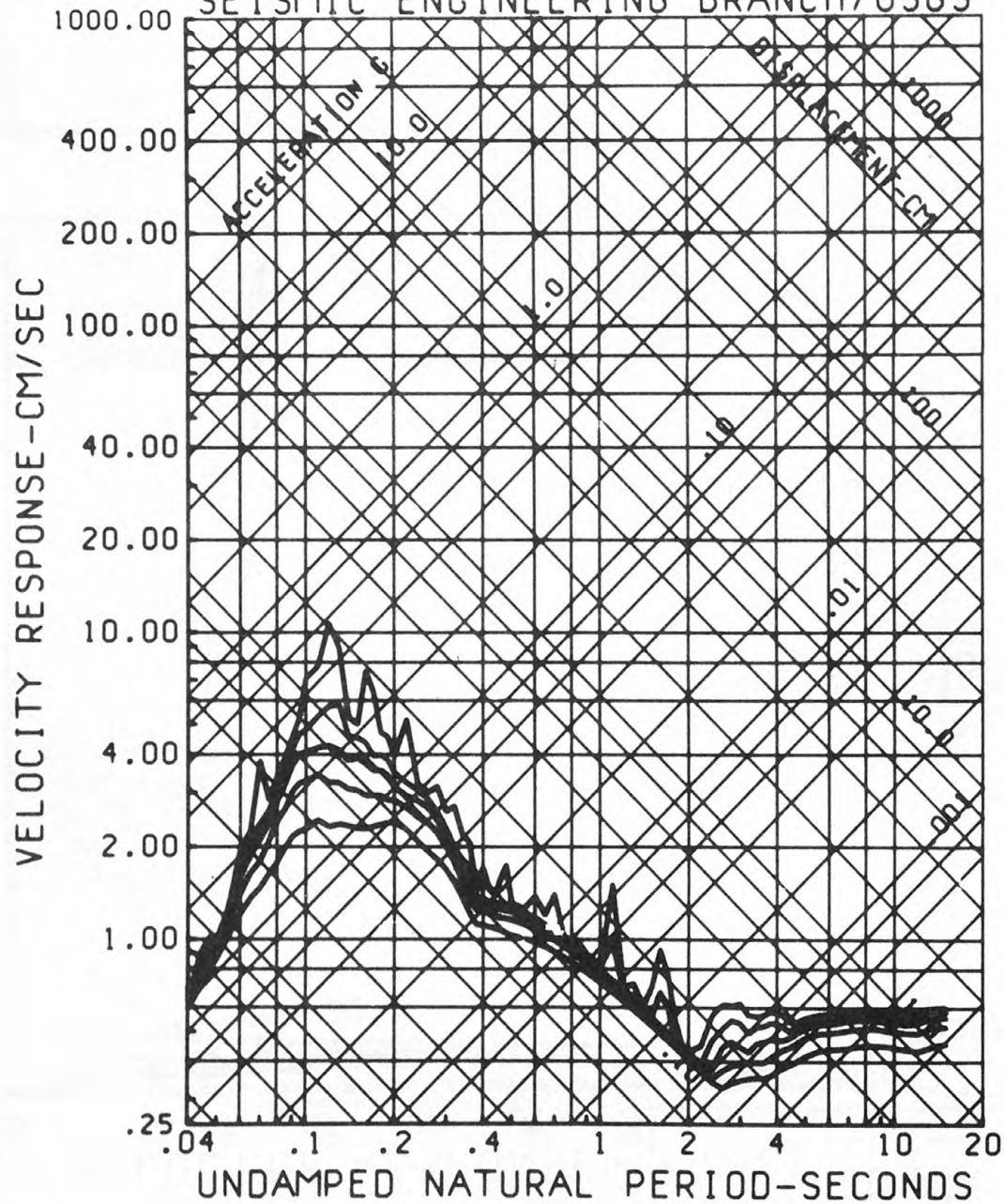
RELATIVE VELOCITY RESPONSE SPECTRUM
 PETROLIA, CALIF. CAPE MENDO, 01/11/75, DOWN
 0.2, 5, 10, 20 PERCENT CRITICAL DAMPING
 SEISMIC ENGINEERING BRANCH/USGS

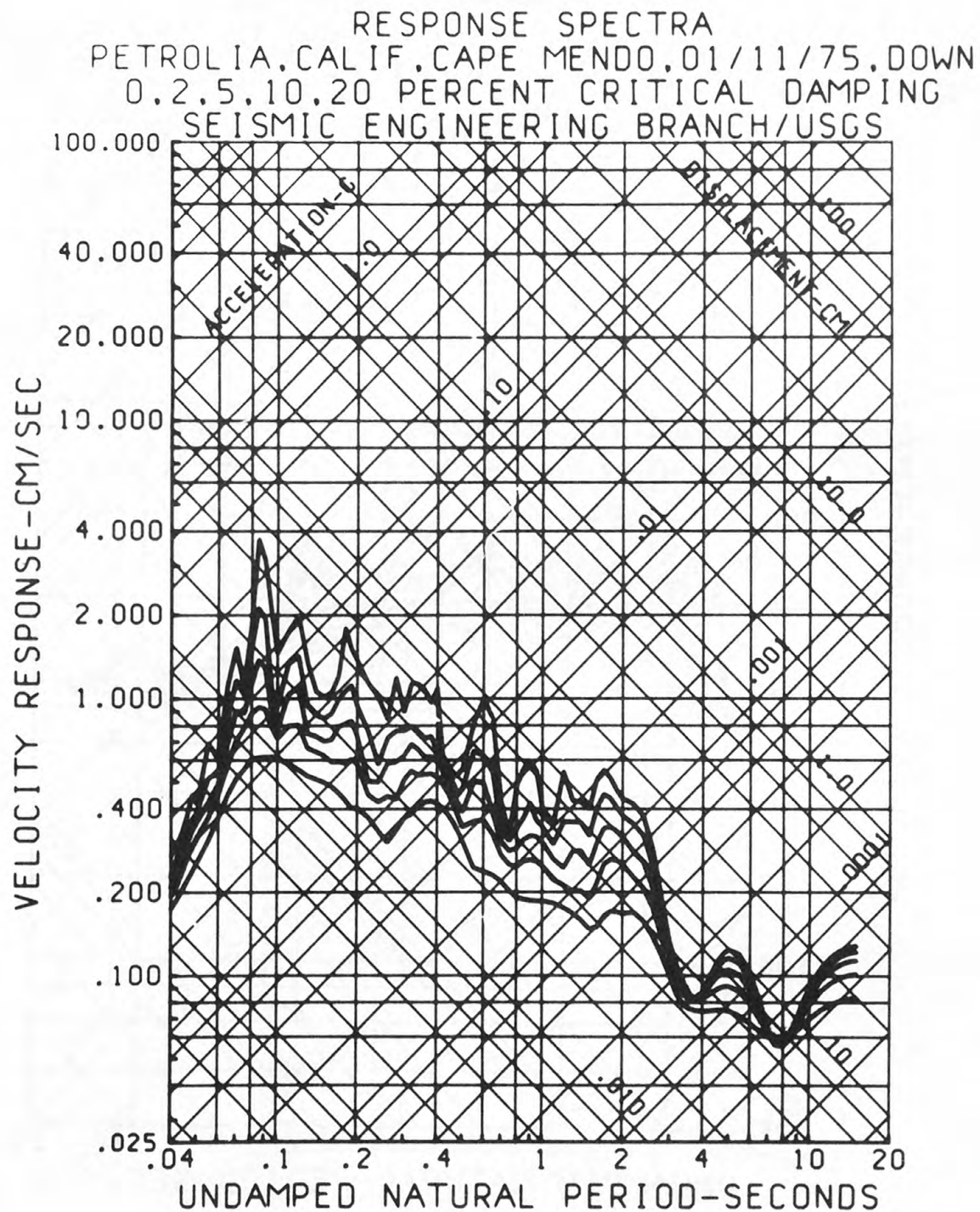


RELATIVE VELOCITY RESPONSE SPECTRUM
 PETROLIA,CALIF,CAPE MENDO,01/11/75,N30E
 0.2,5,10,20 PERCENT CRITICAL DAMPING
 SEISMIC ENGINEERING BRANCH/USGS

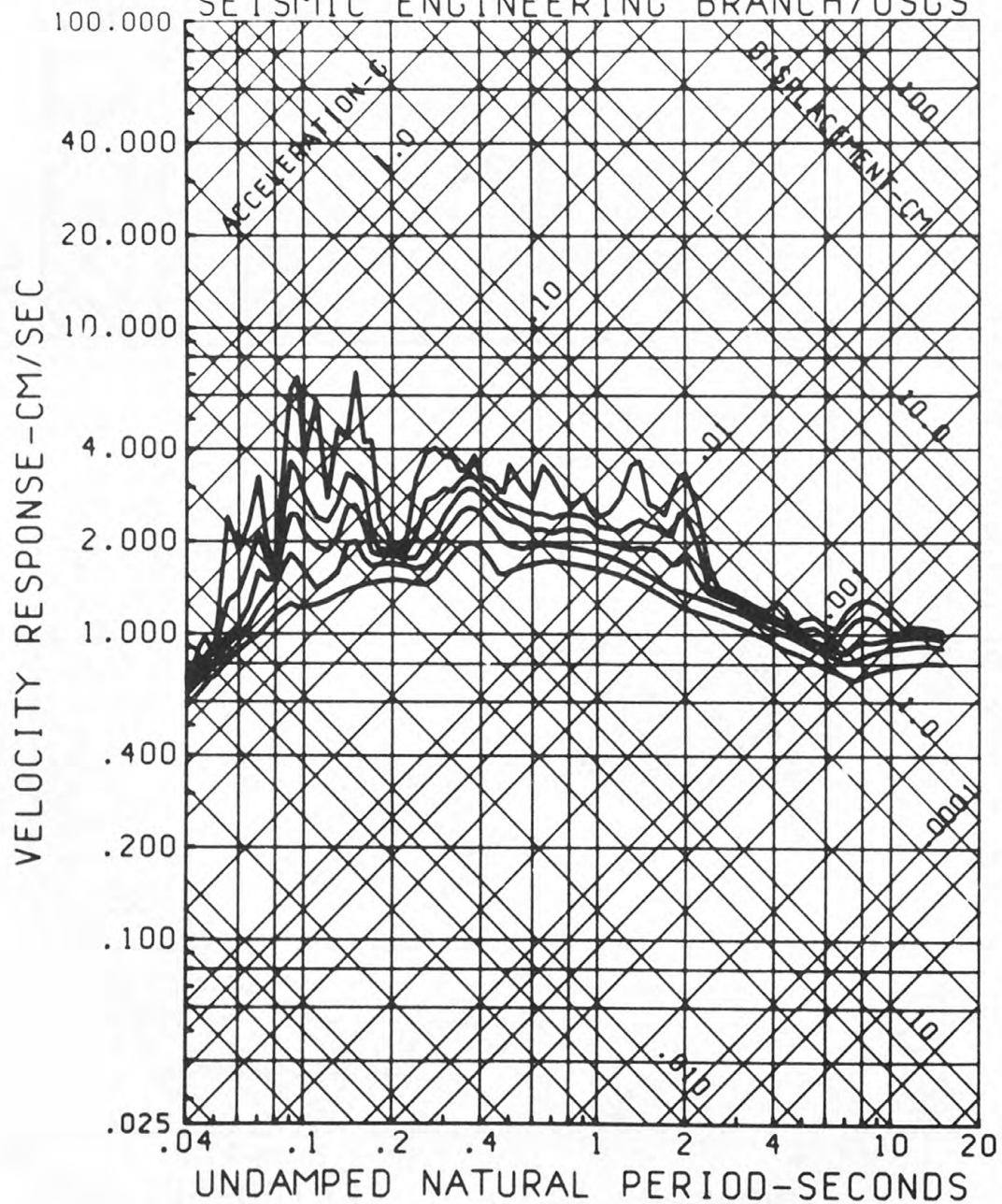


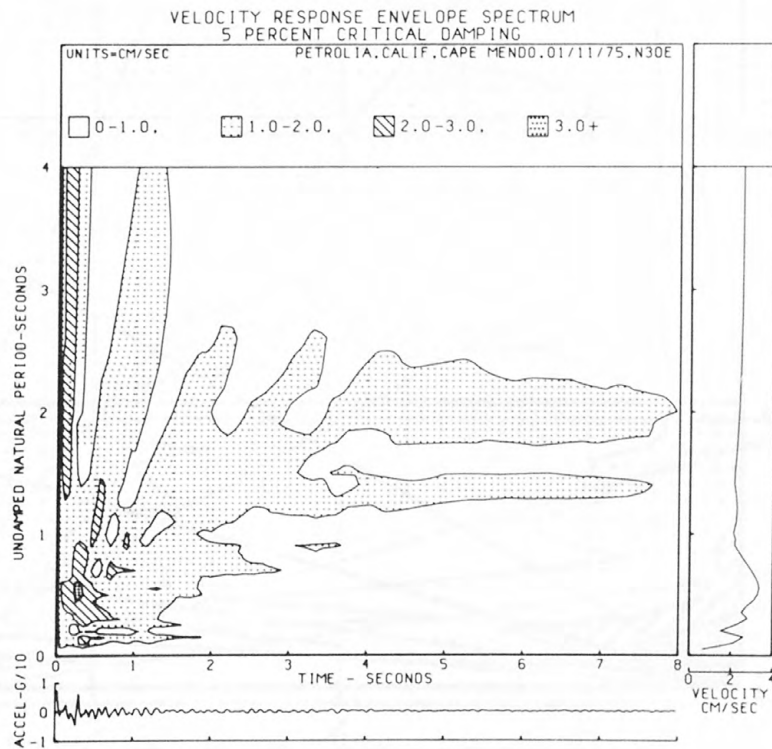
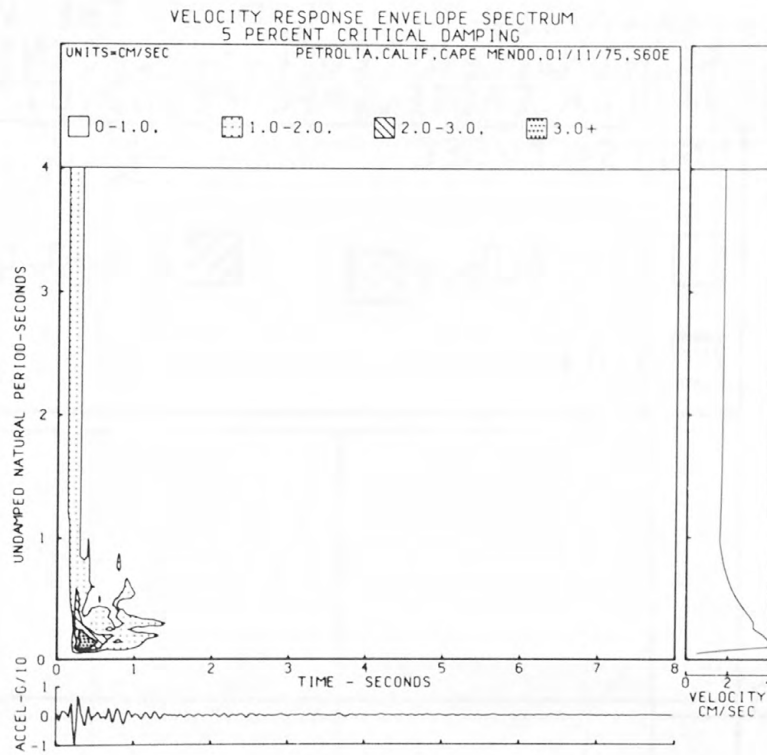
RESPONSE SPECTRA
 PETROLIA,CALIF,CAPE MENDO,01/11/75,S60E
 0.2,5,10,20 PERCENT CRITICAL DAMPING
 SEISMIC ENGINEERING BRANCH/USGS



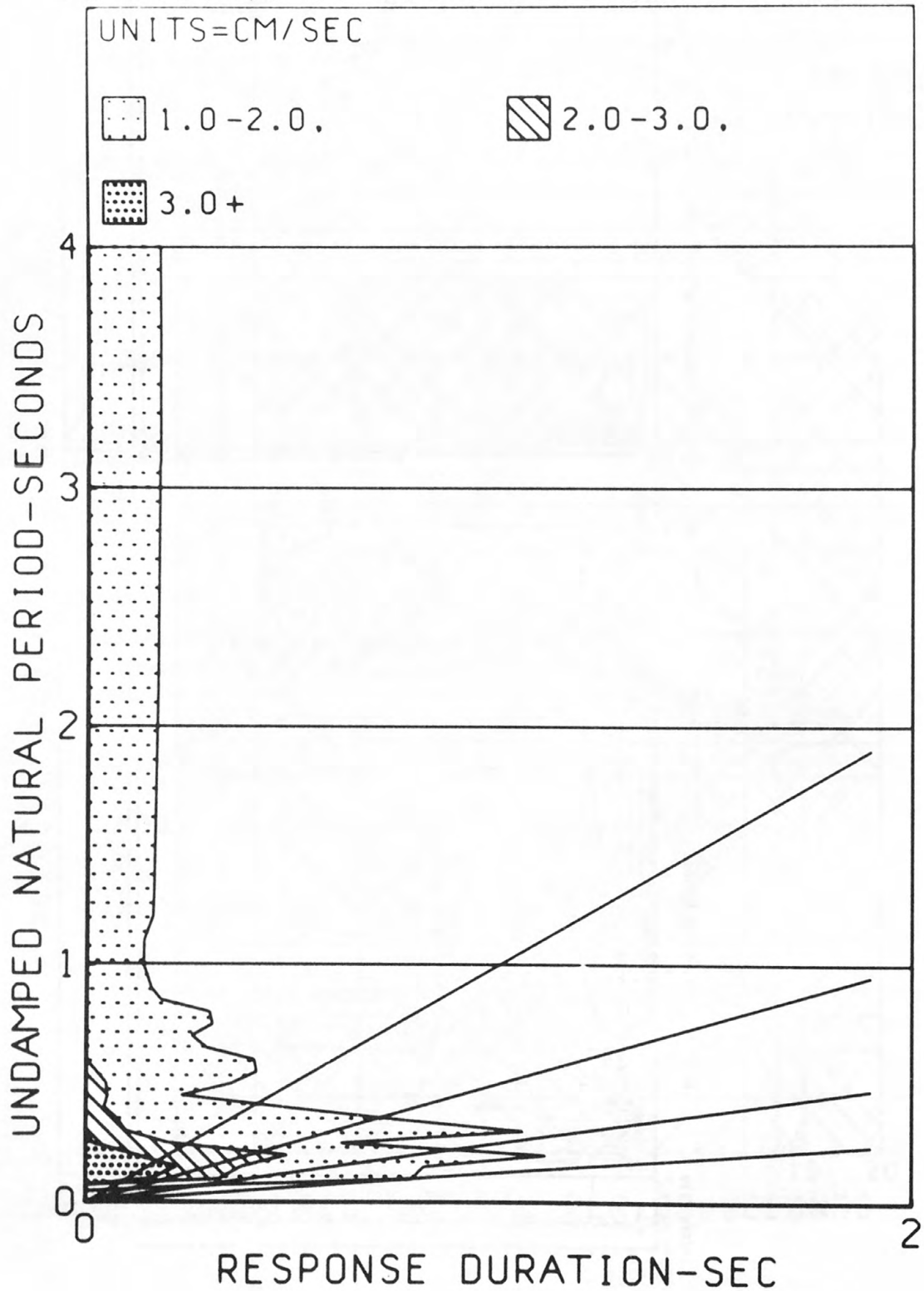


RESPONSE SPECTRA
 PETROLIA,CALIF,CAPE MENDO,01/11/75,N30E
 0.2,5,10,20 PERCENT CRITICAL DAMPING
 SEISMIC ENGINEERING BRANCH/USGS

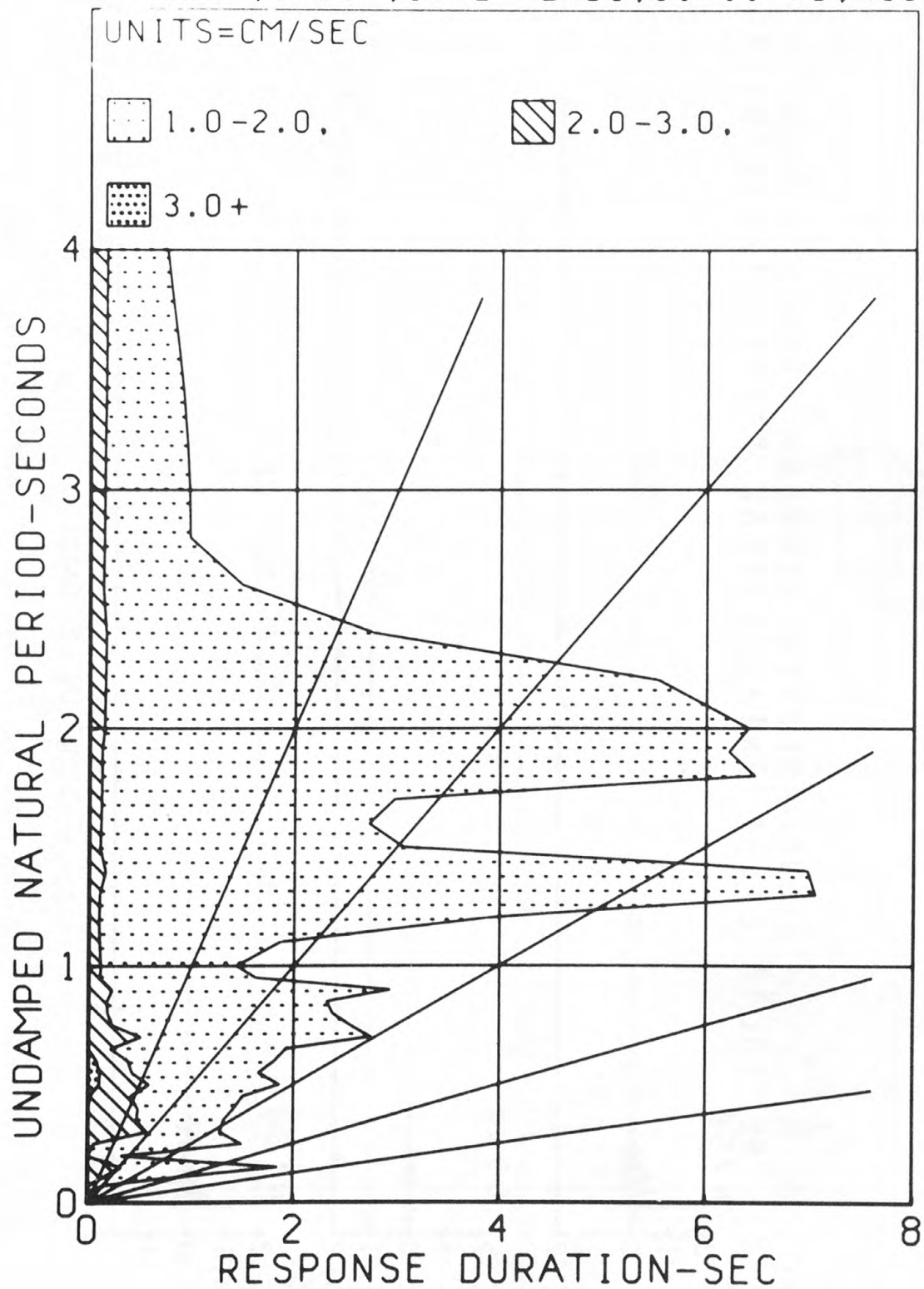




DURATION SPECTRUM OF THE VELOCITY
 RESPONSE ENVELOPE, 5 PERCENT DAMPING
 PETROLIA, CALIF, CAPE MENDO, 01/11/75, S60E

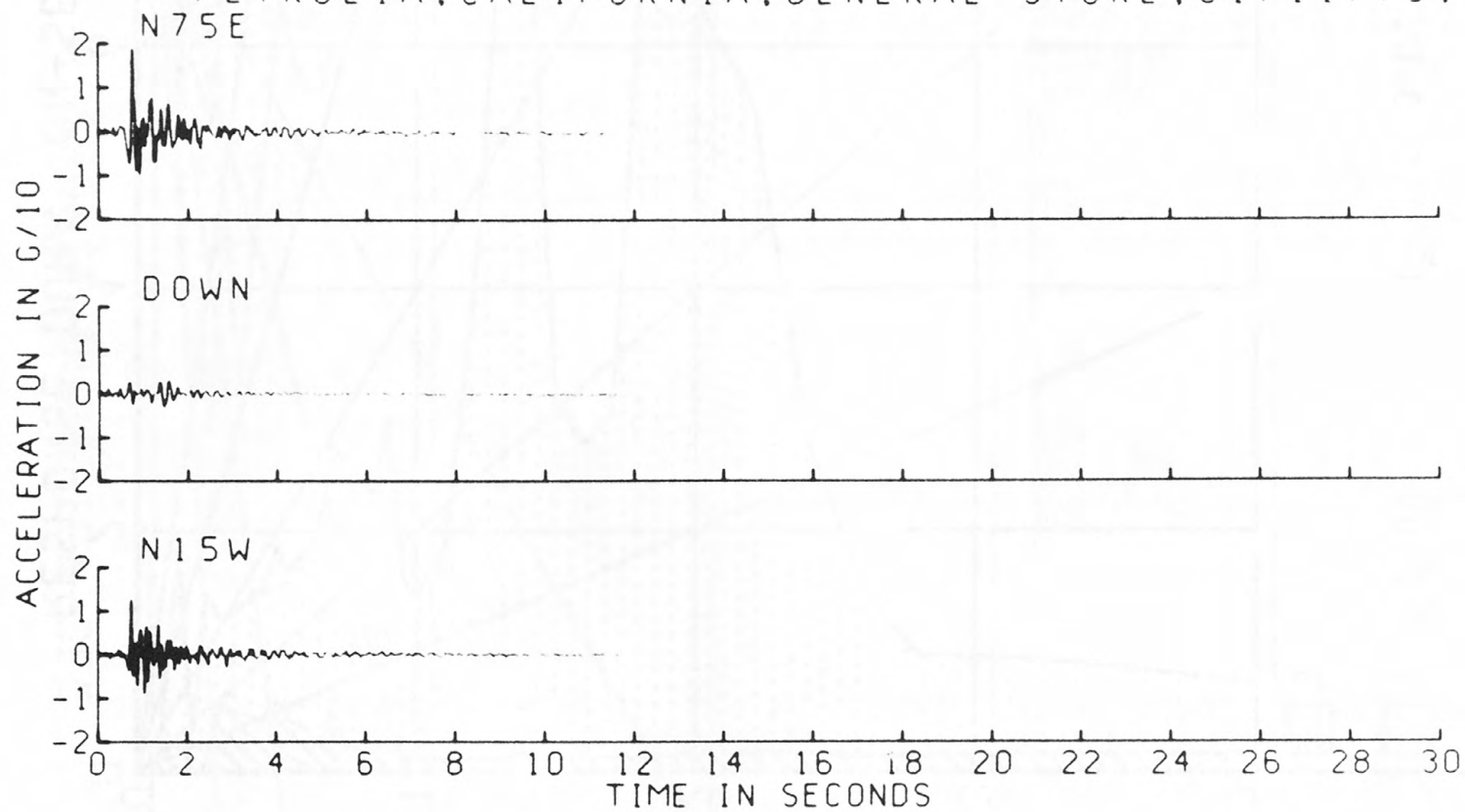


VIBRATION SPECTRUM OF THE VELOCITY
 RESPONSE ENVELOPE, 5 PERCENT DAMPING
 PETROLIA, CALIF, CAPE MENDO, 01/11/75, N30E



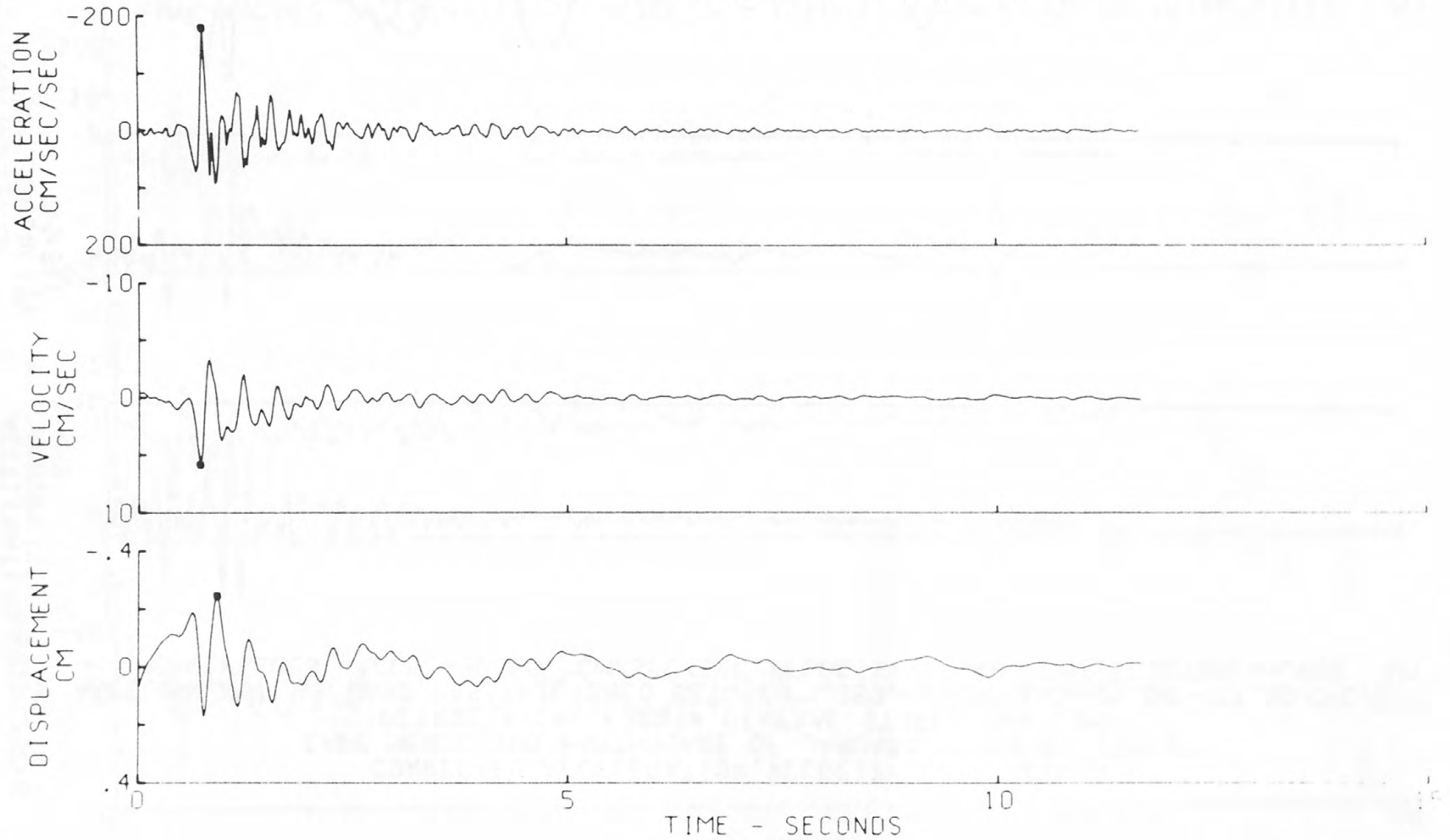
UNCORRECTED ACCELEROGRAM

PETROLIA, CALIFORNIA, GENERAL STORE, 01/11/75, 1737 PST



CORRECTED ACCELERATION, VELOCITY, DISPLACEMENT
CAPE MENDOCINO EARTHQUAKE OF JANUARY 11, 1975, 1737 PST
PETROLIA, CALIFORNIA, GENERAL STORE, N75E COMP

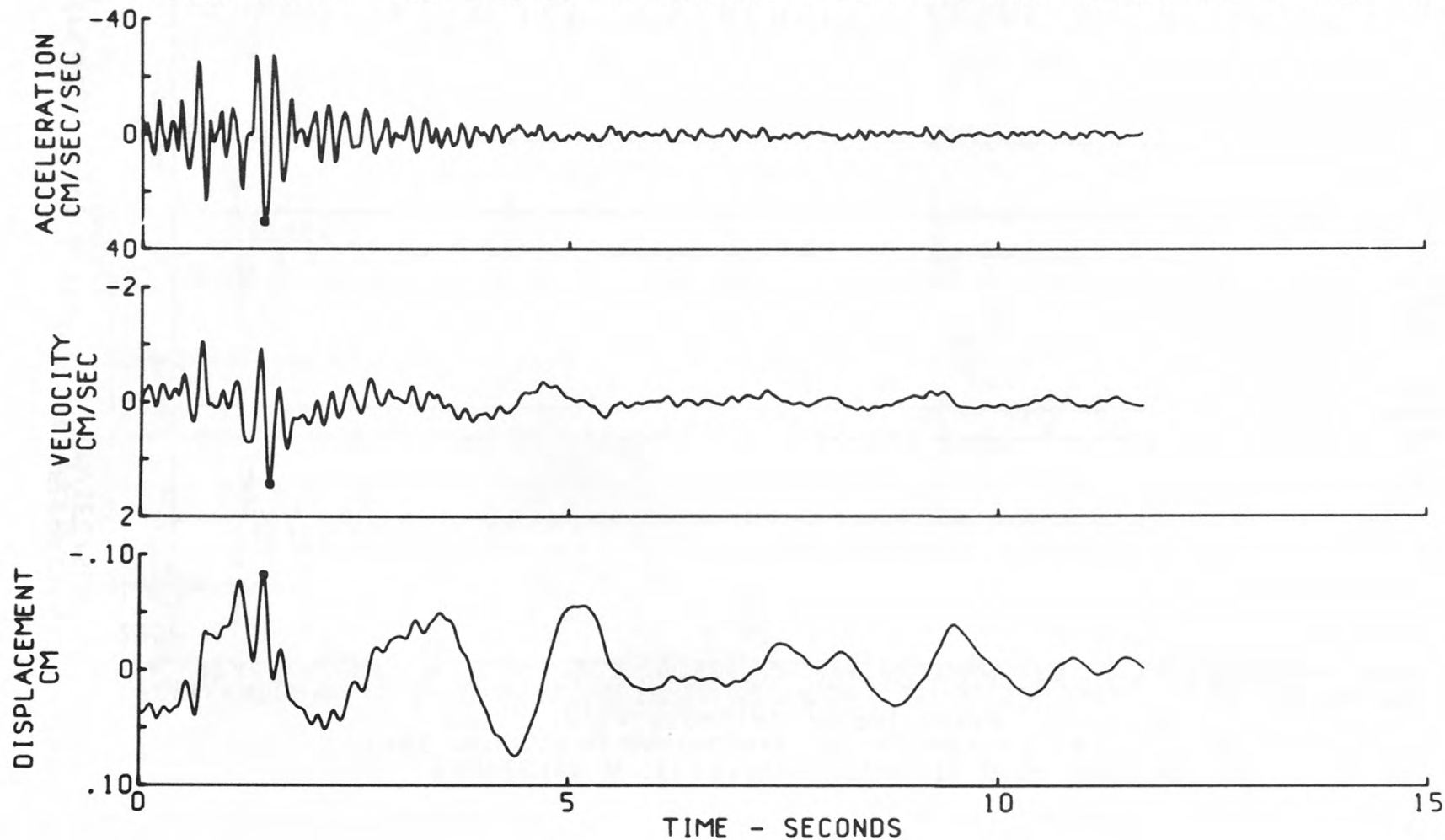
ACCELEROGRAM IS BAND PASS FILTERED BETWEEN .350 - .500 AND 25.00 - 27.00 CYC/SEC
• PEAK VALUES ACCEL=-179.7 CM/SEC/SEC, VELOCITY=5.869 CM/SEC, DISPL=-.244 CM



CORRECTED ACCELERATION, VELOCITY, DISPLACEMENT
CAPE MENDOCINO EARTHQUAKE OF JANUARY 11, 1975, 1737PST
PETROLIA, CALIFORNIA, GENERAL STORE, DOWN COMP

ACCELEROGRAM IS BAND PASS FILTERED BETWEEN .350 - .500 AND 25.00 - 27.00 CYC/SEC

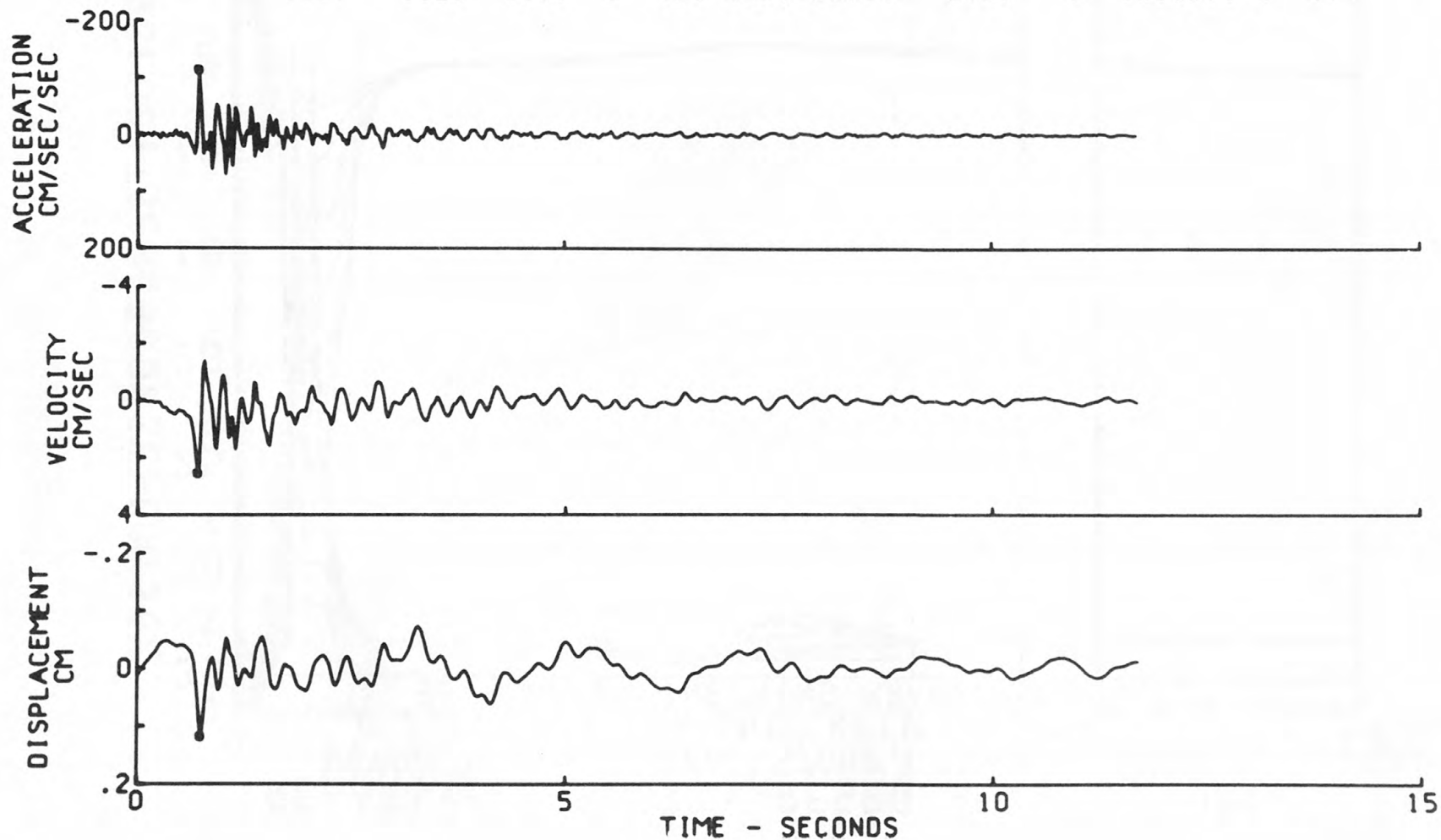
• PEAK VALUES ACCEL=30.50 CM/SEC/SEC, VELOCITY=1.440 CM/SEC, DISPL=-.082 CM



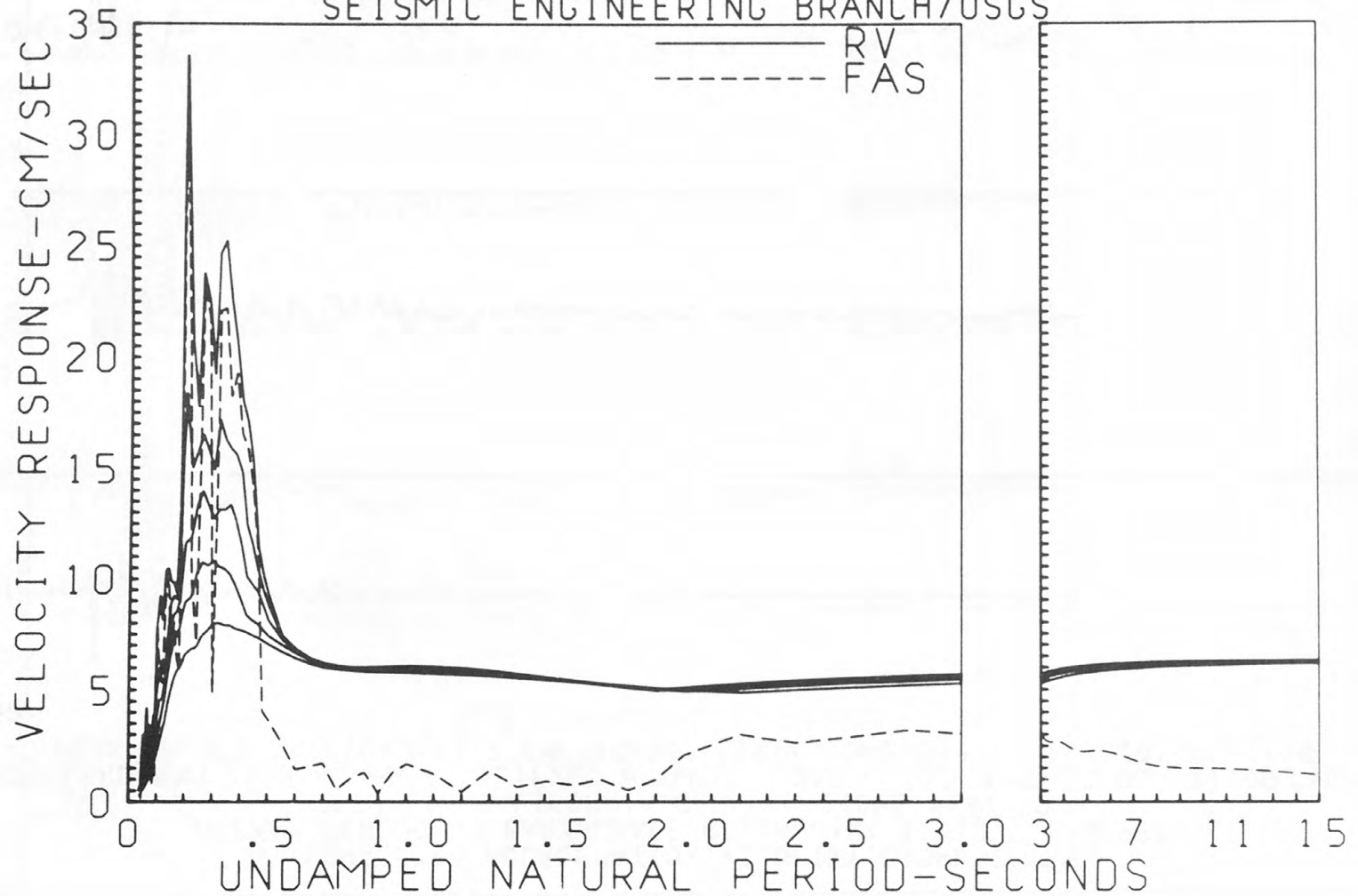
CORRECTED ACCELERATION, VELOCITY, DISPLACEMENT
CAPE MENDOCINO EARTHQUAKE OF JANUARY 11, 1975, 1737PST
PETROLIA, CALIFORNIA, GENERAL STORE, N15W COMP

ACCELEROGRAM IS BAND PASS FILTERED BETWEEN .350 - .500 AND 25.00 - 27.00 CYC/SEC

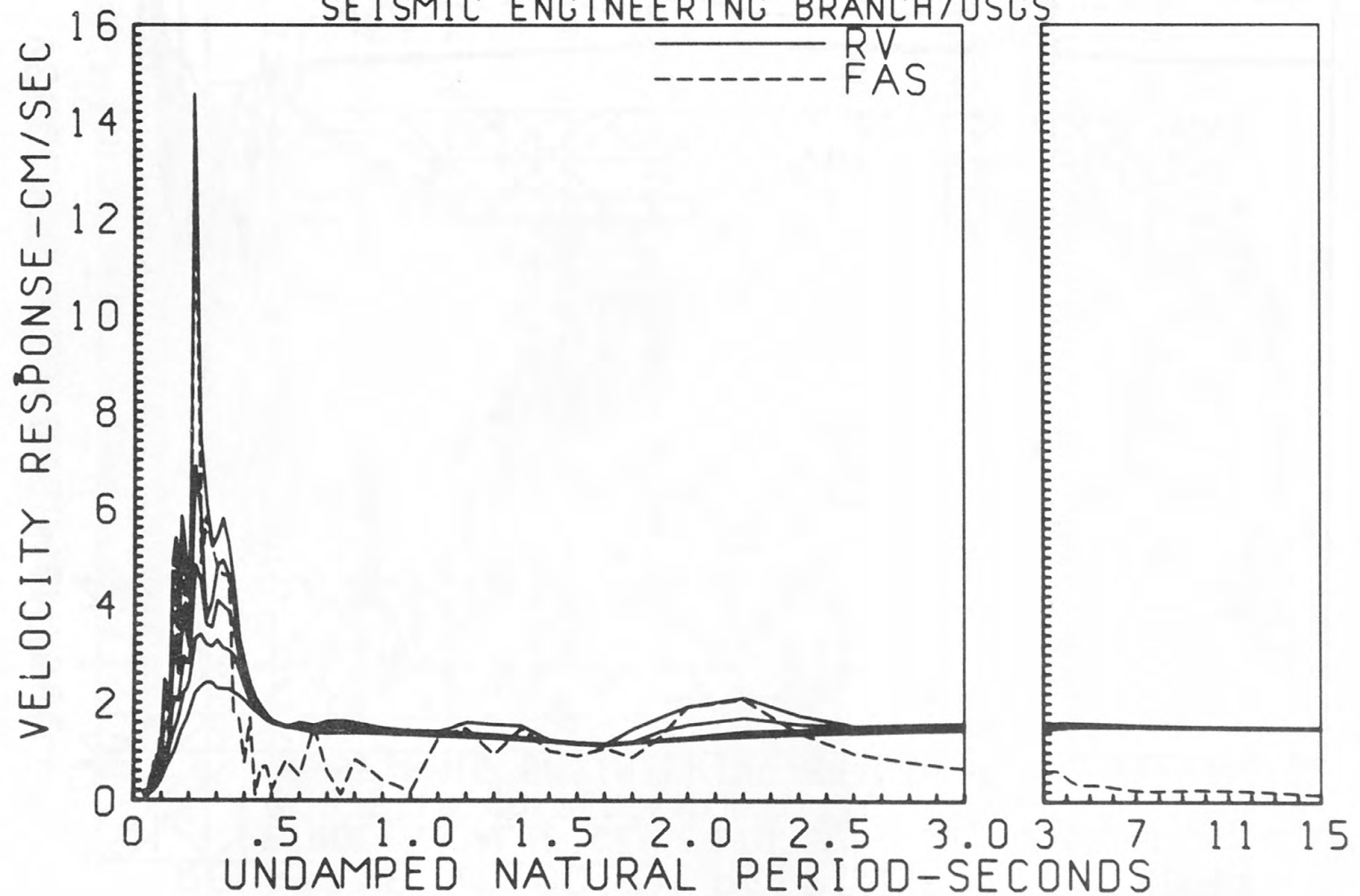
• PEAK VALUES ACCEL=-113.7 CM/SEC/SEC, VELOCITY=2.564 CM/SEC, DISPL=.119 CM



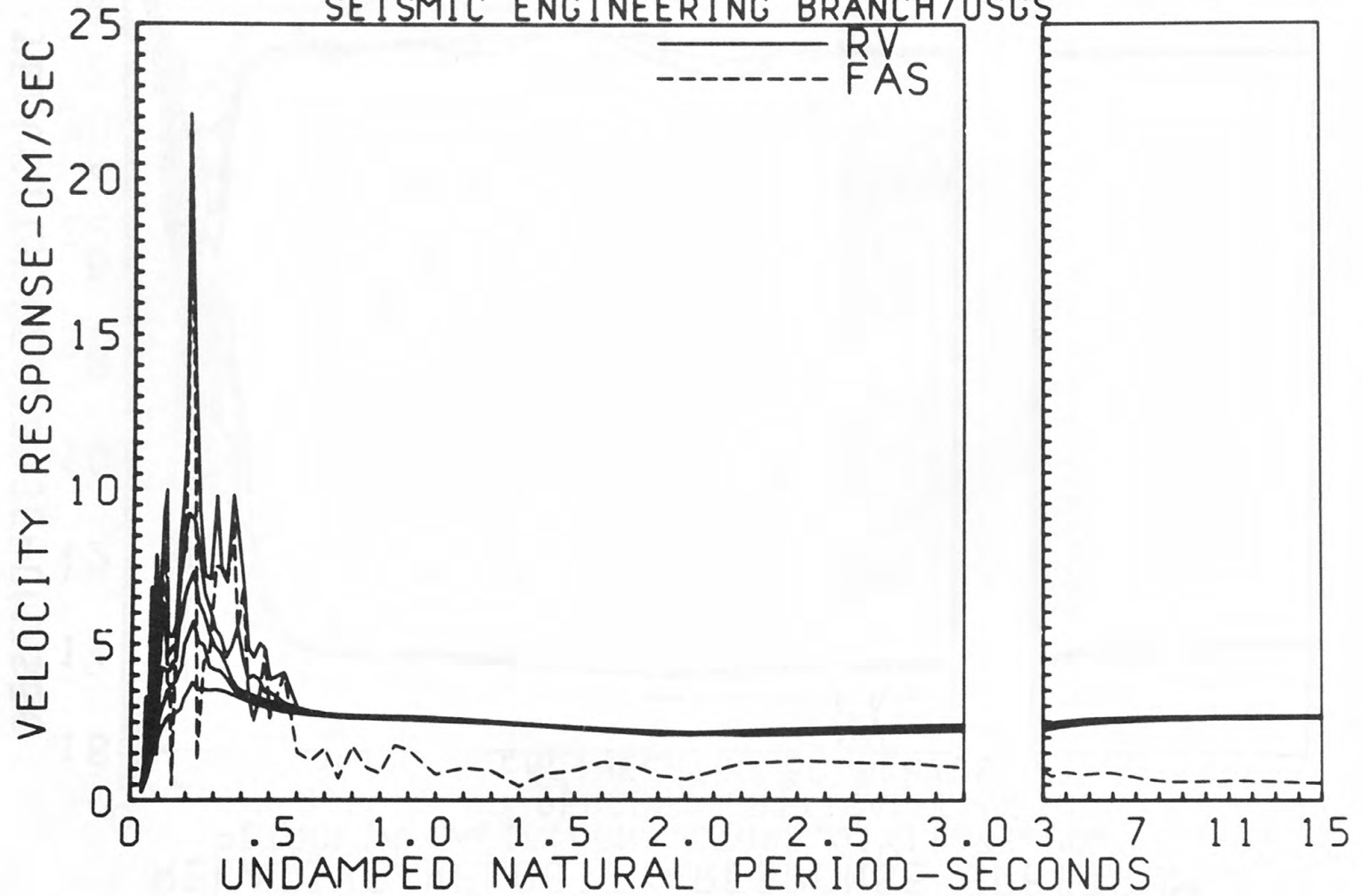
RELATIVE VELOCITY RESPONSE SPECTRUM
PETROLIA,CALIF.GEN. STORE,01/11/75,N75E
0.2,5,10,20 PERCENT CRITICAL DAMPING
SEISMIC ENGINEERING BRANCH/USGS



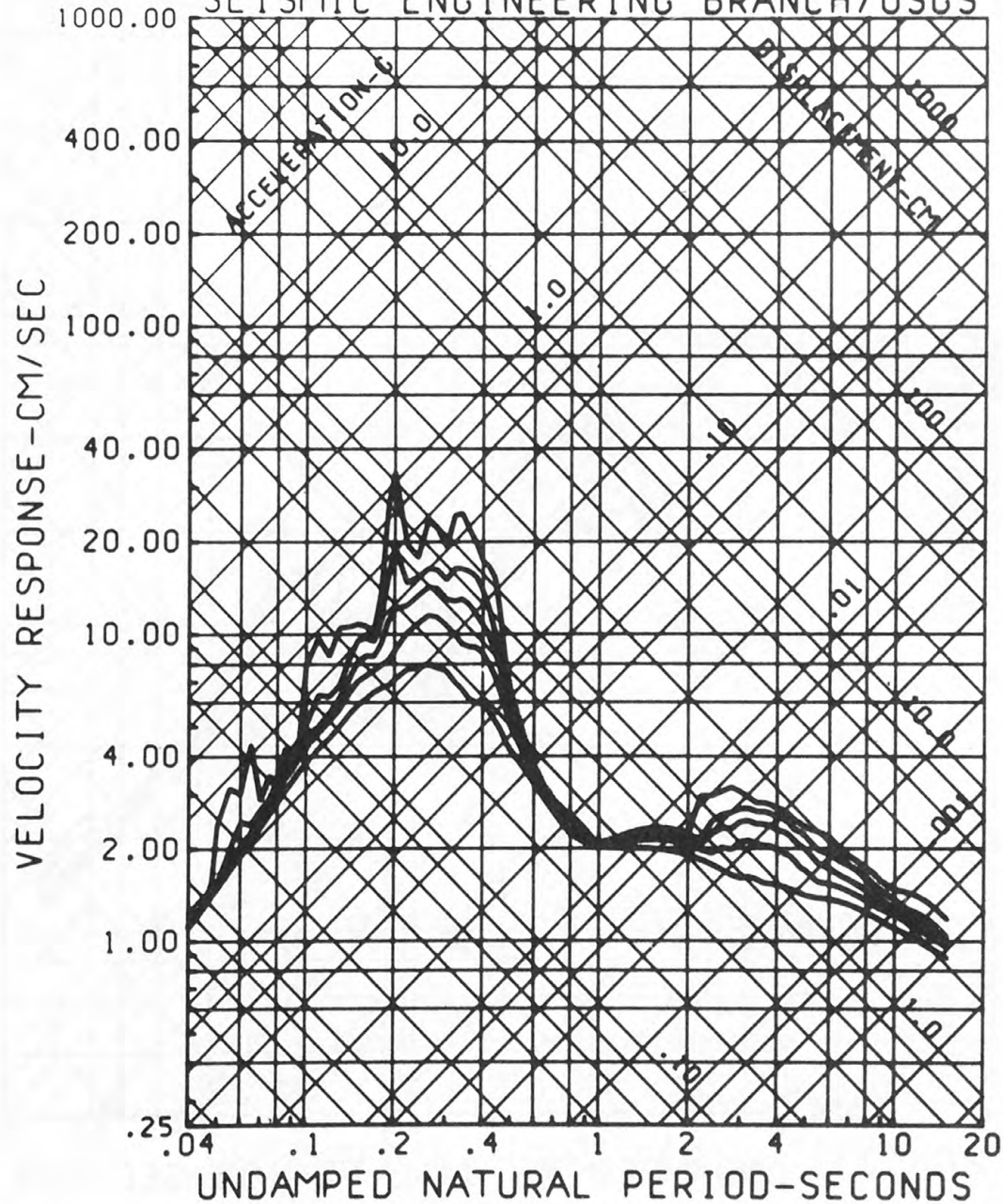
RELATIVE VELOCITY RESPONSE SPECTRUM
 PETROLIA, CALIF. GEN. STORE, 01/11/75, DOWN
 0, 2, 5, 10, 20 PERCENT CRITICAL DAMPING
 SEISMIC ENGINEERING BRANCH/USGS



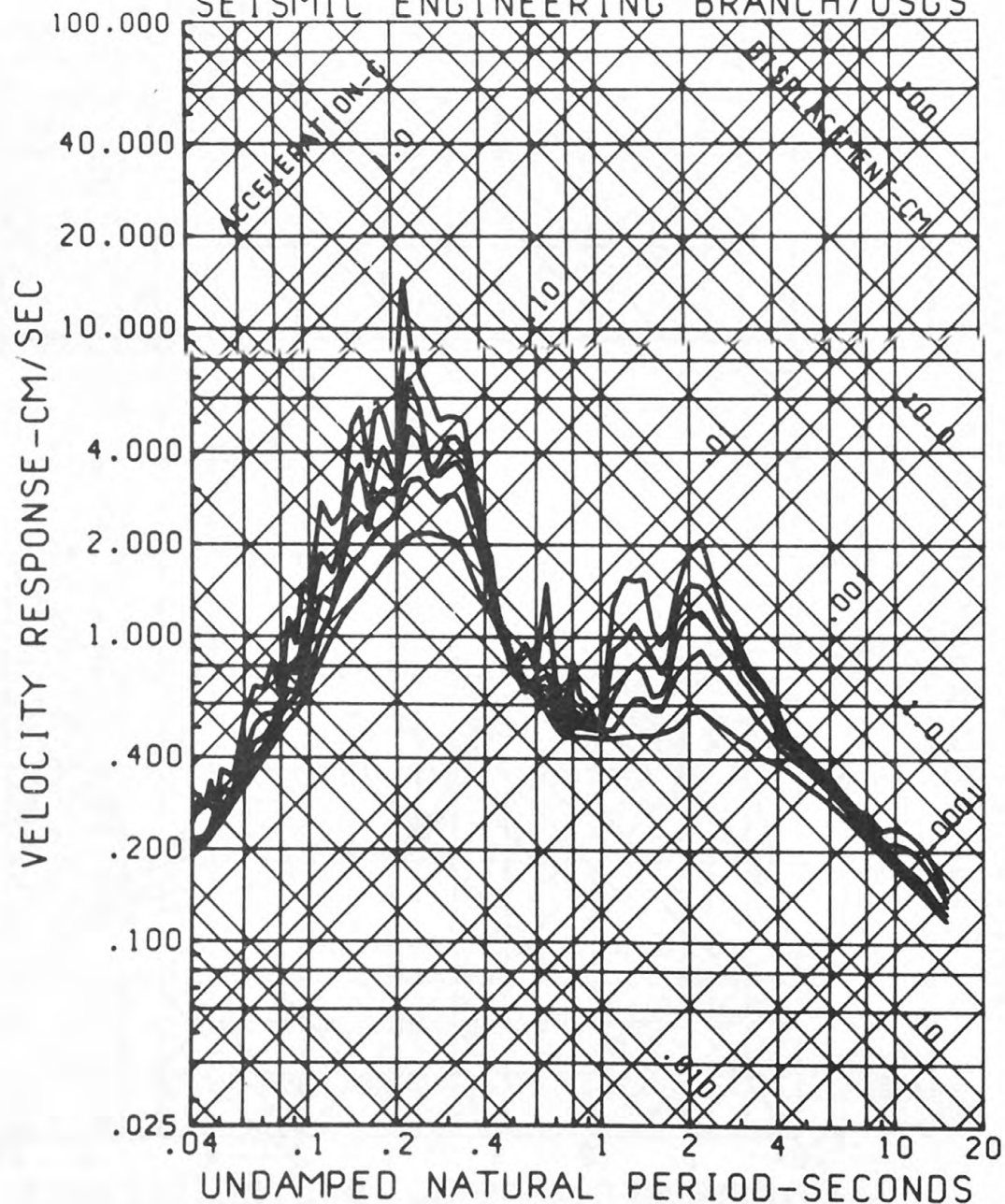
RELATIVE VELOCITY RESPONSE SPECTRUM
PETROLIA,CALIF,GEN. STORE,01/11/75,N15W
0.2,5,10,20 PERCENT CRITICAL DAMPING
SEISMIC ENGINEERING BRANCH/USGS

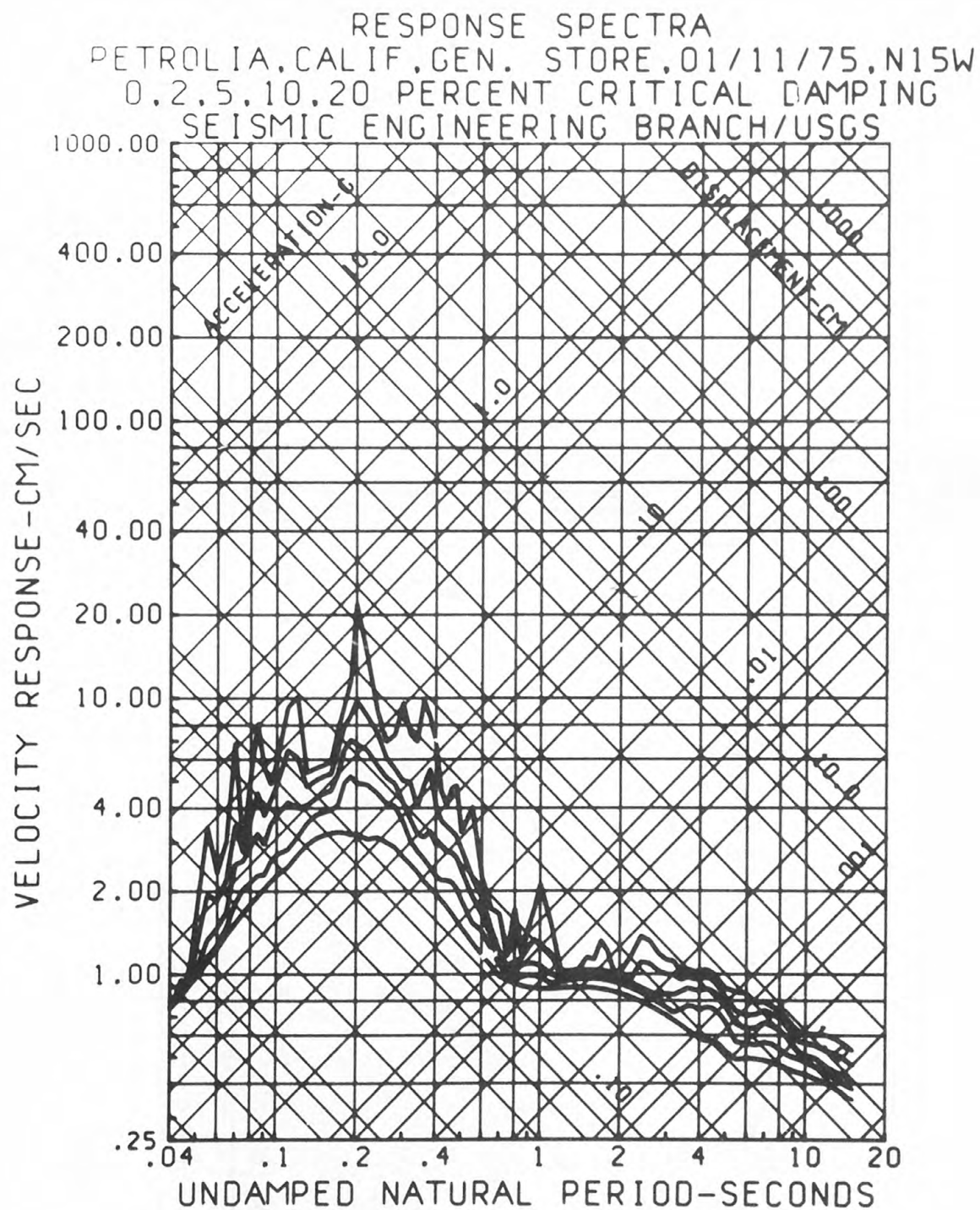


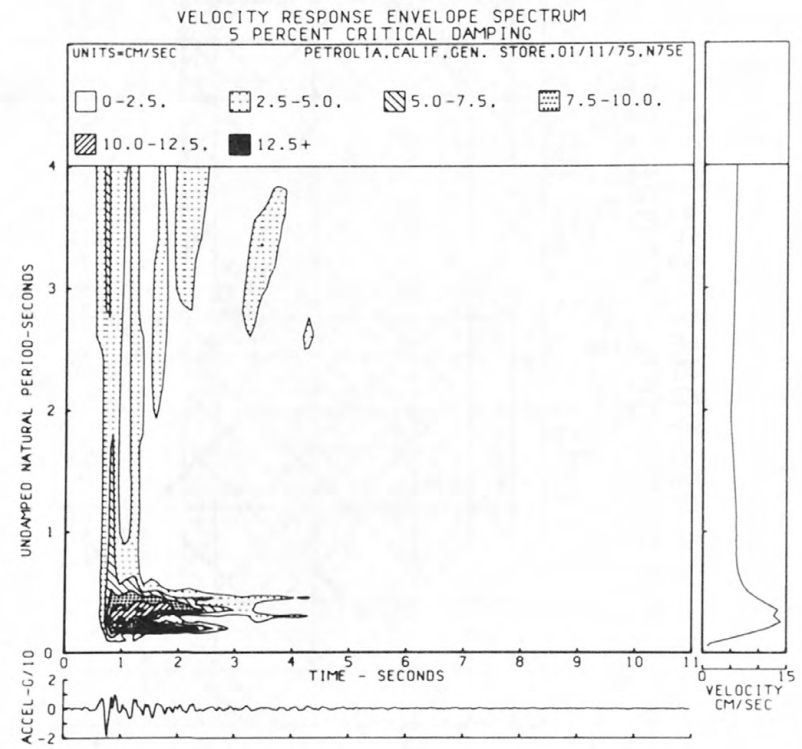
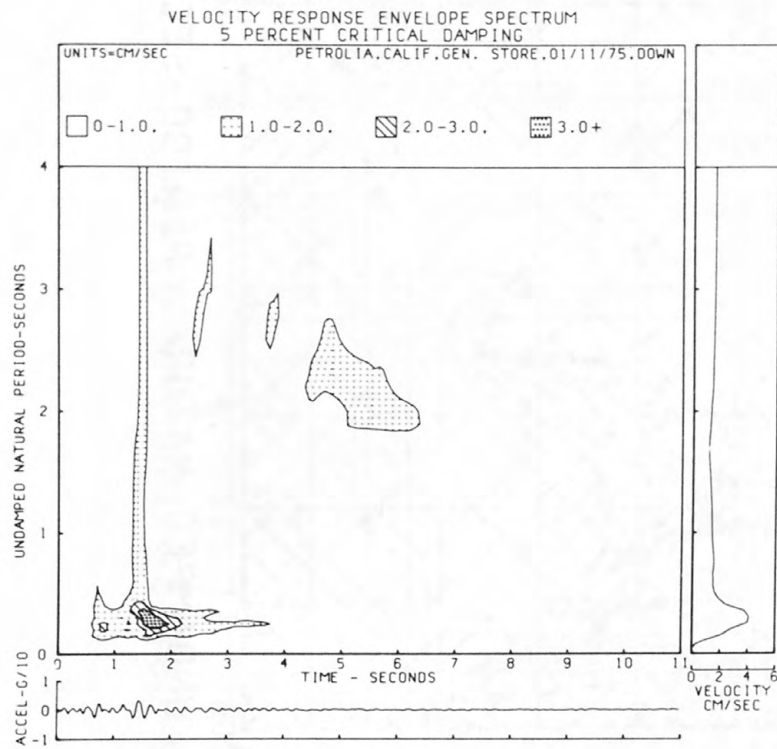
RESPONSE SPECTRA
 PETROLIA, CALIF. GEN. STORE. 01/11/75. N75E
 0.2, 5, 10, 20 PERCENT CRITICAL DAMPING
 SEISMIC ENGINEERING BRANCH/USGS

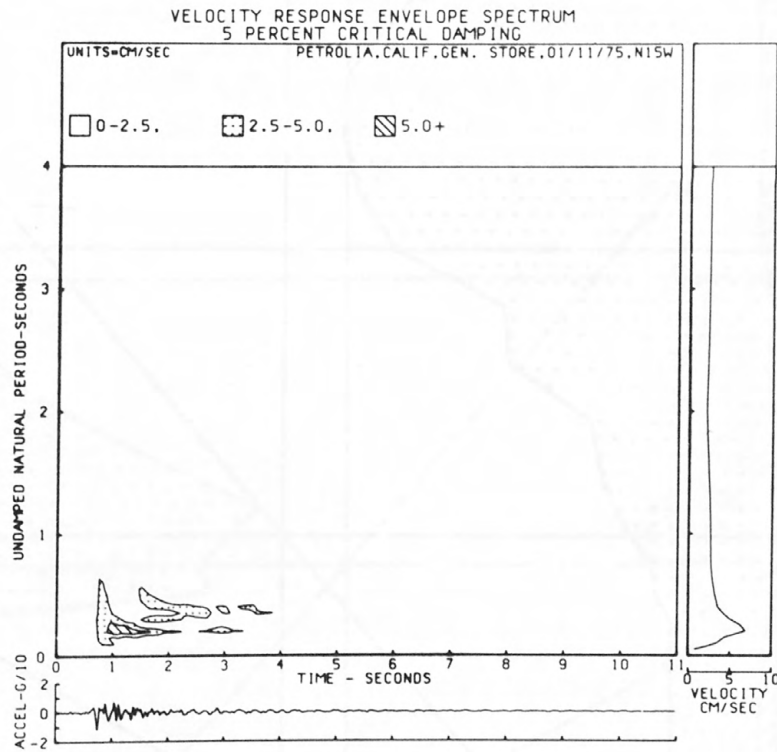


RESPONSE SPECTRA
 PETROLIA, CALIF. GEN. STORE, 01/11/75, DOWN
 0.2, 5, 10, 20 PERCENT CRITICAL DAMPING
 SEISMIC ENGINEERING BRANCH/USGS

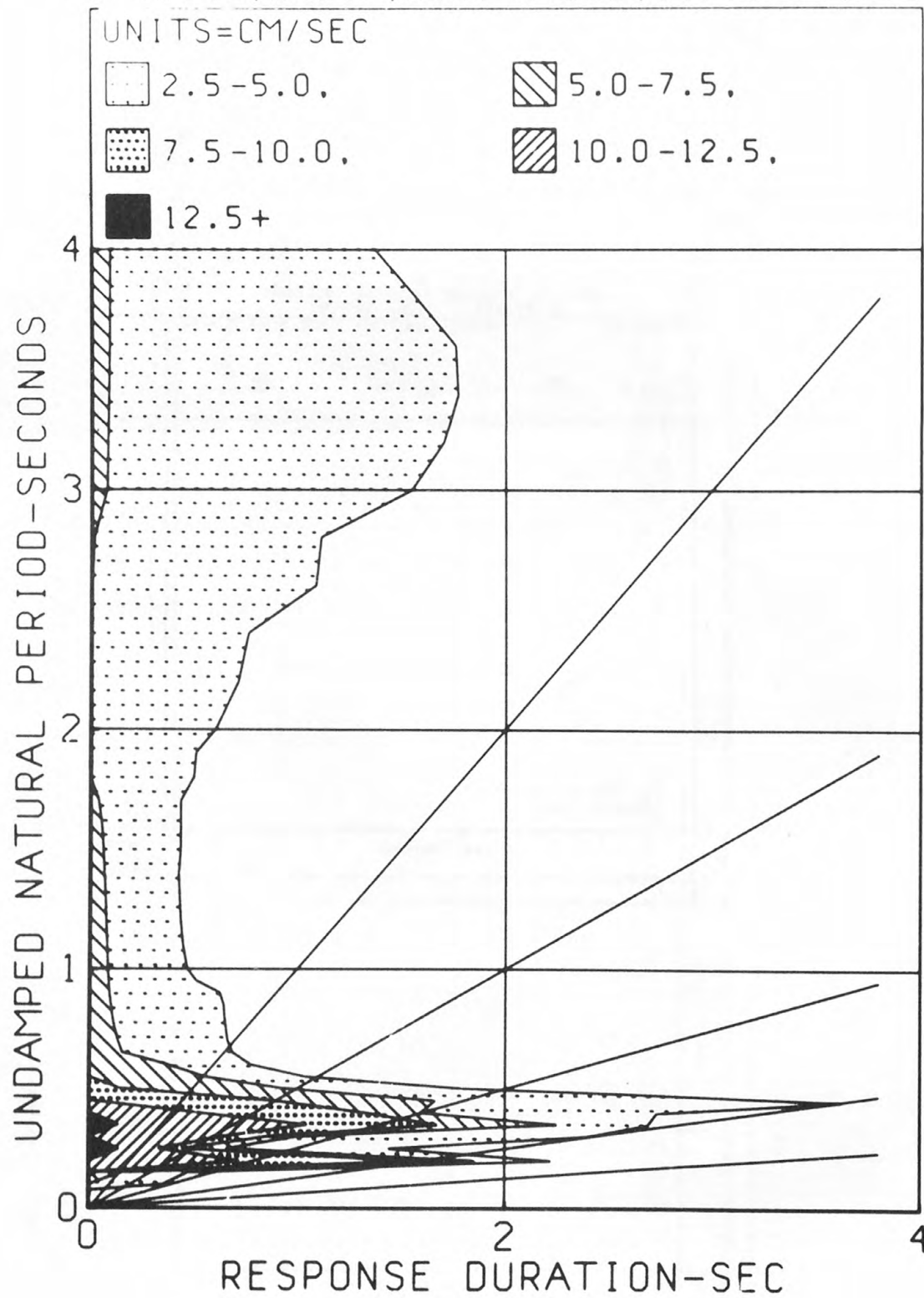




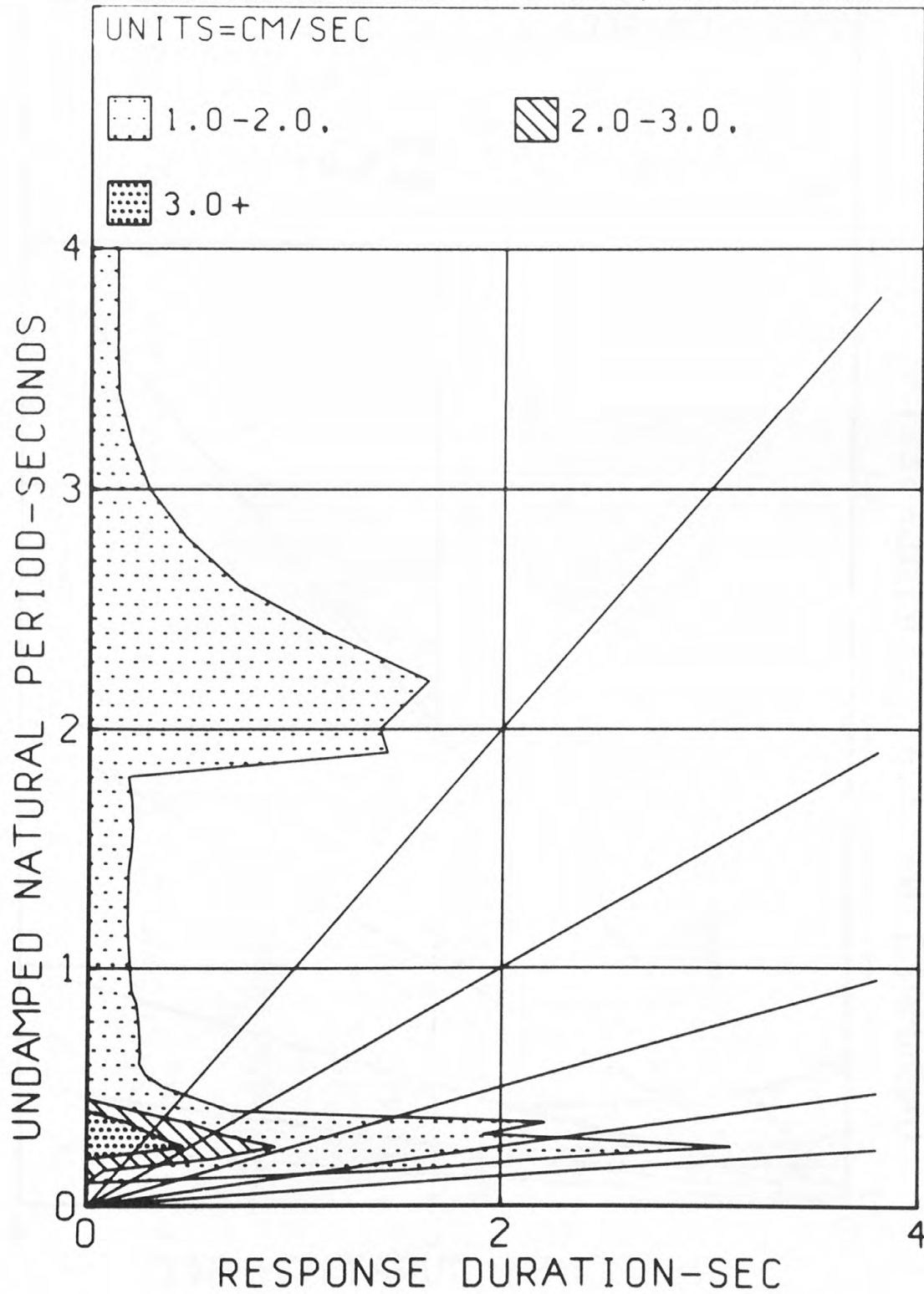




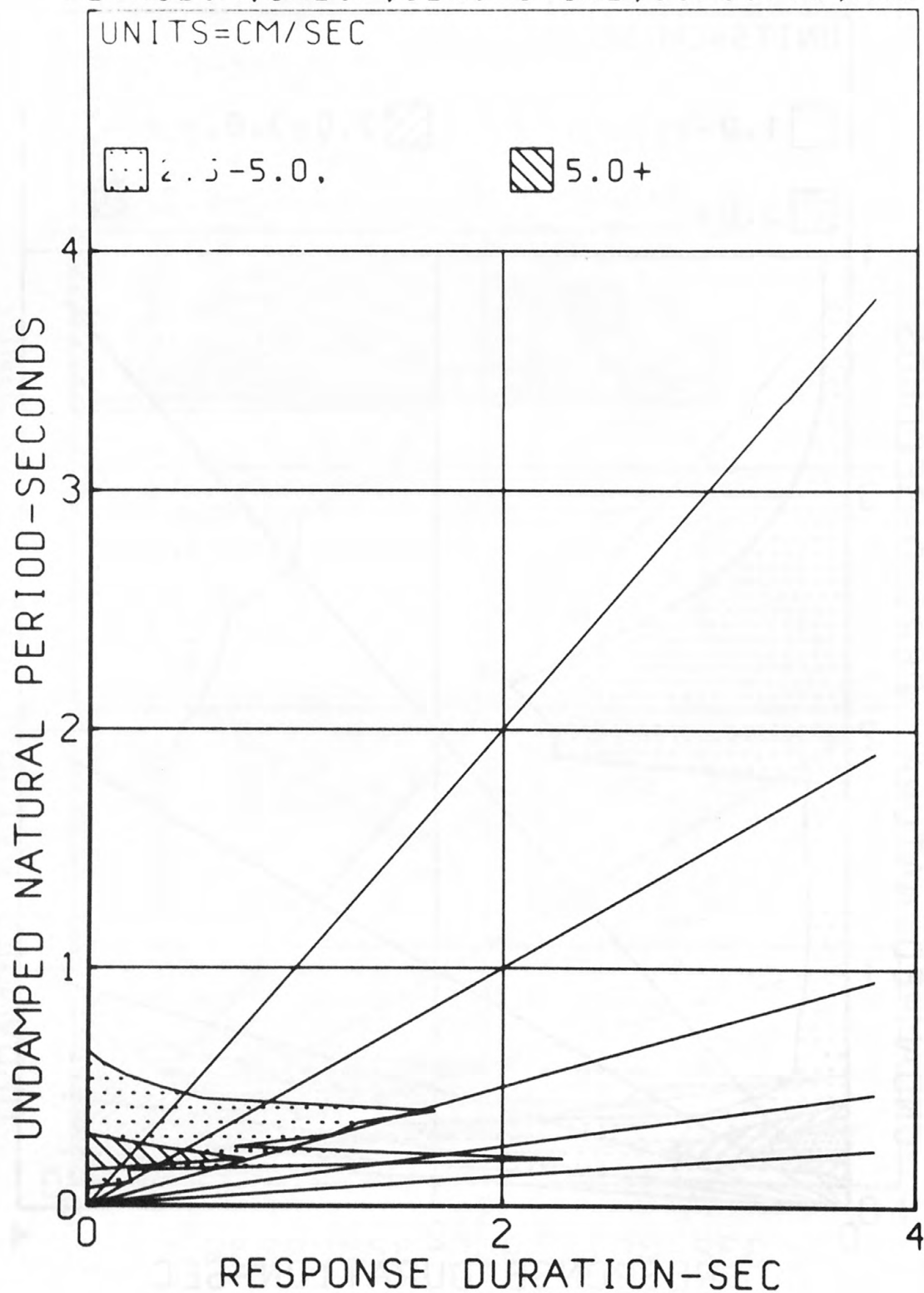
VIBRATION SPECTRUM OF THE VELOCITY
 RESPONSE ENVELOPE, 5 PERCENT DAMPING
 PETROLIA, CALIF, GEN. STORE, 01/11/75, N75E

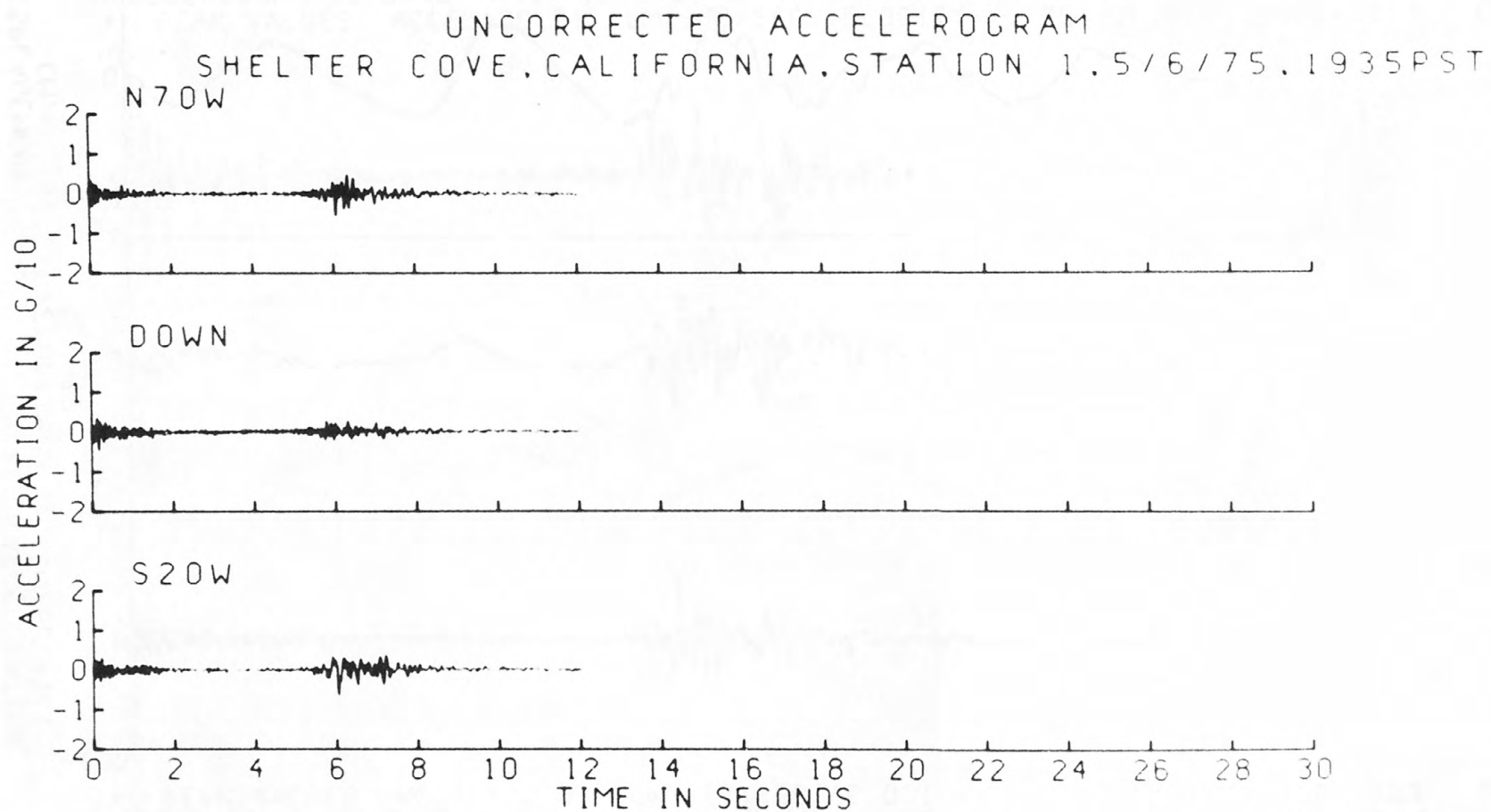


DURATION SPECTRUM OF THE VELOCITY
 RESPONSE ENVELOPE, 5 PERCENT DAMPING
 PETROLIA, CALIF, GEN. STORE, 01/11/75, DOWN

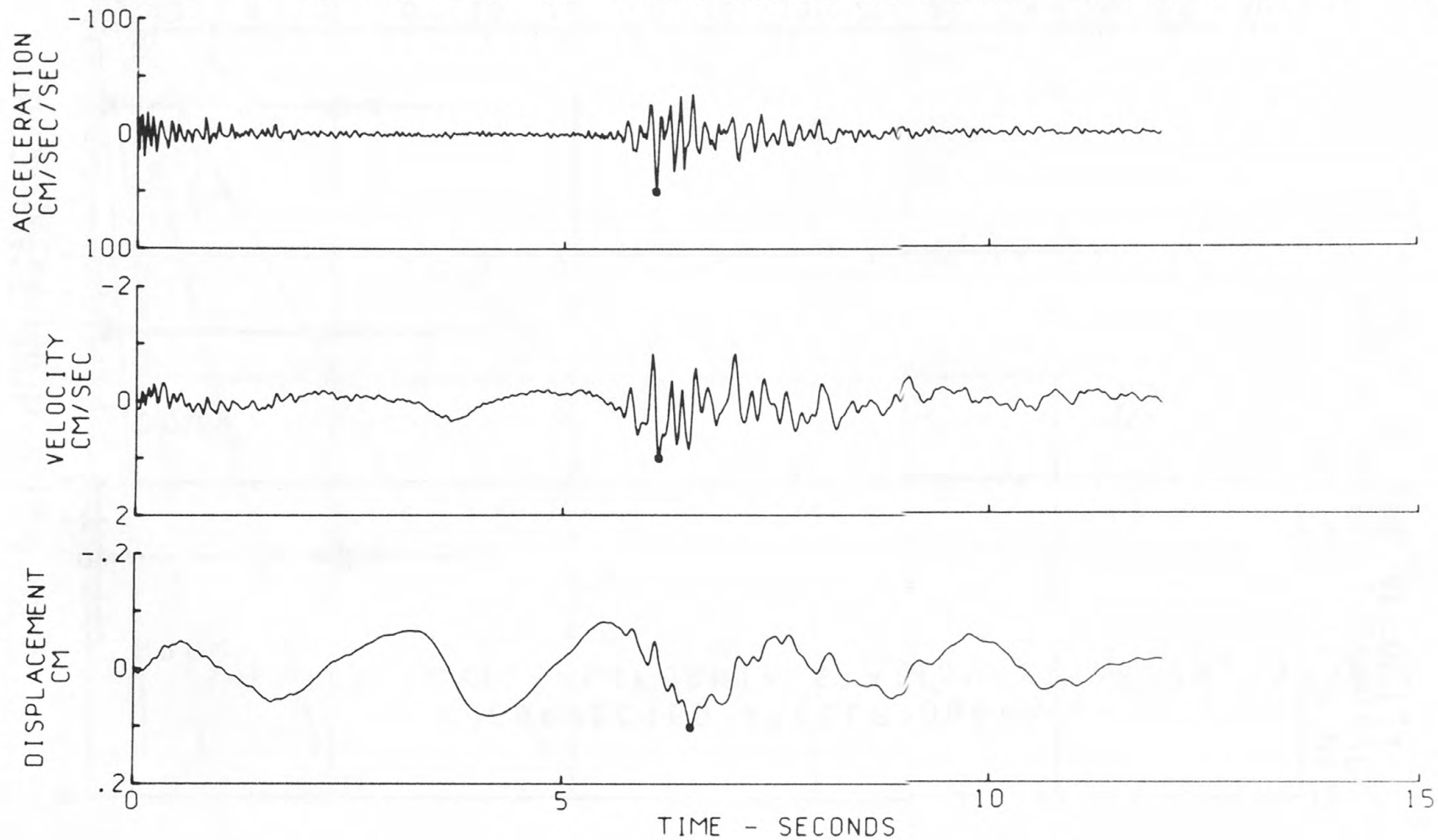


DURATION SPECTRUM OF THE VELOCITY
 RESPONSE ENVELOPE, 5 PERCENT DAMPING
 PETROLIA, CALIF, GEN. STORE, 01/11/75, N15W



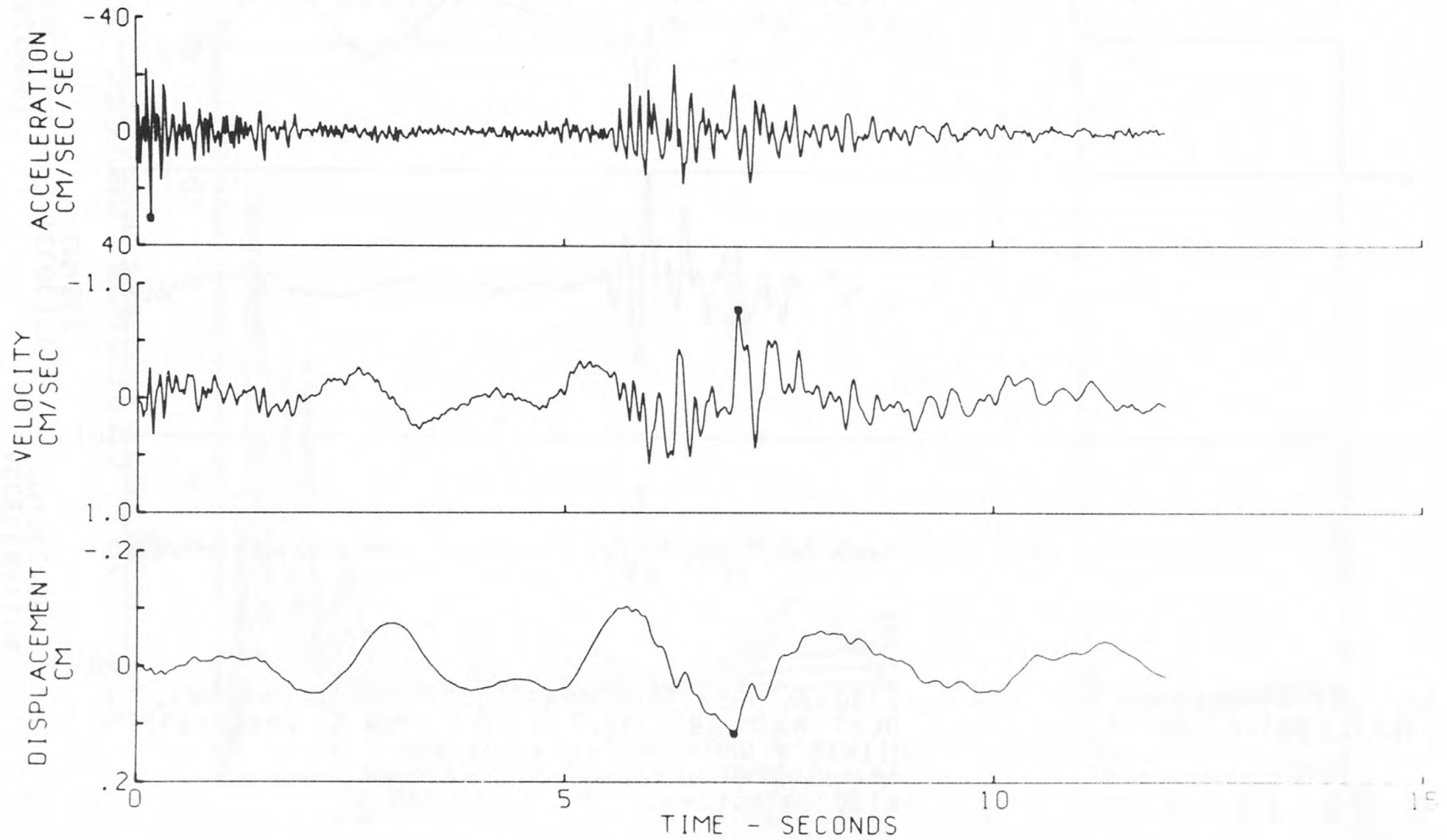


CORRECTED ACCELERATION, VELOCITY, DISPLACEMENT
 CAPE MENDOCINO EARTHQUAKE OF MAY 6, 1975, 1935 PST
 SHELTER COVE, CALIFORNIA, STATION 1, N70W COMP
 ACCELEROGRAM IS BAND PASS FILTERED BETWEEN .230 - .330 AND 25.00 - 27.00 CYC/SEC
 • PEAK VALUES ACCEL=50.03 CM/SEC/SEC, VELOCITY=1.010 CM/SEC, DISPL=.103 CM

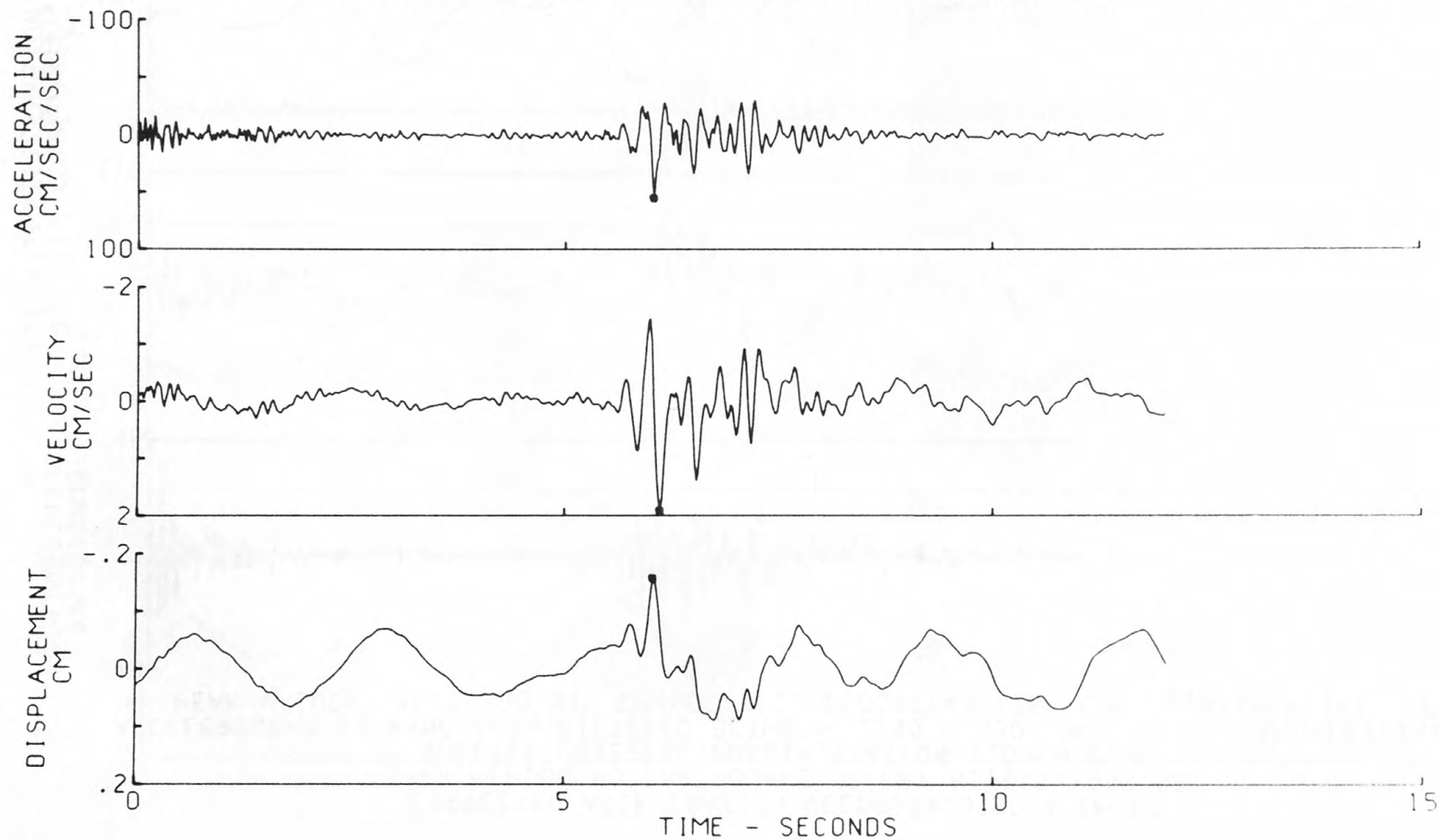


CORRECTED ACCELERATION, VELOCITY, DISPLACEMENT
 CAPE MENDOCINO EARTHQUAKE OF MAY 6, 1975, 1935 PST
 SHELTER COVE, CALIFORNIA, STATION 1, DOWN COMP

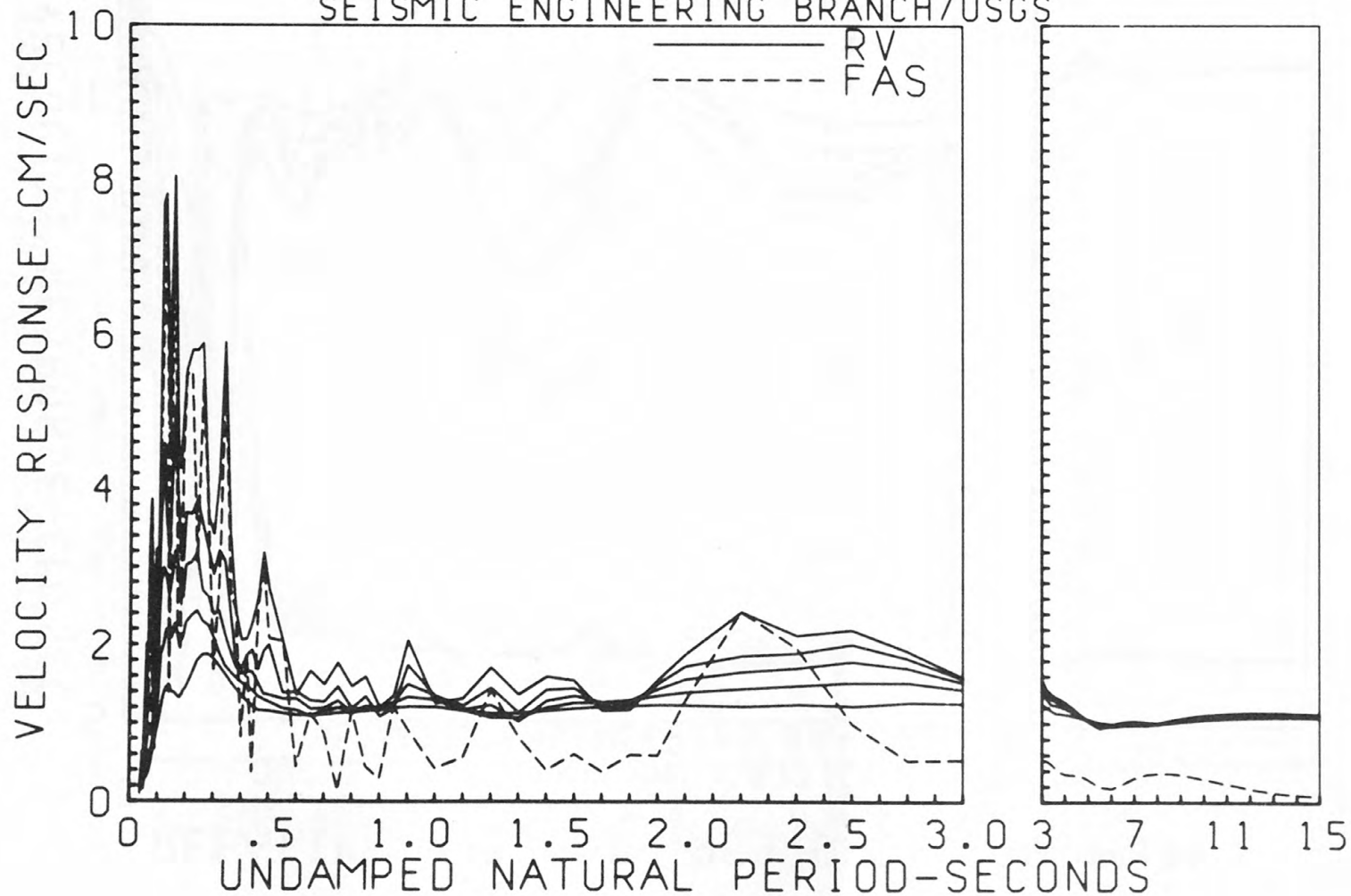
ACCELEROGRAM IS BAND PASS FILTERED BETWEEN .230 - .330 AND 25.00 - 27.00 CYC/SEC
 • PEAK VALUES ACCEL=30.51 CM/SEC/SEC, VELOCITY=-.776 CM/SEC, DISPL=.117 CM



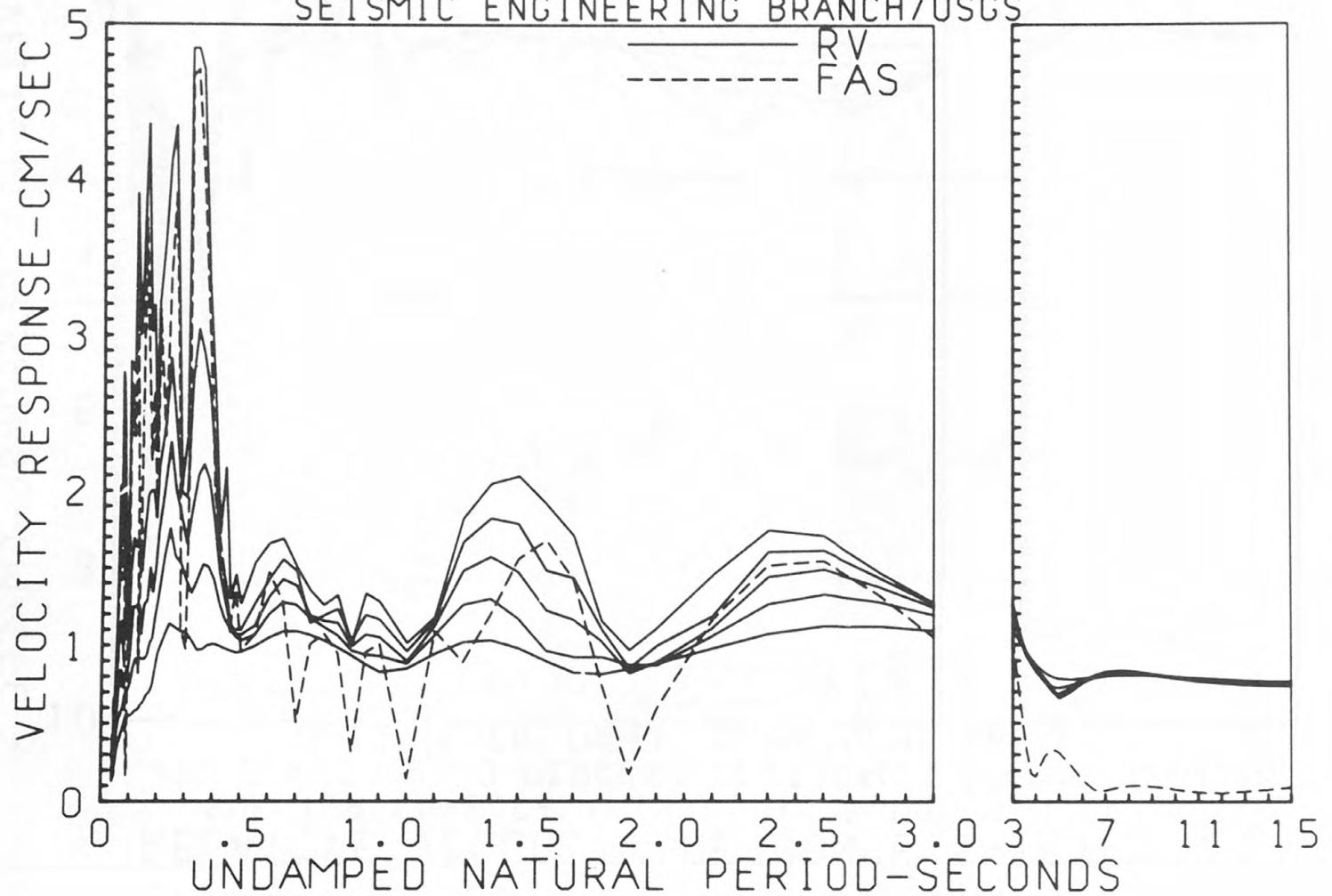
CORRECTED ACCELERATION, VELOCITY, DISPLACEMENT
 CAPE MENDOCINO EARTHQUAKE OF MAY 6, 1975, 1935PST
 SHELTER COVE, CALIFORNIA, STATION 1.520W COMP
 ACCELEROGRAM IS BAND PASS FILTERED BETWEEN .230 - .330 AND 25.00 - 27.00 CYC/SEC
 • PEAK VALUES ACCEL=54.88 CM/SEC/SEC, VELOCITY=1.925 CM/SEC, DISPL=-.157 CM



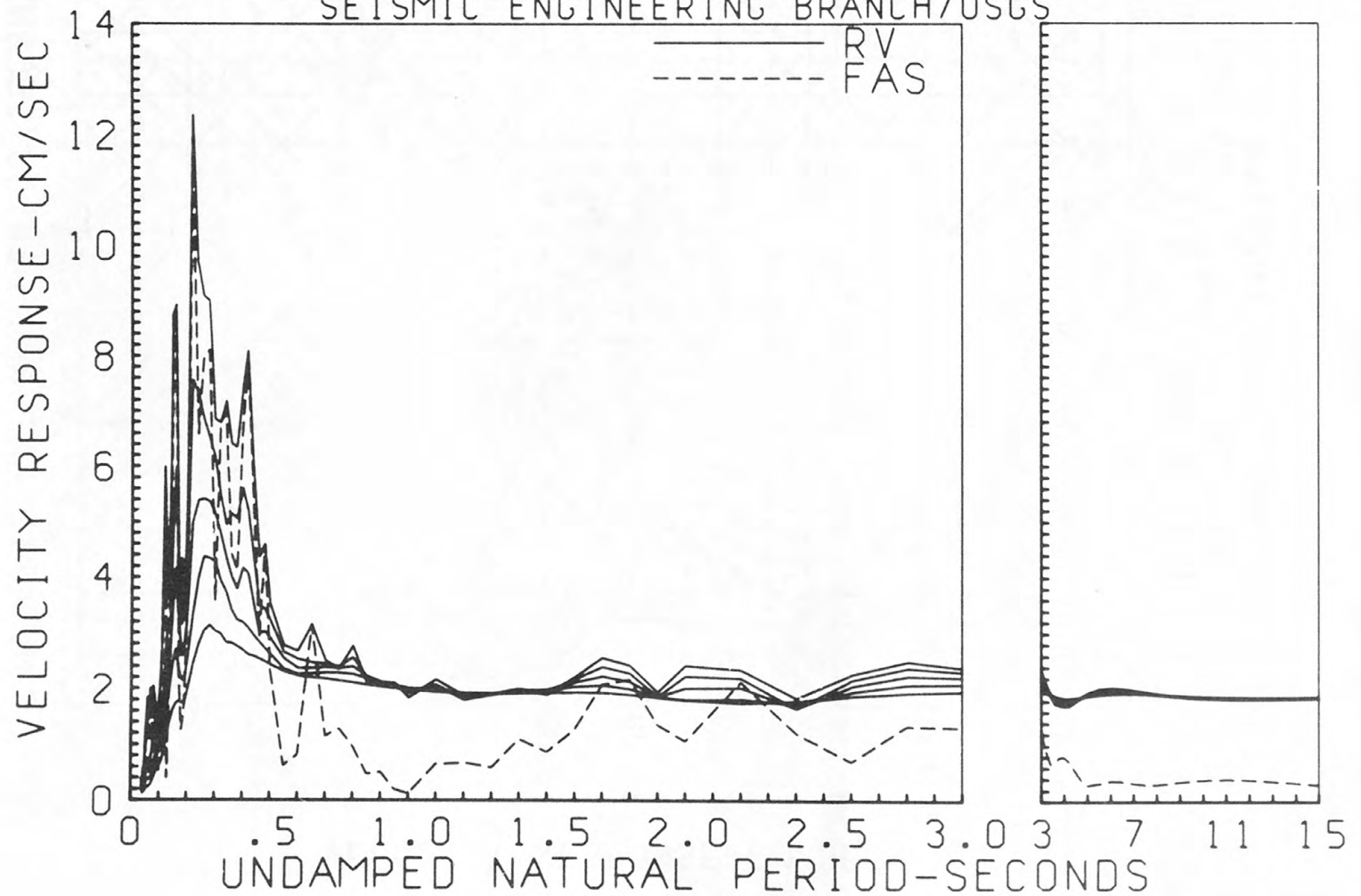
RELATIVE VELOCITY RESPONSE SPECTRUM
 SHELTER COVE, CALIF., S.C. 1.05/06/75, N70W
 0.2, 5, 10, 20 PERCENT CRITICAL DAMPING
 SEISMIC ENGINEERING BRANCH/USGS



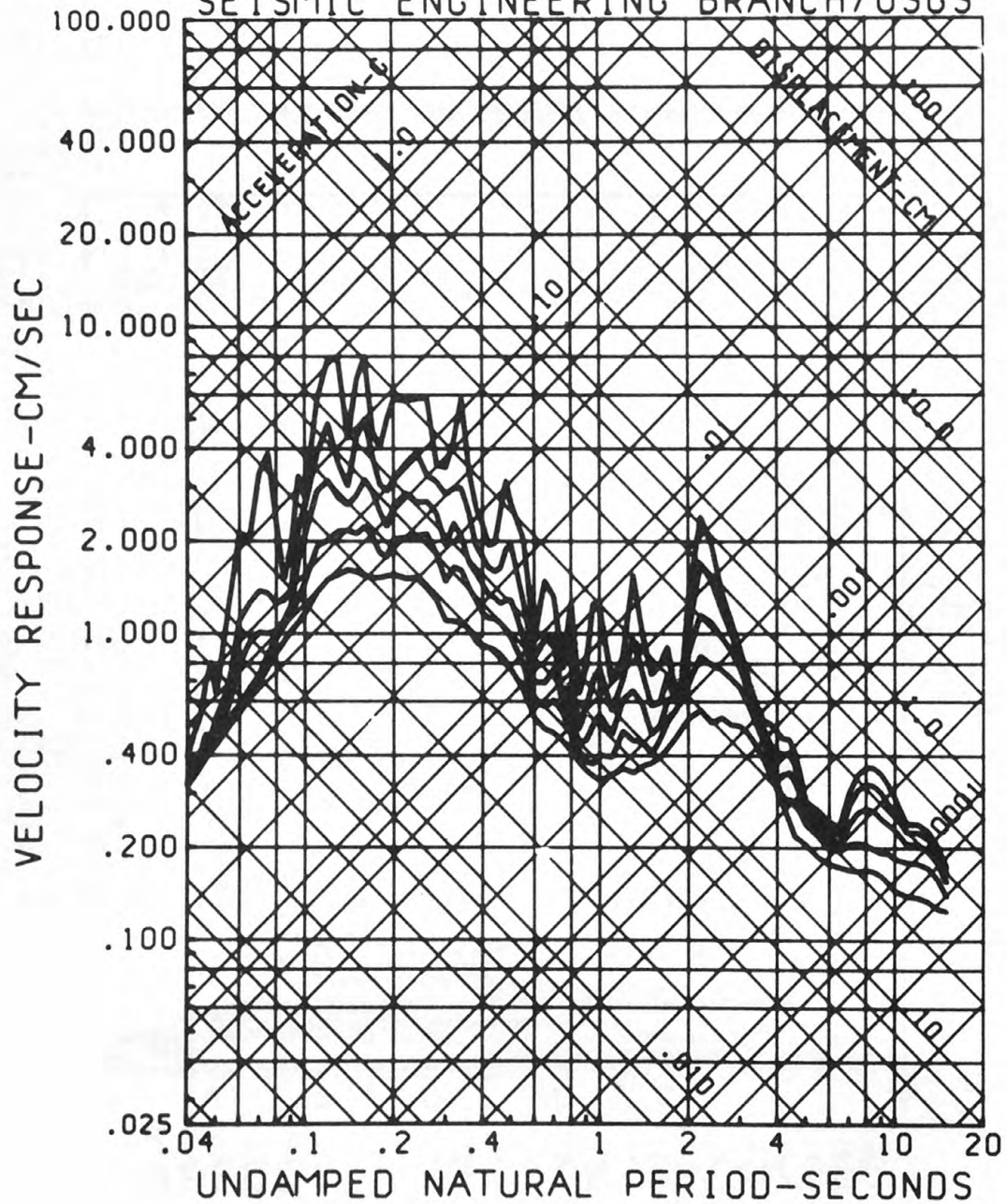
RELATIVE VELOCITY RESPONSE SPECTRUM
 SHELTER COVE, CALIF, S.C. 1,05/06/75, DOWN
 0,2,5,10,20 PERCENT CRITICAL DAMPING
 SEISMIC ENGINEERING BRANCH/USGS

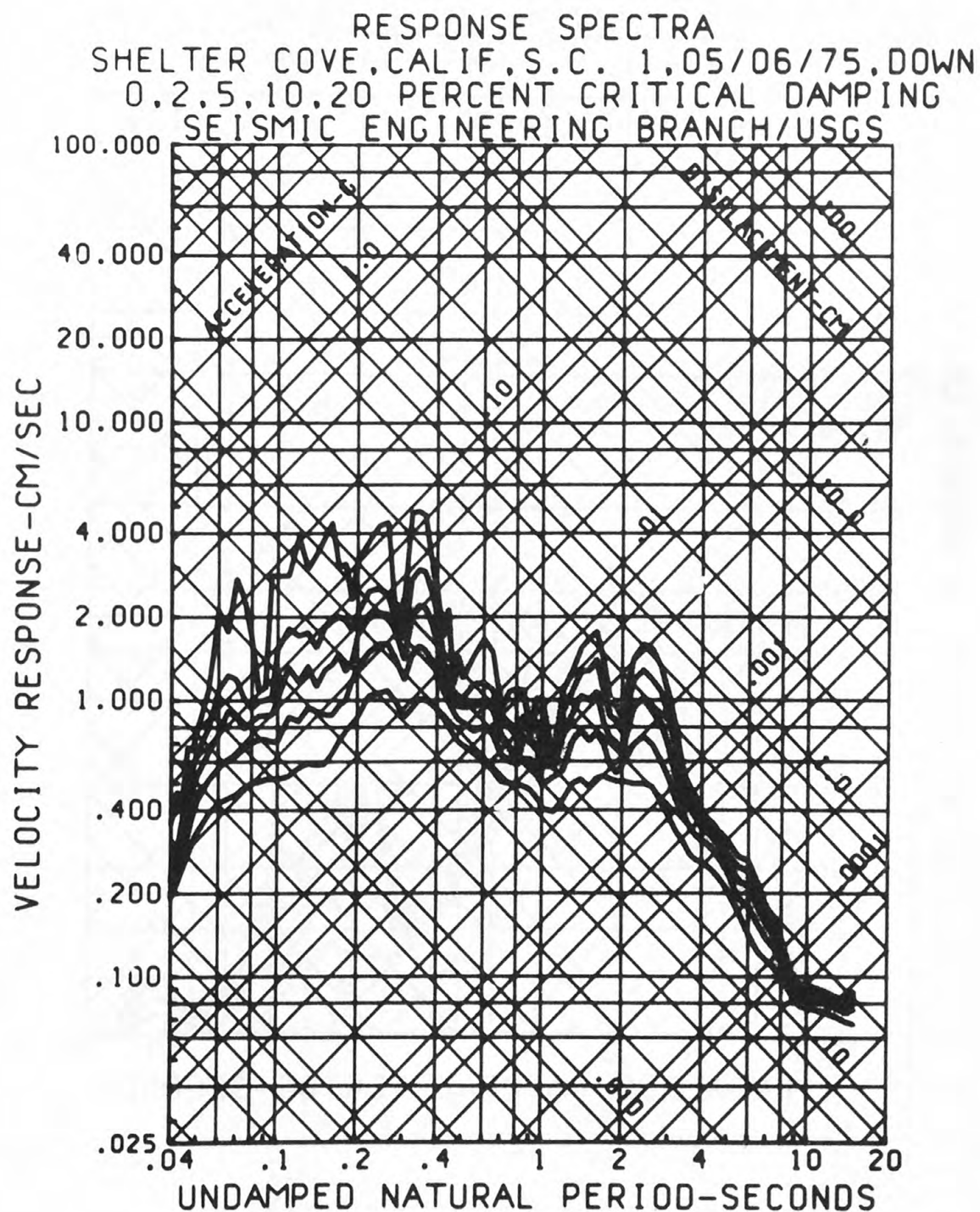


RELATIVE VELOCITY RESPONSE SPECTRUM
 SHELTER COVE, CALIF, S.C. 1,05/06/75, S20W
 0.2, 5, 10, 20 PERCENT CRITICAL DAMPING
 SEISMIC ENGINEERING BRANCH/USGS

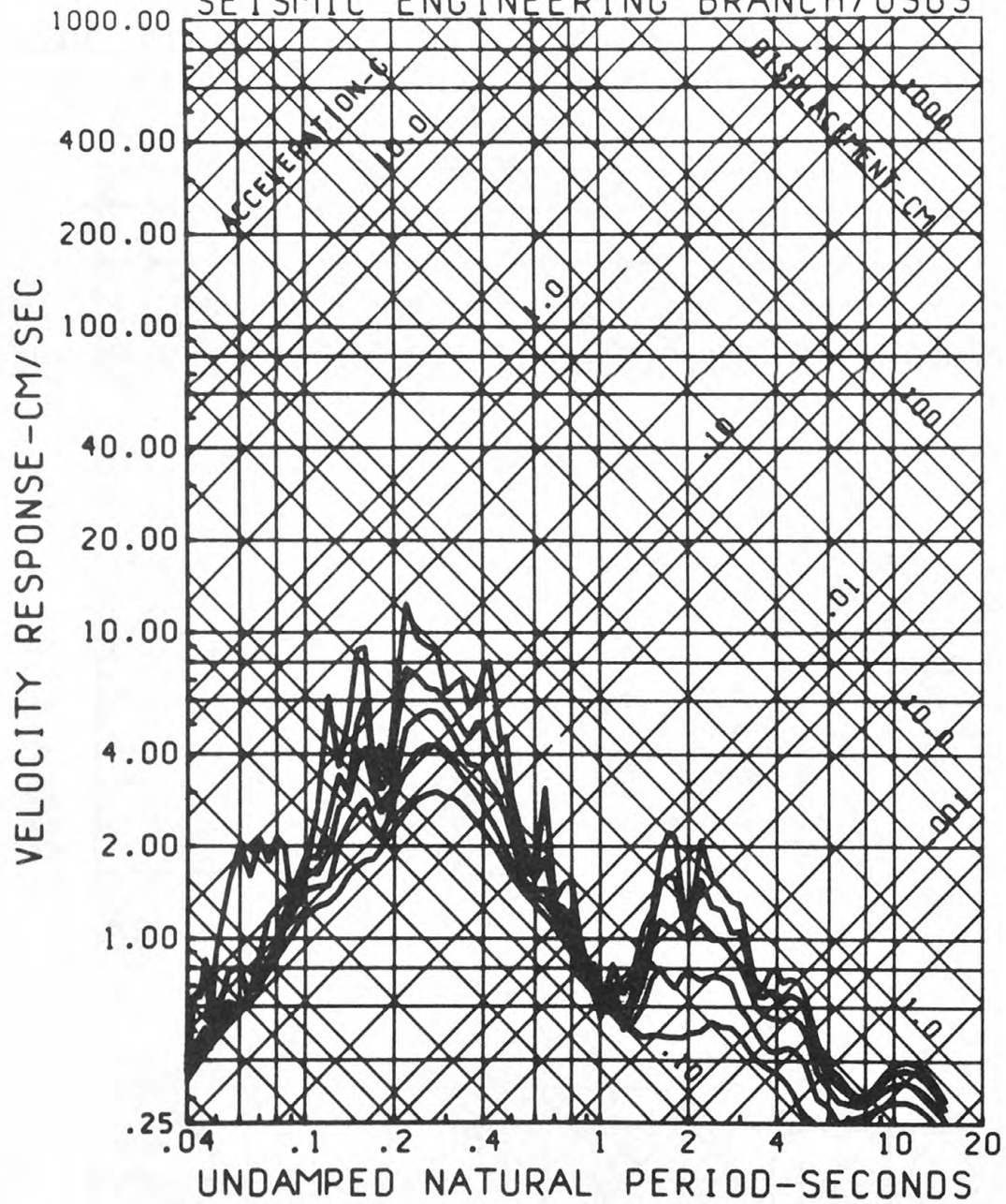


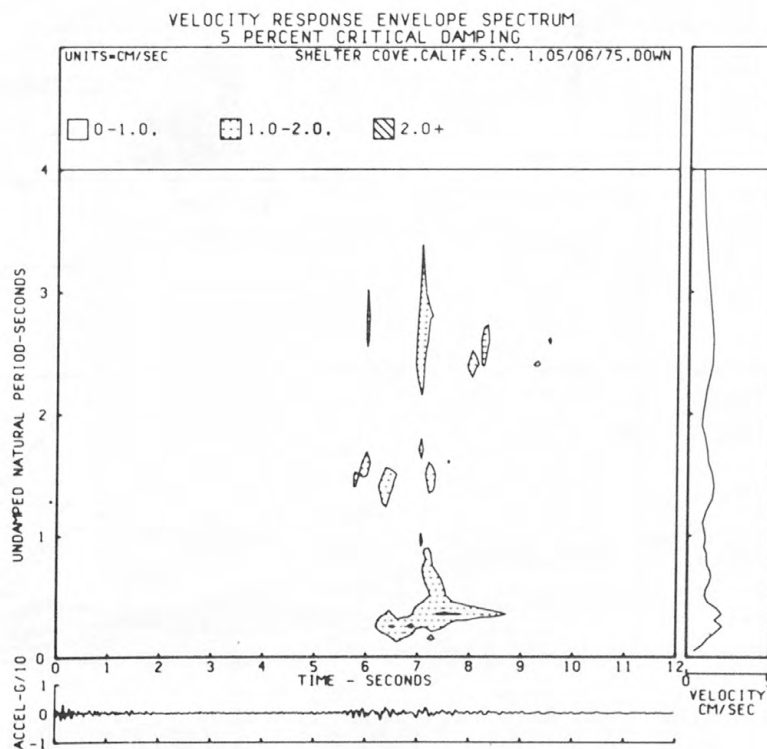
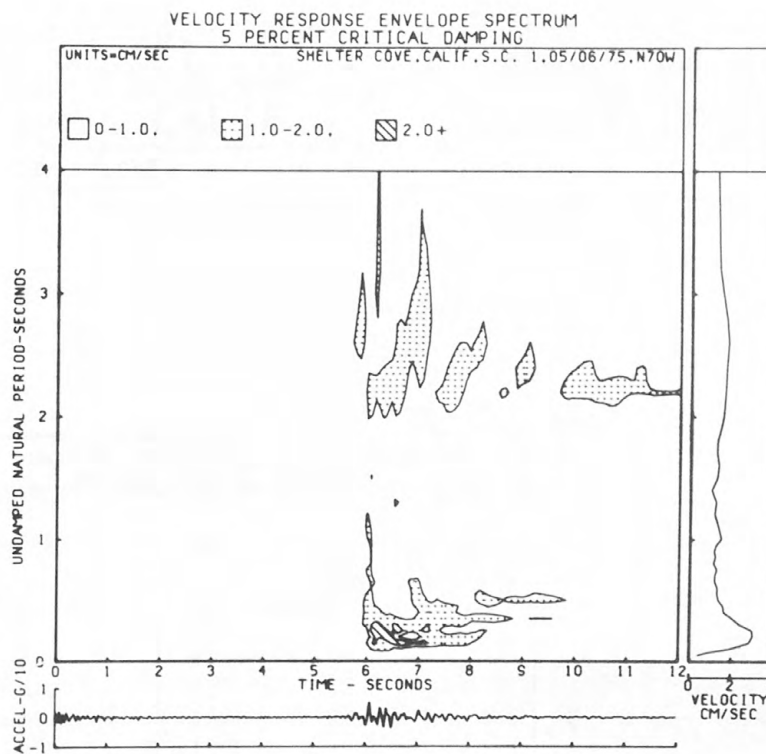
RESPONSE SPECTRA
 SHELTER COVE, CALIF, S.C. 1,05/06/75, N70W
 0, 2, 5, 10, 20 PERCENT CRITICAL DAMPING
 SEISMIC ENGINEERING BRANCH/USGS

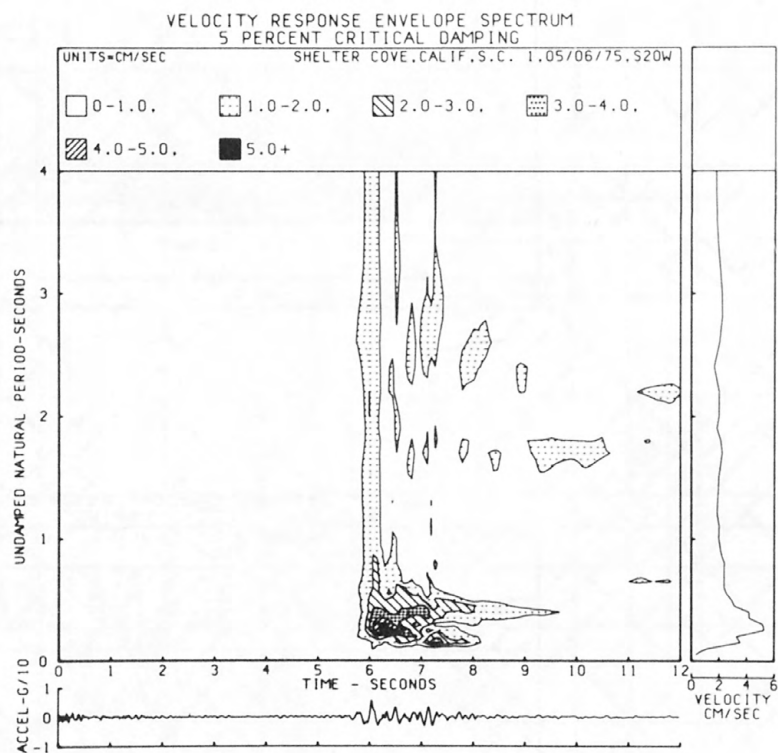




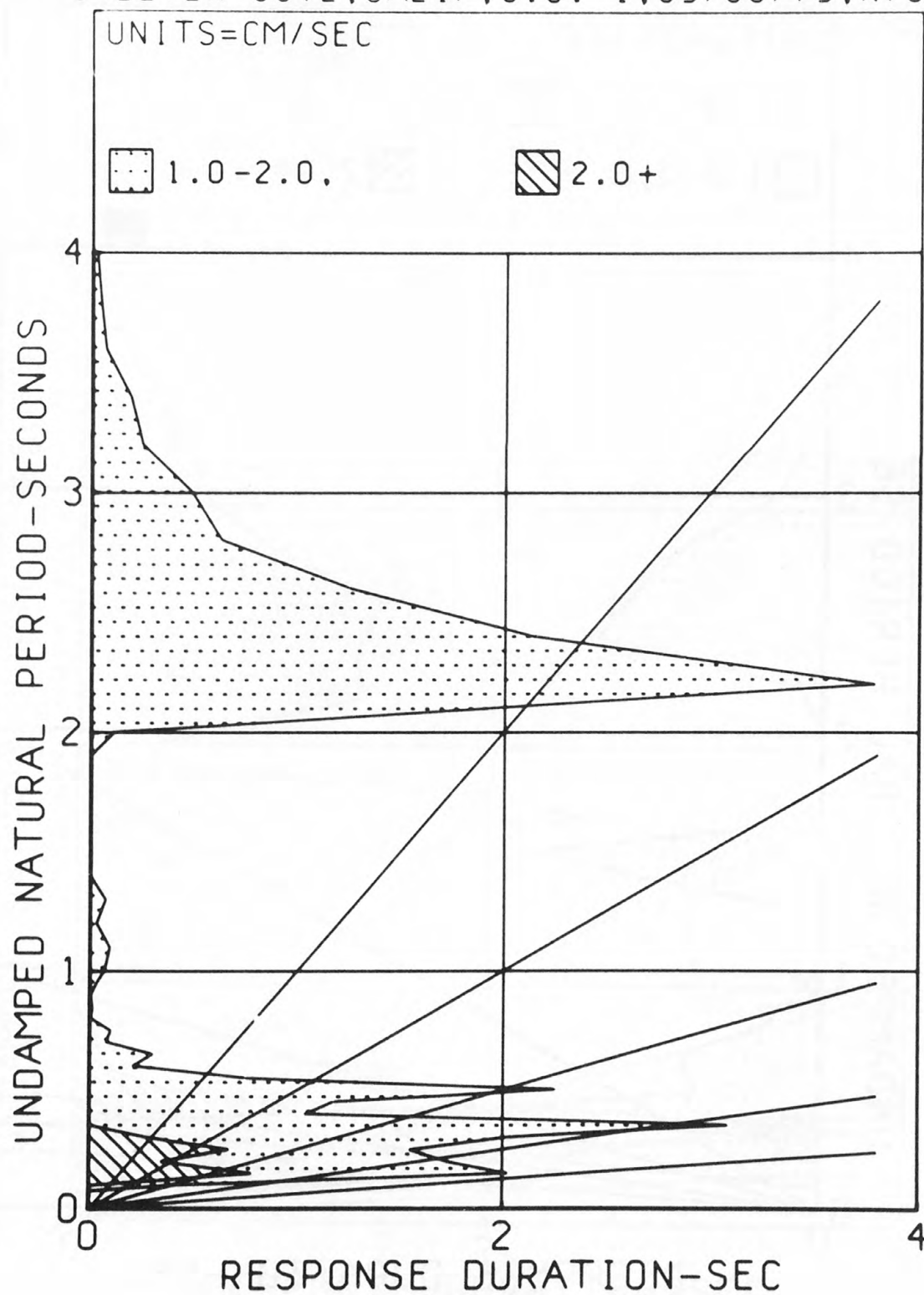
RESPONSE SPECTRA
 SHELTER COVE, CALIF. S.C. 1,05/06/75, S20W
 0.2, 5, 10, 20 PERCENT CRITICAL DAMPING
 SEISMIC ENGINEERING BRANCH/USGS



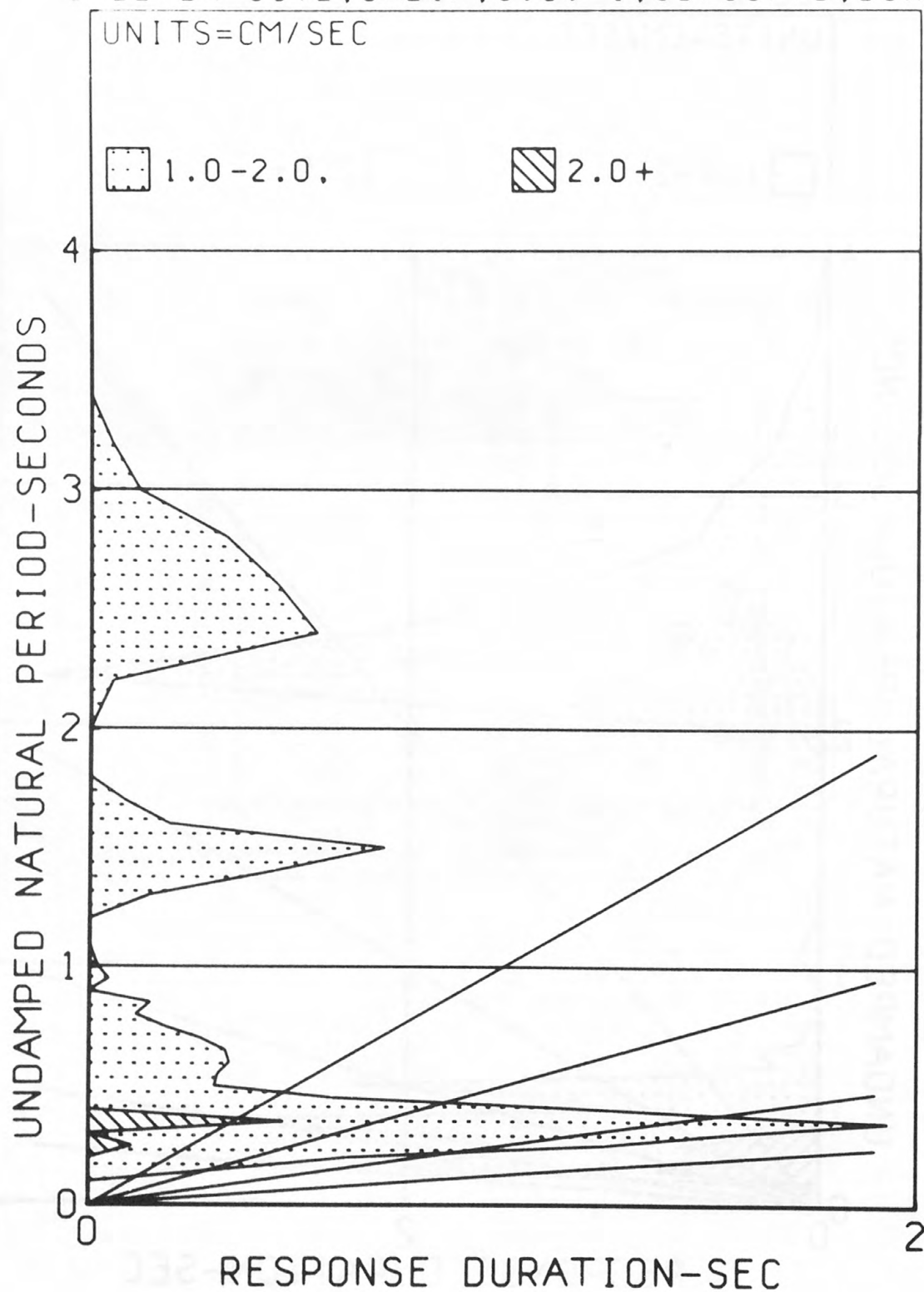




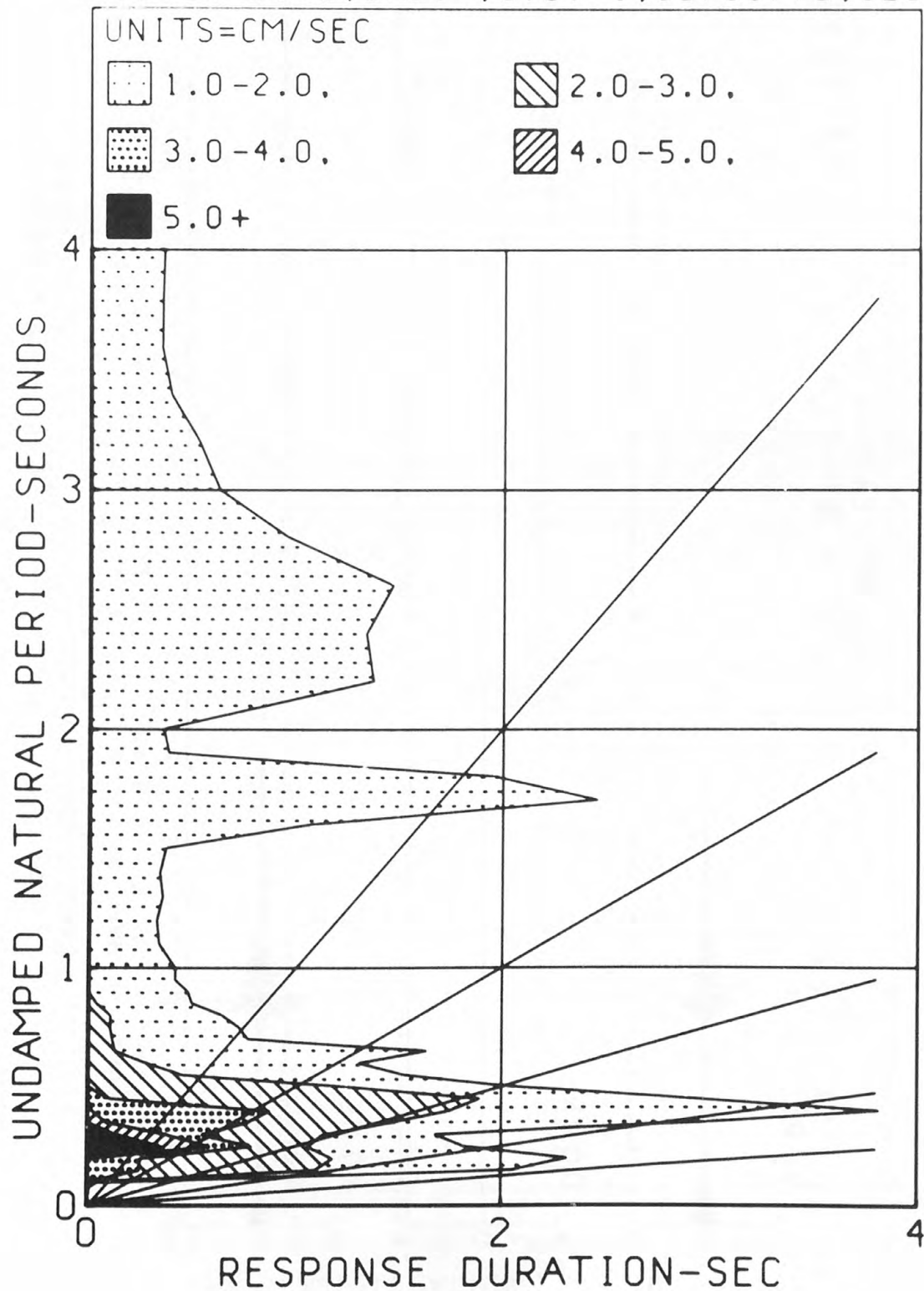
DURATION SPECTRUM OF THE VELOCITY
 RESPONSE ENVELOPE, 5 PERCENT DAMPING
 SHELTER COVE, CALIF, S.C. 1,05/06/75, N70W

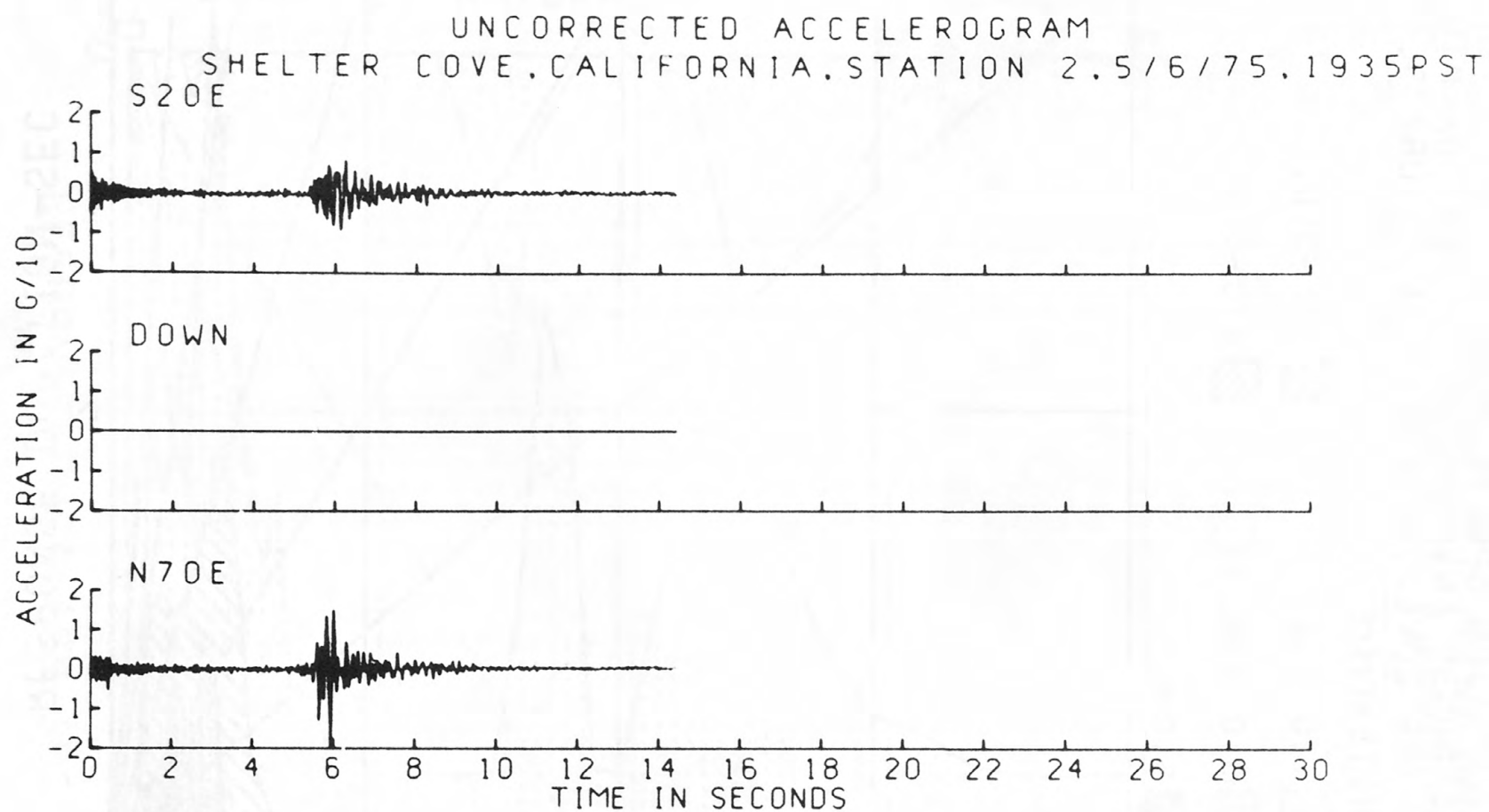


VIBRATION SPECTRUM OF THE VELOCITY
 RESPONSE ENVELOPE, 5 PERCENT DAMPING
 SHELTER COVE, CALIF, S.C. 1,05/06/75, DOWN



VIBRATION SPECTRUM OF THE VELOCITY
 RESPONSE ENVELOPE, 5 PERCENT DAMPING
 SHELTER COVE, CALIF, S.C. 1,05/06/75, S20W

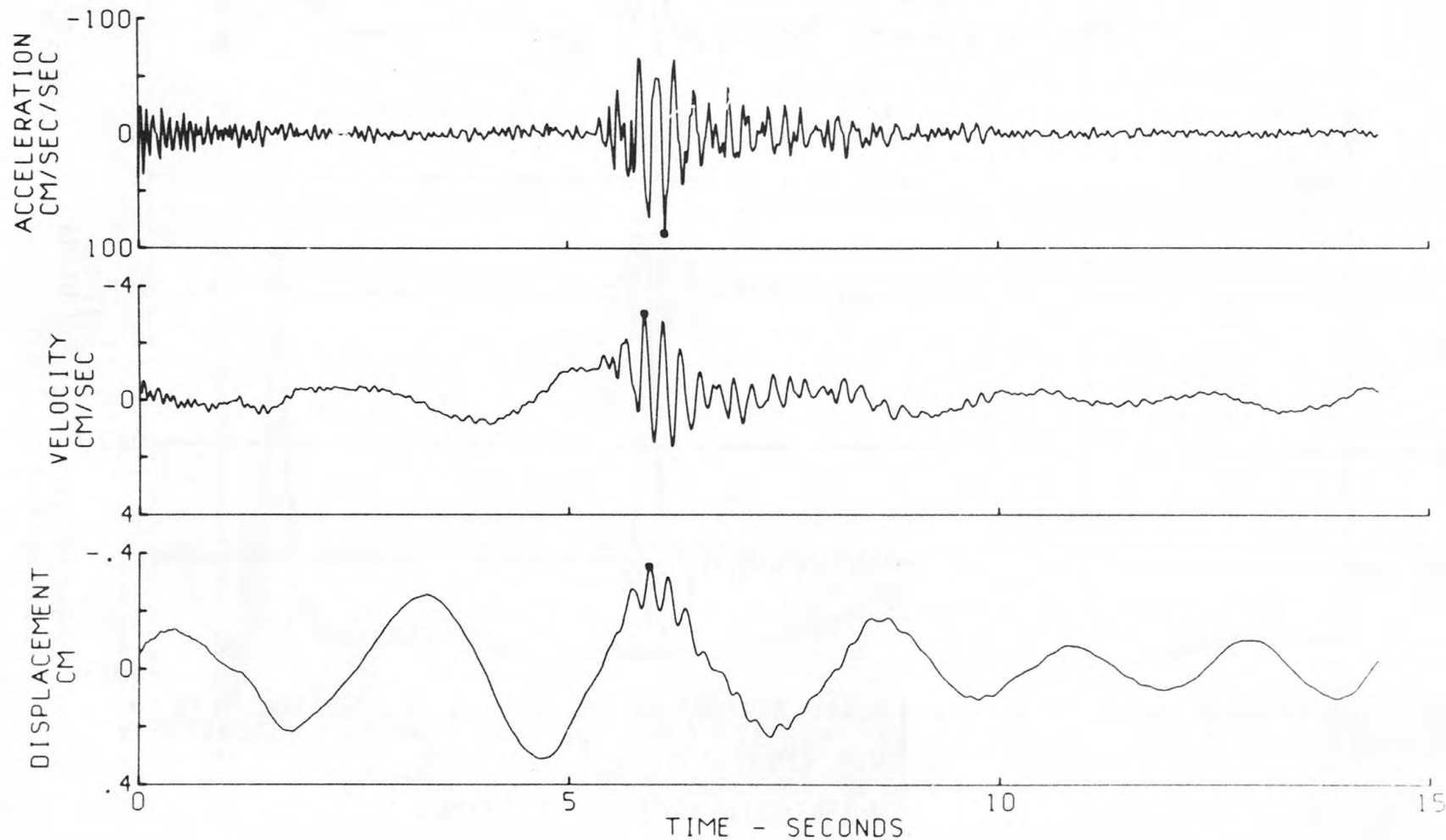




CORRECTED ACCELERATION, VELOCITY, DISPLACEMENT
CAPE MENDOCINO EARTHQUAKE OF MAY 6, 1975, 1935PST
SHELTER COVE, CALIFORNIA, STATION 2, S20E COMP

ACCELEROGRAM IS BAND PASS FILTERED BETWEEN .230 - .330 AND 25.00 - 27.00 CYC/SEC

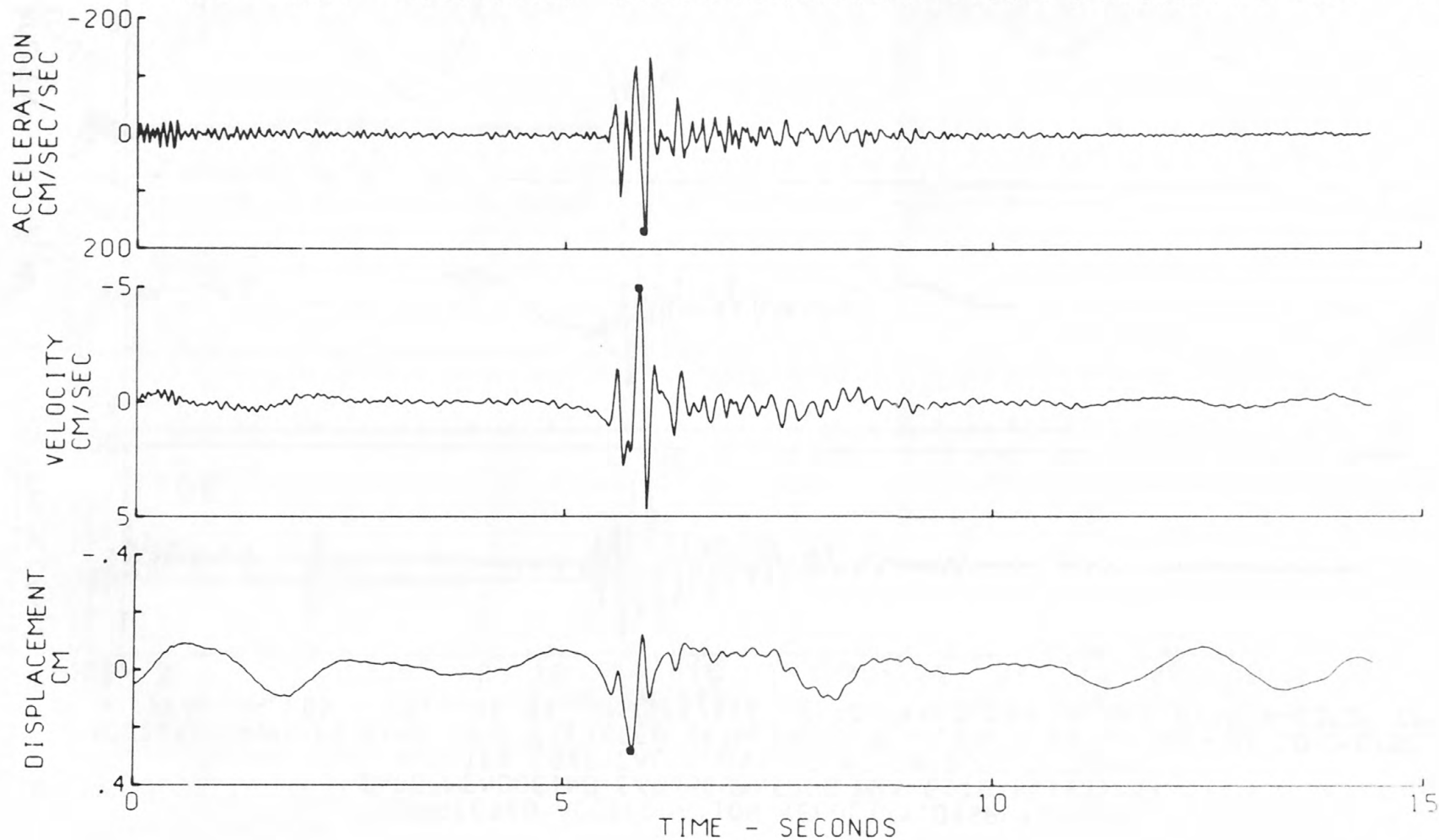
• PEAK VALUES ACCEL=86.84 CM/SEC/SEC, VELOCITY=-3.044 CM/SEC, DISPL=-.352 CM



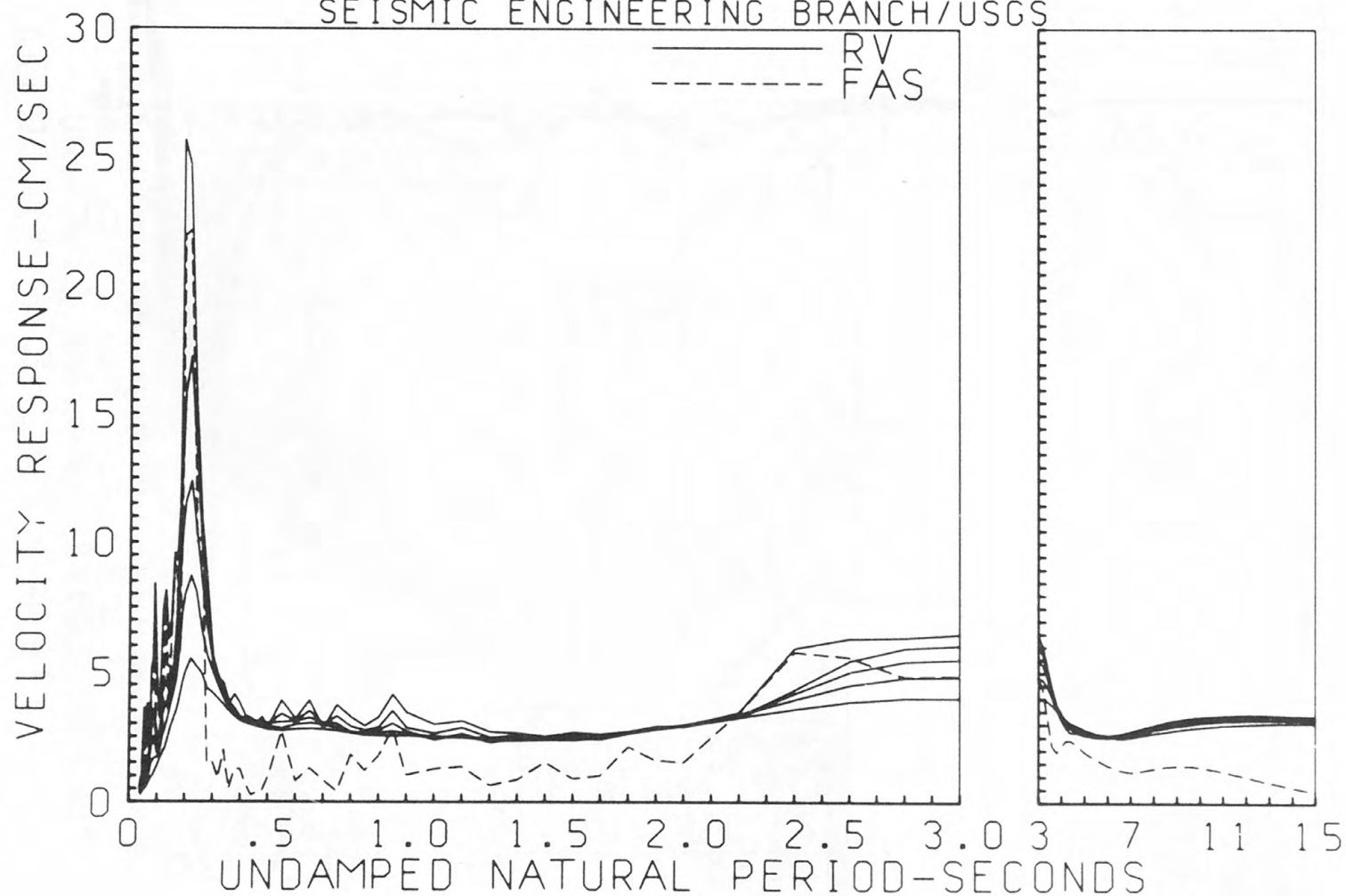
CORRECTED ACCELERATION, VELOCITY, DISPLACEMENT
CAPE MENDOCINO EARTHQUAKE OF MAY 6, 1975, 1935 PST
SHELTER COVE, CALIFORNIA, STATION 2, N70E COMP

ACCELEROGRAM IS BAND PASS FILTERED BETWEEN .230 - .330 AND 25.00 - 27.00 CYC/SEC

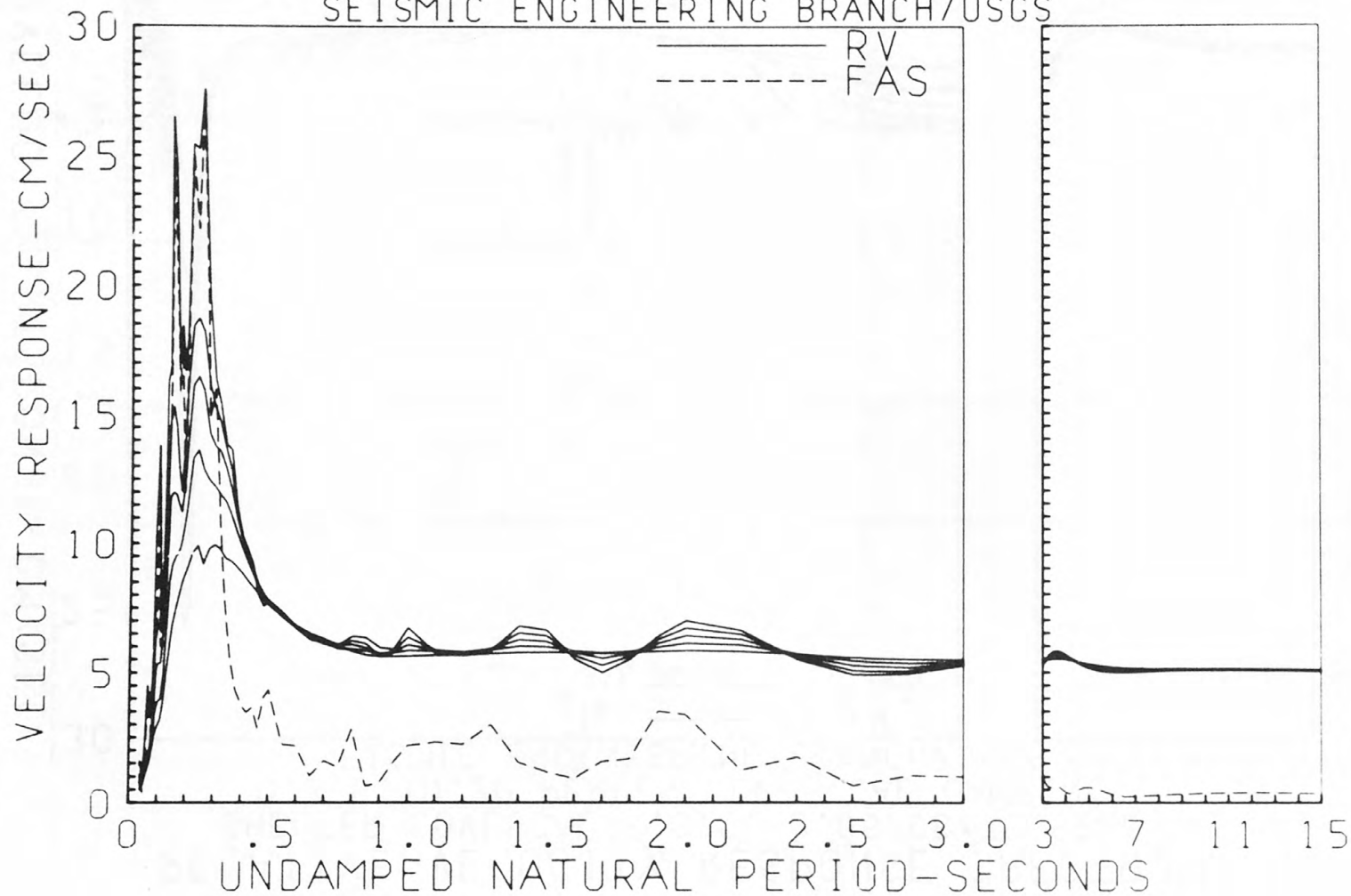
• PEAK VALUES ACCEL=168.3 CM/SEC/SEC, VELOCITY=-4.972 CM/SEC, DISPL=.283 CM



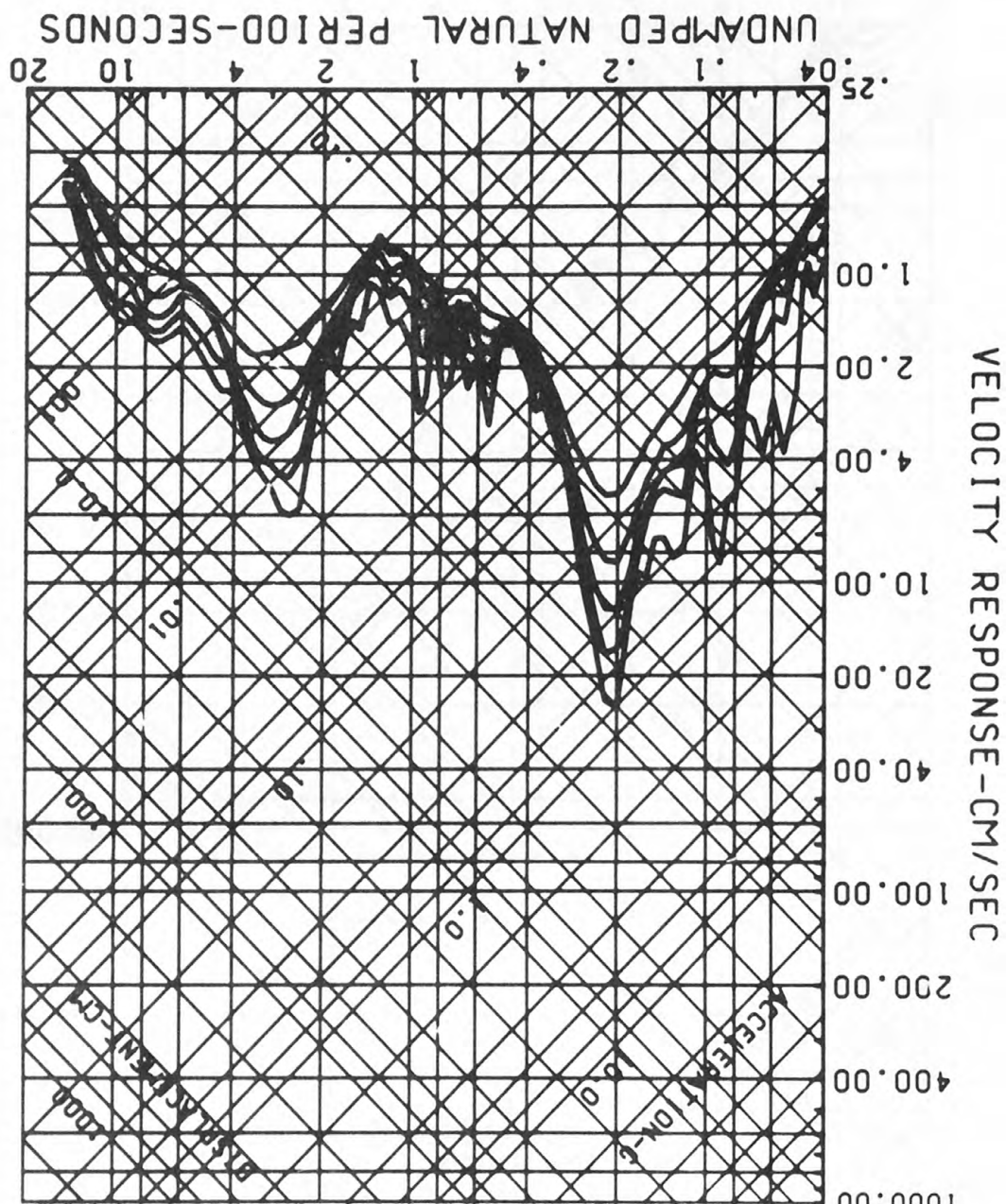
RELATIVE VELOCITY RESPONSE SPECTRUM
 SHELTER COVE, CALIF, S.C. 2,05/06/75, S20E
 0.2, 5, 10, 20 PERCENT CRITICAL DAMPING
 SEISMIC ENGINEERING BRANCH/USGS

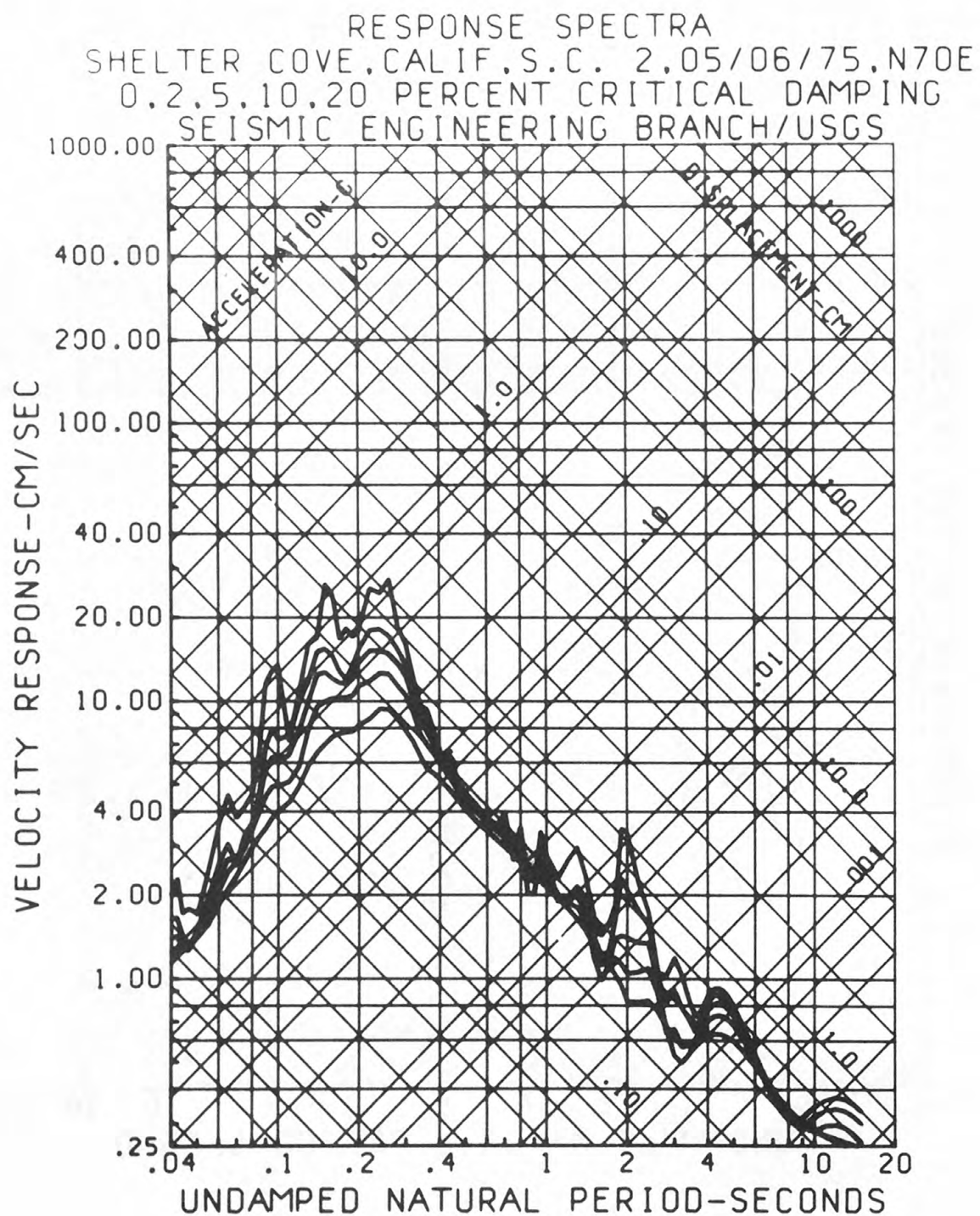


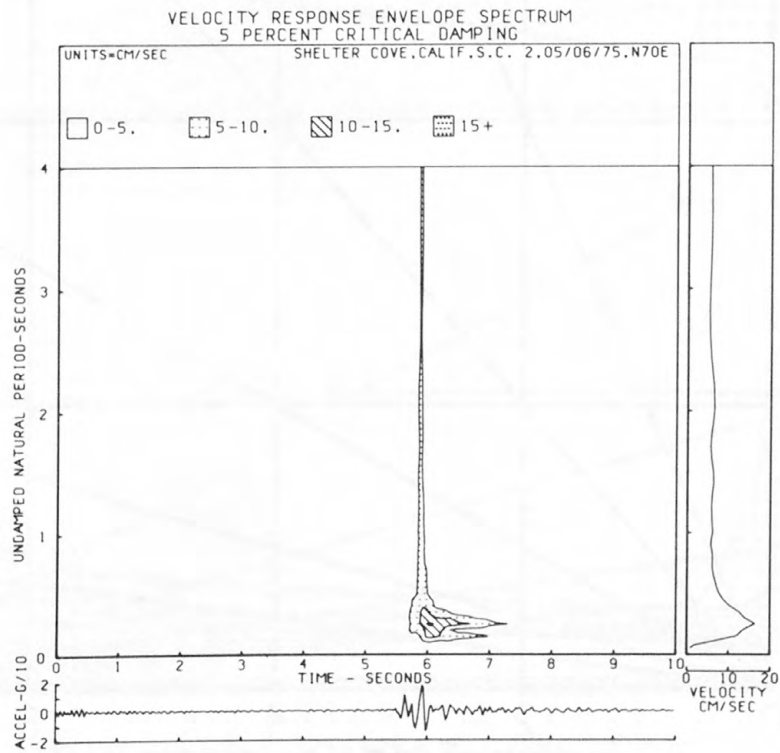
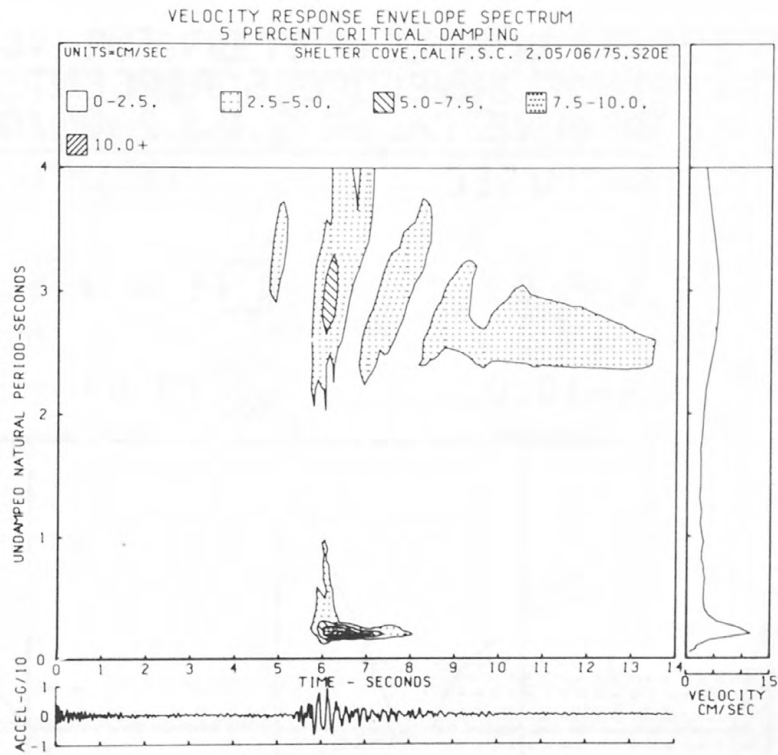
RELATIVE VELOCITY RESPONSE SPECTRUM
SHELTER COVE, CALIF, S.C. 2,05/06/75, N70E
0,2,5,10,20 PERCENT CRITICAL DAMPING
SEISMIC ENGINEERING BRANCH/USGS



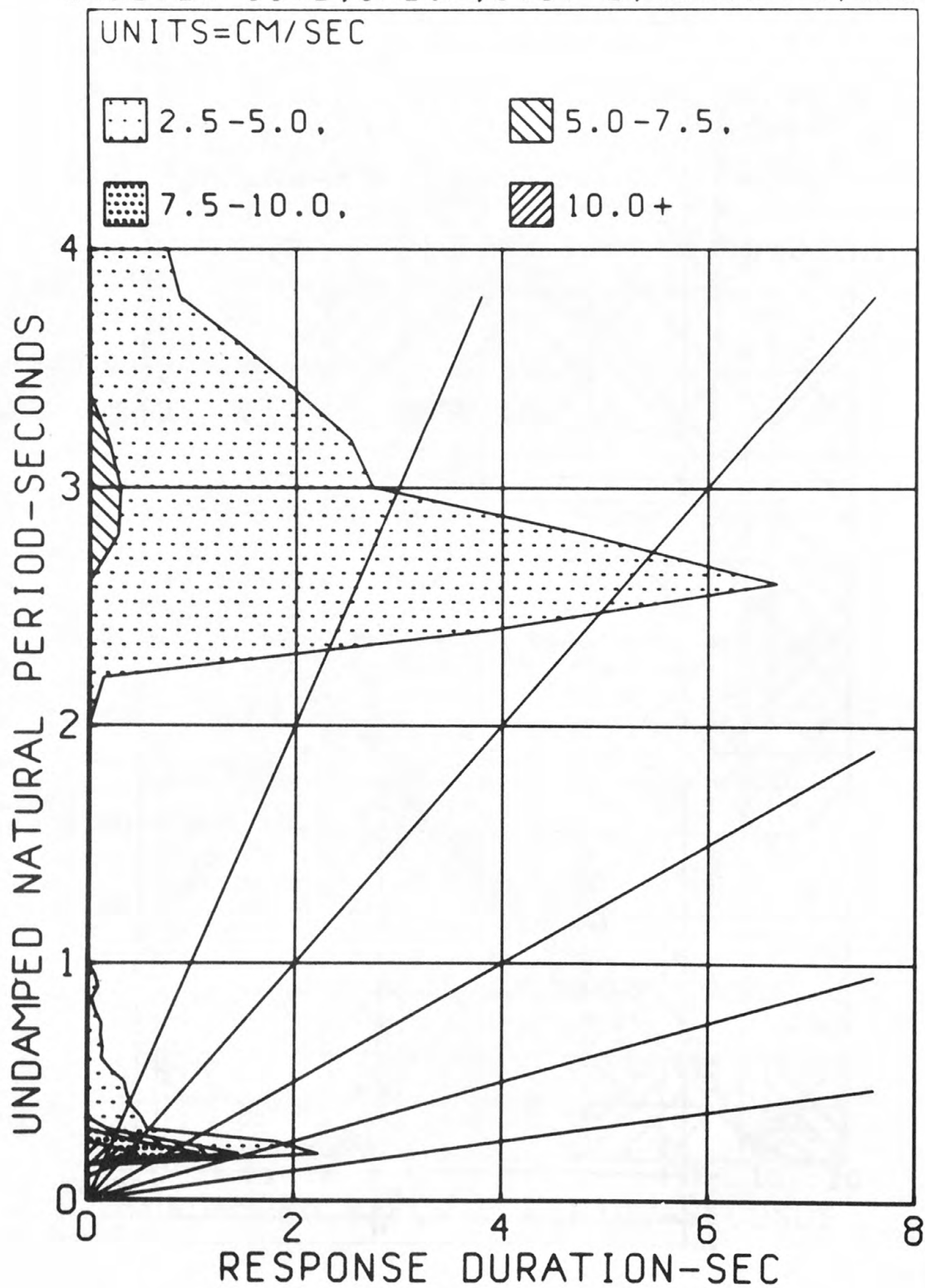
RESPONSE SPECTRA
 SHELTER COVE, CALIF., S.C. 2.05/06/75, S20E
 0.2, 5, 10, 20 PERCENT CRITICAL DAMPING
 SEISMIC ENGINEERING BRANCH/USGS



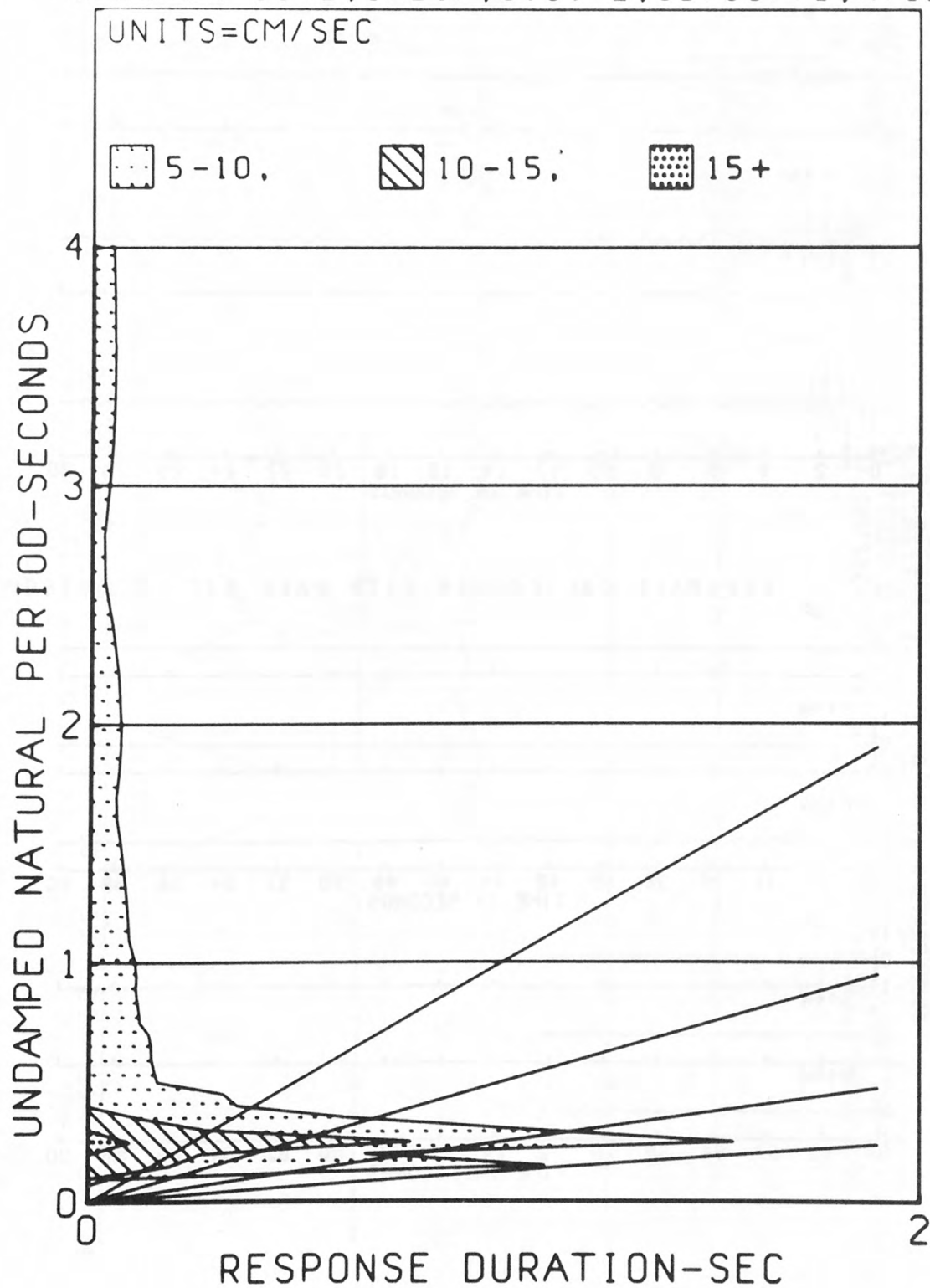




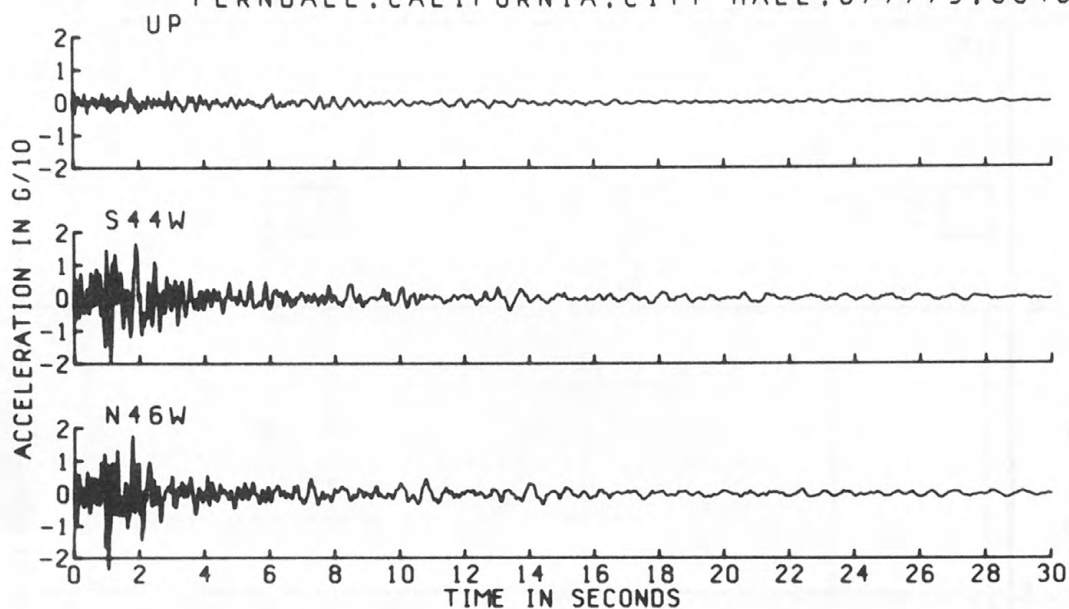
DURATION SPECTRUM OF THE VELOCITY
RESPONSE ENVELOPE, 5 PERCENT DAMPING
SHELTER COVE, CALIF, S.C. 2,05/06/75, S20E



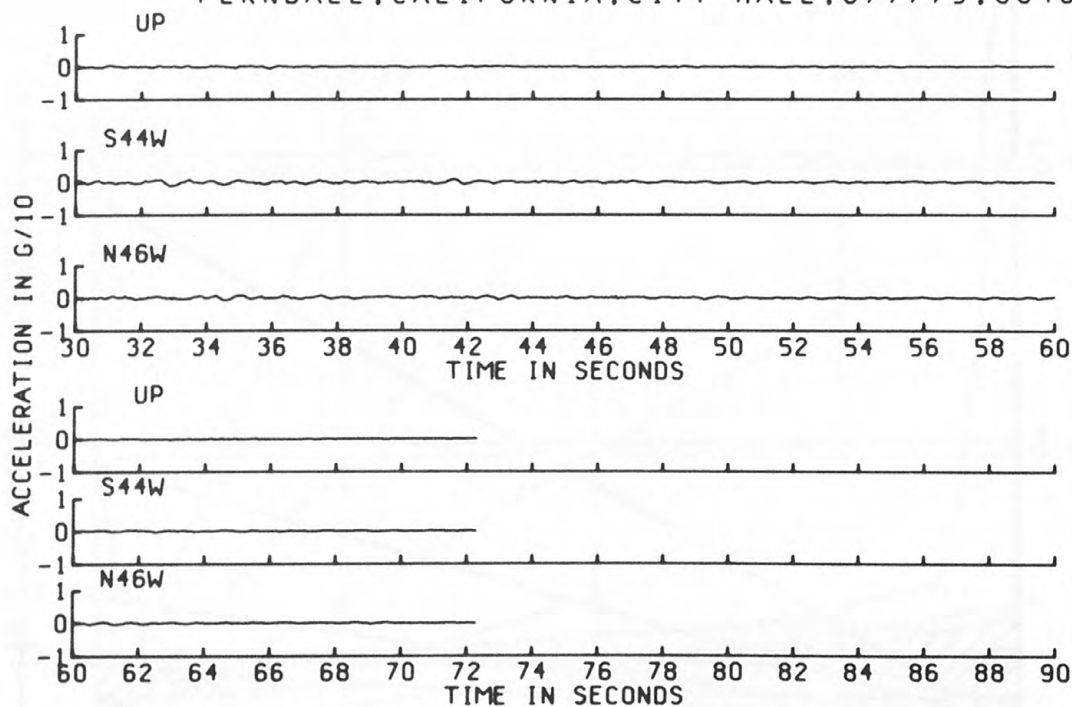
DURATION SPECTRUM OF THE VELOCITY
 RESPONSE ENVELOPE, 5 PERCENT DAMPING
 SHELTER COVE, CALIF, S.C. 2,05/06/75, N70E



UNCORRECTED ACCELEROGRAM
 FERNDAL, CALIFORNIA, CITY HALL, 6/7/75, 0846GMT



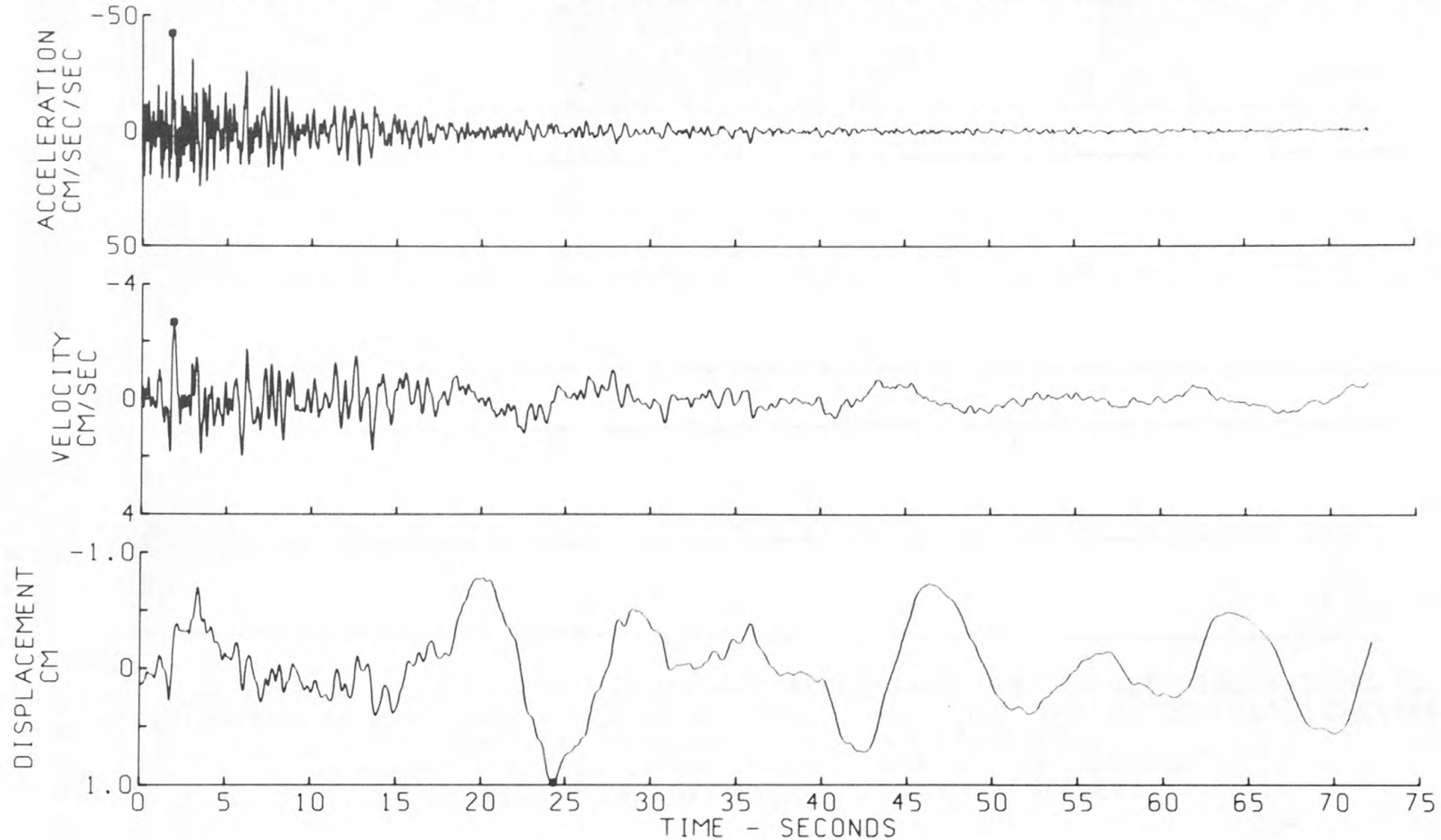
FERNDAL, CALIFORNIA, CITY HALL, 6/7/75, 0846GMT



CORRECTED ACCELERATION, VELOCITY, DISPLACEMENT
HUMBOLDT COUNTY EARTHQUAKE OF JUNE 7, 1975, 0846GMT
FERNDAL, CALIFORNIA, CITY HALL, UP COMP

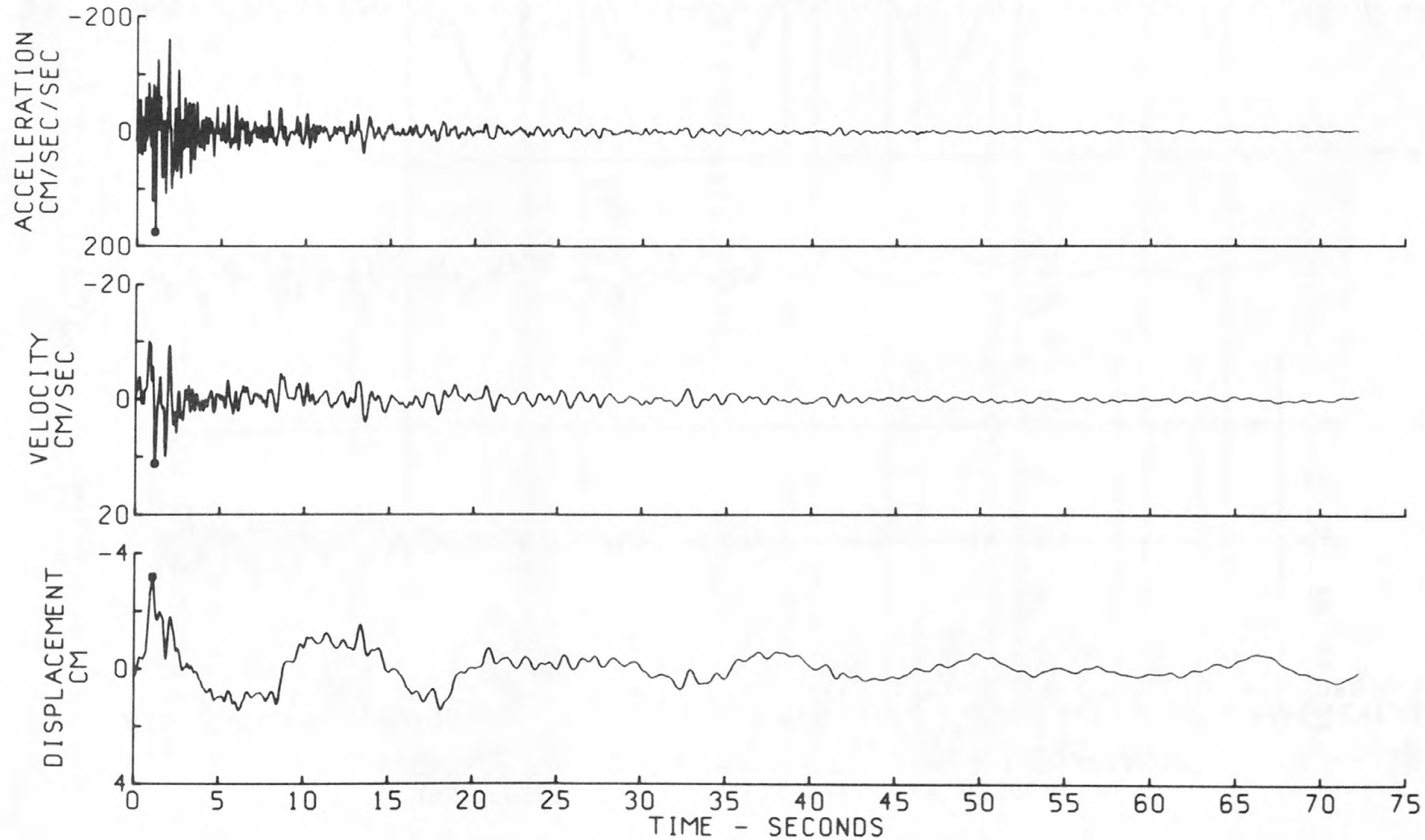
ACCELEROGRAM IS BAND PASS FILTERED BETWEEN .050 - .070 AND 25.00 - 27.00 CYC/SEC

• PEAK VALUES ACCEL=-42.37 CM/SEC/SEC, VELOCITY=-2.660 CM/SEC, DISPL=.980 CM

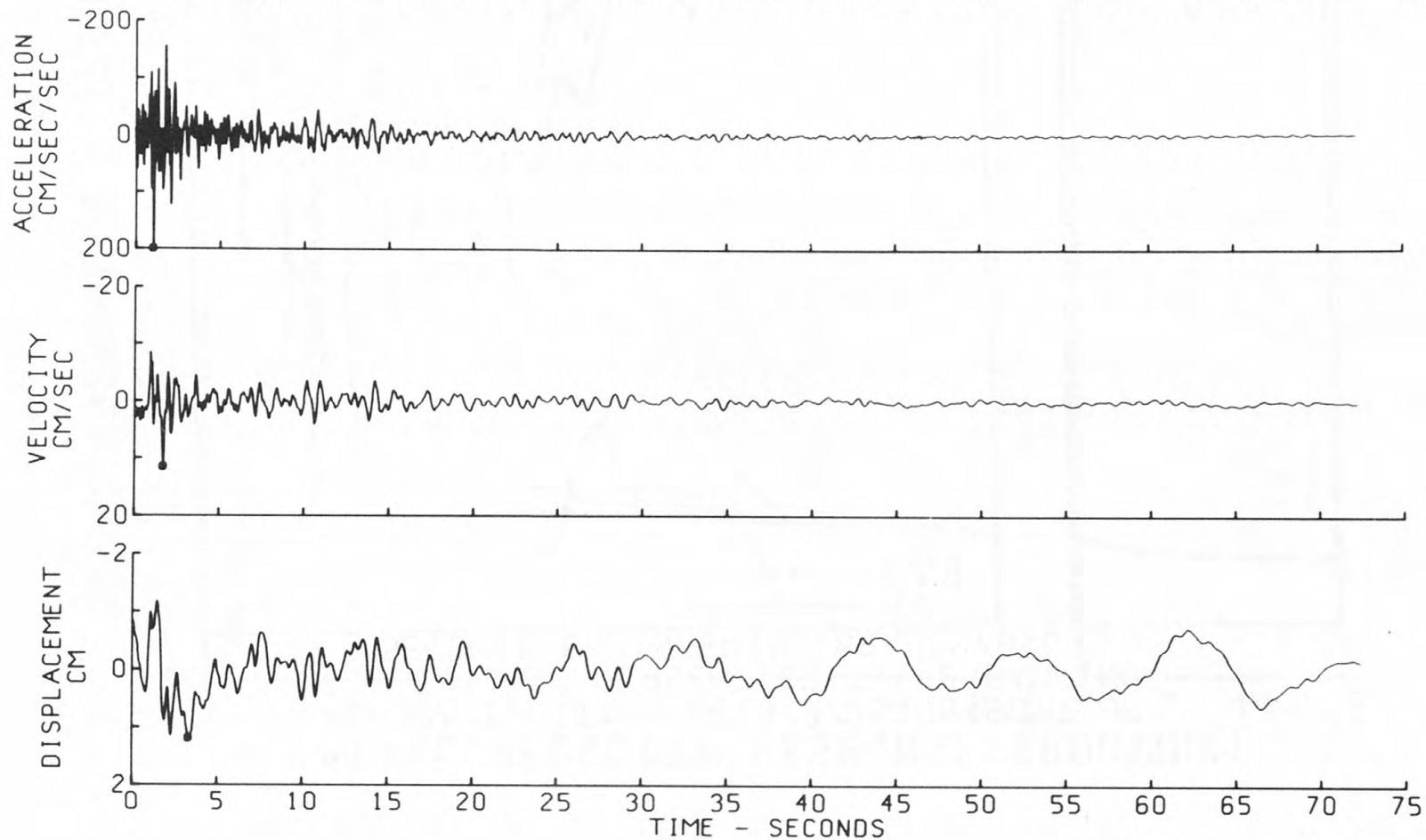


CORRECTED ACCELERATION, VELOCITY, DISPLACEMENT
HUMBOLDT COUNTY EARTHQUAKE OF JUNE 7, 1975, 0846GMT
FERNDAL, CALIFORNIA, CITY HALL, S44W COMP

ACCELEROGRAM IS BAND PASS FILTERED BETWEEN .050 - .070 AND 25.00 - 27.00 CYC/SEC
• PEAK VALUES ACCEL=174.8 CM/SEC/SEC, VELOCITY=11.19 CM/SEC, DISPL=-3.180 CM



CORRECTED ACCELERATION, VELOCITY, DISPLACEMENT
HUMBOLDT COUNTY EARTHQUAKE OF JUNE 7, 1975, 0846GMT
FERNDALE, CALIFORNIA, CITY HALL, N46W COMP
ACCELEROGRAM IS BAND PASS FILTERED BETWEEN .050 - .070 AND 25.00 - 27.00 CYC/SEC
• PEAK VALUES ACCEL=199.4 CM/SEC/SEC, VELOCITY=11.45 CM/SEC, DISPL=1.190 CM

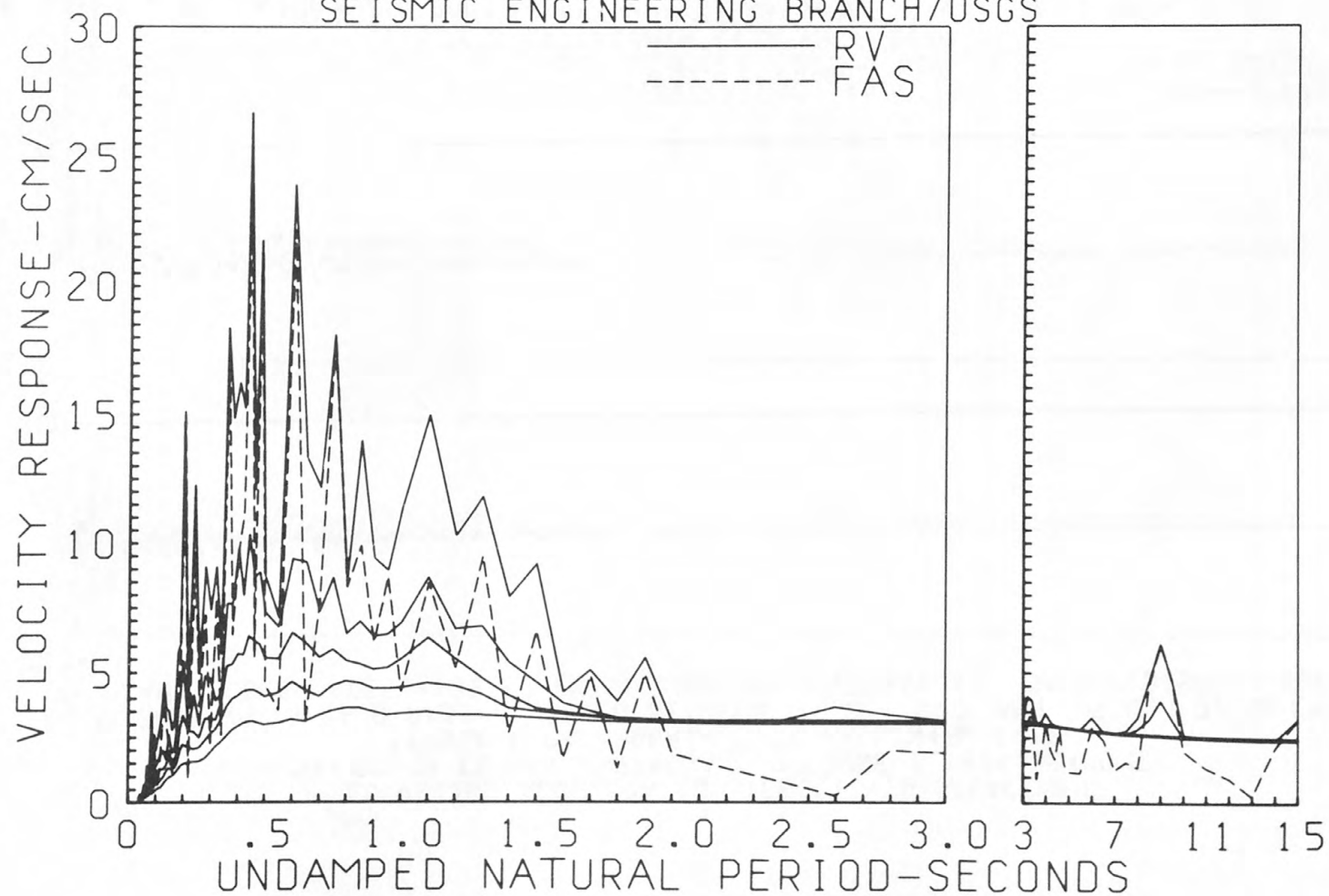


RELATIVE VELOCITY RESPONSE SPECTRUM

FERNDAL, CITY HALL, 6/7/75, 0846GMT, UP

0.2, 5, 10, 20 PERCENT CRITICAL DAMPING

SEISMIC ENGINEERING BRANCH/USGS

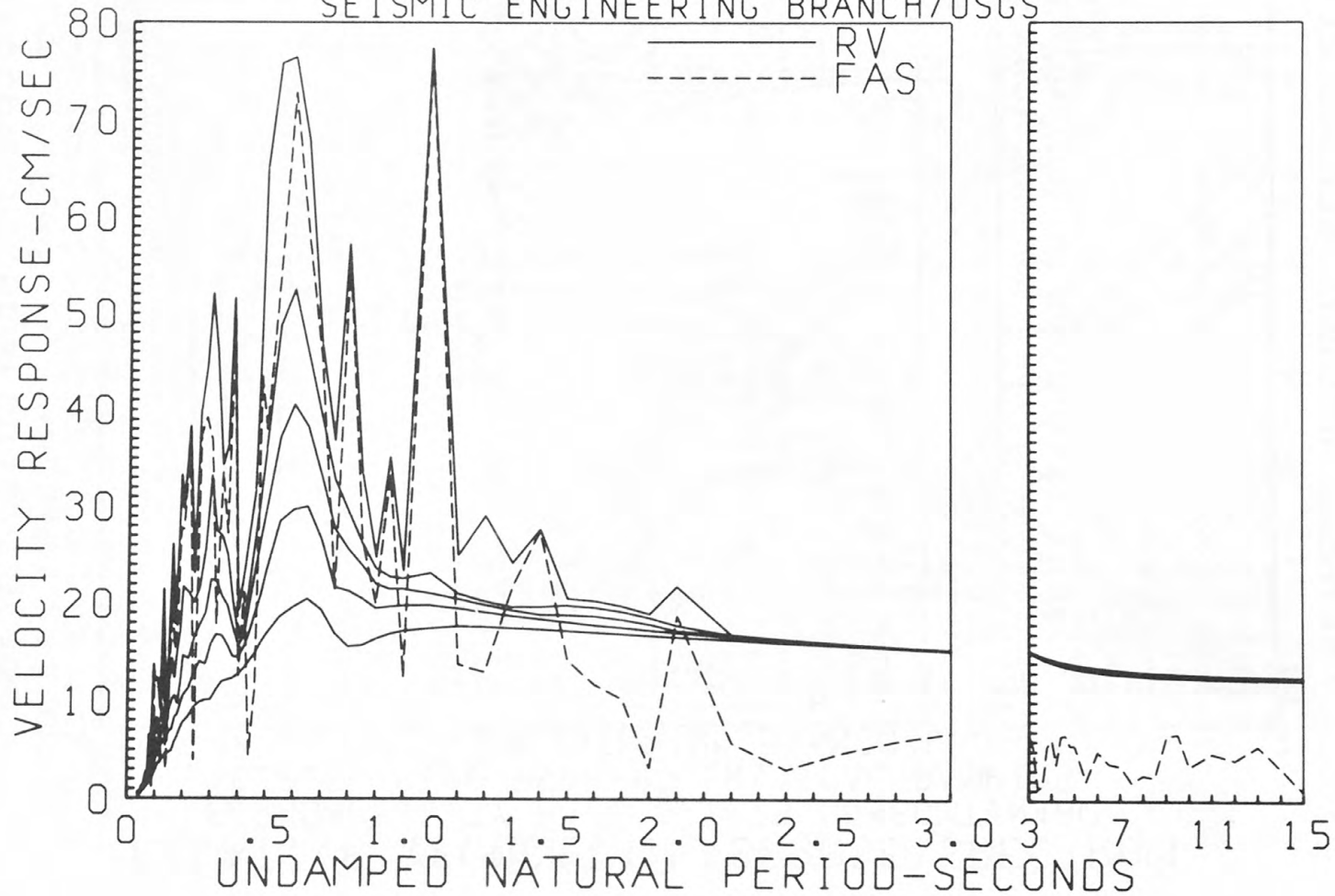


RELATIVE VELOCITY RESPONSE SPECTRUM

FERNDALDE.CITY HALL.6/7/75.0846GMT.S44W

0.2.5.10.20 PERCENT CRITICAL DAMPING

SEISMIC ENGINEERING BRANCH/USGS

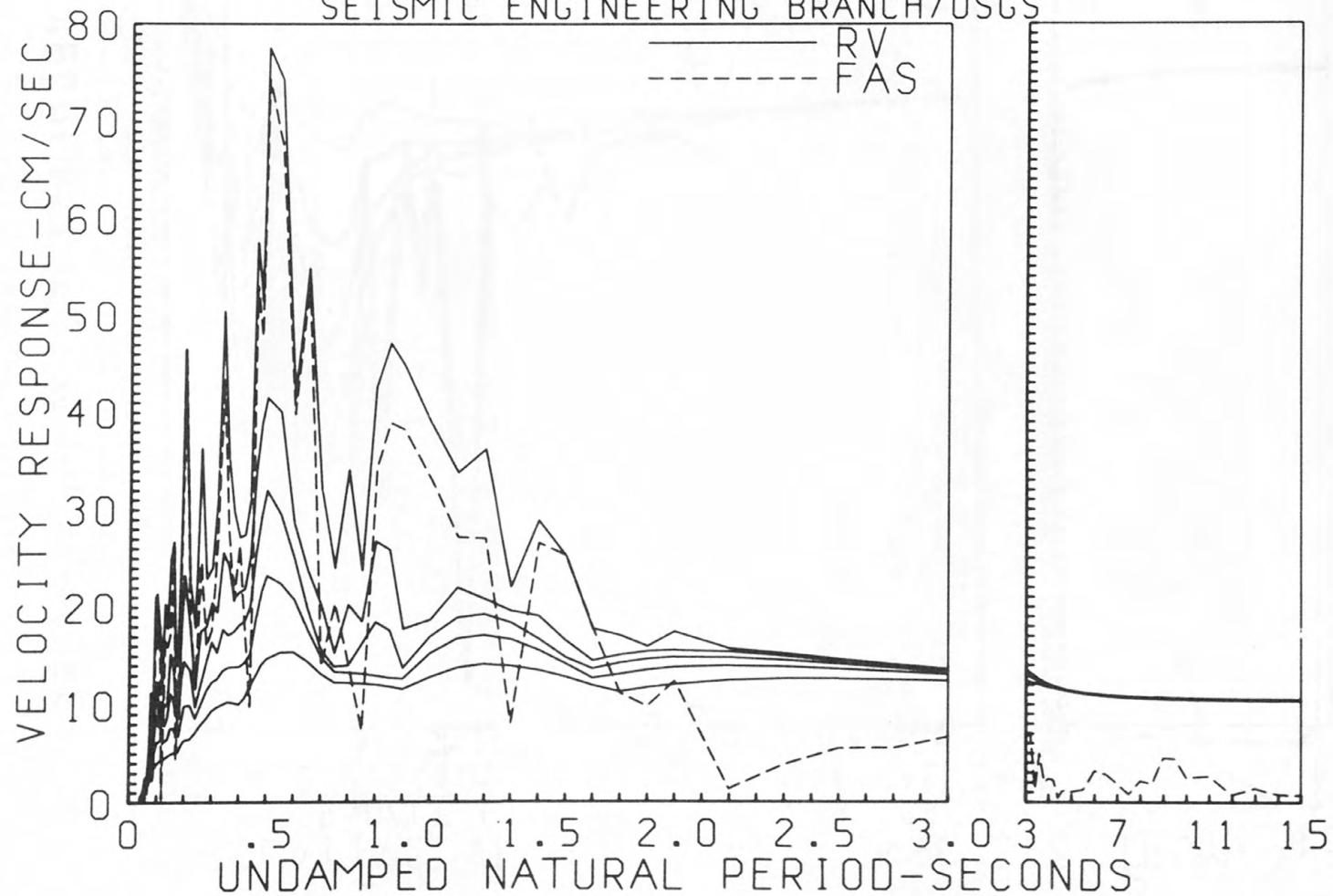


RELATIVE VELOCITY RESPONSE SPECTRUM

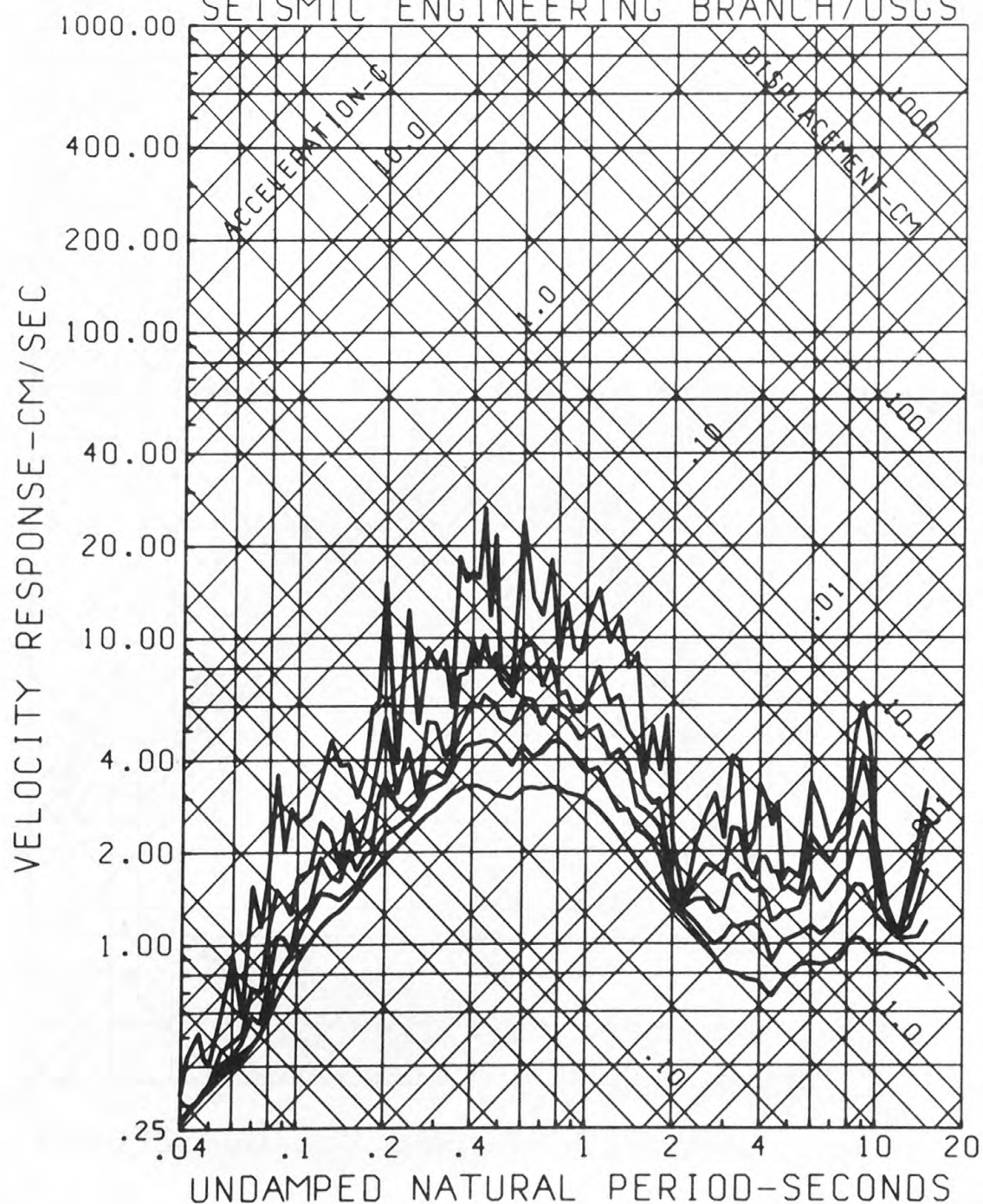
FERNDAL, CITY HALL, 6/7/75, 0846GMT, N46W

0.2, 5, 10, 20 PERCENT CRITICAL DAMPING

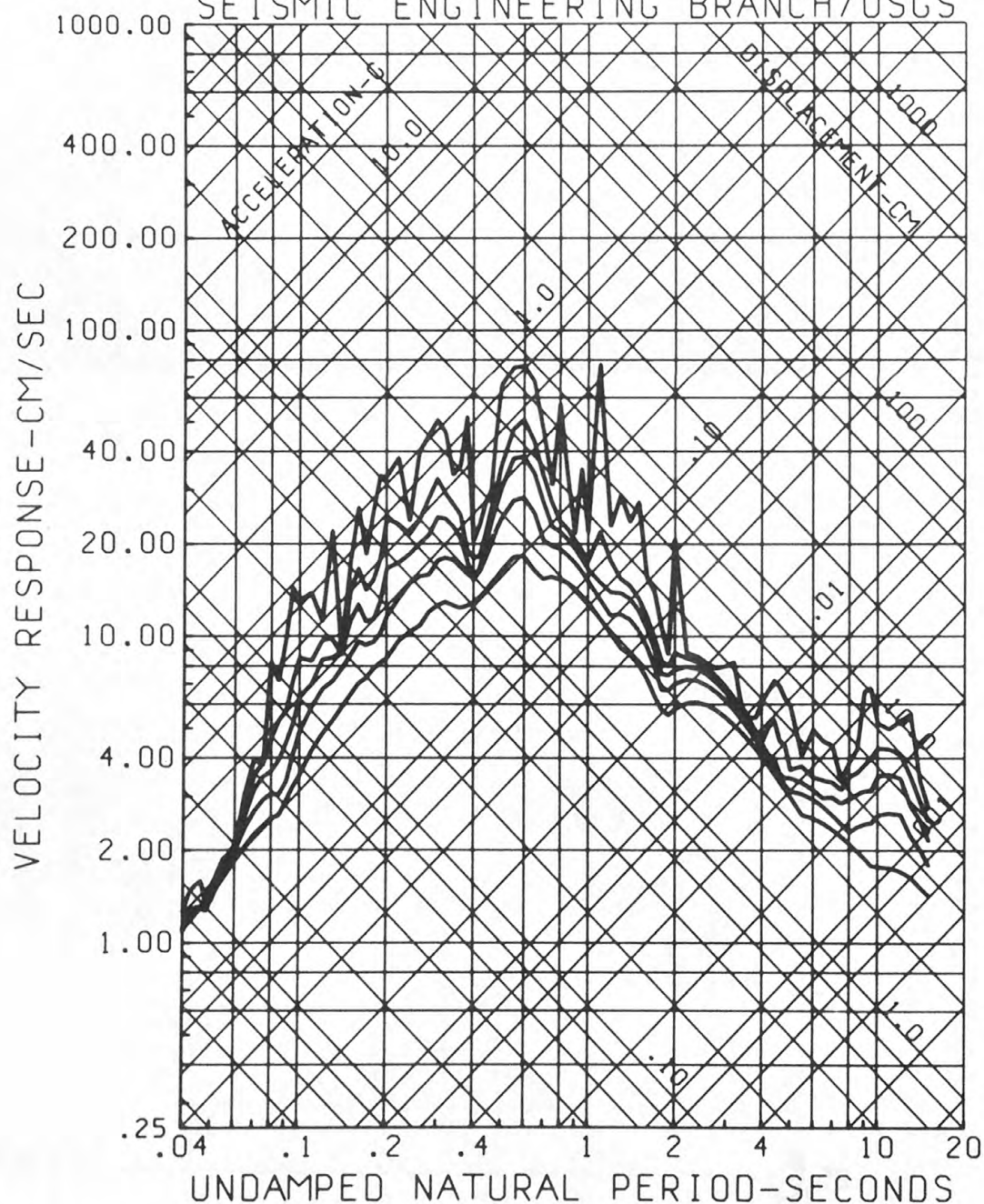
SEISMIC ENGINEERING BRANCH/USGS



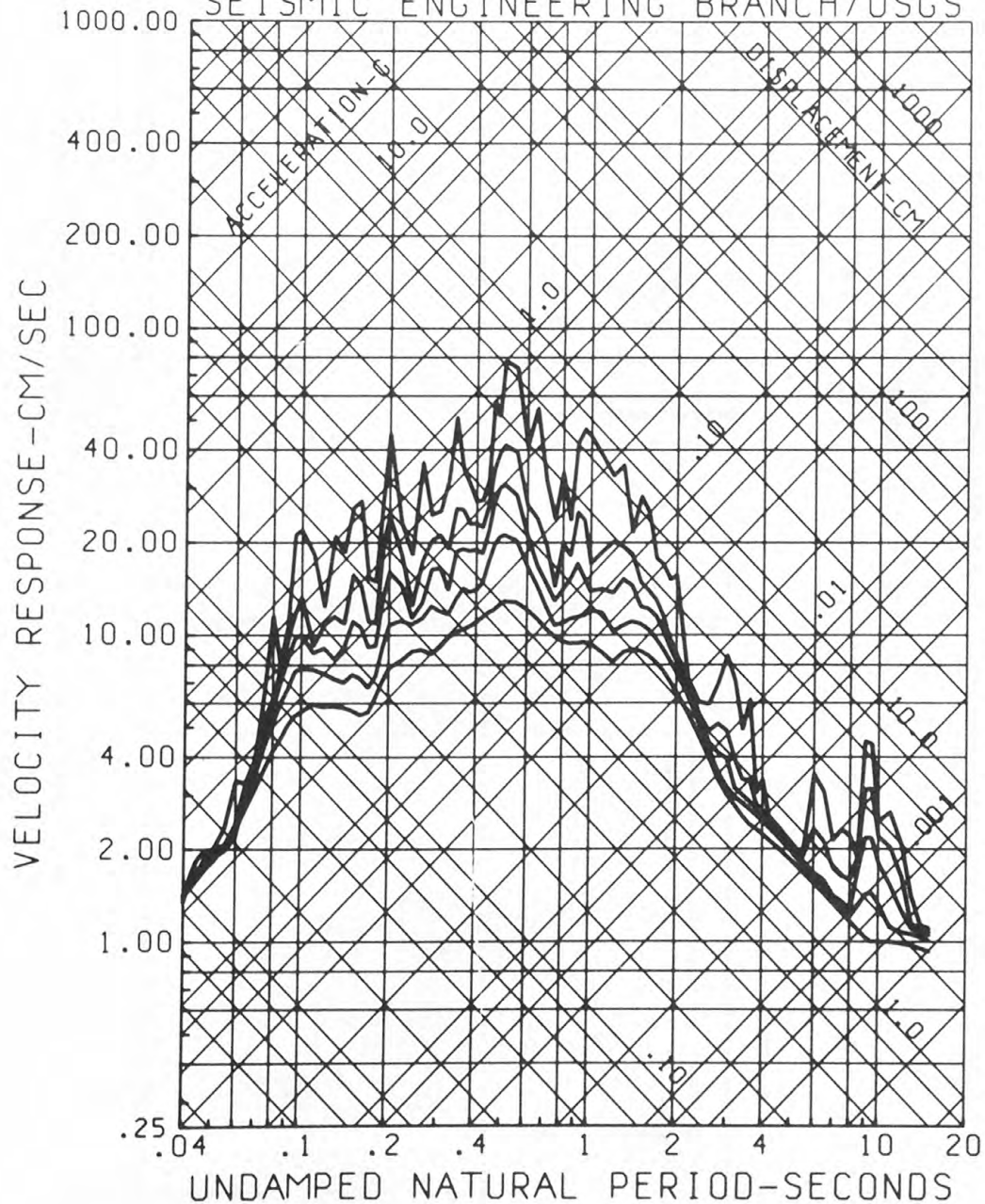
RESPONSE SPECTRA
 FERNDAL, CITY HALL, 6/7/75, 0846GMT, UP
 0.2, 5, 10, 20 PERCENT CRITICAL DAMPING
 SEISMIC ENGINEERING BRANCH/USGS

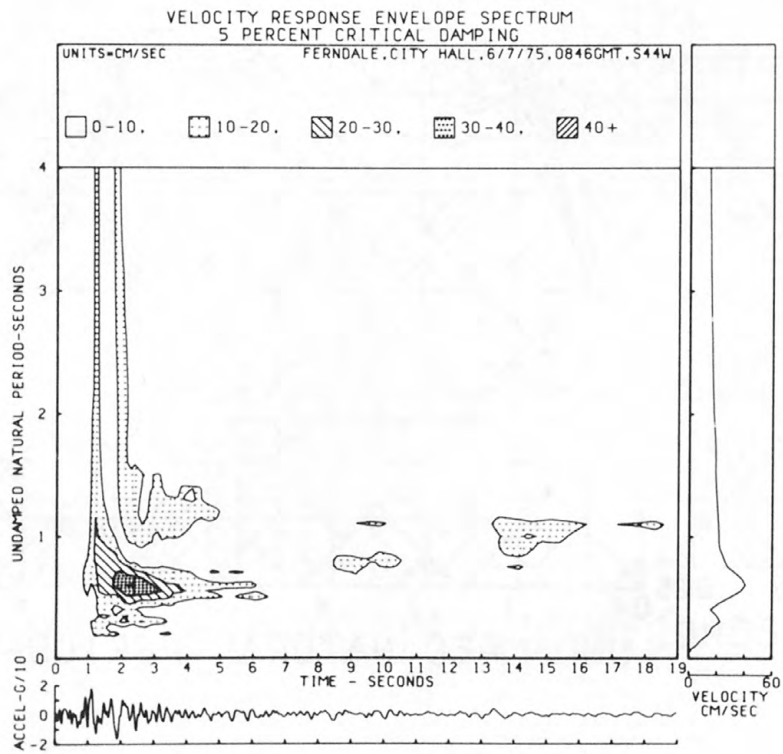
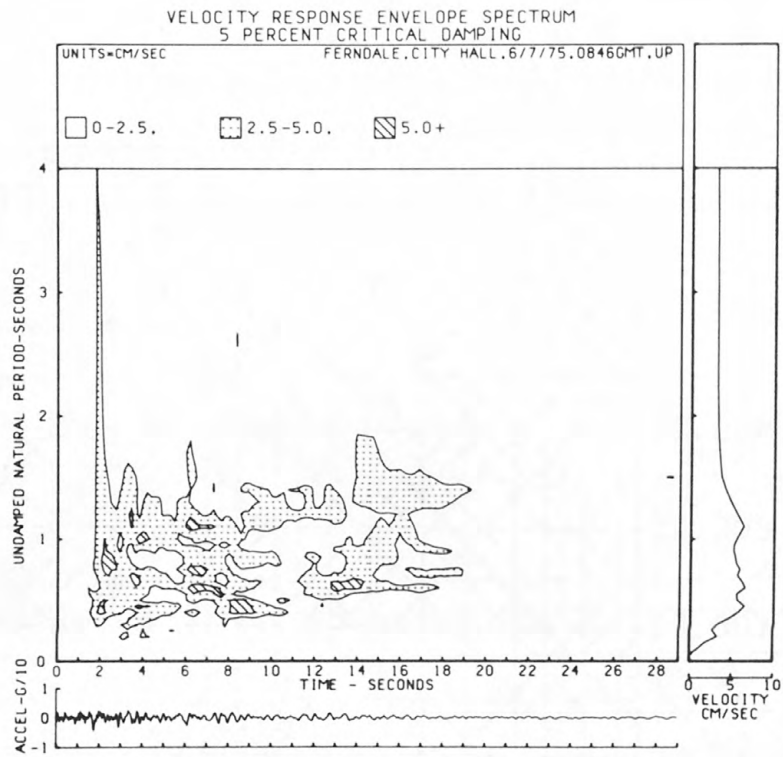


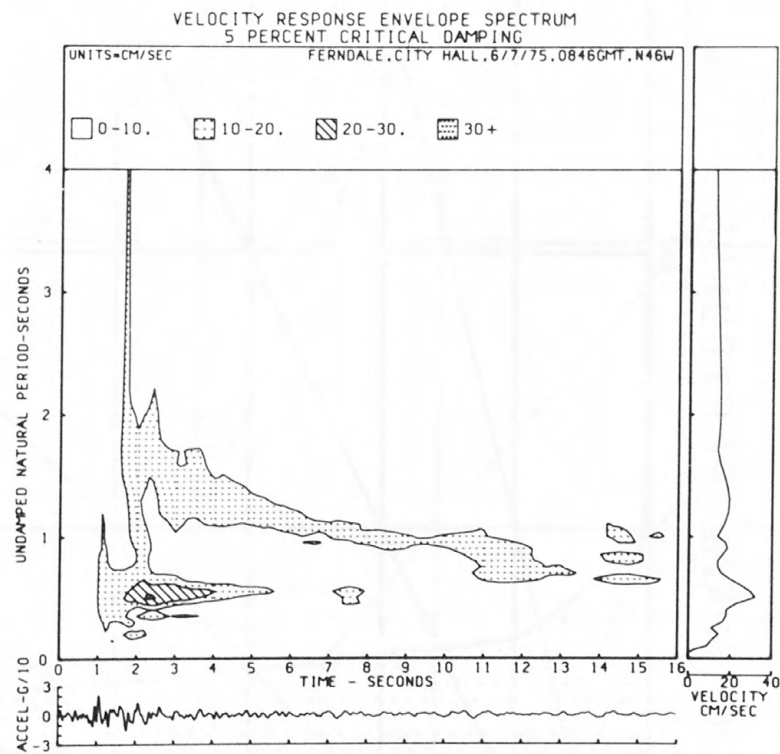
RESPONSE SPECTRA
 FERNDAL, CITY HALL, 6/7/75, 0846GMT, S44W
 0.2, 5, 10, 20 PERCENT CRITICAL DAMPING
 SEISMIC ENGINEERING BRANCH/USGS



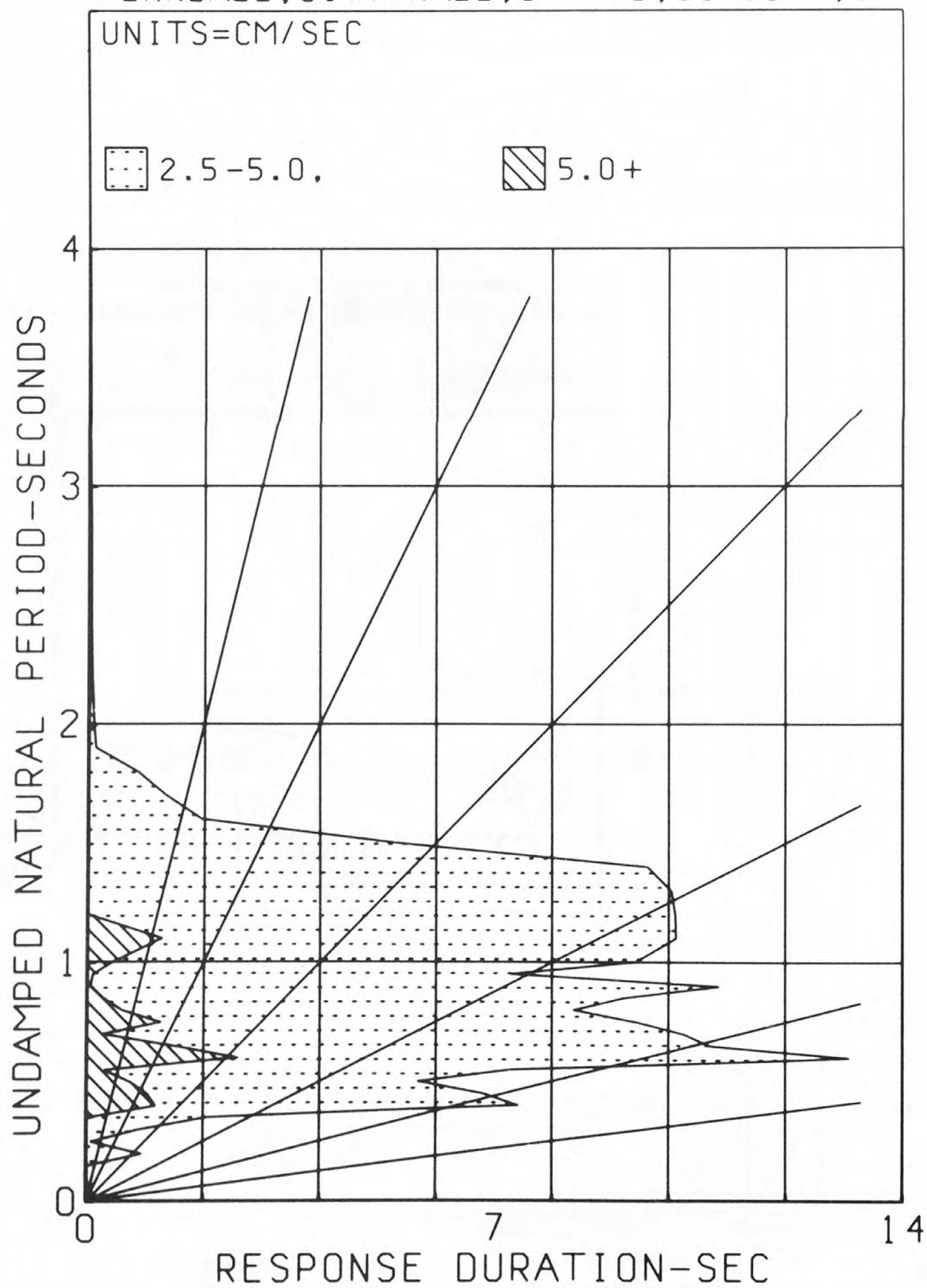
RESPONSE SPECTRA
 FERNDAL.E.CITY HALL.6/7/75.0846GMT.N46W
 0.2.5.10.20 PERCENT CRITICAL DAMPING
 SEISMIC ENGINEERING BRANCH/USGS



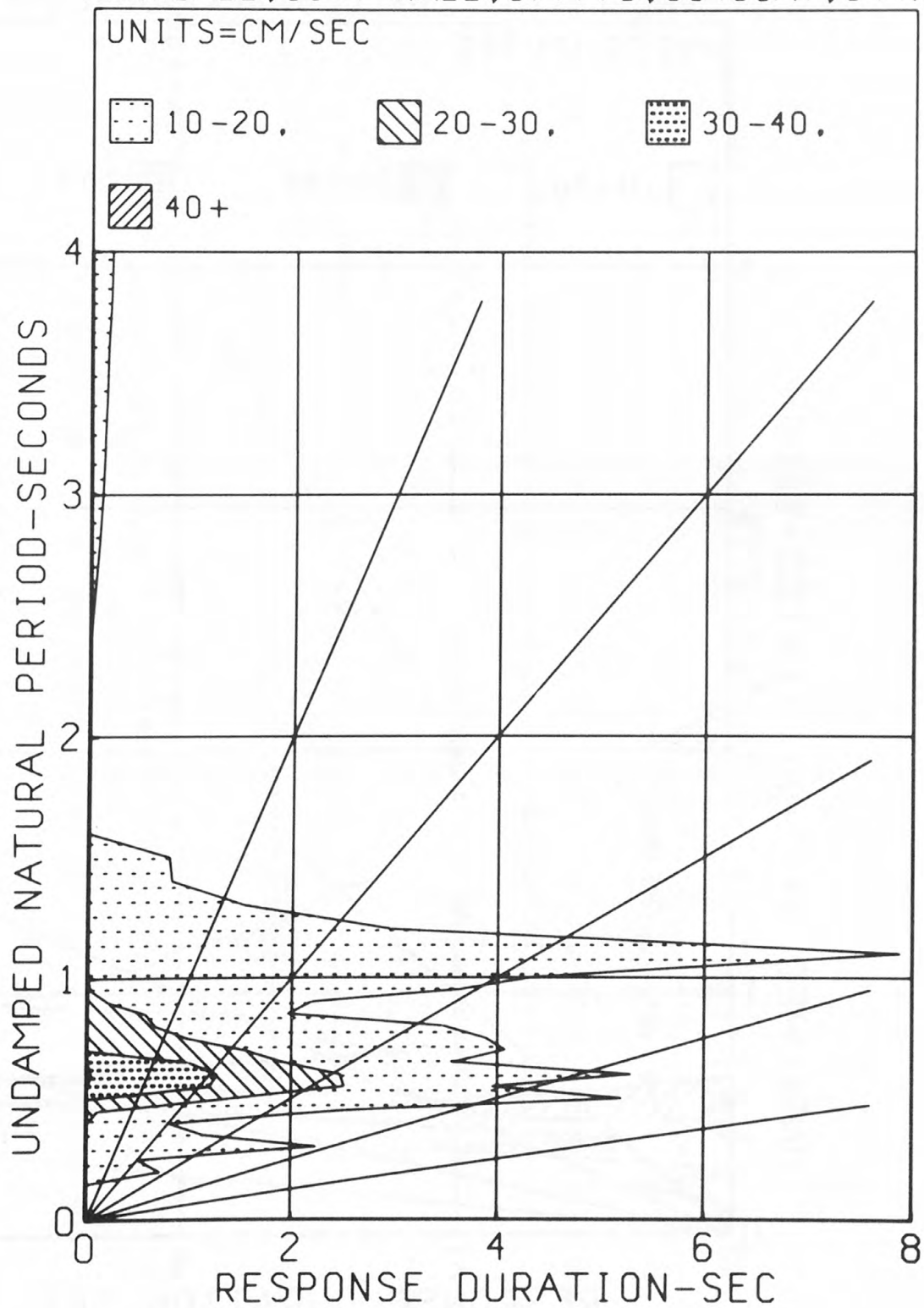




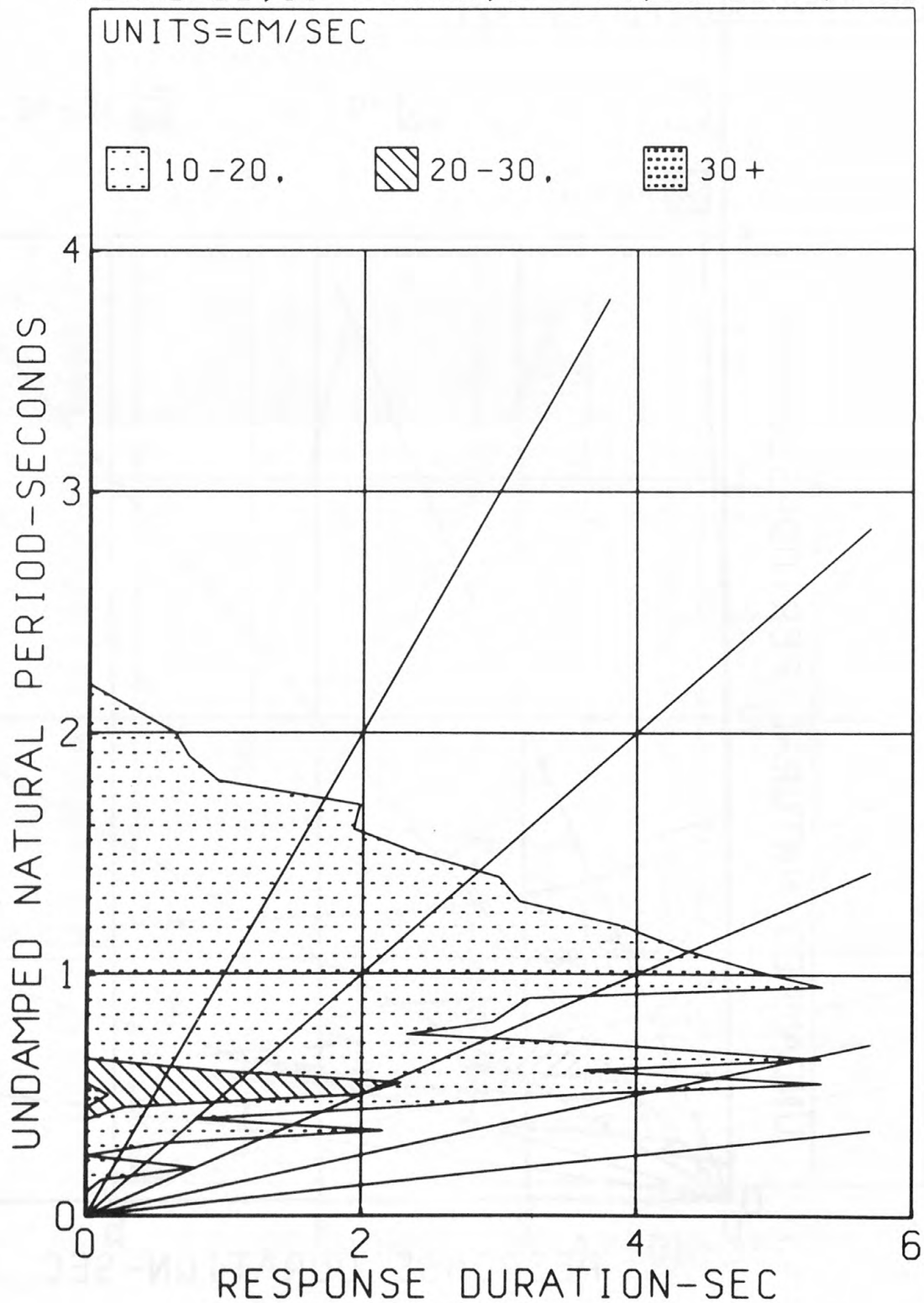
DURATION SPECTRUM OF THE VELOCITY
 RESPONSE ENVELOPE, 5 PERCENT DAMPING
 FERNDAL, CITY HALL, 6/7/75, 0846GMT, UP

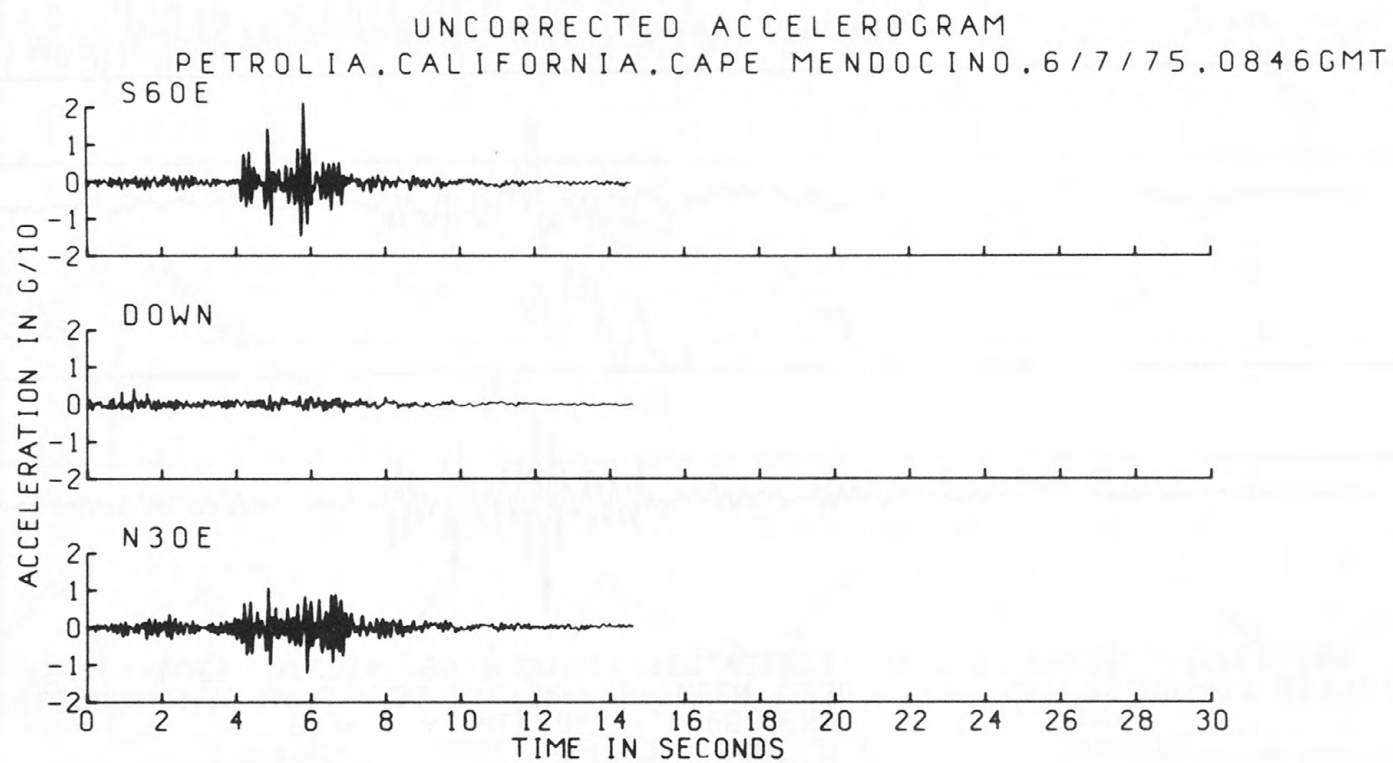


DURATION SPECTRUM OF THE VELOCITY
 RESPONSE ENVELOPE, 5 PERCENT DAMPING
 FERNDALE, CITY HALL, 6/7/75, 0846GMT, S44W

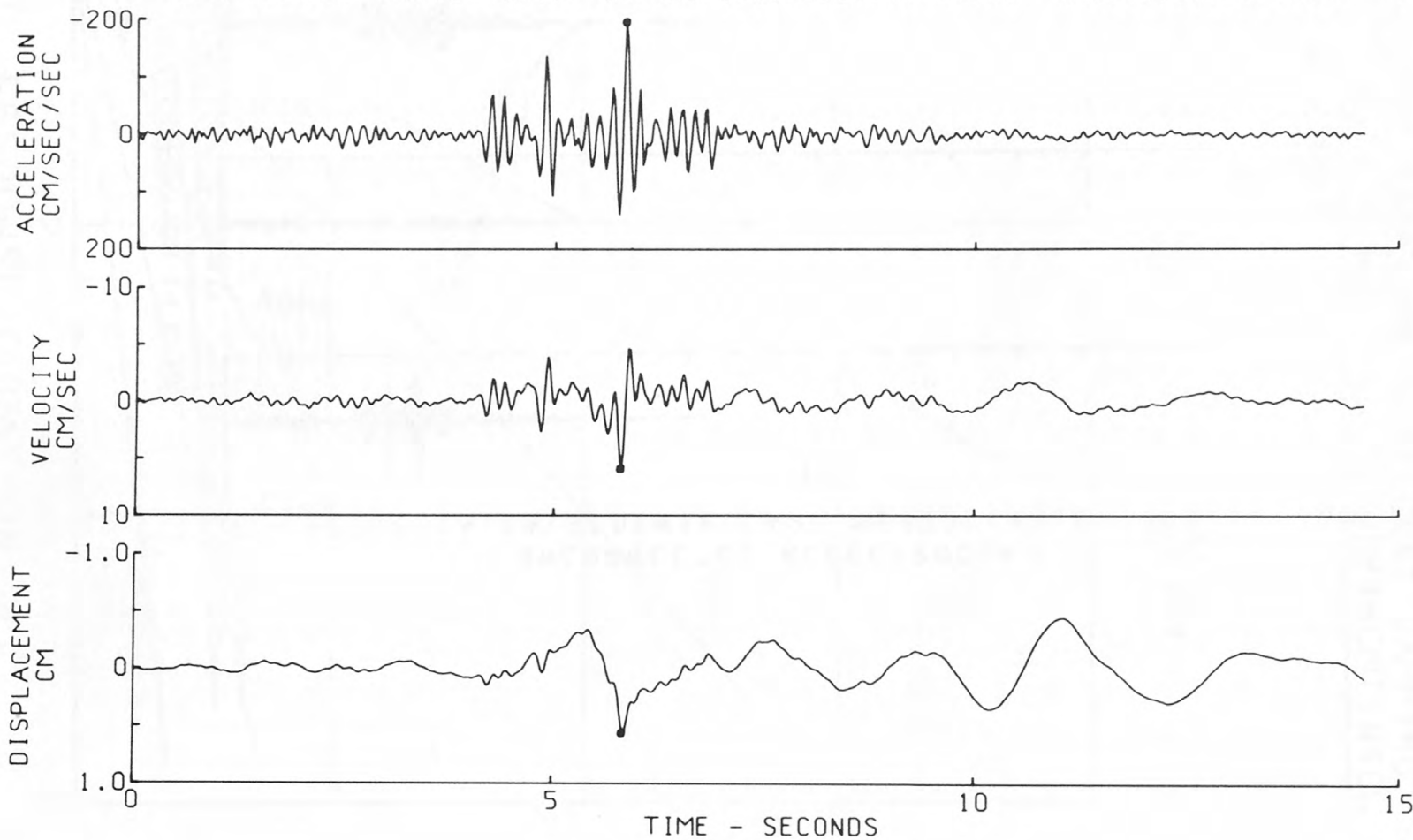


DURATION SPECTRUM OF THE VELOCITY
RESPONSE ENVELOPE, 5 PERCENT DAMPING
FERNDAL, CITY HALL, 6/7/75, 0846GMT, N46W

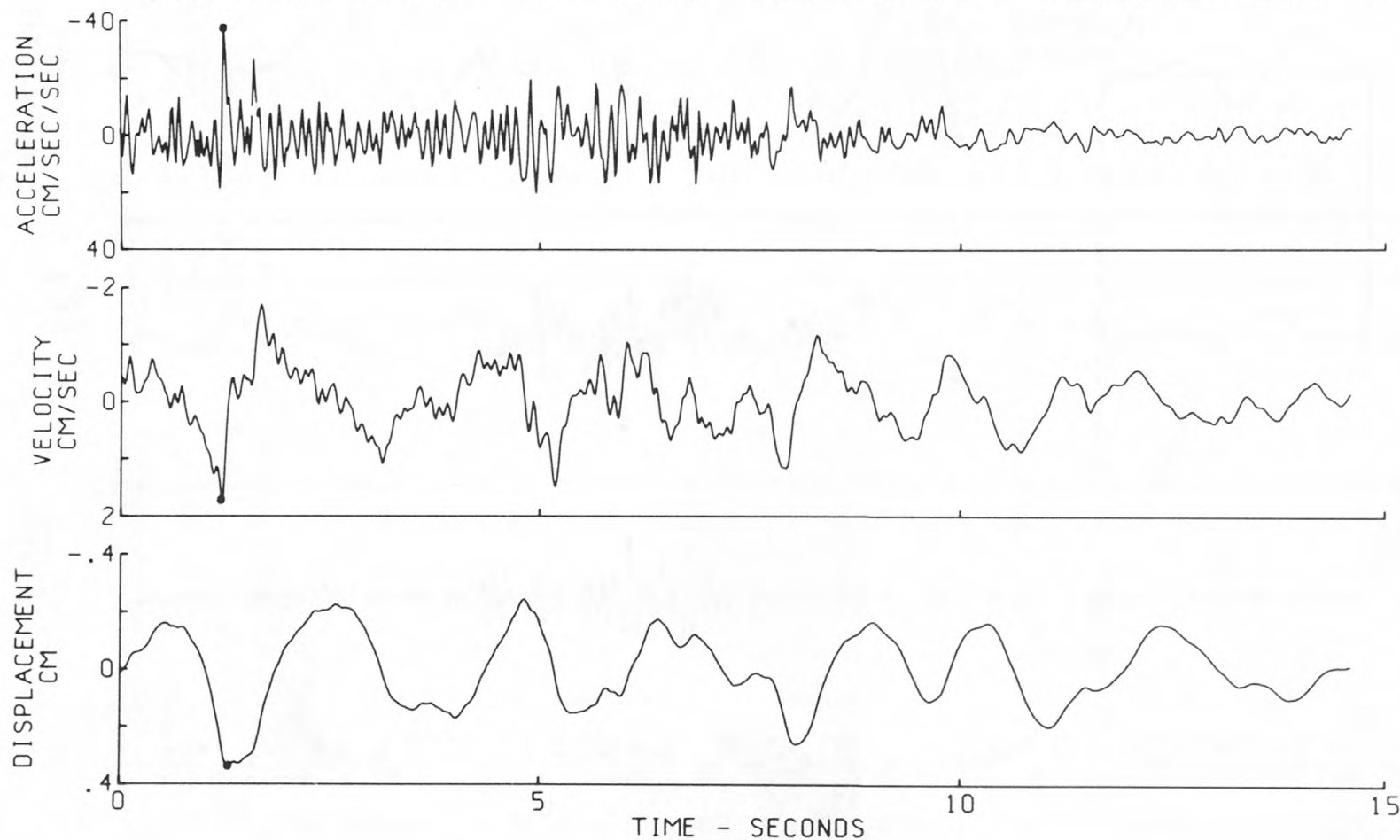




CORRECTED ACCELERATION, VELOCITY, DISPLACEMENT
HUMBOLDT COUNTY EARTHQUAKE OF JUNE 7, 1975, 0846GMT
PETROLIA, CALIFORNIA, CAPE MENDOCINO, S60E COMP
ACCELEROGRAM IS BAND PASS FILTERED BETWEEN .230 - .330 AND 25.00 - 27.00 CYC/SEC
• PEAK VALUES ACCEL=-198.7 CM/SEC/SEC, VELOCITY=5.910 CM/SEC, DISPL=.560 CM

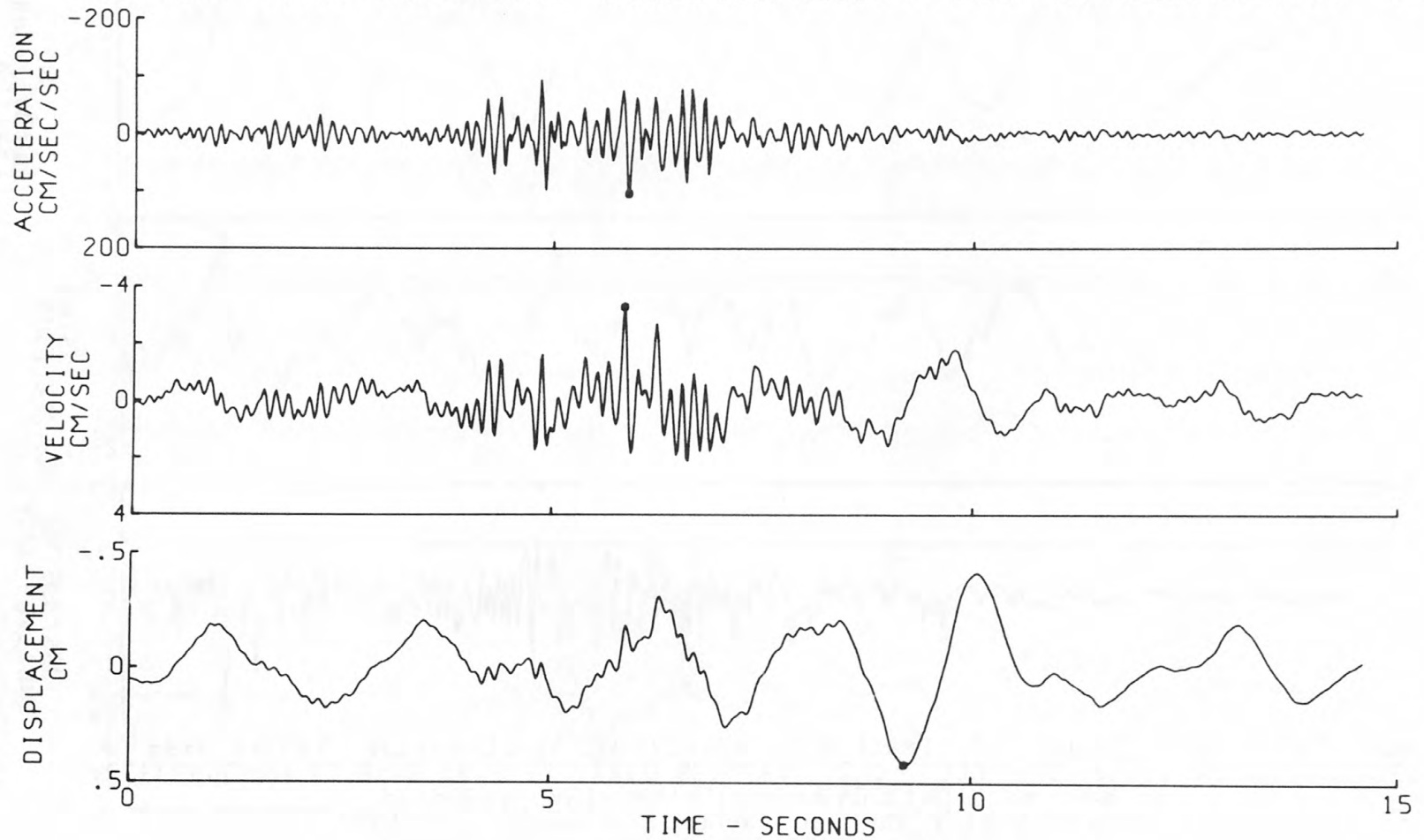


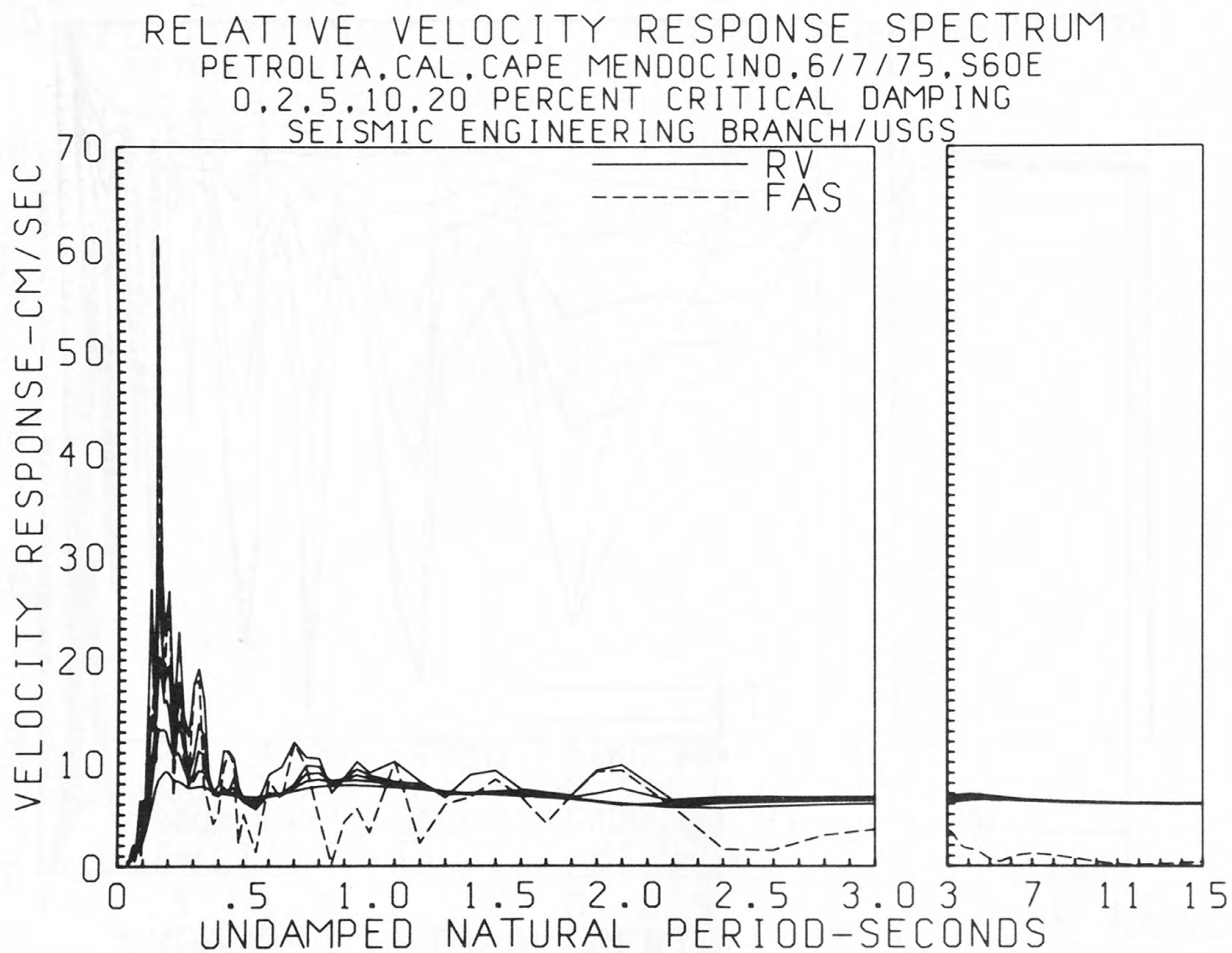
CORRECTED ACCELERATION, VELOCITY, DISPLACEMENT
 HUMBOLDT COUNTY EARTHQUAKE OF JUNE 7, 1975, 0846GMT
 PETROLIA, CALIFORNIA, CAPE MENDOCINO, DOWN COMP
 ACCELEROGRAM IS BAND PASS FILTERED BETWEEN .230 - .330 AND 25.00 - 27.00 CYC/SEC
 • PEAK VALUES ACCEL=-37.75 CM/SEC/SEC, VELOCITY=1.720 CM/SEC, DISPL=.340 CM



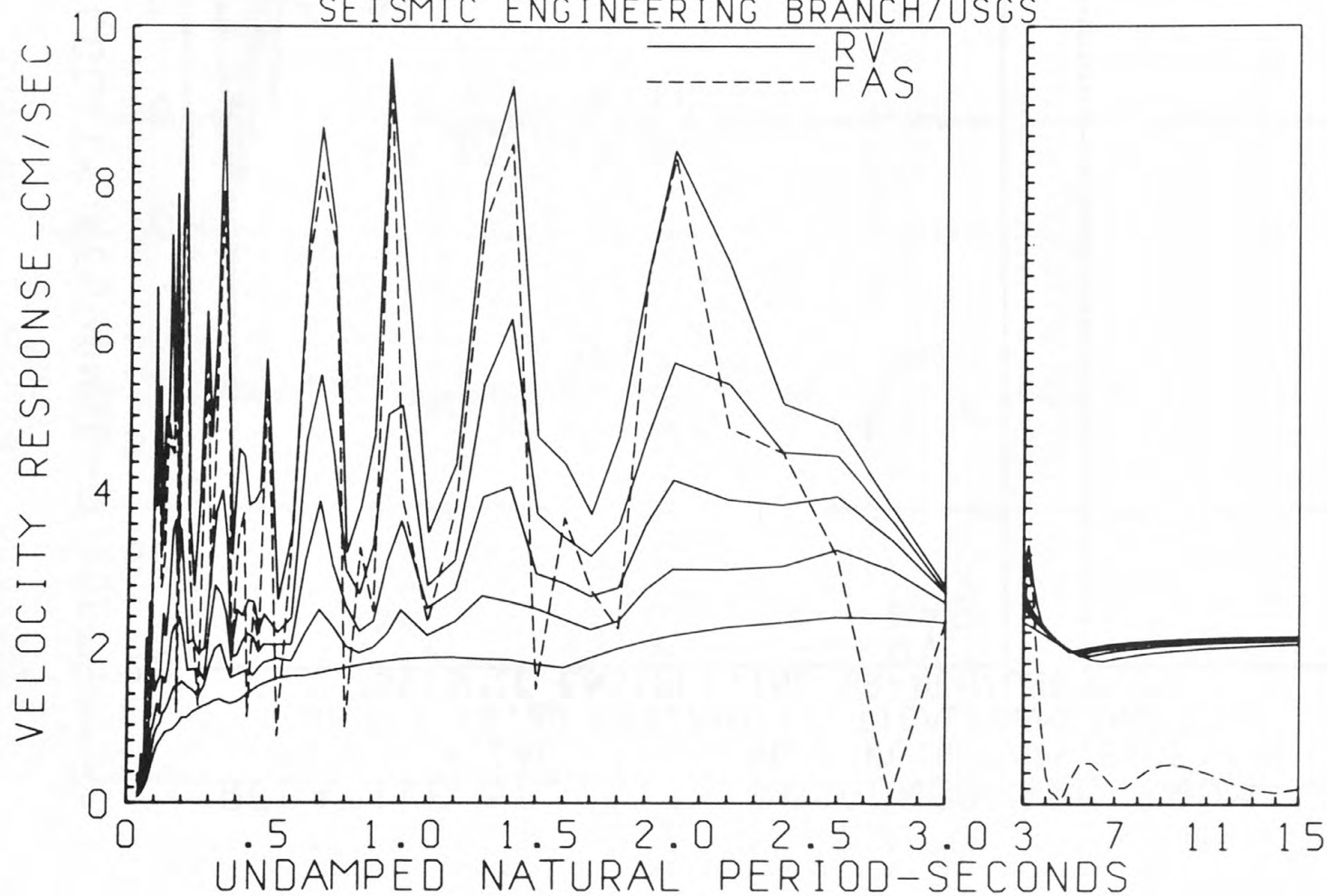
CORRECTED ACCELERATION, VELOCITY, DISPLACEMENT
HUMBOLDT COUNTY EARTHQUAKE OF JUNE 7, 1975, 0846GMT
PETROLIA, CALIFORNIA, CAPE MENDOCINO, N30E COMP

ACCELEROGRAM IS BAND PASS FILTERED BETWEEN .230 - .330 AND 25.00 - 27.00 CYC/SEC
• PEAK VALUES ACCEL=103.0 CM/SEC/SEC, VELOCITY=-3.300 CM/SEC, DISPL=.420 CM

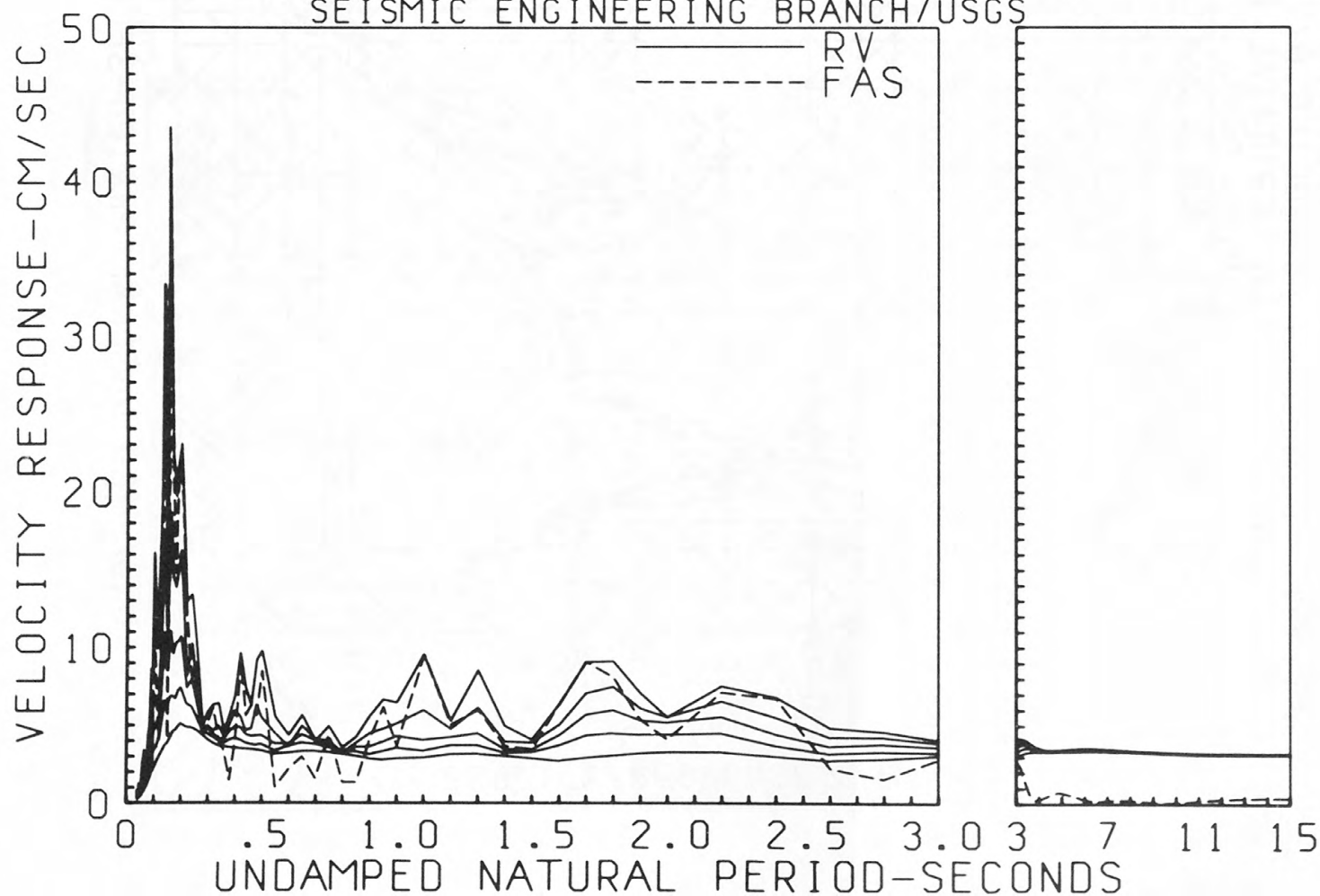




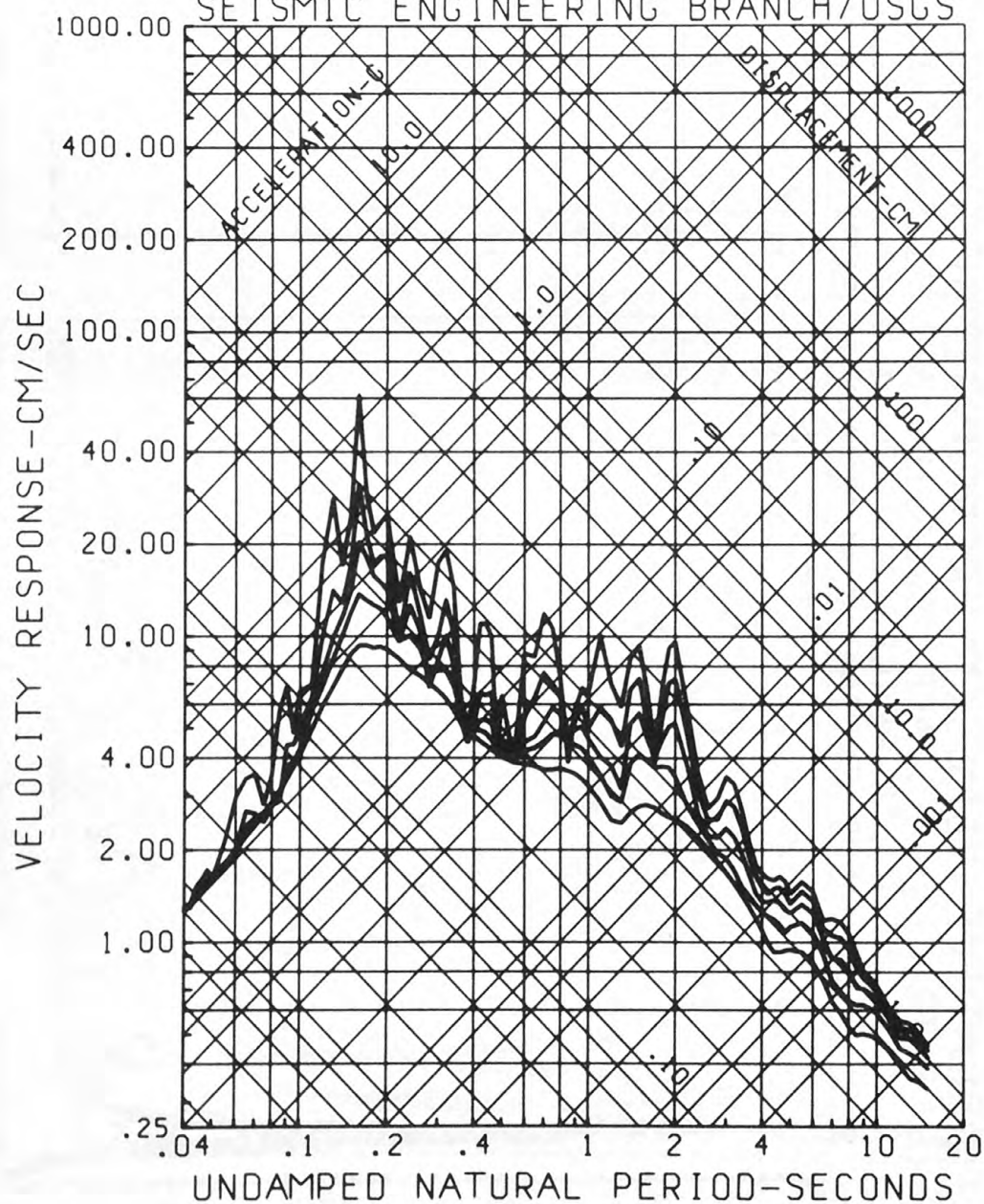
RELATIVE VELOCITY RESPONSE SPECTRUM
 PETROLIA, CAL, CAPE MENDOCINO, 6/7/75, DOWN
 0.2, 5, 10, 20 PERCENT CRITICAL DAMPING
 SEISMIC ENGINEERING BRANCH/USGS

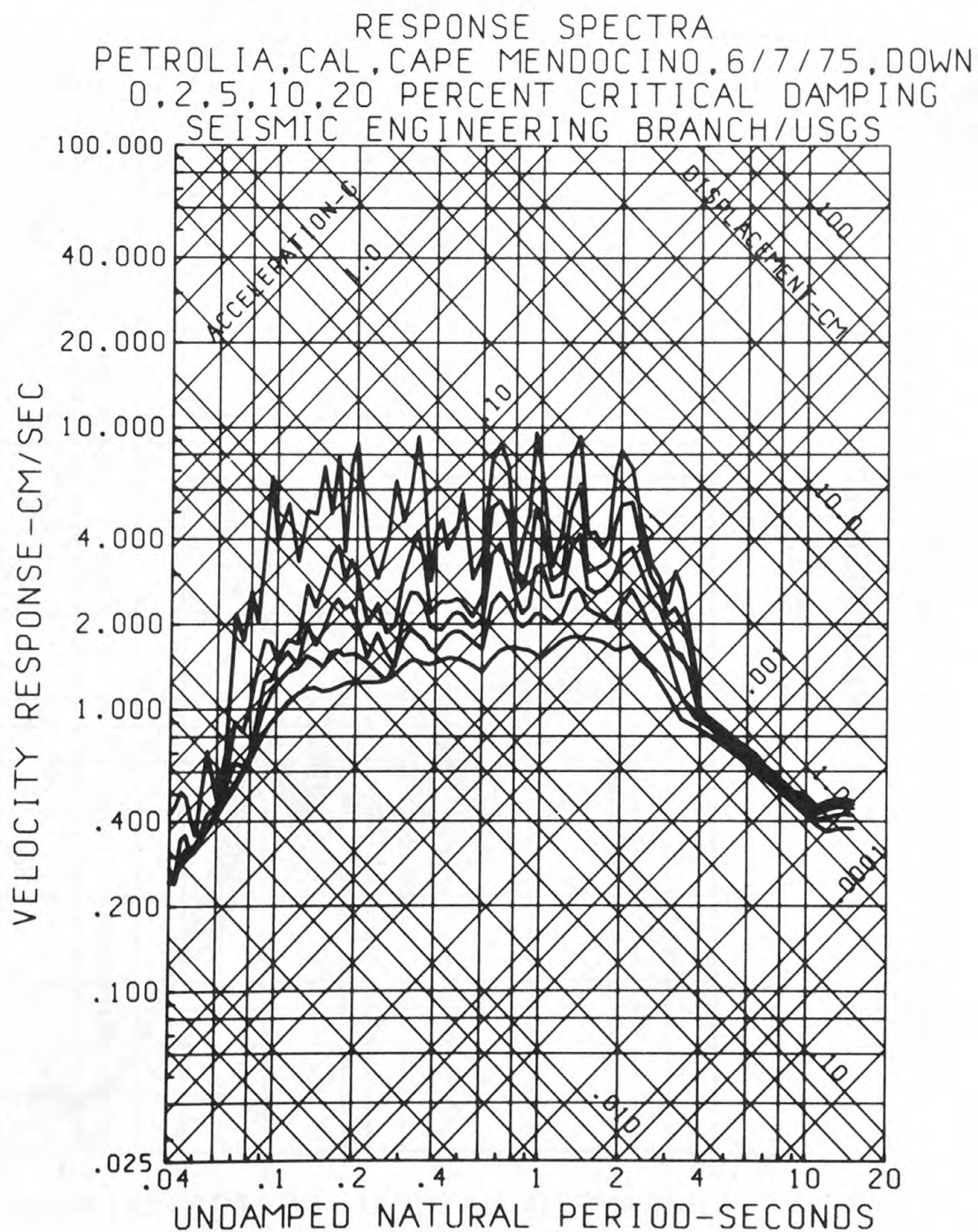


RELATIVE VELOCITY RESPONSE SPECTRUM
 PETROLIA,CAL,CAPE MENDOCINO,6/7/75,N30E
 0,2,5,10,20 PERCENT CRITICAL DAMPING
 SEISMIC ENGINEERING BRANCH/USGS

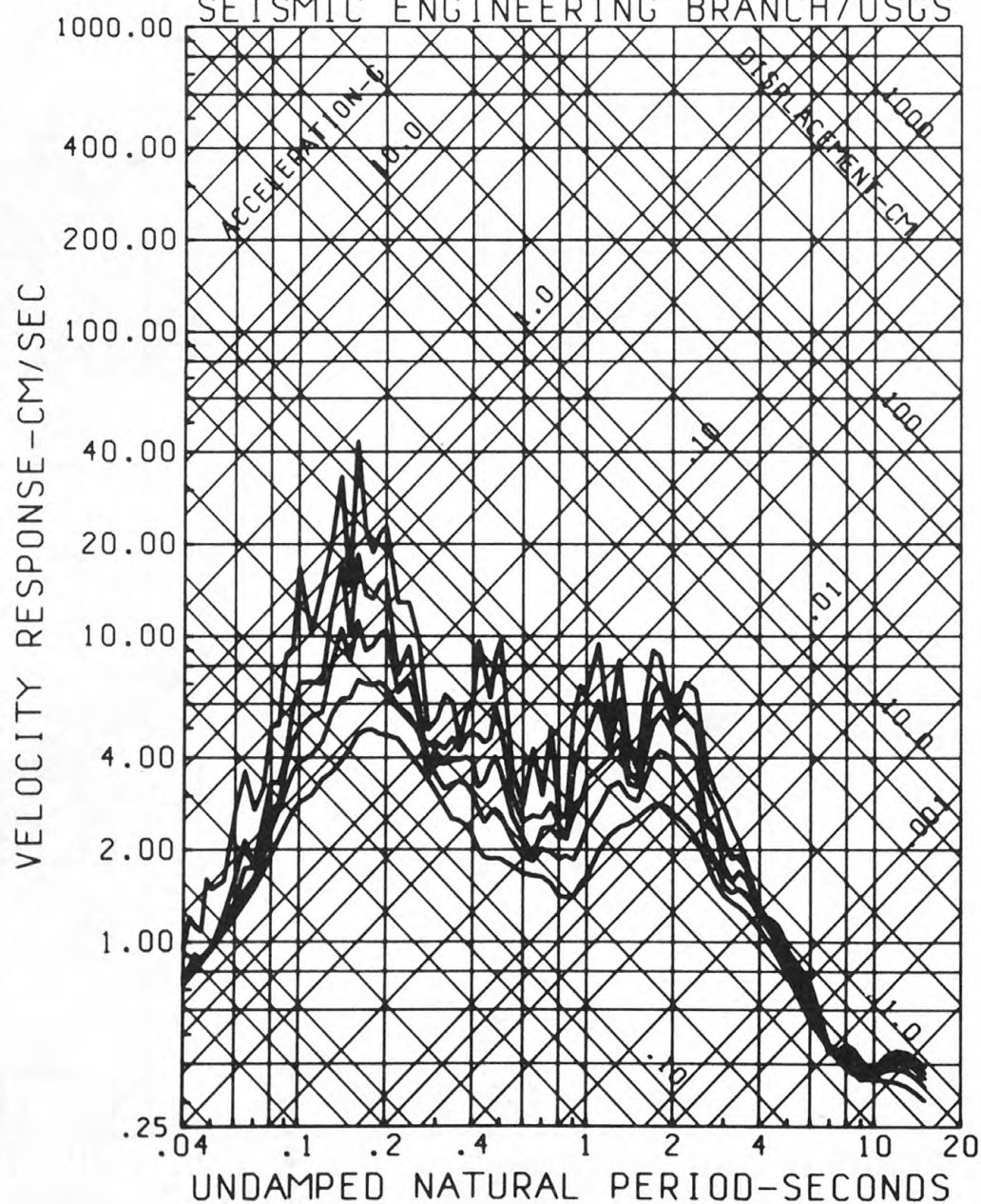


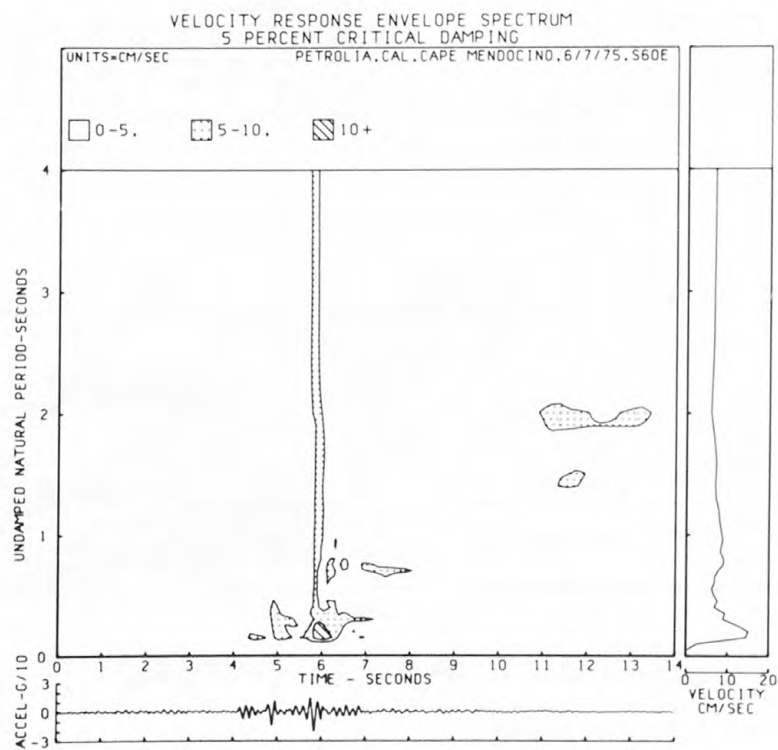
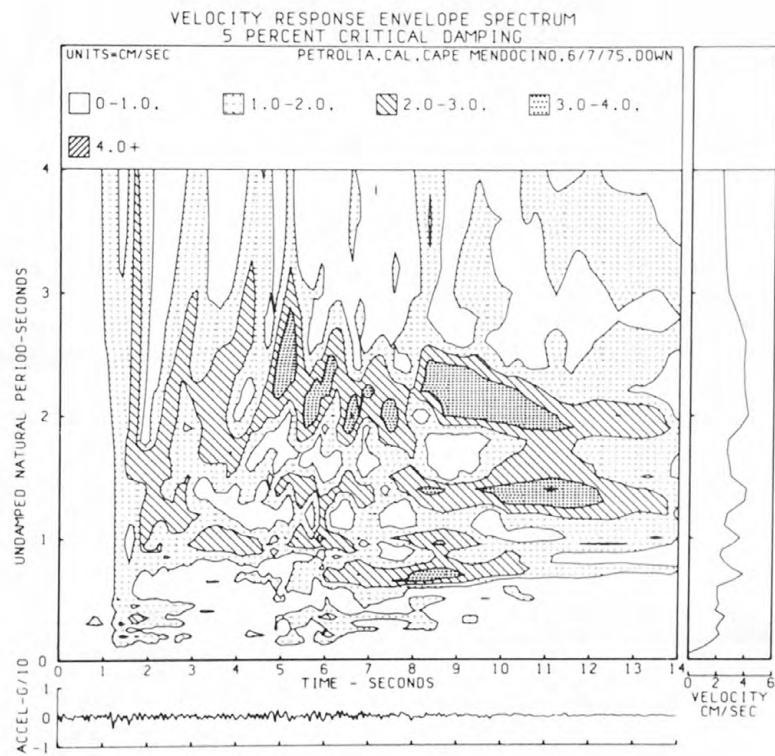
RESPONSE SPECTRA
 PETROLIA.CAL,CAPE MENDOCINO,6/7/75,S60E
 0.2,5,10,20 PERCENT CRITICAL DAMPING
 SEISMIC ENGINEERING BRANCH/USGS

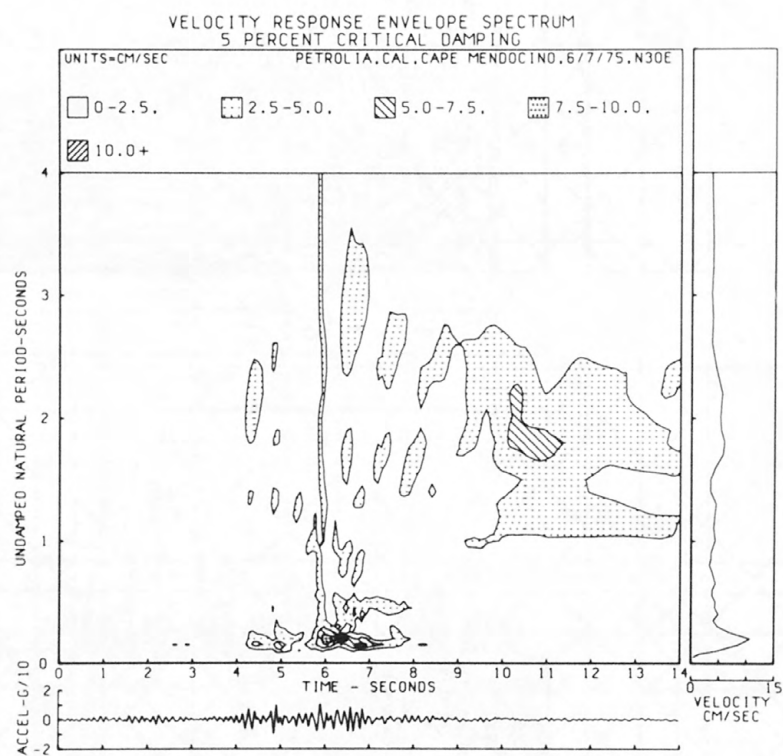




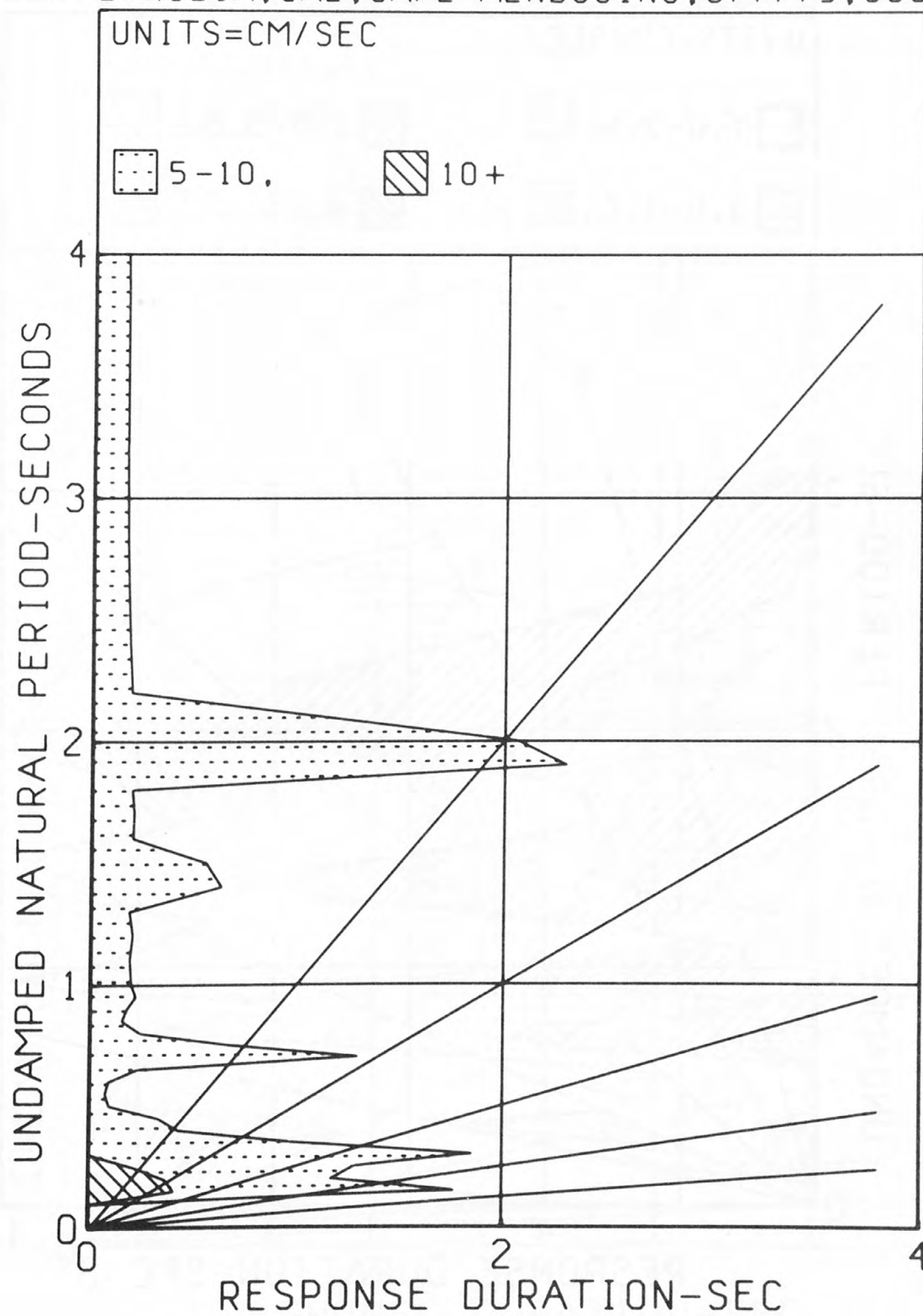
RESPONSE SPECTRA
 PETROLIA,CAL,CAPE MENDOCINO,6/7/75,N30E
 0,2,5,10,20 PERCENT CRITICAL DAMPING
 SEISMIC ENGINEERING BRANCH/USGS



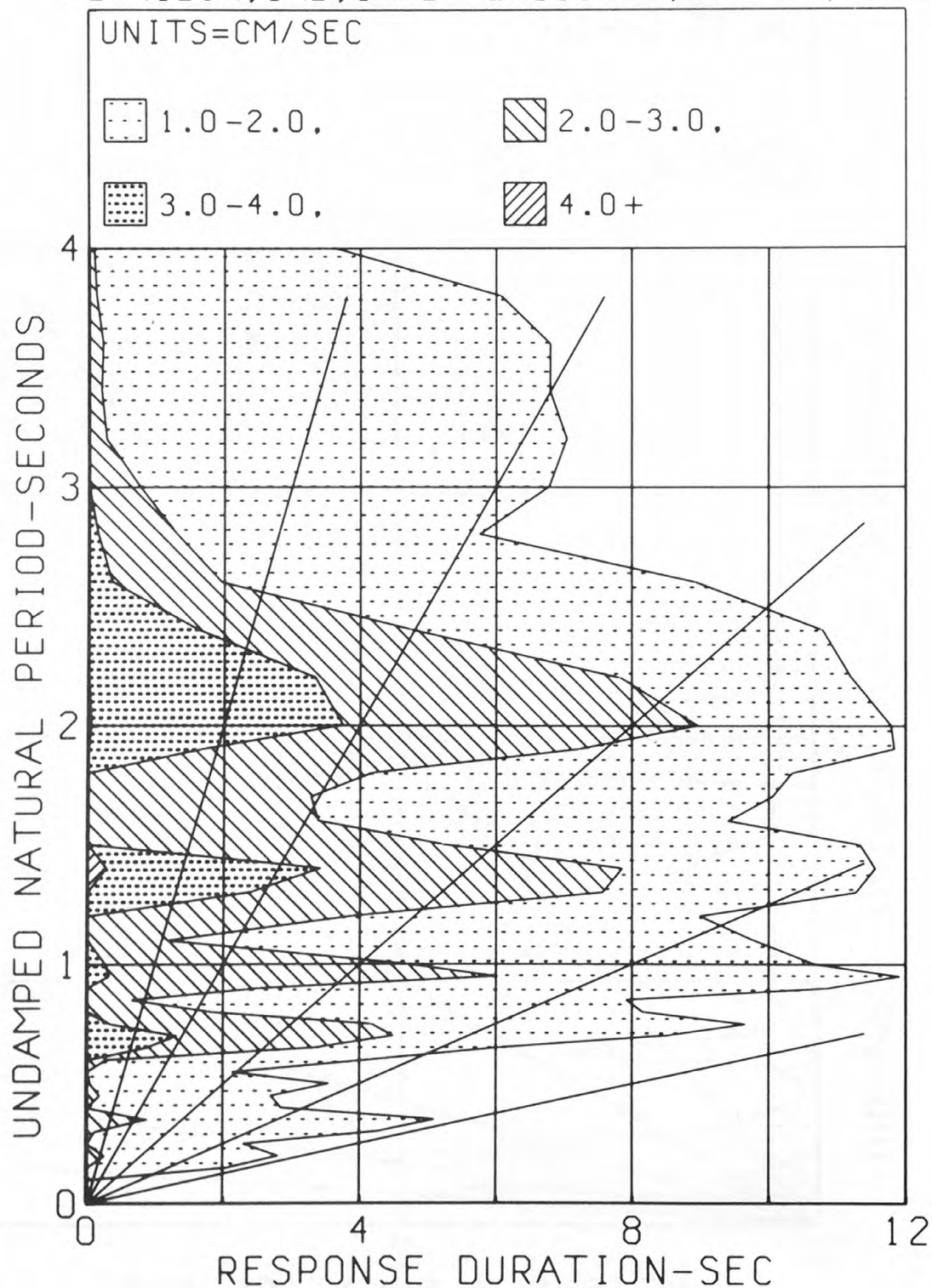




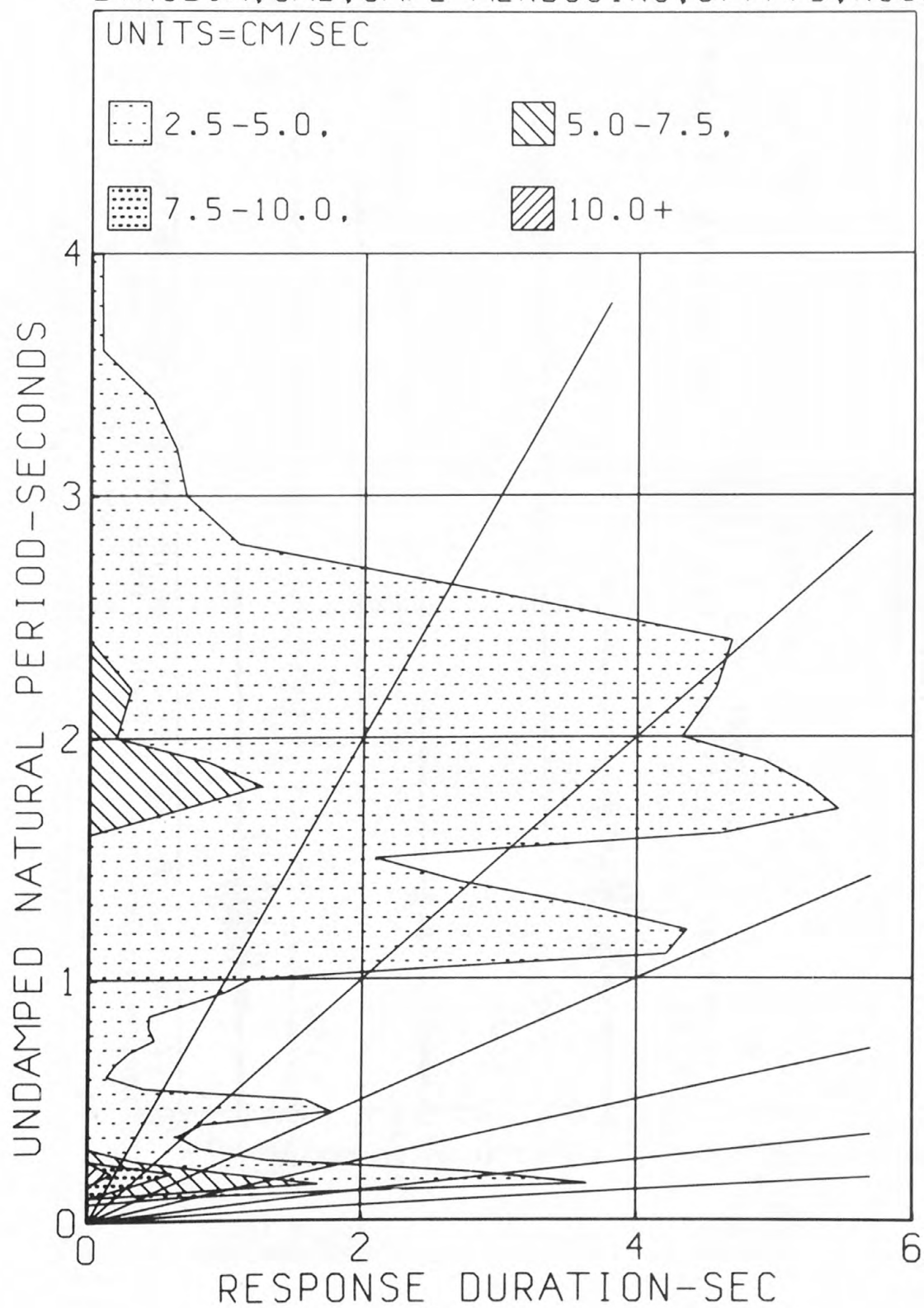
DURATION SPECTRUM OF THE VELOCITY
RESPONSE ENVELOPE, 5 PERCENT DAMPING
PETROLIA, CAL, CAPE MENDOCINO, 6/7/75, S60E

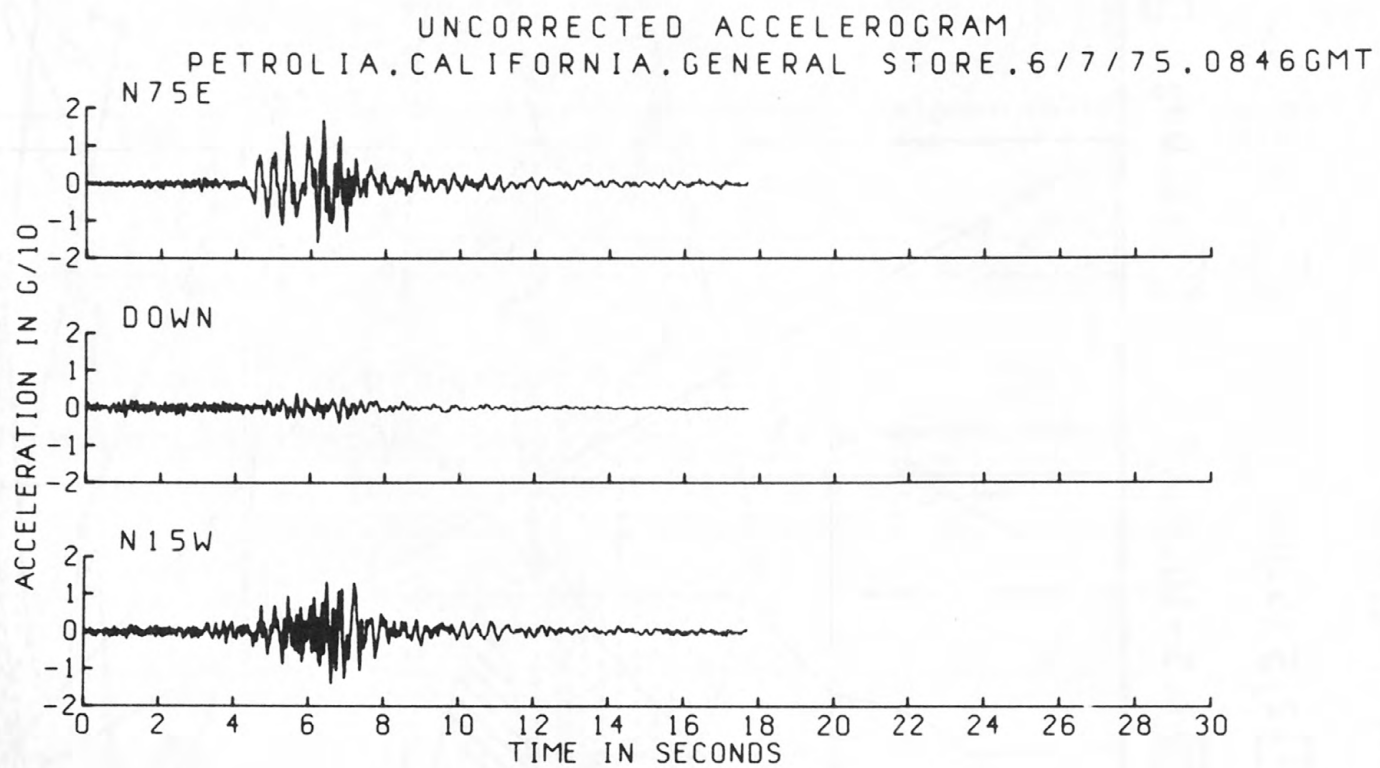


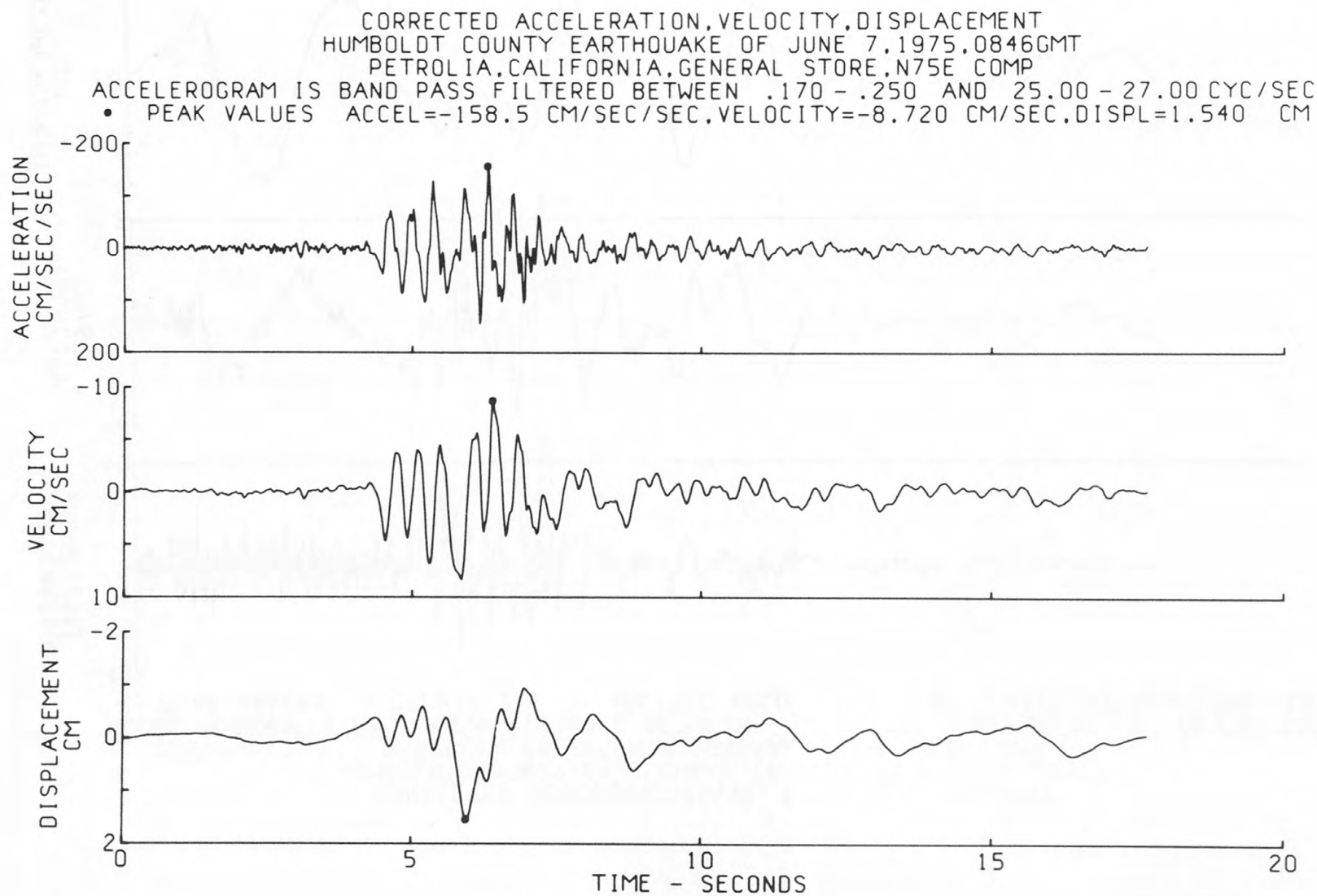
DURATION SPECTRUM OF THE VELOCITY
 RESPONSE ENVELOPE, 5 PERCENT DAMPING
 PETROLIA, CAL, CAPE MENDOCINO, 6/7/75, DOWN

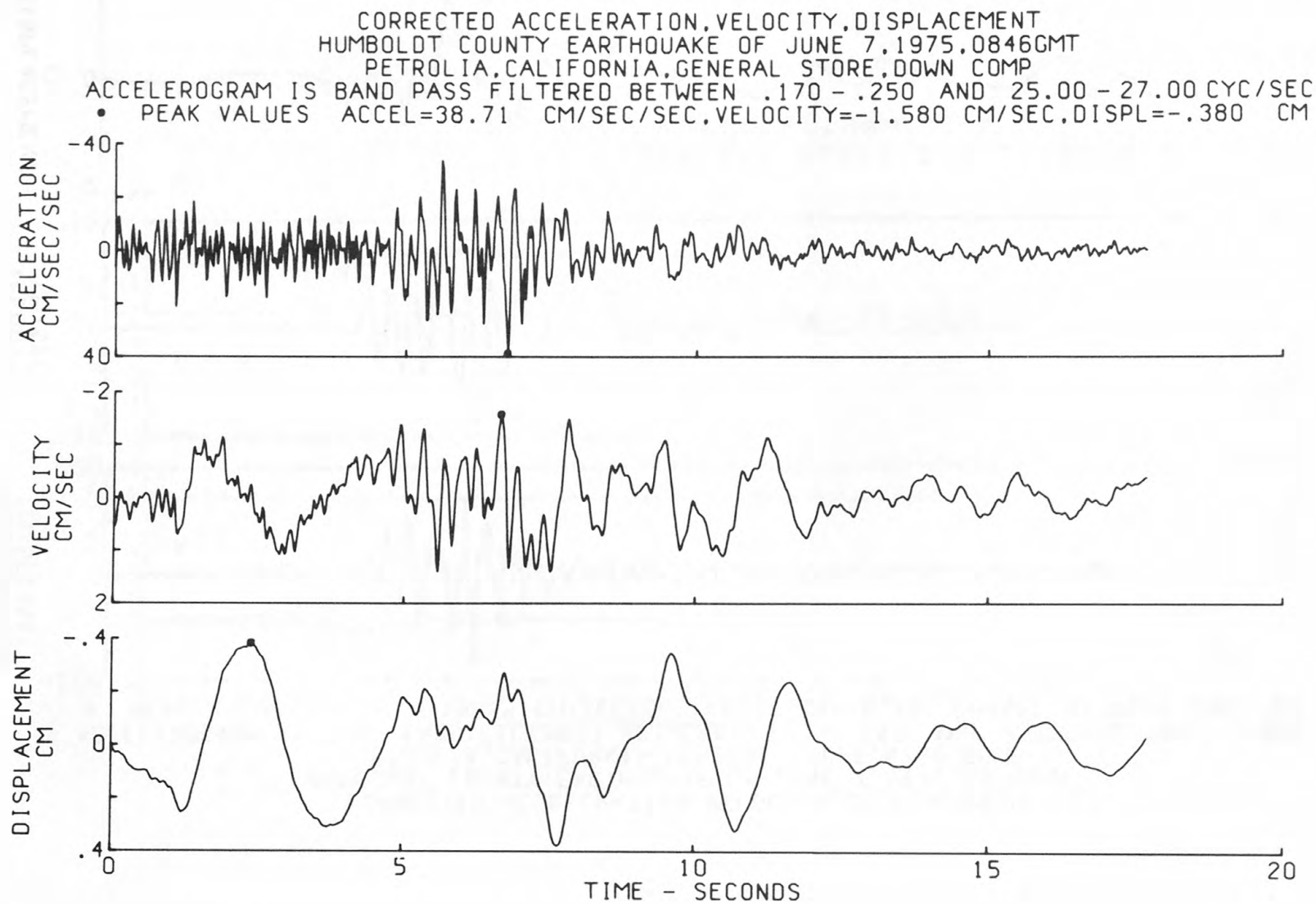


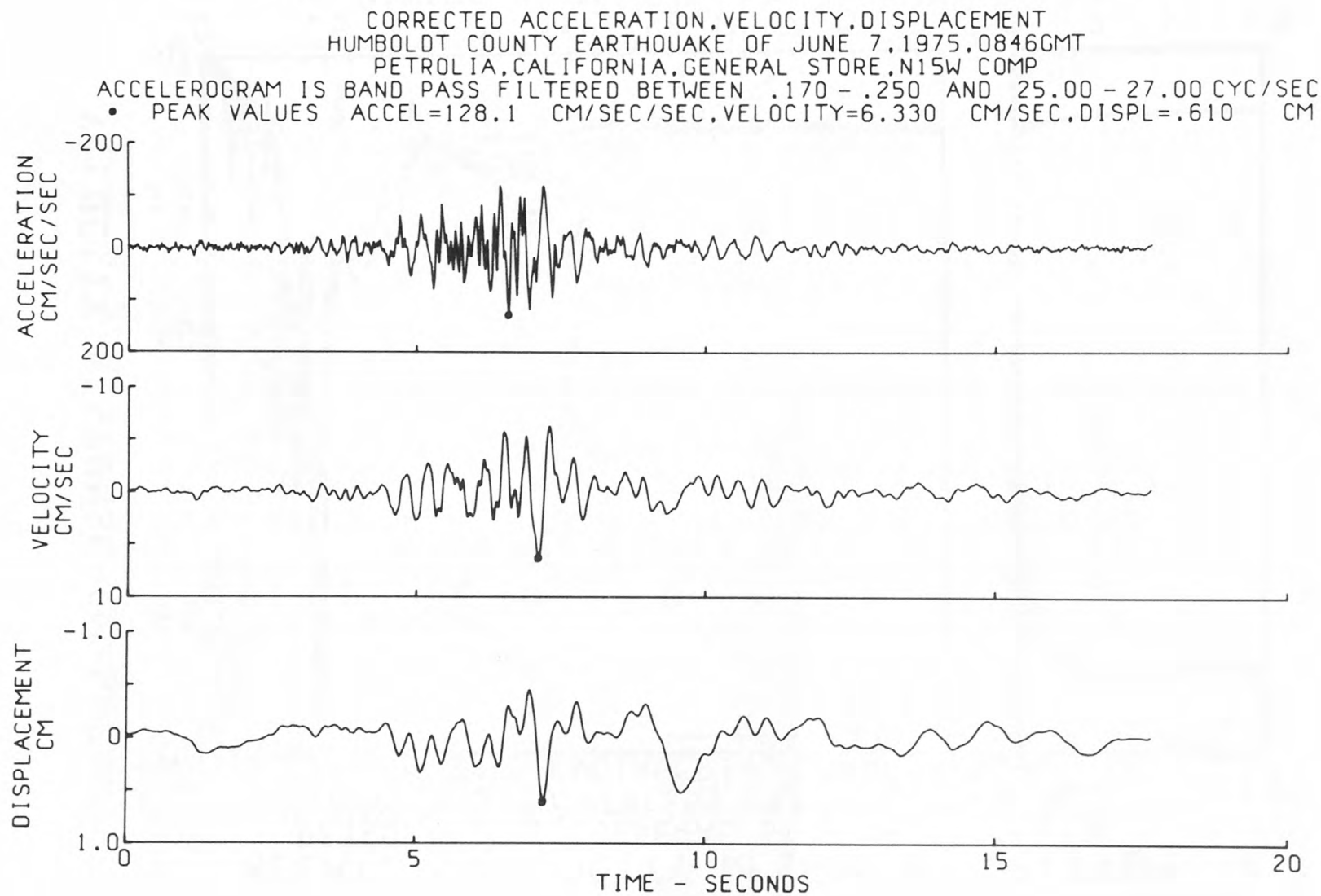
DURATION SPECTRUM OF THE VELOCITY
RESPONSE ENVELOPE, 5 PERCENT DAMPING
PETROLIA, CAL, CAPE MENDOCINO, 6/7/75, N30E



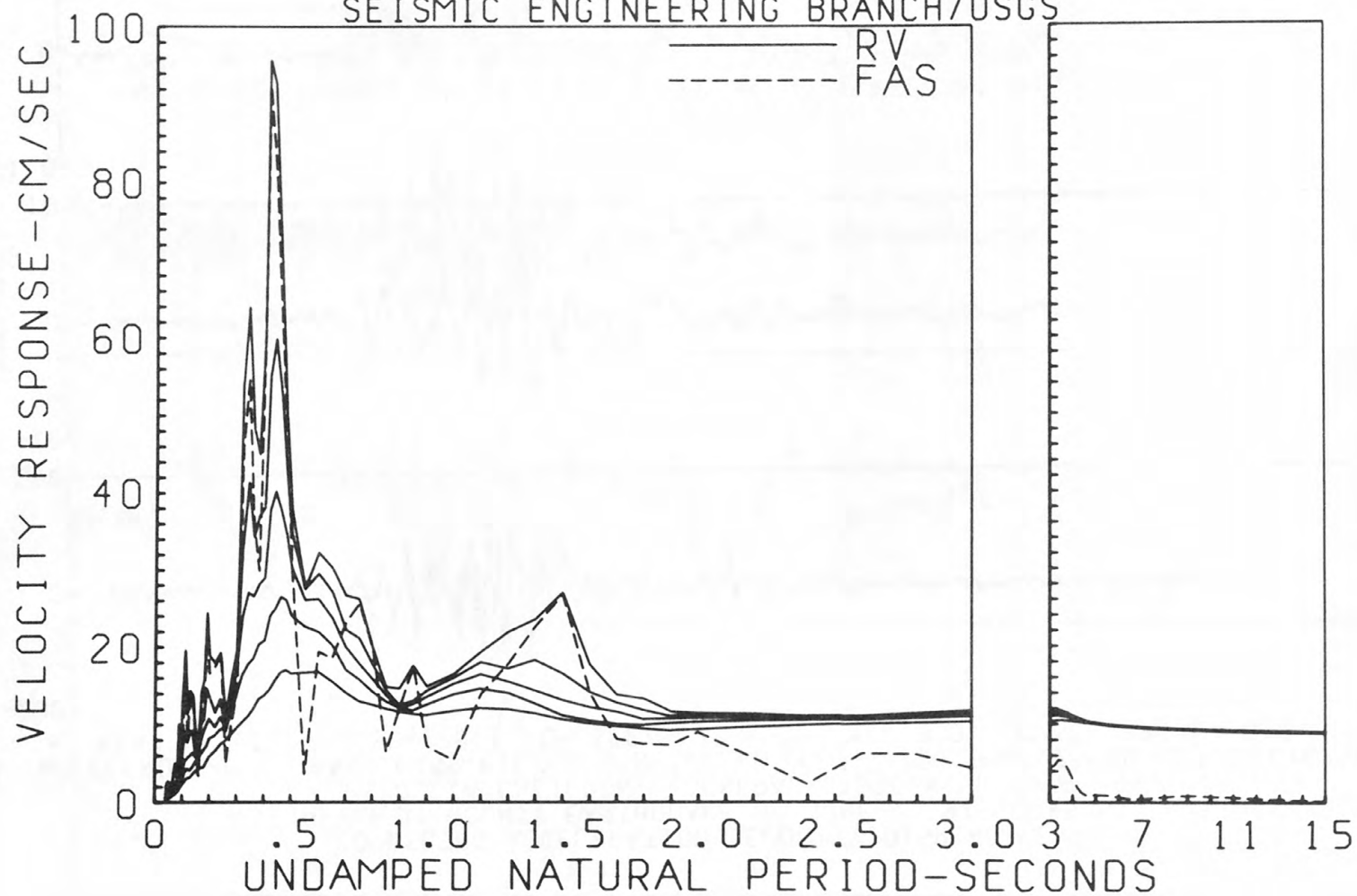




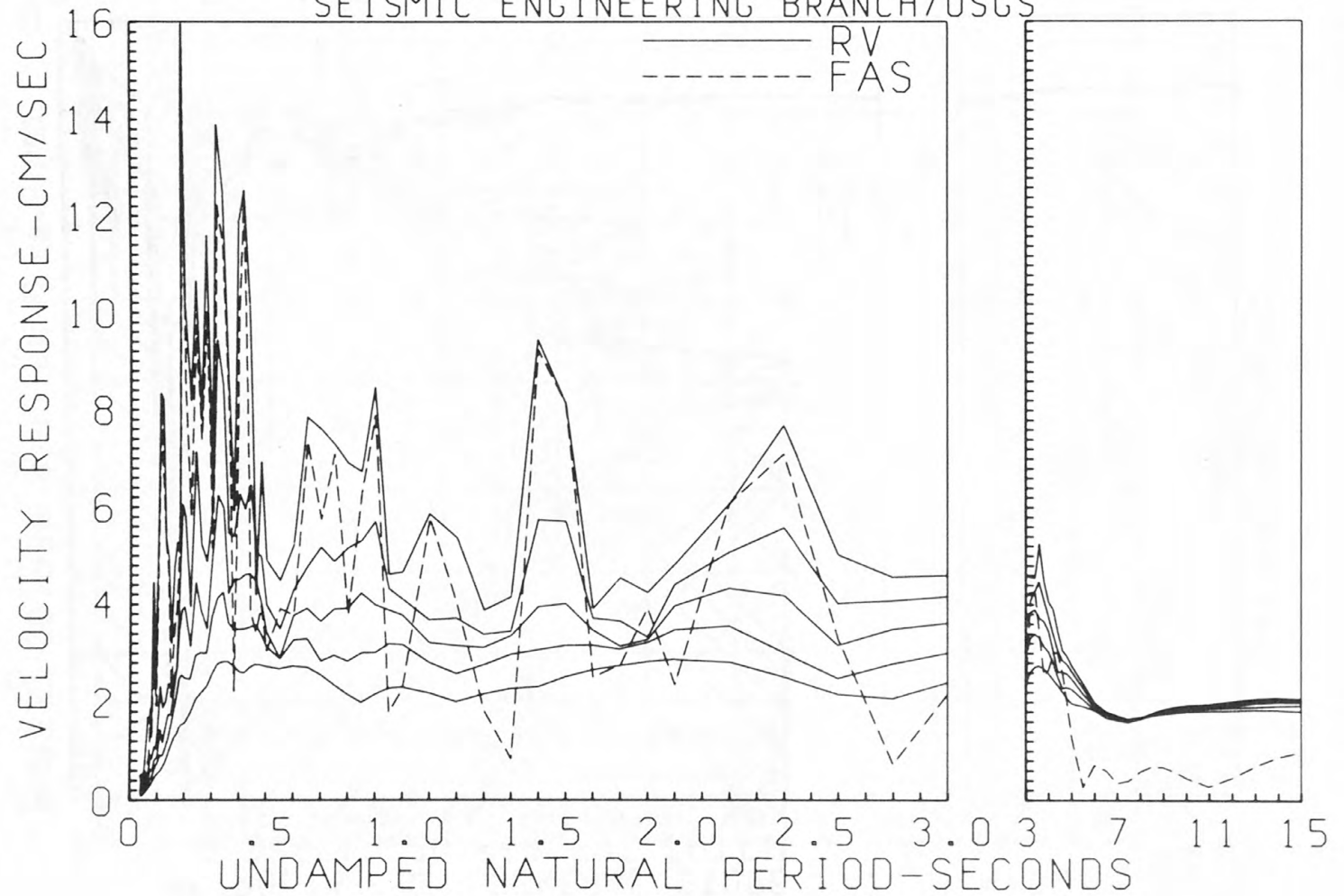




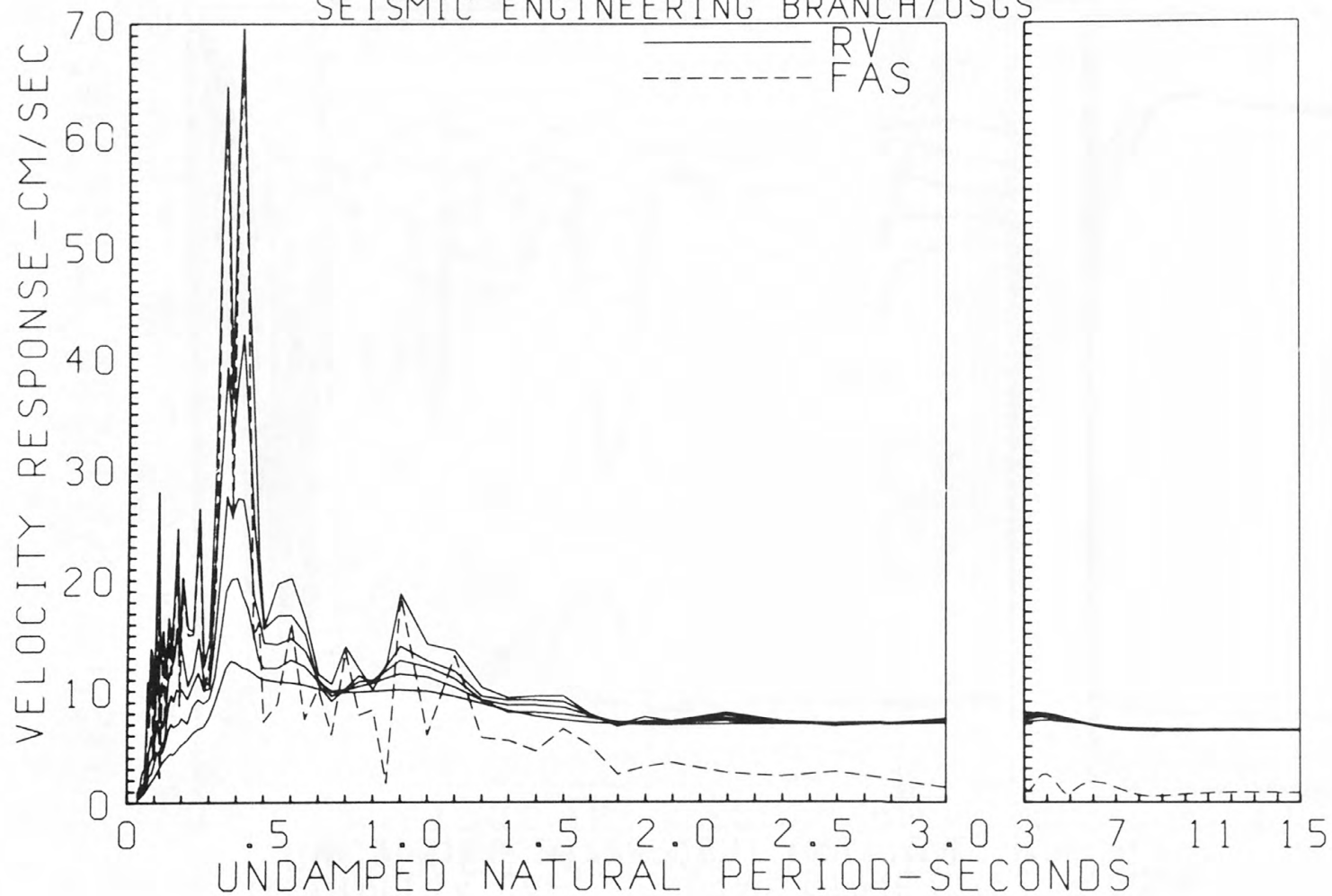
RELATIVE VELOCITY RESPONSE SPECTRUM
PETROLIA,CAL.,GENERAL STORE,6/7/75,N75E
0,2,5,10,20 PERCENT CRITICAL DAMPING
SEISMIC ENGINEERING BRANCH/USGS



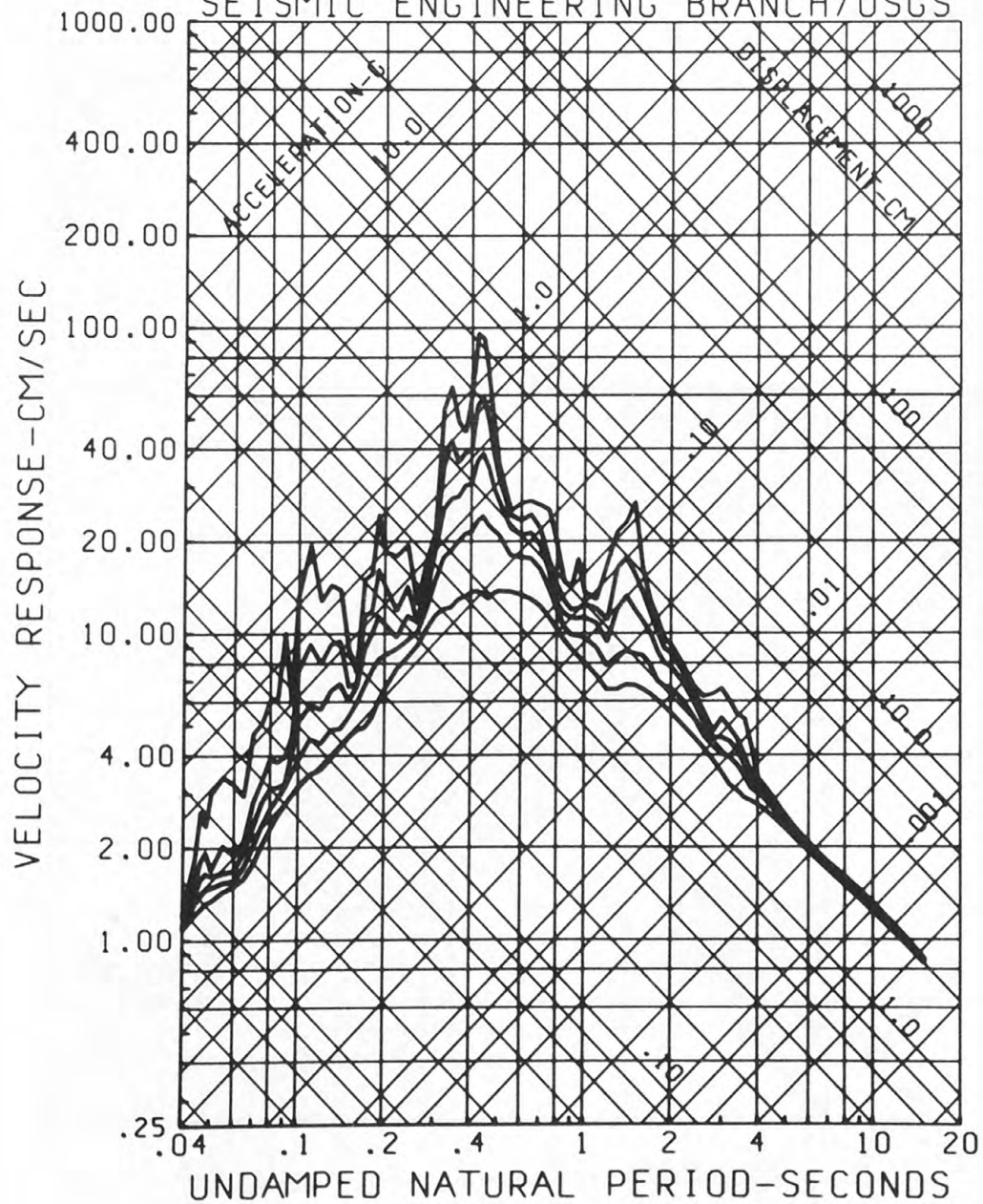
RELATIVE VELOCITY RESPONSE SPECTRUM
 PETROLIA,CAL.,GENERAL STORE,6/7/75,DOWN
 0,2,5,10,20 PERCENT CRITICAL DAMPING
 SEISMIC ENGINEERING BRANCH/USGS



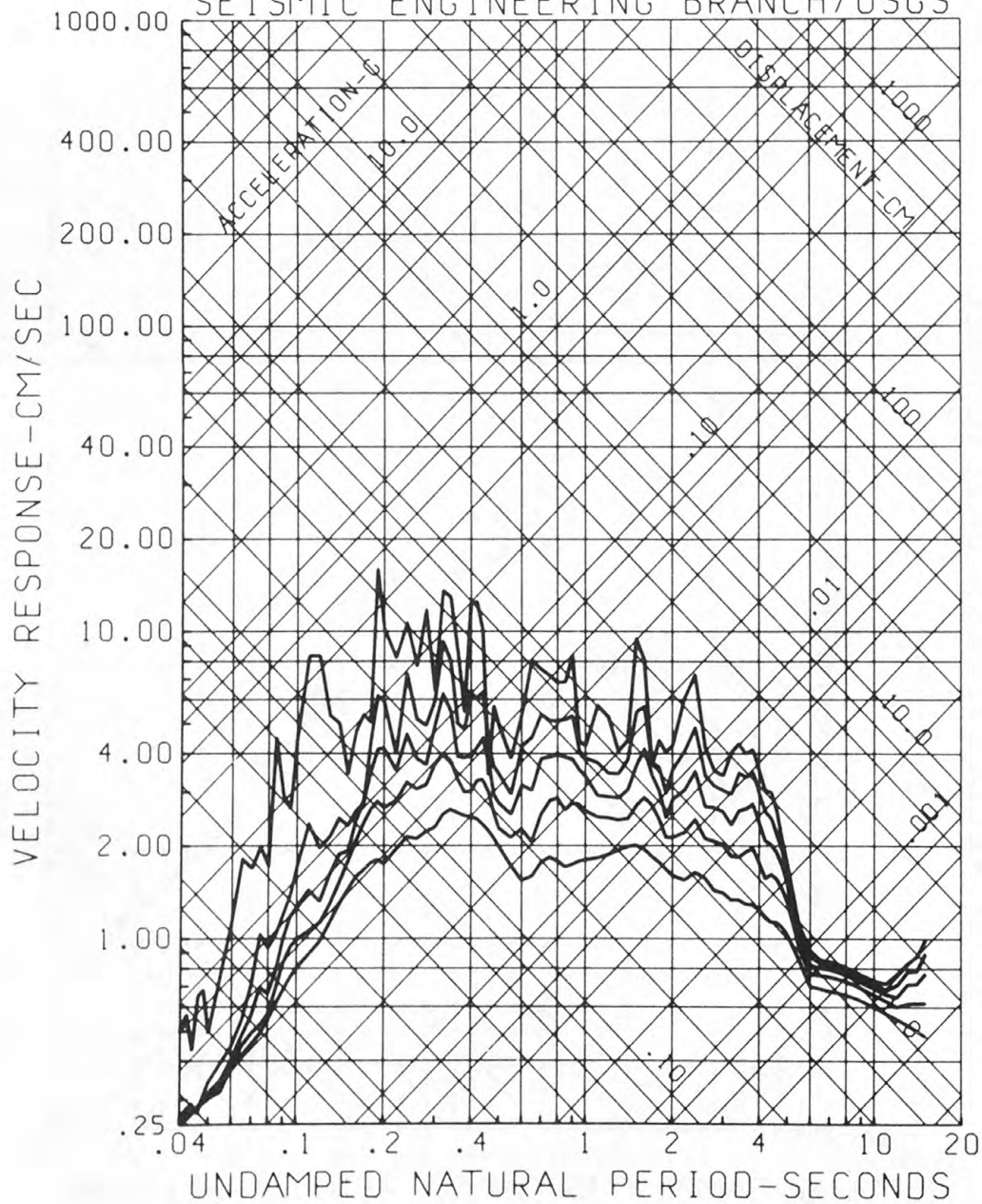
RELATIVE VELOCITY RESPONSE SPECTRUM
 PETROLIA,CAL.,GENERAL STORE,6/7/75,N15W
 0.2,5,10,20 PERCENT CRITICAL DAMPING
 SEISMIC ENGINEERING BRANCH/USGS

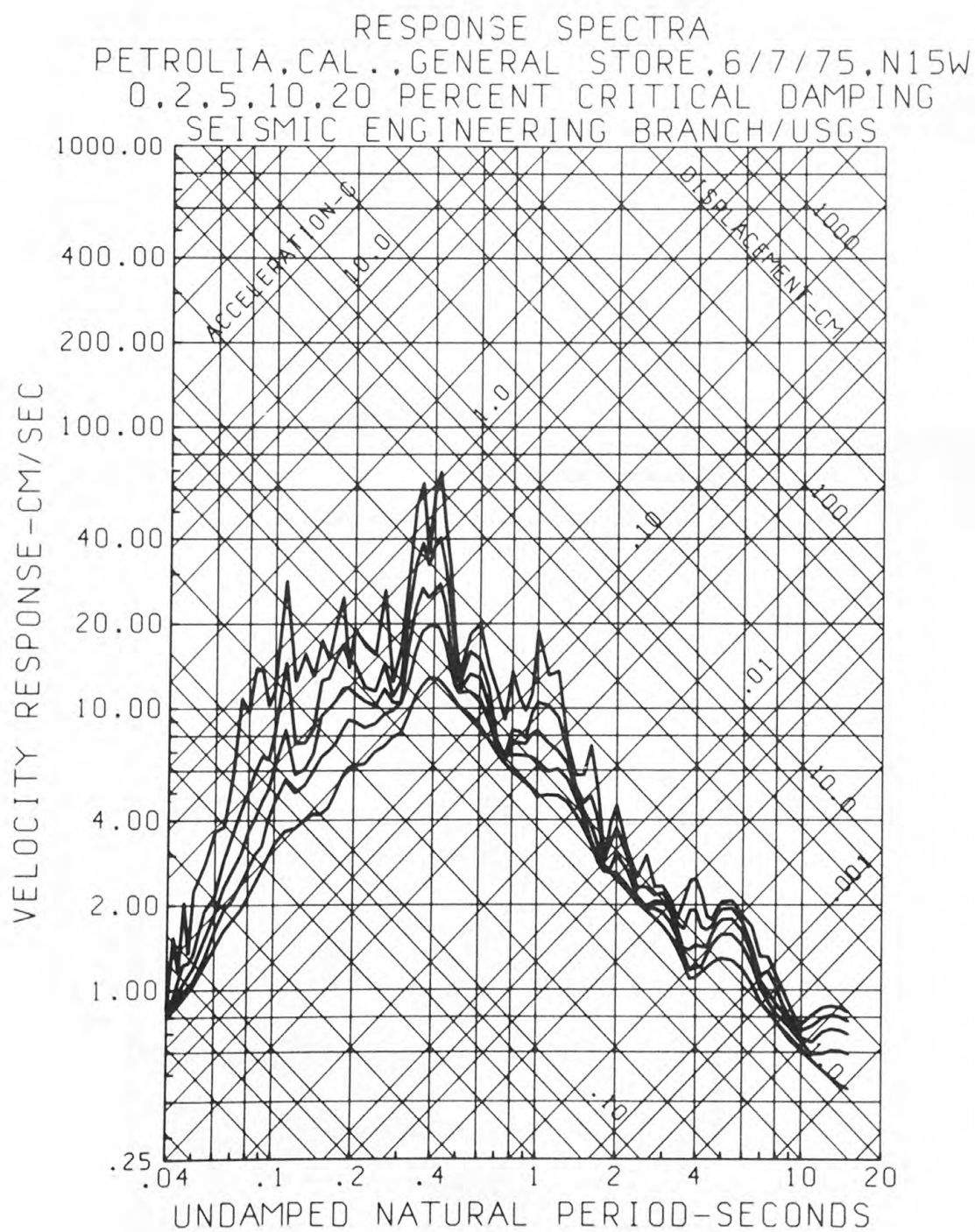


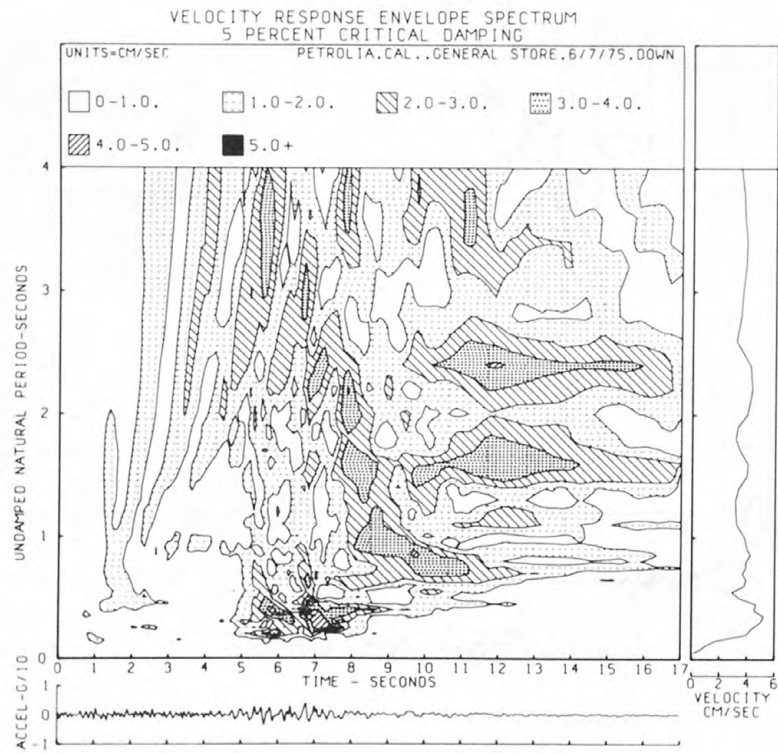
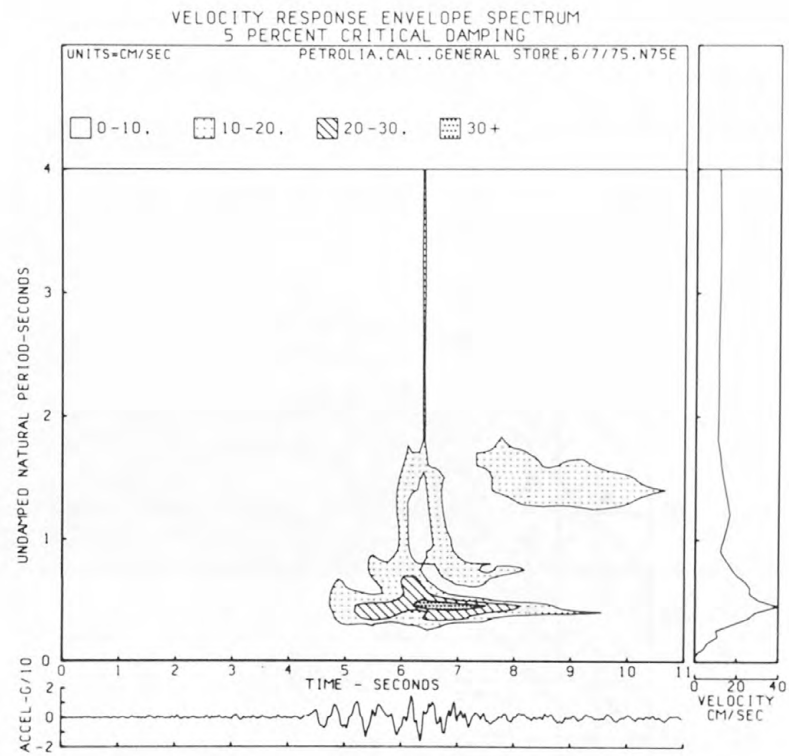
RESPONSE SPECTRA
 PETROLIA, CAL., GENERAL STORE, 6/7/75, N75E
 0.2, 5, 10, 20 PERCENT CRITICAL DAMPING
 SEISMIC ENGINEERING BRANCH/USGS

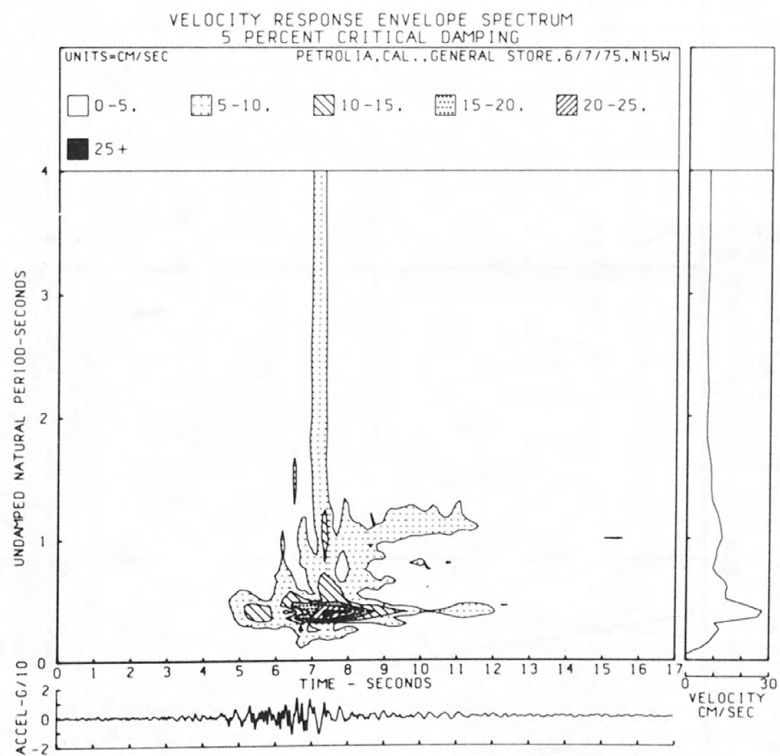


RESPONSE SPECTRA
 PETROLIA.CAL.,GENERAL STORE.6/7/75.DOWN
 0.2,5,10,20 PERCENT CRITICAL DAMPING
 SEISMIC ENGINEERING BRANCH/USGS

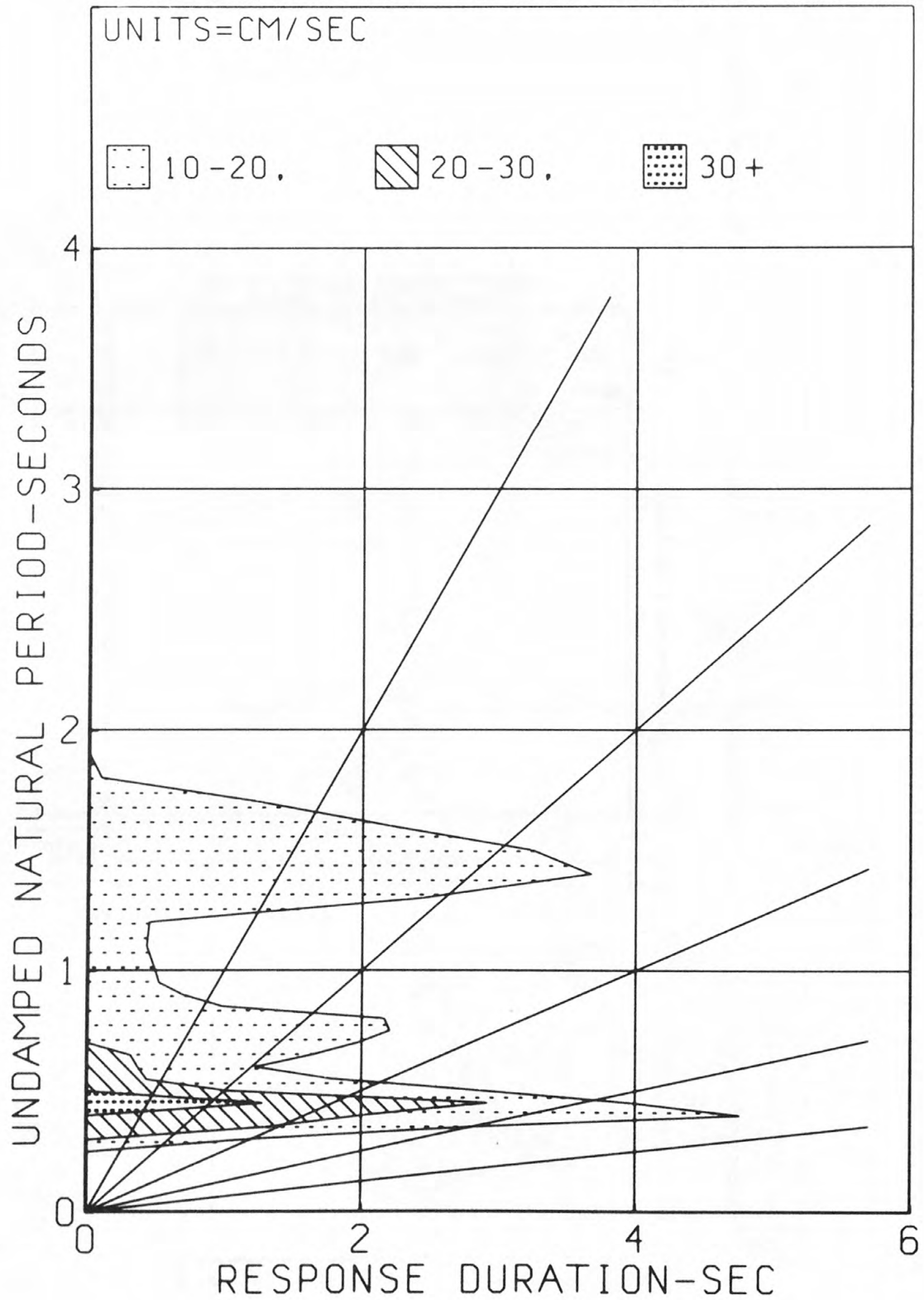




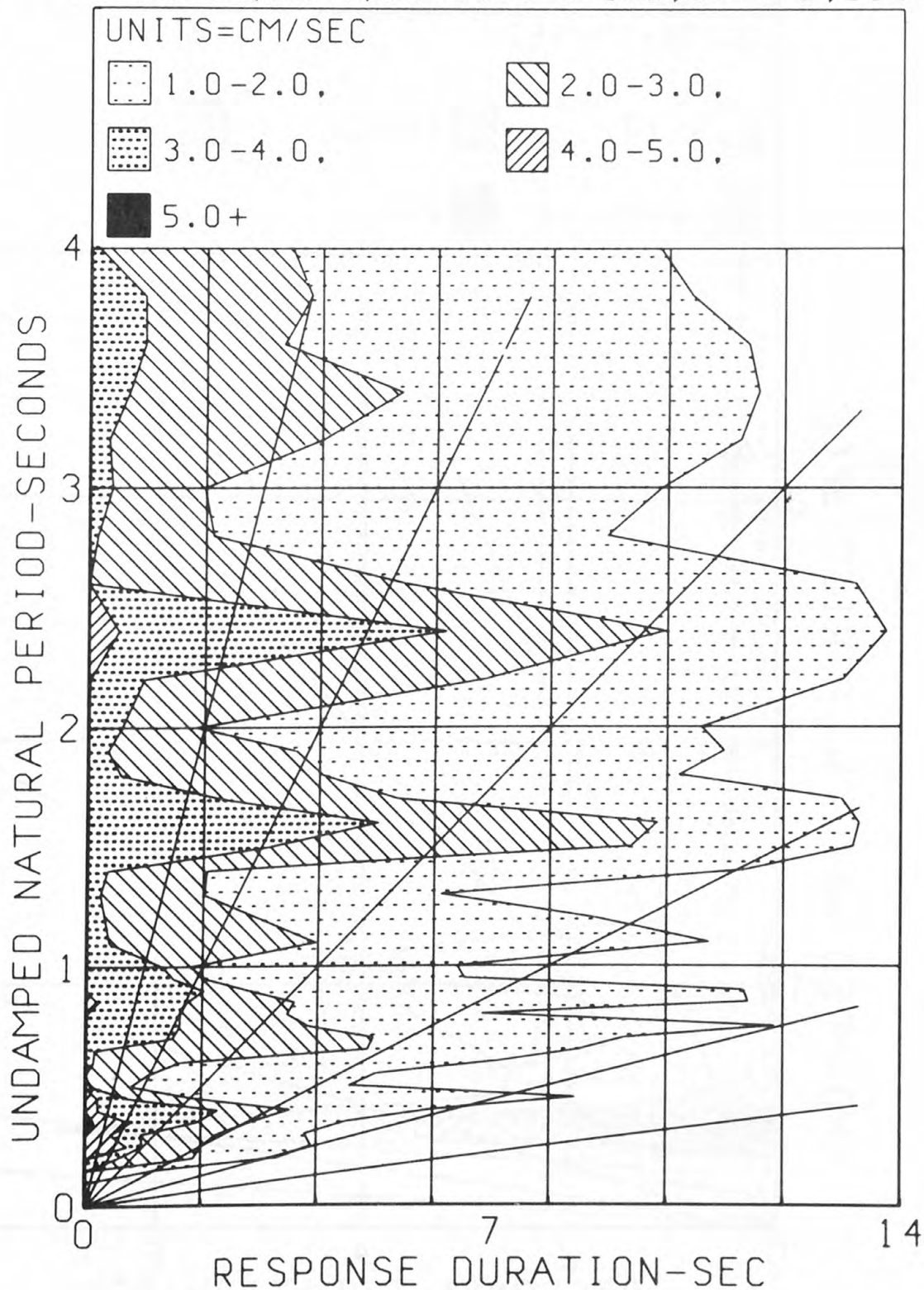




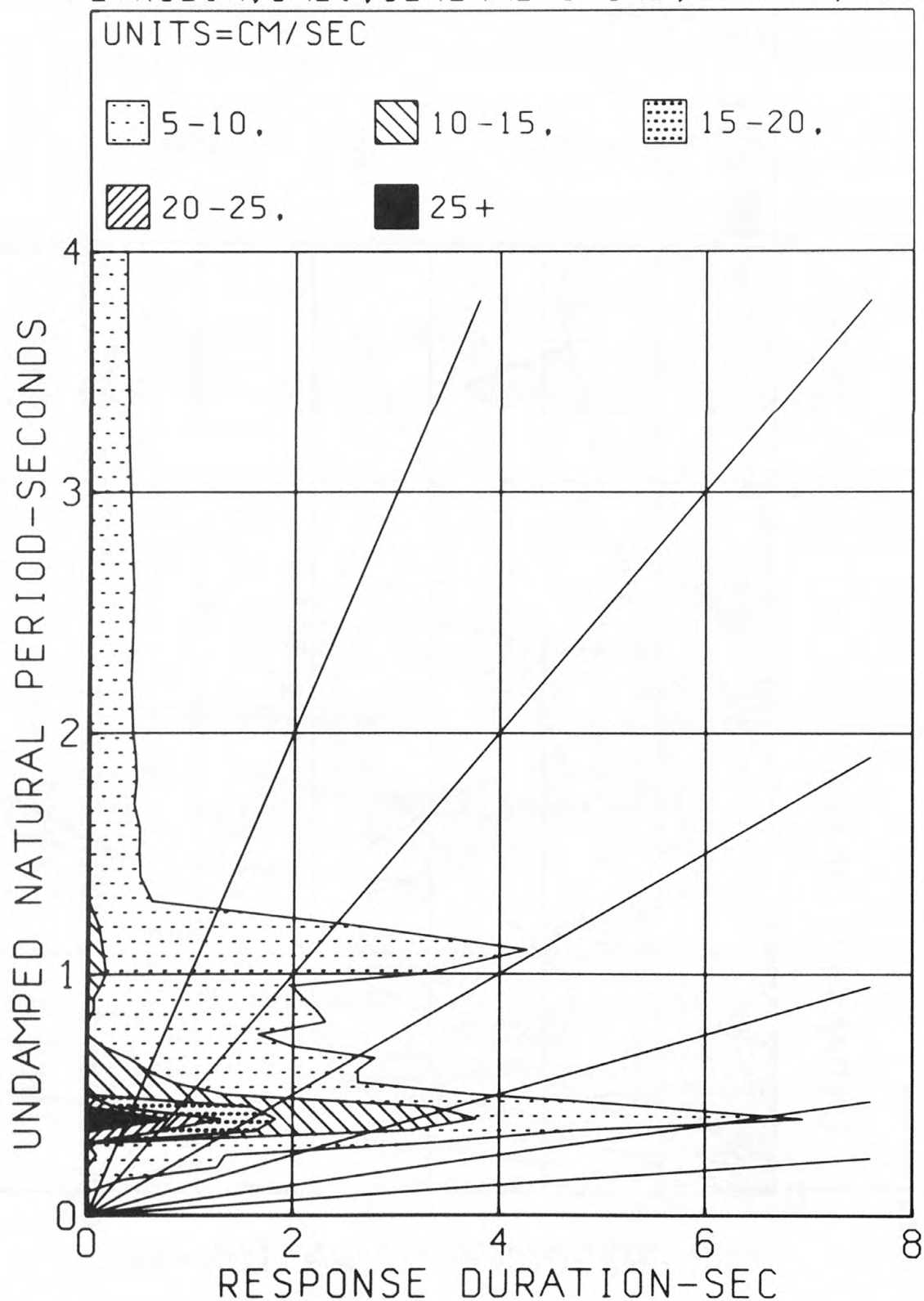
DURATION SPECTRUM OF THE VELOCITY
 RESPONSE ENVELOPE, 5 PERCENT DAMPING
 PETROLIA, CAL., GENERAL STORE, 6/7/75, N75E

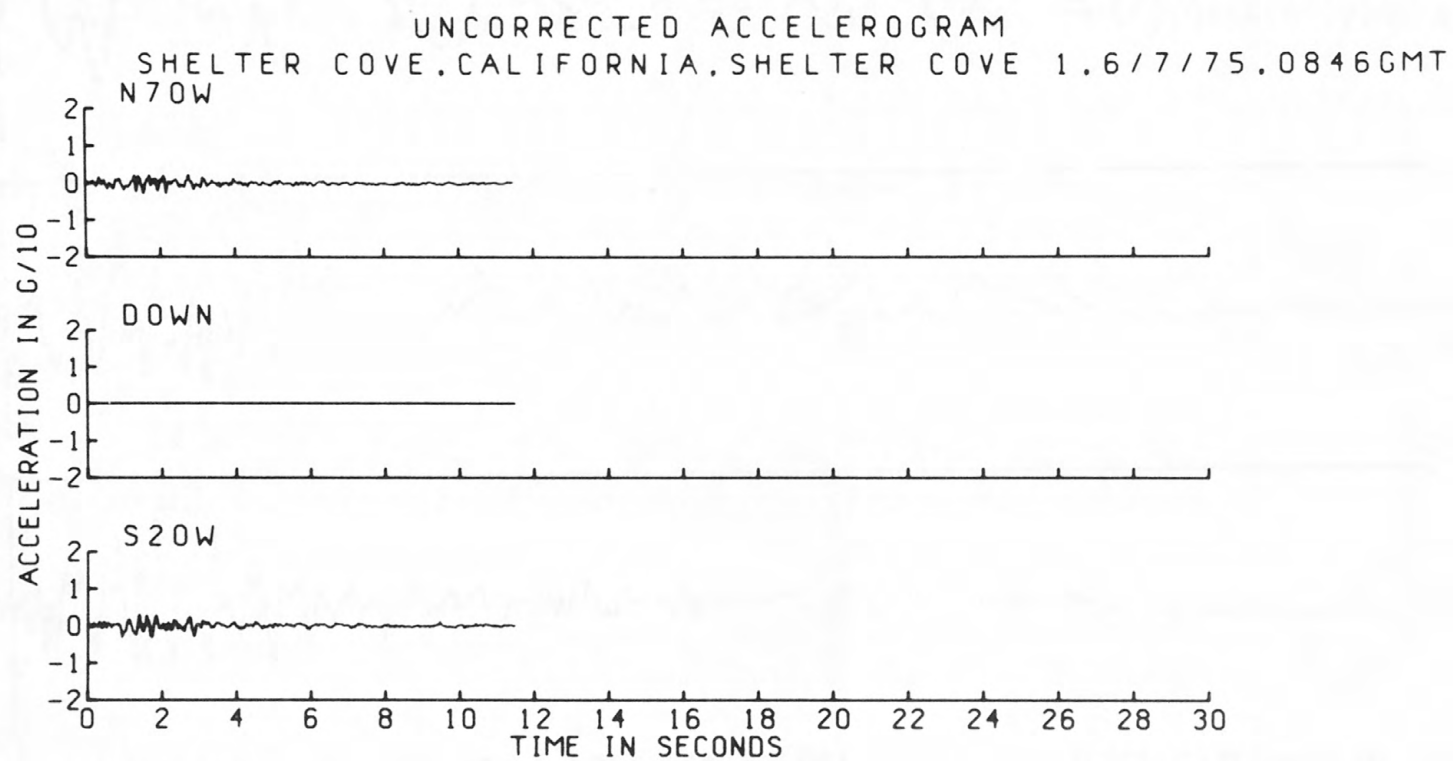


DURATION SPECTRUM OF THE VELOCITY
 RESPONSE ENVELOPE, 5 PERCENT DAMPING
 PETROLIA, CAL., GENERAL STORE, 6/7/75, DOWN



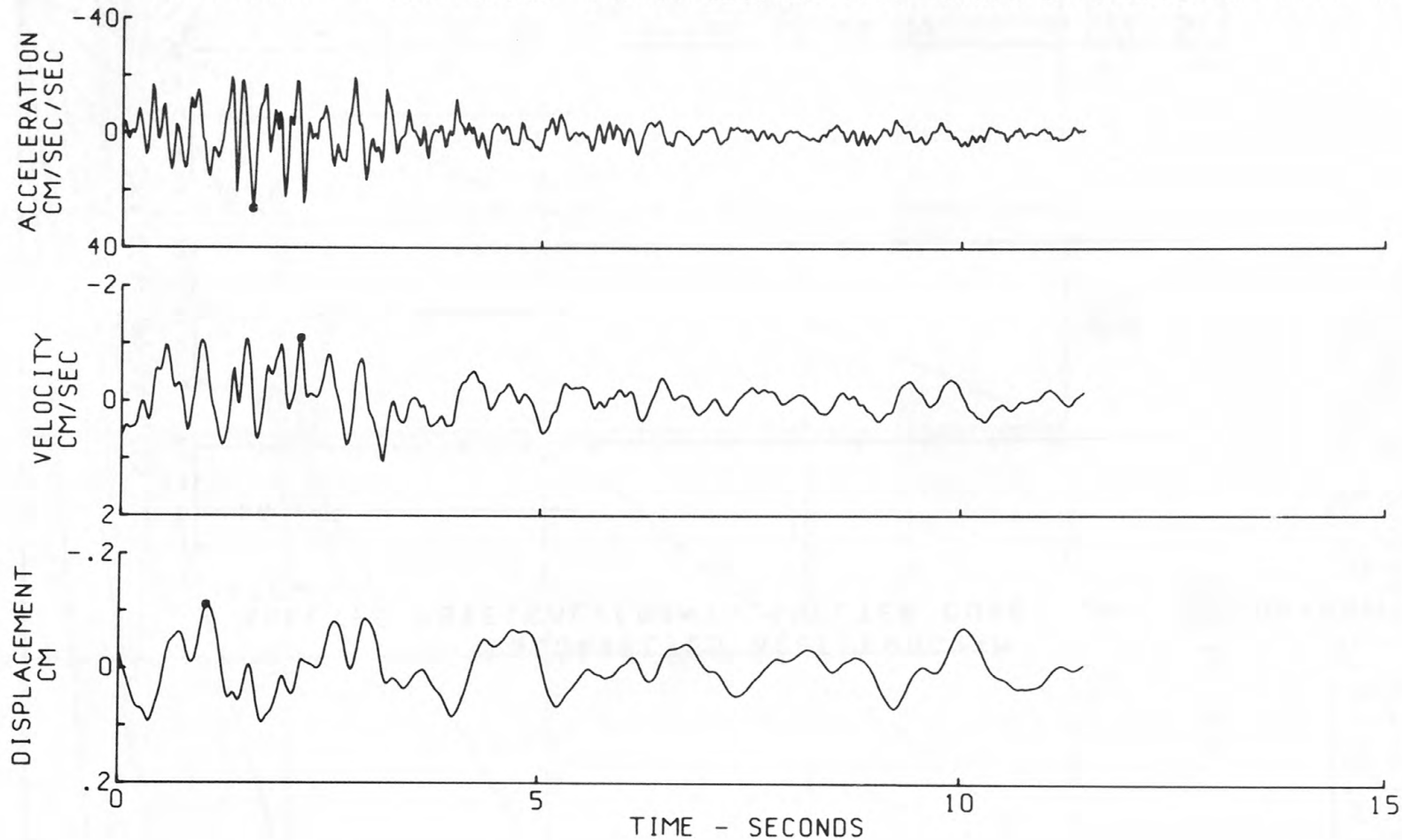
DURATION SPECTRUM OF THE VELOCITY
RESPONSE ENVELOPE, 5 PERCENT DAMPING
PETROLIA, CAL., GENERAL STORE, 6/7/75, N15W





CORRECTED ACCELERATION, VELOCITY, DISPLACEMENT
HUMBOLDT COUNTY EARTHQUAKE OF JUNE 7, 1975, 0846GMT
SHELTER COVE, CALIFORNIA, SHELTER COVE 1.N70W COMP

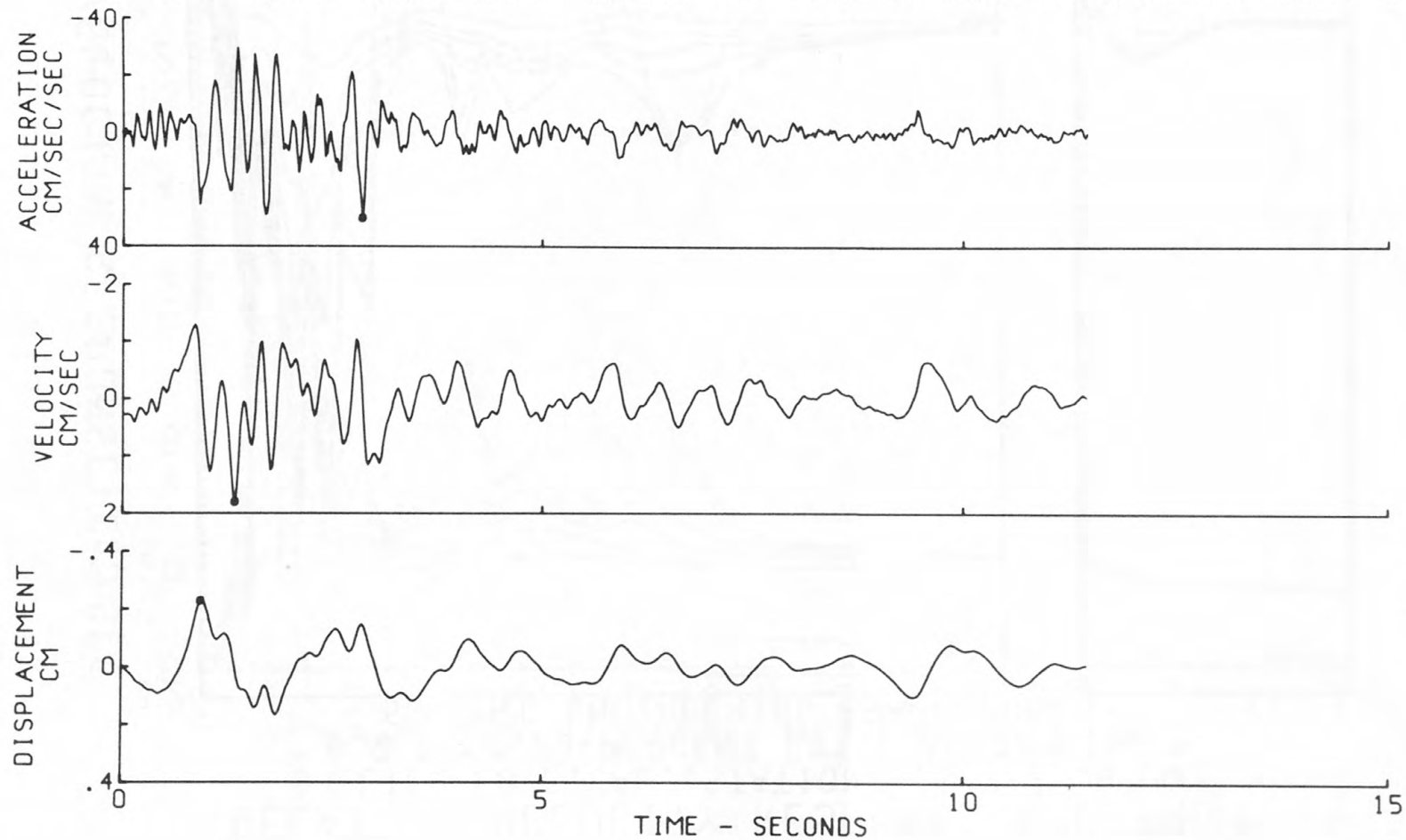
ACCELEROGRAM IS BAND PASS FILTERED BETWEEN .350 - .500 AND 25.00 - 27.00 CYC/SEC
• PEAK VALUES ACCEL=26.26 CM/SEC/SEC, VELOCITY=-1.100 CM/SEC, DISPL=-.110 CM

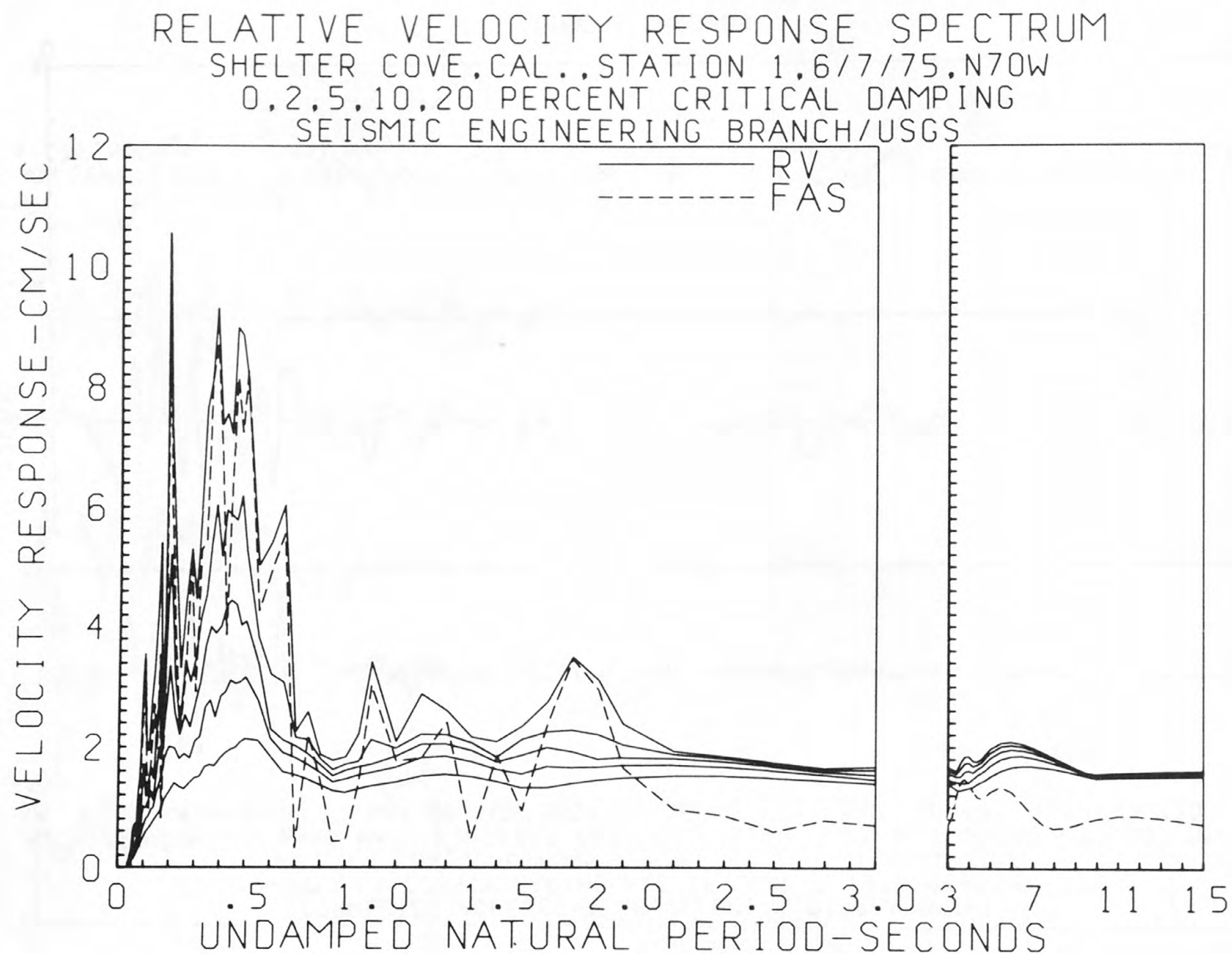


CORRECTED ACCELERATION, VELOCITY, DISPLACEMENT
HUMBOLDT COUNTY EARTHQUAKE OF JUNE 7, 1975, 0846GMT
SHELTER COVE, CALIFORNIA, SHELTER COVE 1, S20W COMP

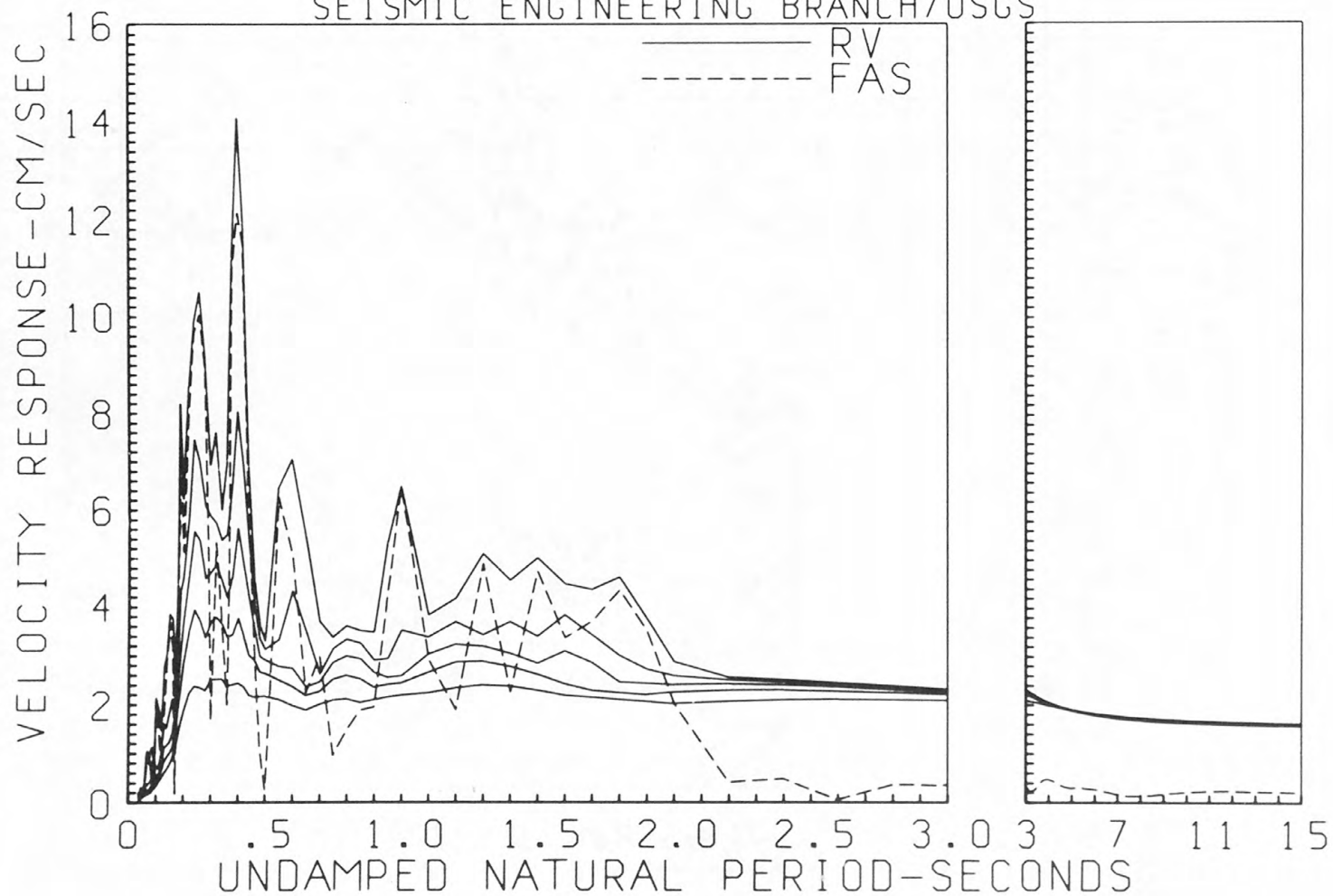
ACCELEROGRAM IS BAND PASS FILTERED BETWEEN .350 - .500 AND 25.00 - 27.00 CYC/SEC

• PEAK VALUES ACCEL=29.96 CM/SEC/SEC, VELOCITY=1.800 CM/SEC, DISPL=-.230 CM

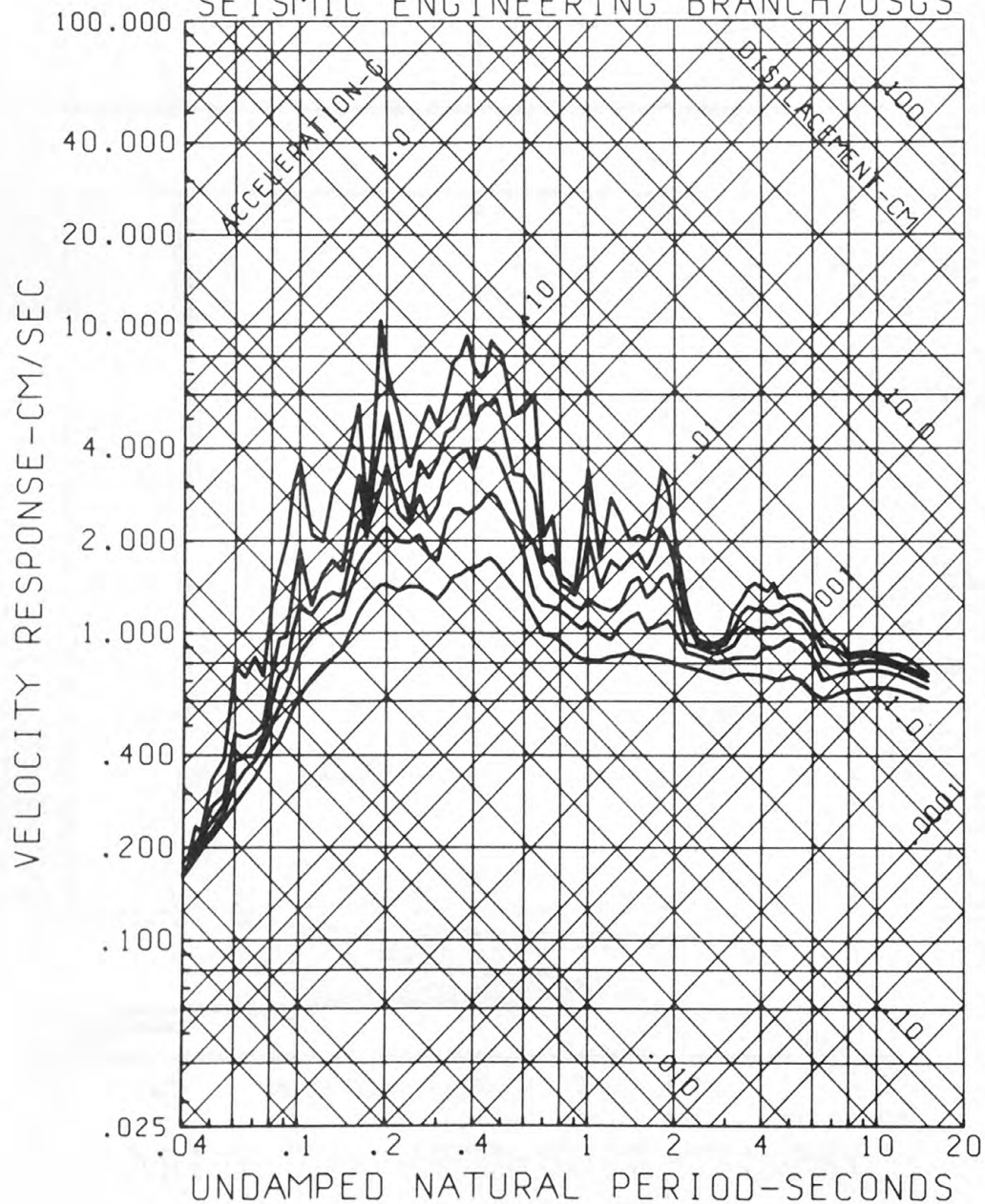




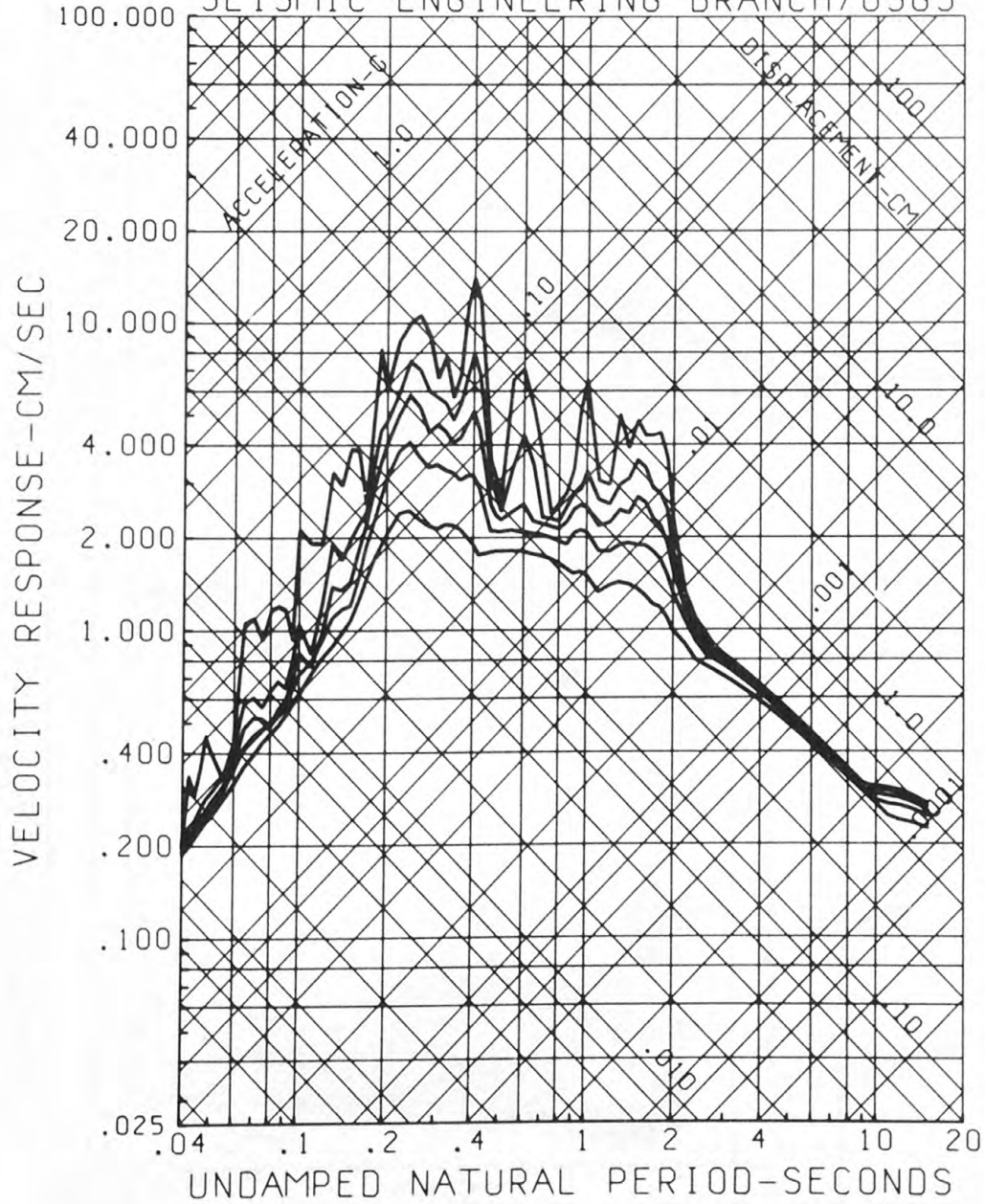
RELATIVE VELOCITY RESPONSE SPECTRUM
 SHELTER COVE, CAL., STATION 1.6/7/75, S20W
 0.2, 5, 10, 20 PERCENT CRITICAL DAMPING
 SEISMIC ENGINEERING BRANCH/USGS

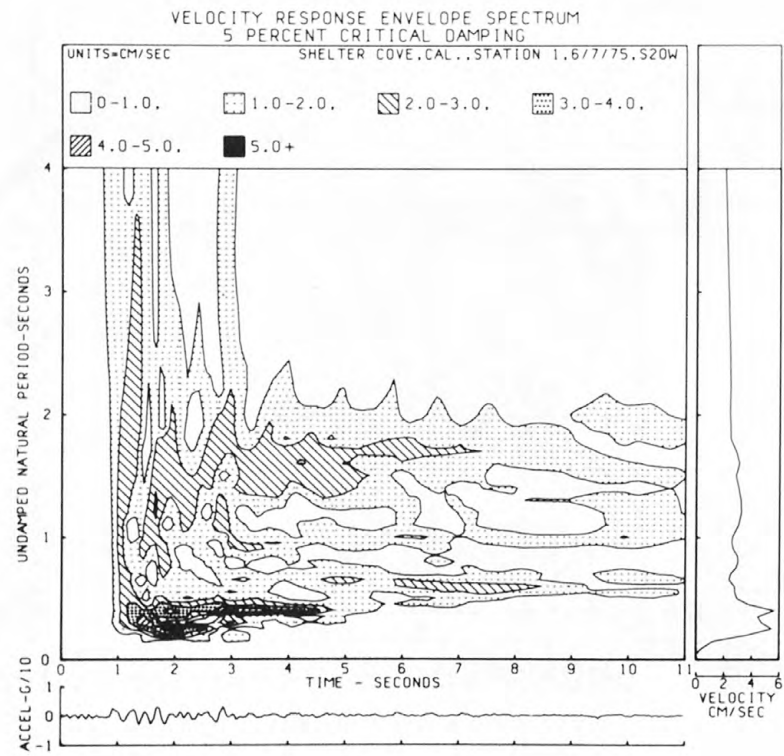
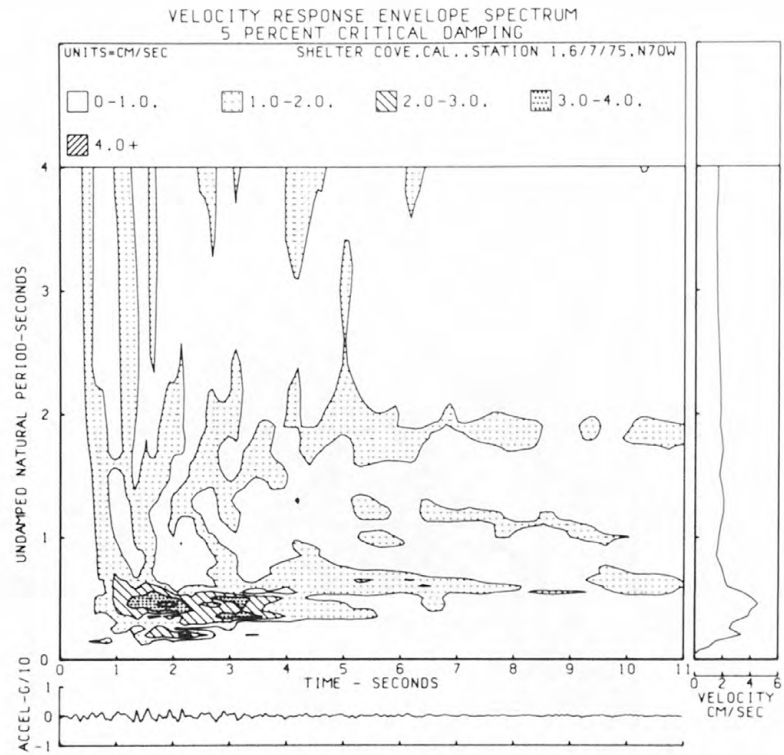


RESPONSE SPECTRA
 SHELTER COVE, CAL., STATION 1.6/7/75, N70W
 0, 2, 5, 10, 20 PERCENT CRITICAL DAMPING
 SEISMIC ENGINEERING BRANCH/USGS

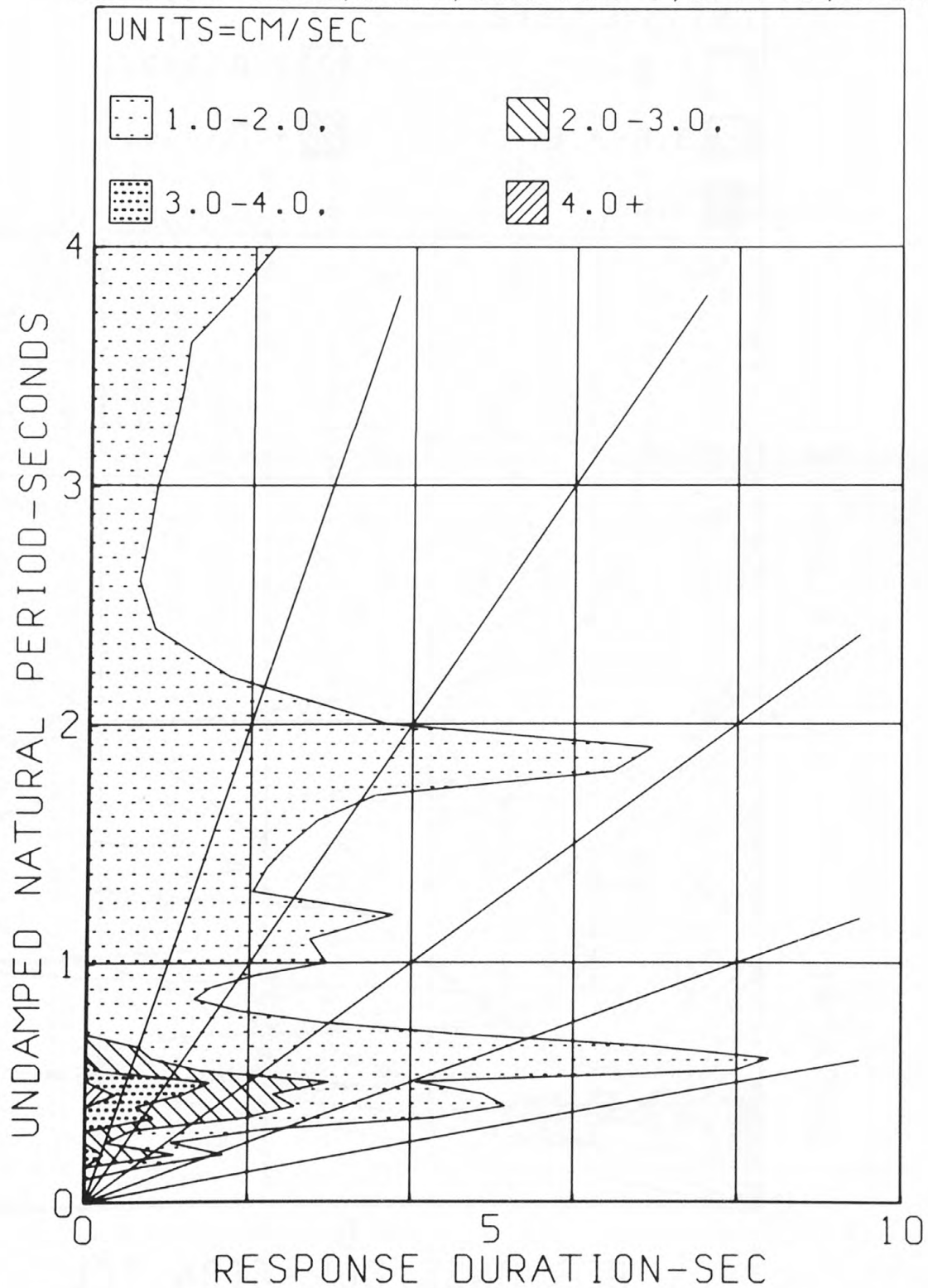


RESPONSE SPECTRA
 SHELTER COVE, CAL., STATION 1.6/7/75, S20W
 0.2, 5, 10, 20 PERCENT CRITICAL DAMPING
 SEISMIC ENGINEERING BRANCH/USGS

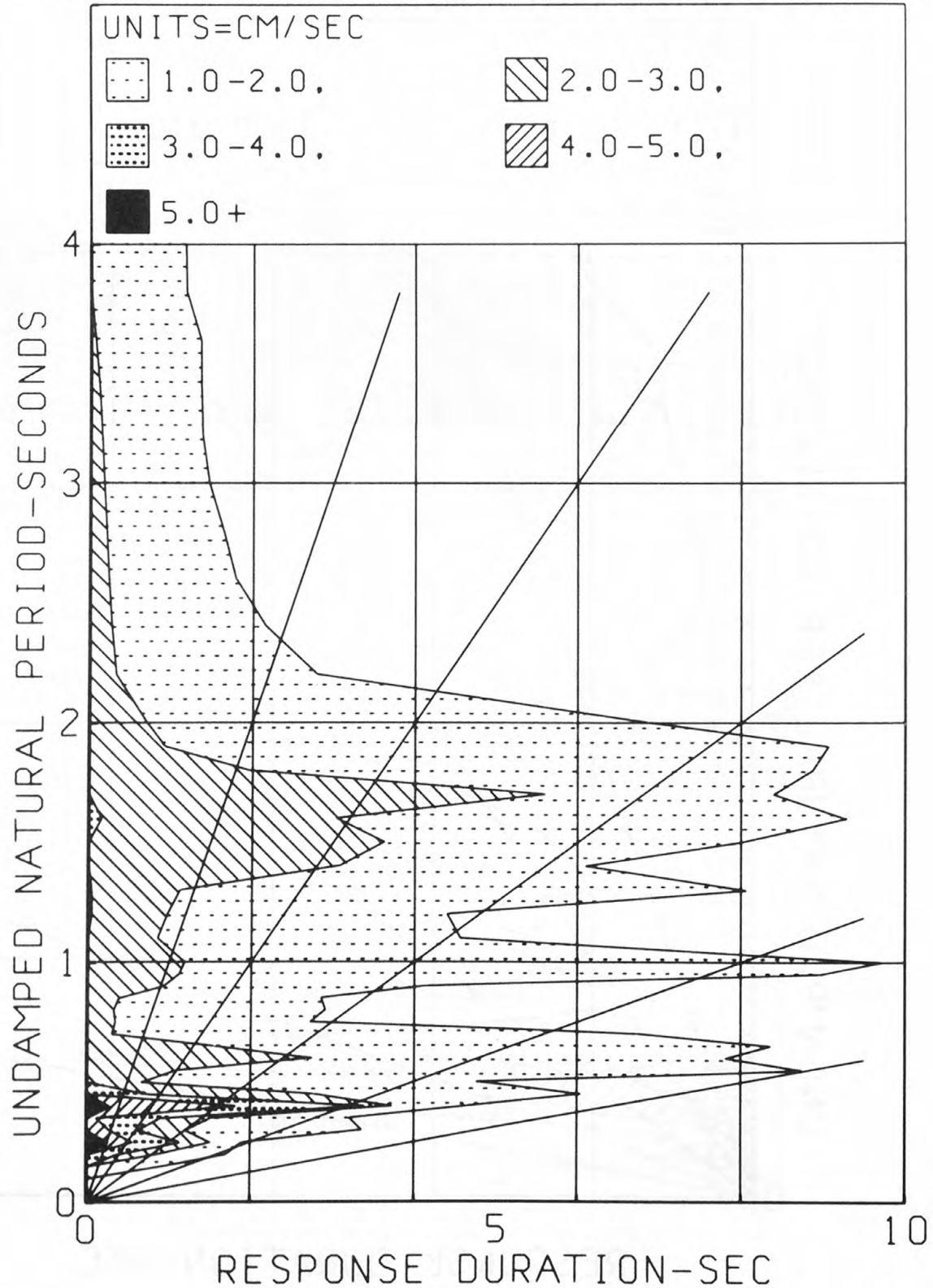


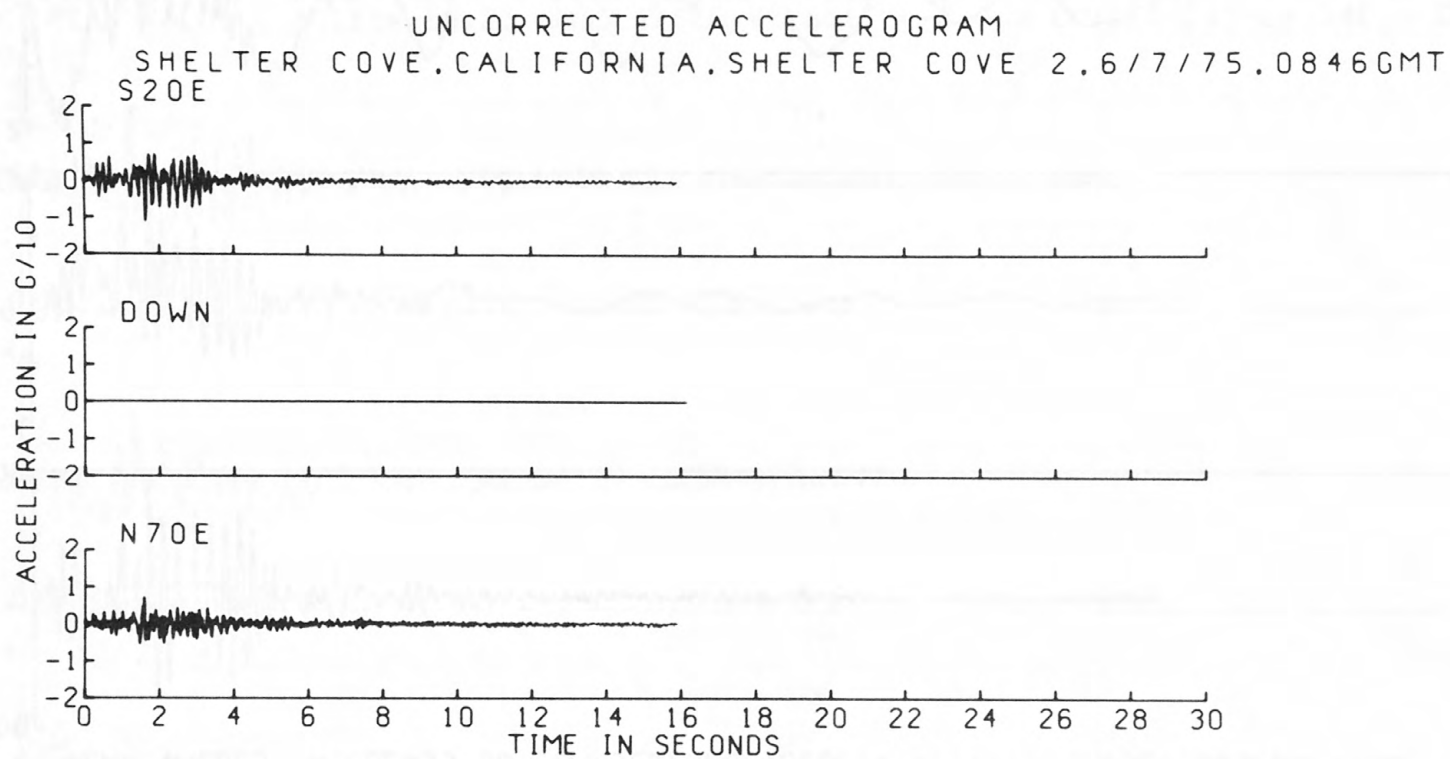


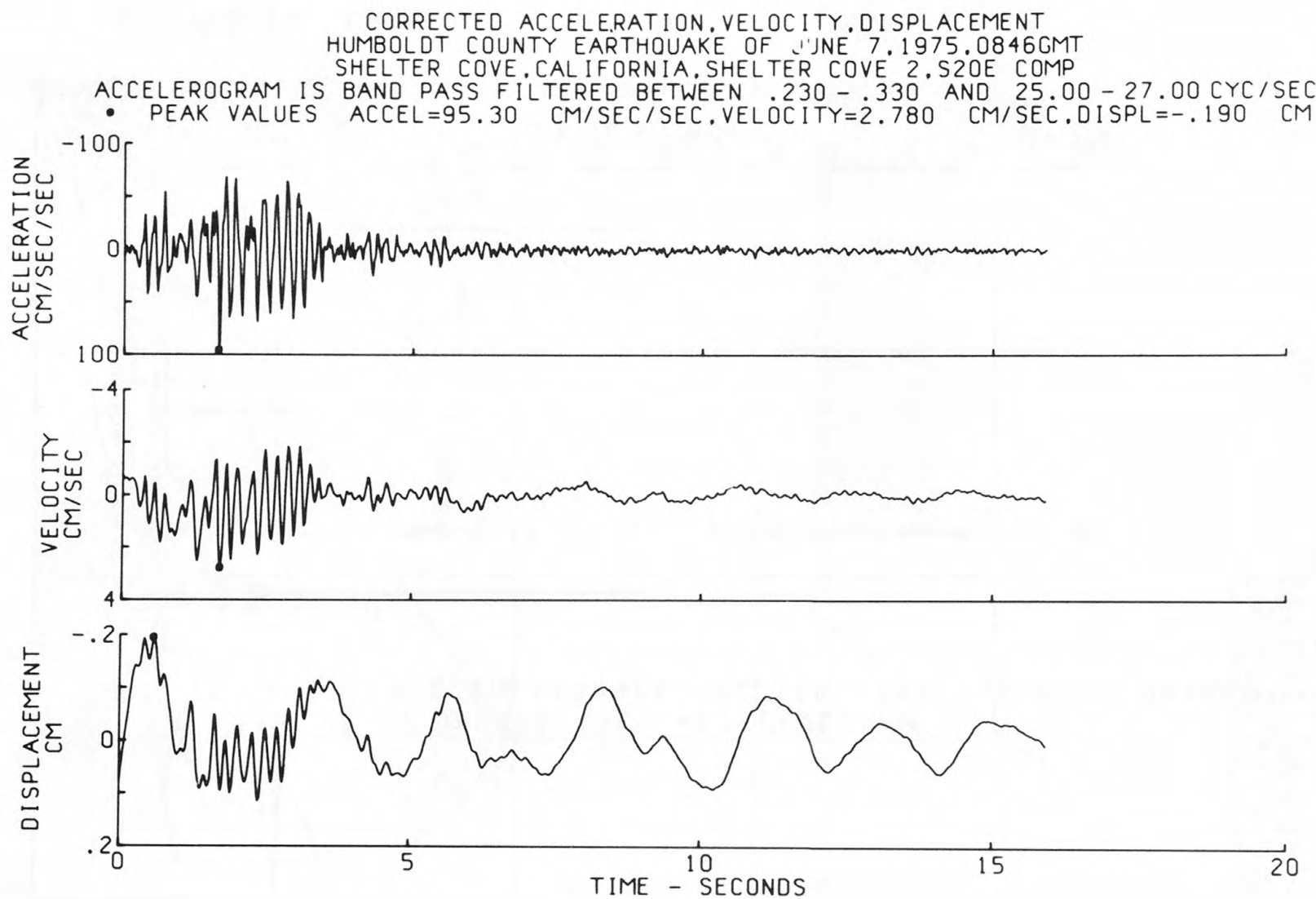
DURATION SPECTRUM OF THE VELOCITY
 RESPONSE ENVELOPE, 5 PERCENT DAMPING
 SHELTER COVE, CAL., STATION 1, 6/7/75, N70W



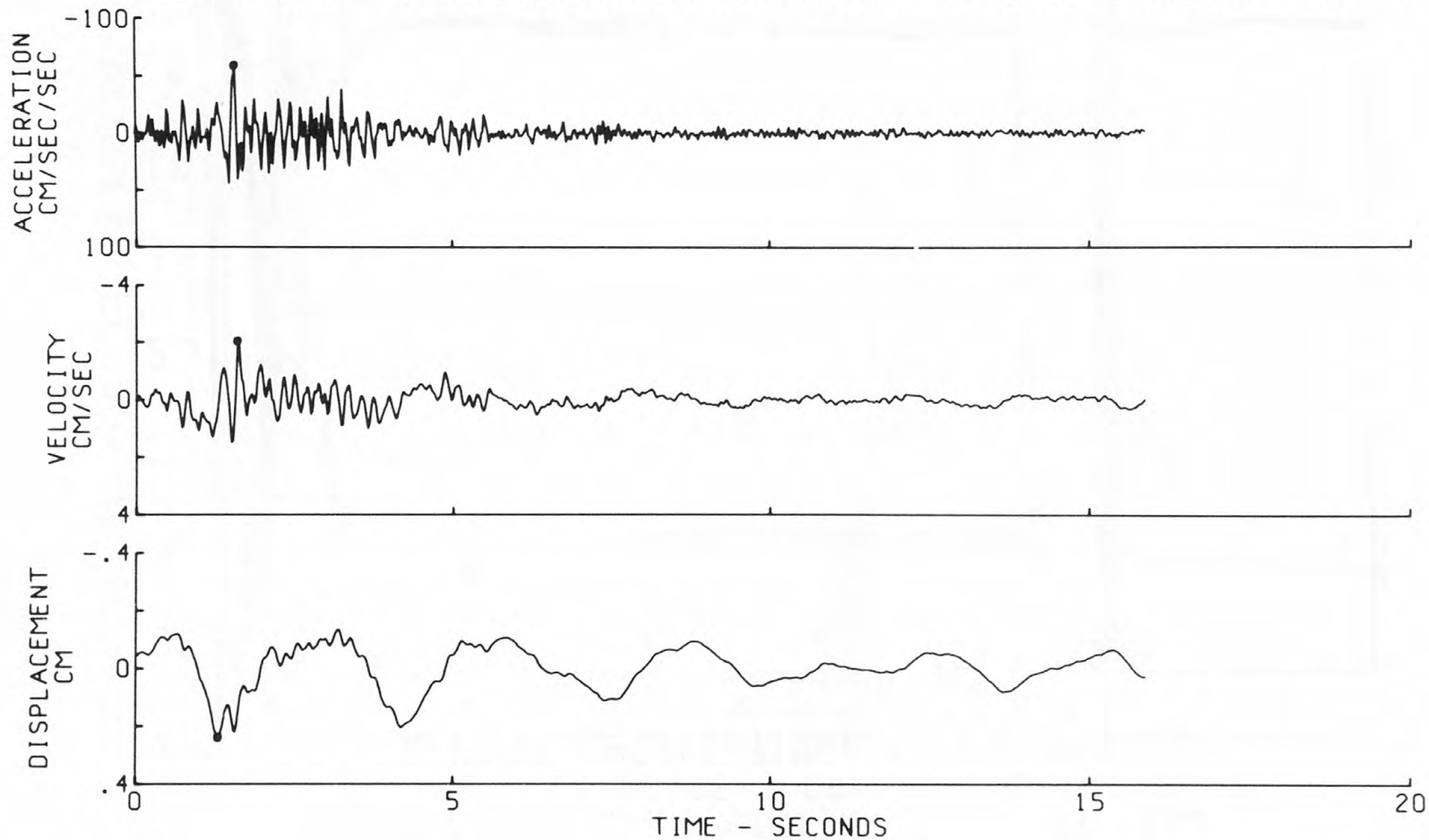
DURATION SPECTRUM OF THE VELOCITY
 RESPONSE ENVELOPE, 5 PERCENT DAMPING
 SHELTER COVE, CAL., STATION 1, 6/7/75, S20W



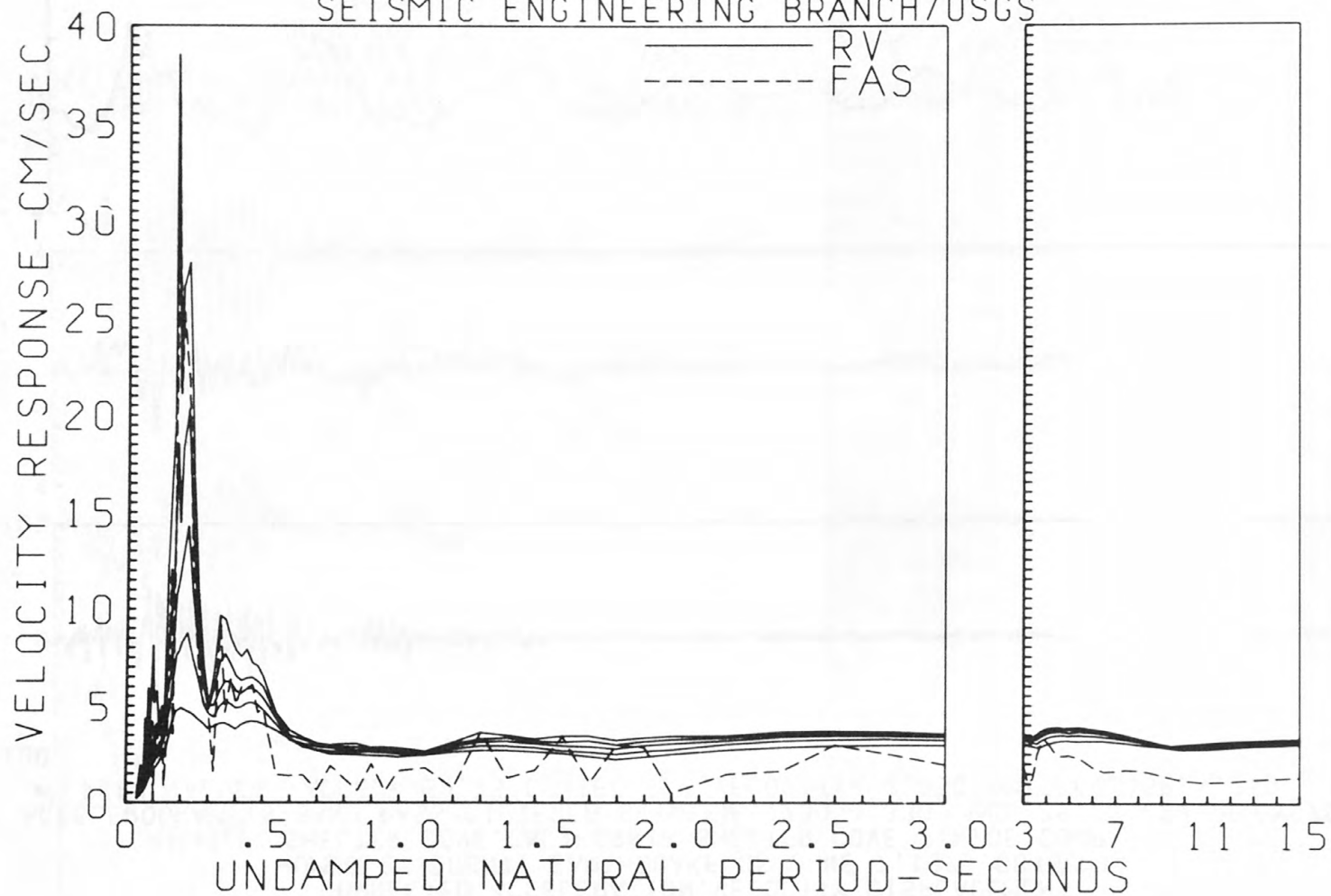




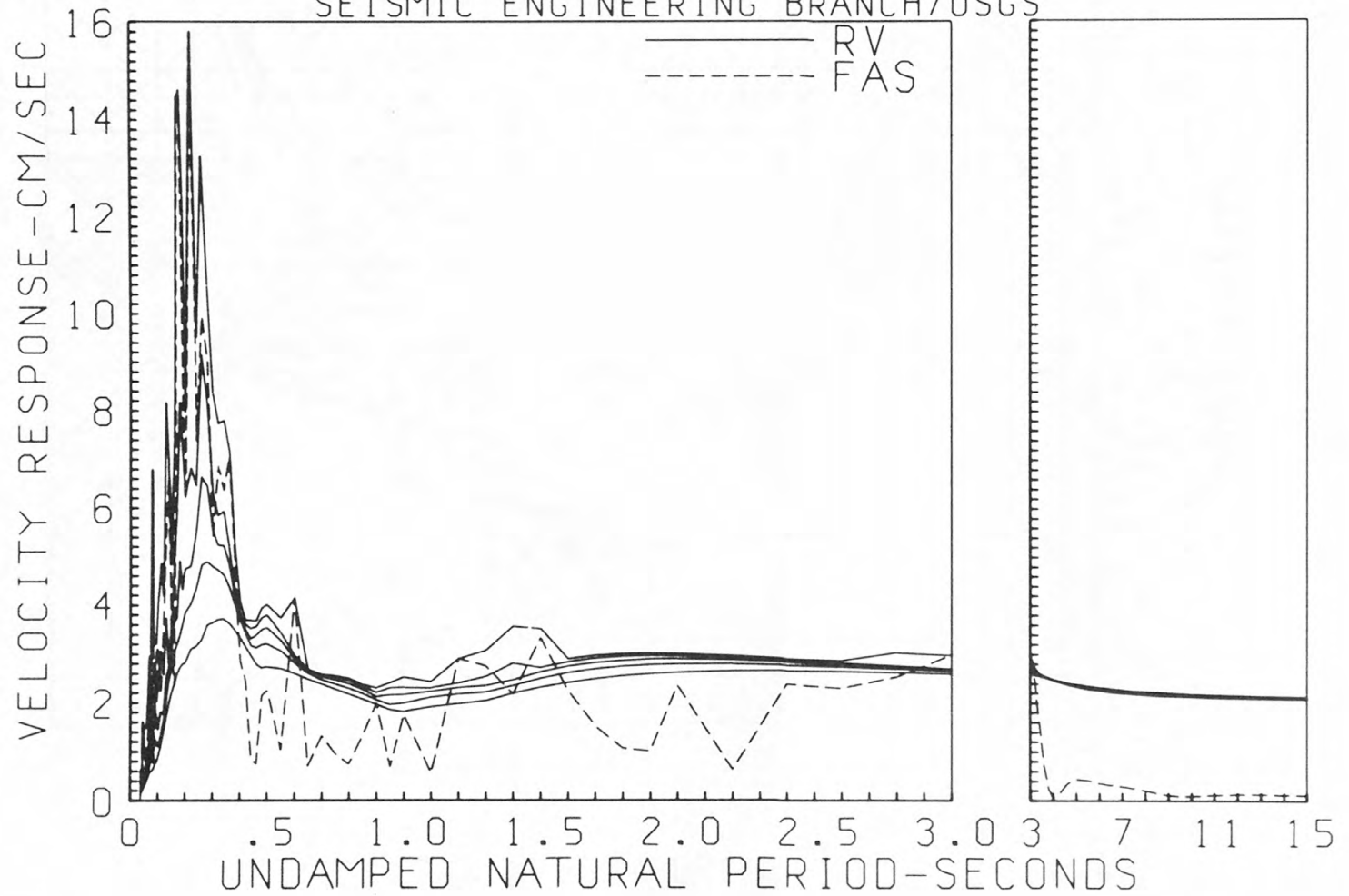
CORRECTED ACCELERATION, VELOCITY, DISPLACEMENT
HUMBOLDT COUNTY EARTHQUAKE OF JUNE 7, 1975, 0846GMT
SHELTER COVE, CALIFORNIA, SHELTER COVE 2.N70E COMP
ACCELEROGRAM IS BAND PASS FILTERED BETWEEN .230 - .330 AND 25.00 - 27.00 CYC/SEC
• PEAK VALUES ACCEL=-59.25 CM/SEC/SEC, VELOCITY=-2.050 CM/SEC, DISPL=.240 CM



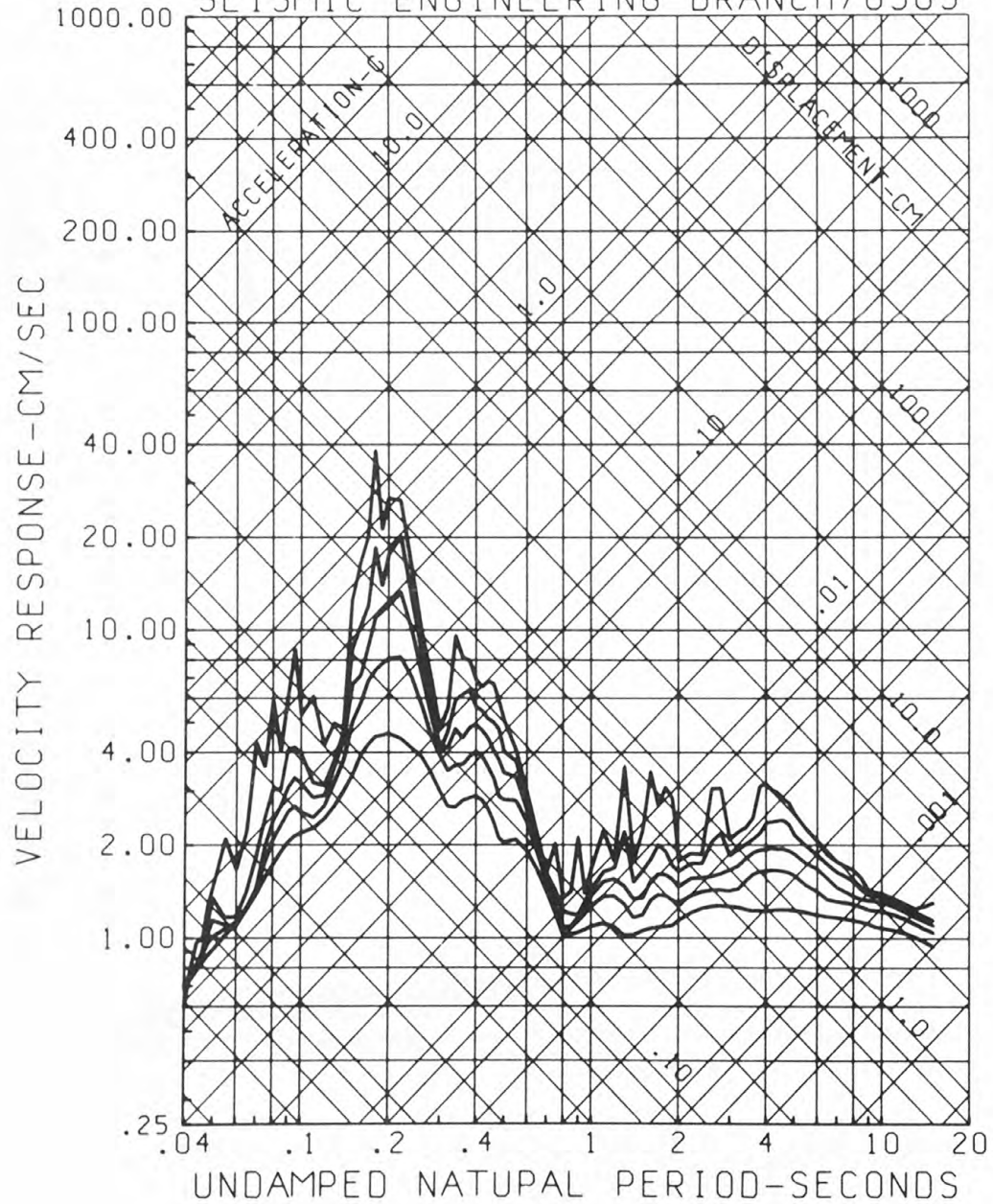
RELATIVE VELOCITY RESPONSE SPECTRUM
SHELTER COVE, CAL., STATION 2, 6/7/75, S20W
0, 2, 5, 10, 20 PERCENT CRITICAL DAMPING
SEISMIC ENGINEERING BRANCH/USGS



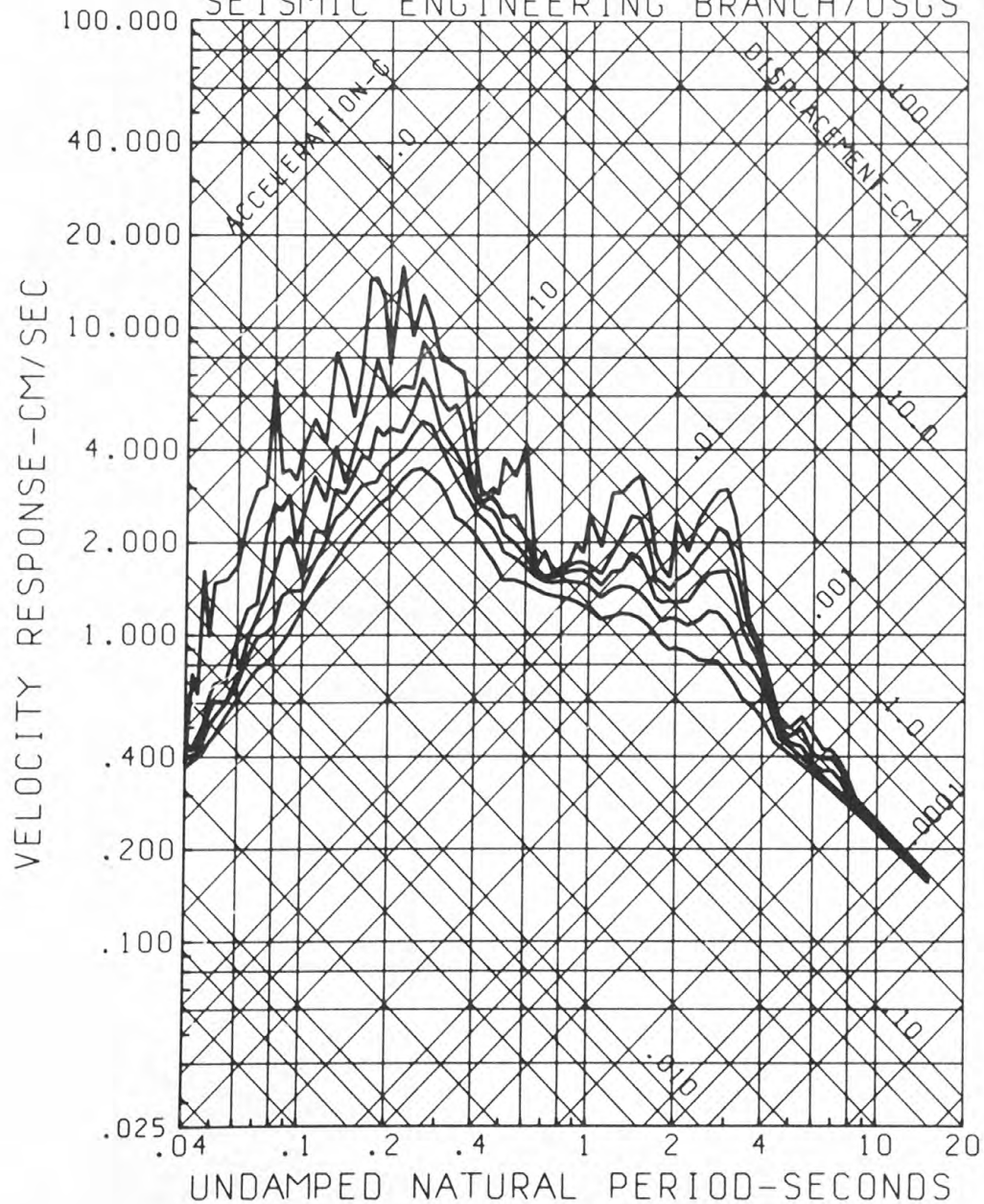
RELATIVE VELOCITY RESPONSE SPECTRUM
 SHELTER COVE, CAL., STATION 2, 6/7/75, N70E
 0.2, 5, 10, 20 PERCENT CRITICAL DAMPING
 SEISMIC ENGINEERING BRANCH/USGS

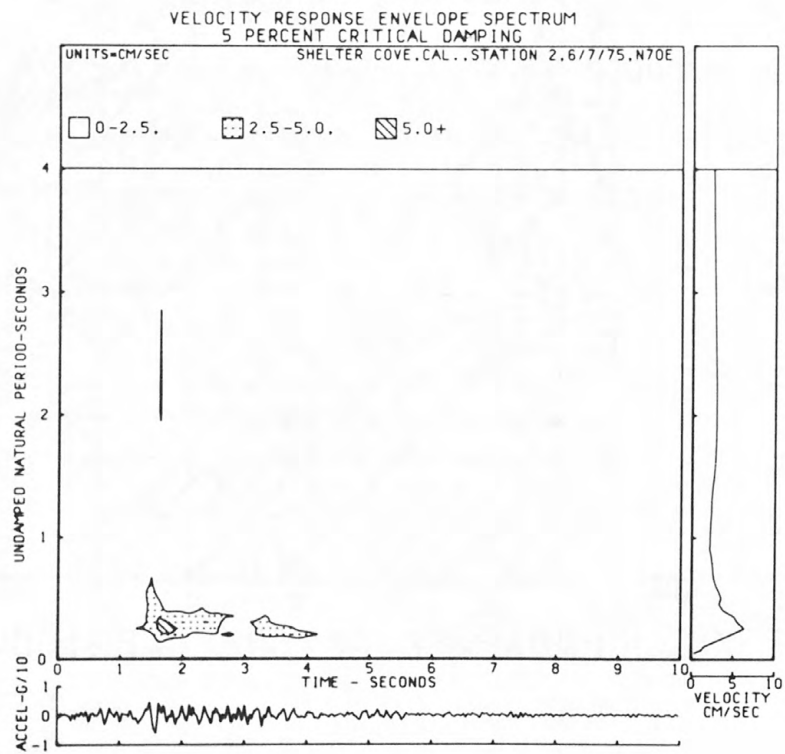
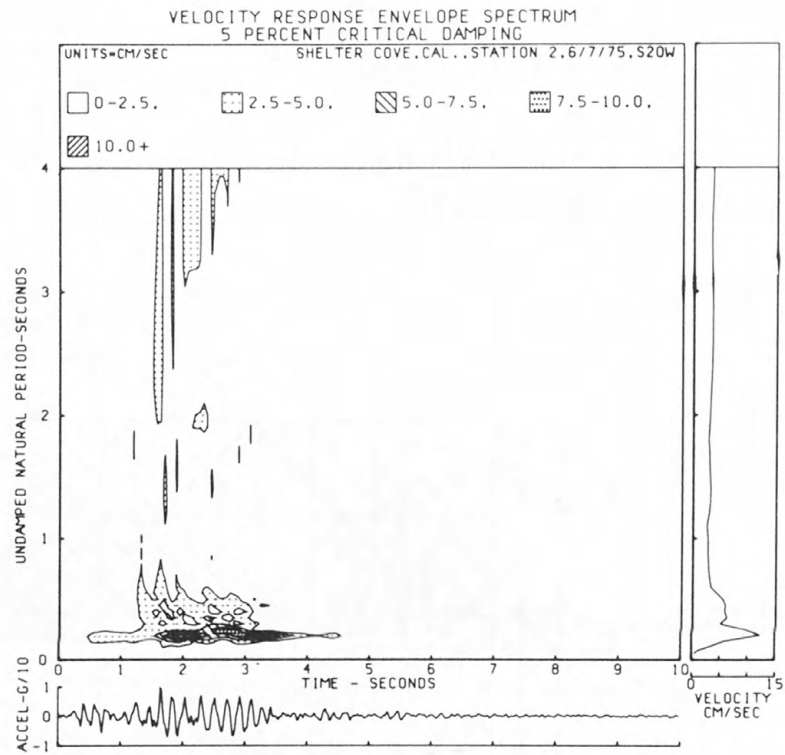


RESPONSE SPECTRA
 SHELTER COVE, CAL..STATION 2,6/7/75,S20W
 0.2,5,10,20 PERCENT CRITICAL DAMPING
 SEISMIC ENGINEERING BRANCH/USGS

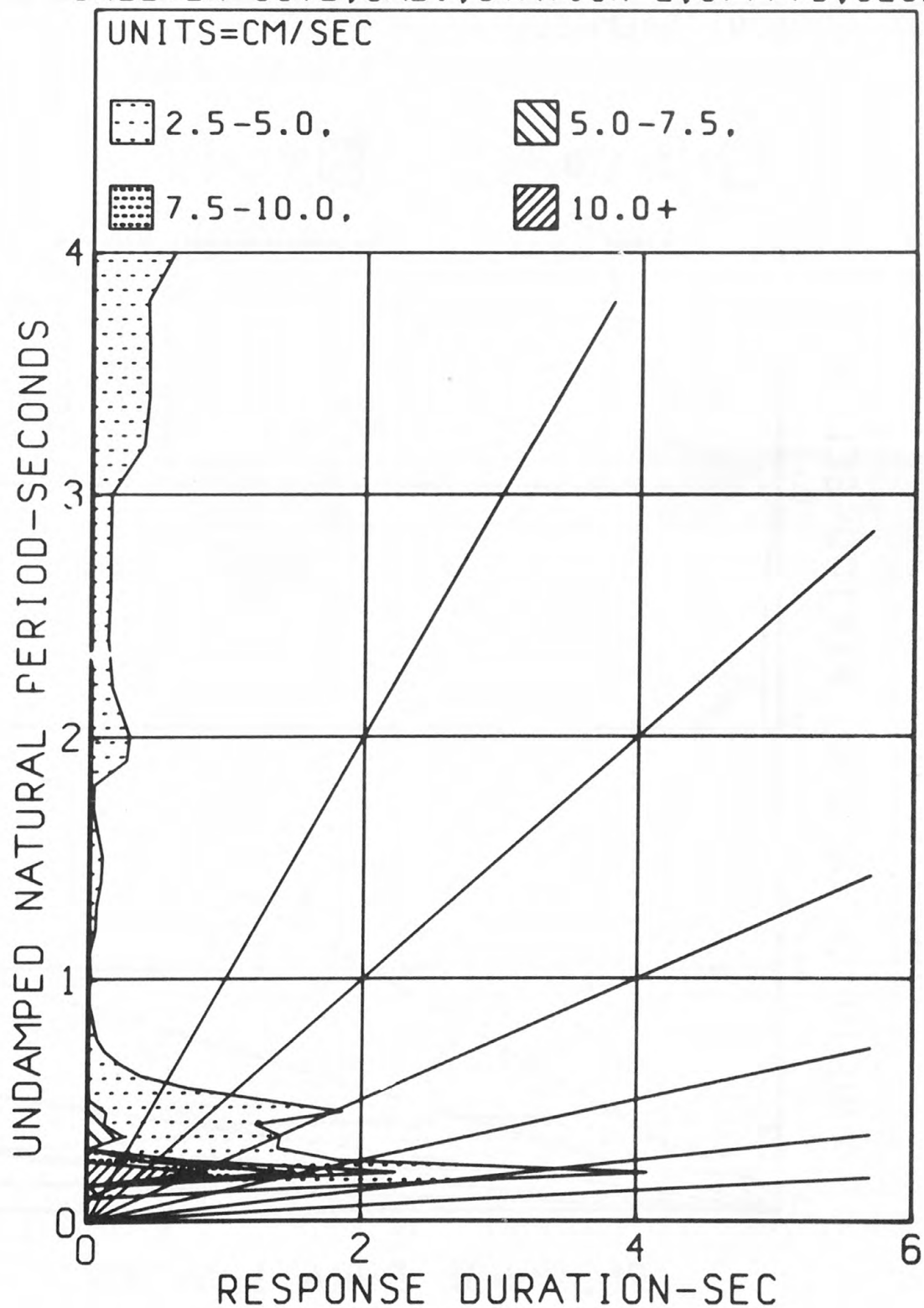


RESPONSE SPECTRA
 SHELTER COVE, CAL., STATION 2.6/7/75, N70E
 0.2, 5, 10, 20 PERCENT CRITICAL DAMPING
 SEISMIC ENGINEERING BRANCH/USGS

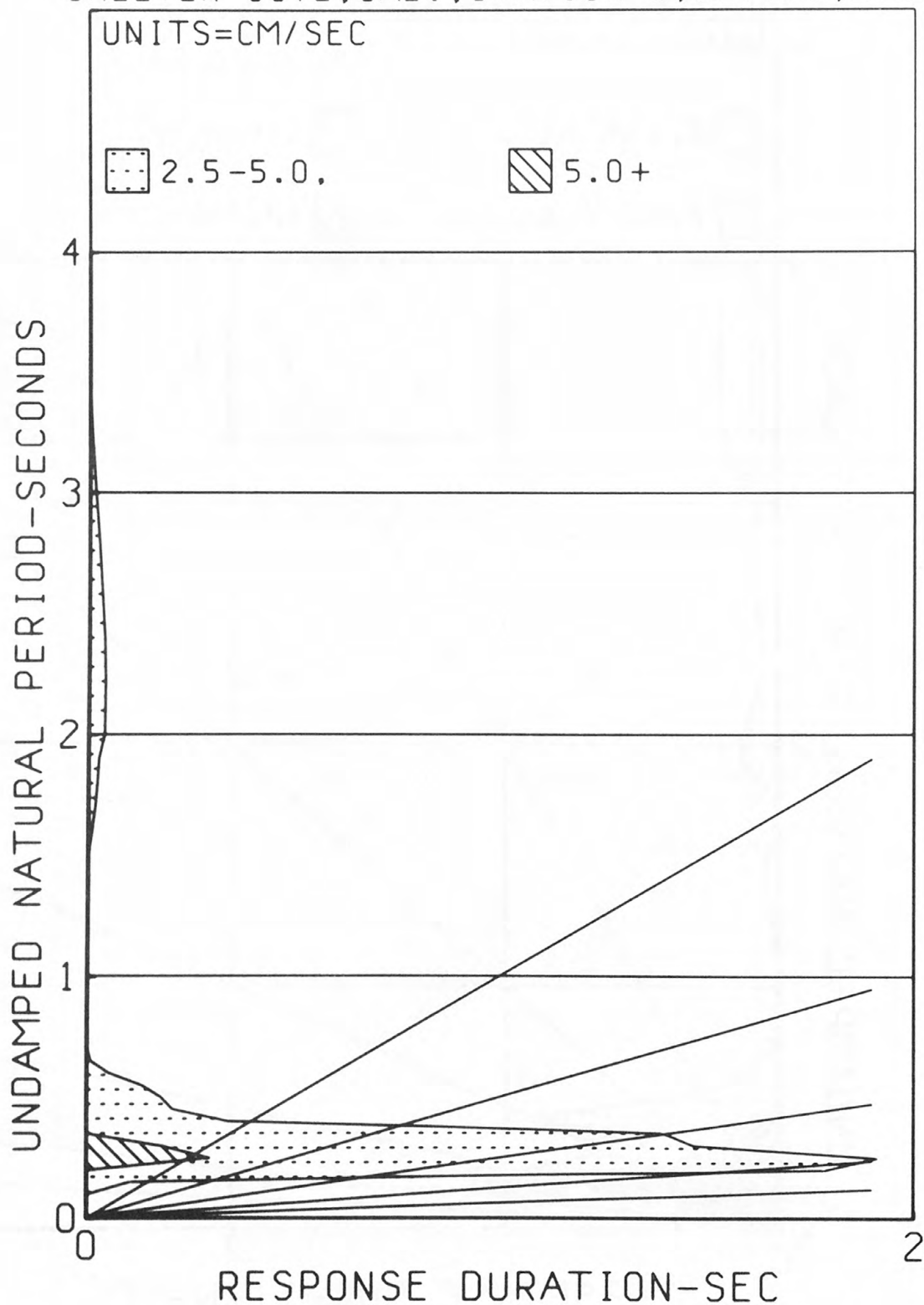




DURATION SPECTRUM OF THE VELOCITY
 RESPONSE ENVELOPE, 5 PERCENT DAMPING
 SHELTER COVE, CAL., STATION 2, 6/7/75, S20W



DURATION SPECTRUM OF THE VELOCITY
 RESPONSE ENVELOPE, 5 PERCENT DAMPING
 SHELTER COVE, CAL., STATION 2.6/7/75, N70E



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