

Modified Level II Streambed-Scour Analysis for Structure I-65-81-5523 Crossing Big Blue River in Shelby County, Indiana

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CONVERSION FACTORS AND ABBREVIATIONS

Multiply	By	To obtain
inch (in.)	25.4	millimeter
foot (ft)	0.3048	meter
square foot (ft ²)	929.0	square centimeter
feet per second (ft/s)	0.3048	meters per second
cubic foot per second (ft ³ /s)	0.02832	cubic meter per second
mile (mi)	1.609	kilometer
square mile (mi ²)	2.590	square kilometer

Abbreviations used in this report:

D ₅₀	median diameter of bed material
Q100	100-year discharge
FEMA	Federal Emergency Management Agency
HEC	Hydraulic Engineering Circular
IDNR	Indiana Department of Natural Resources
INDOT	Indiana Department of Transportation
USGS	U. S. Geological Survey
WSPRO	Water Surface PROfile model

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ABSTRACT

Level II scour evaluations follow a process in which hydrologic, hydraulic, and sediment-transport data are evaluated to calculate the depth of scour that may result when a given discharge is routed through a bridge opening. The results of the modified Level II analysis for structure I-65-81-5523 on Interstate 65 crossing Big Blue River in Shelby County, Indiana, are presented. The site is near the town of Mt. Auburn in the southwestern part of Shelby County. Scour depths were computed with the Water Surface PROfile model, version V050196, which incorporates the scour-calculation procedures outlined in Hydraulic Engineering Circular No. 18. Total scour depths at the piers were approximately 30.8 feet for the modeled discharge of 23,900 cubic feet per second and approximately 35.7 feet for the modeled discharge of 31,300 cubic feet per second.

INTRODUCTION

The U.S. Geological Survey (USGS), in cooperation with the Indiana Department of Transportation (INDOT), is conducting Level II scour analyses at a number of bridges throughout Indiana. This report describes the methods applied and the modeling results for bridge I-65-81-5523.

Background and Scope

Level I scour assessment is a process where a large number of bridges are studied as a group. Assessments usually are made by evaluating a combination of geomorphic, hydrologic, and bridge-characteristic data. The results help investigators determine which bridges appear to be most likely to experience streambed-scour problems and which bridges appear to be relatively immune to problems brought on by streambed scour (for example, bridges built on bedrock).

When applied correctly, Level I scour assessments provide an investigator with information to identify those bridges that appear to be relatively safe and those bridges that fall into higher risk categories.

Level II scour evaluations describe the process for an investigator to apply a model to a bridge site and calculate the potential depth of scour that may result from a given flood event. Level II analyses involve the application of basic hydrologic, hydraulic, and sediment-transport engineering concepts and may include an evaluation of flood history, channel hydraulic conditions (for example, water-surface profile analysis), and basic sediment-transport analyses such as scour calculations (Lagasse and others, 1995).

The methods and model outlined in Hydraulic Engineering Circular (HEC) No. 18 (Richardson and Davis, 1995) formulate the basis for Level II scour evaluations. Methods used in this study for Level II scour evaluations are a modification of the HEC-18 standards. These modifications were made to comply with the methodology requested by INDOT (Merril Dougherty, Indiana Department of Transportation, oral commun., 1996). Descriptions of the specific modifications are given in the "Evaluation Methods" section of this report.

This report presents the methods followed for modeling, special considerations for this study site, and the input for and the output from the Water Surface PROfile (WSPRO) model.

Site Description

The study site is located near the town of Mt. Auburn in the southwestern part of Shelby County. The drainage area for the site is approximately 575 mi² (estimated using Hoggatt, 1975, and USGS 7.5-minute topographic data). The predominant land use in the basin is agricultural; in the immediate vicinity of the bridge, the land is predominantly forest.

Within the immediate vicinity of the bridge, Big Blue River has a channel-bed slope of approximately 0.00077 ft/ft. The channel-bed material is gravelly sandy silt-clay, and the channel banks consist of sandy silt-clay. At the time of the Level I site visit on September 25, 1995, the banks were observed to have 0 to 50 percent woody vegetative cover; the field report noted that the banks were experiencing some fluvial erosion.

The Interstate 65 crossing of Big Blue River is a 416-ft-long, multi-lane bridge consisting of five spans supported by concrete and steel piers and riprap covered spill-through abutments. Additional details describing conditions at the site are included in the Level I data base (Hopkins and Robinson, unpub. data, 1997). Photographs of the site, taken at the time of the Level I site visit, are archived at the USGS office in Indianapolis.

EVALUATION METHODS

The methods described in this section apply to a number of bridge sites in Indiana being evaluated for scour and outline the procedures requested by INDOT for these modified Level II scour analyses. The principal modification requested by INDOT was that the input data to the model come from or be estimated from existing data sources; no additional field data were collected. Actual methods used in the scour evaluation at this particular bridge site use the most applicable method possible, given the data available.

To determine drainage area, either published values found in Hoggatt (1975) or 7.5-minute topographic maps with Hoggatt's original drainage-area delineations were used. Where there are no published data, drainage-area segments measured from the maps produced by Hoggatt were either subtracted from downstream sites or added to upstream sites published by Hoggatt (1975).

In Indiana, flood discharges are coordinated by agreement among State and Federal agencies. At sites where flood discharges officially are coordinated among State and Federal agencies in Indiana, the coordinated 100-year discharge (Q100) was modeled. INDOT also provided an additional flood discharge for these coordinated sites in excess of the Q100 to be modeled.

If a flood discharge was not coordinated, the USGS examined Federal Emergency Management Agency (FEMA) studies for Q100 determinations. Where FEMA studies did not produce a Q100, the USGS contacted IDNR for an estimated Q100 in the vicinity of the site being studied. If IDNR did not have a Q100, data from nearby USGS streamflow-gaging stations were analyzed with nearby and similar drainage basins that have been coordinated. At sites having no coordinated discharge data, the two discharges used in the model were 1) the approximated Q100 and 2) a discharge equal to 1.7 times the approximated Q100.

Most of the cross-section and bridge-opening geometry data were taken from the bridge plans (Indiana State Highway Commission, 1968) provided by INDOT. Bridge plans are presumed to be representative of current conditions at the site. To determine the cross-section geometry, a line was drawn on the bridge plans parallel to the bridge stationing and approximately one bridge width from the bridge. For sites where the bridge plans did not extend far enough laterally for collection of all cross-section data required for WSPRO model analysis, additional data were collected from 7.5-minute topographic maps.

The roadway and embankment profile was taken from the bridge and highway plans for those sites where roadway overtopping was expected. The INDOT bridge plans and 7.5-minute topographic maps were used as a guide, based on the water-surface elevations calculated by the WSPRO model, to determine if roadway overtopping might occur.

Roughness values (*n*-values) for the main channel were estimated by viewing photographs archived from the Level I scour assessments. The *n*-values for the overbanks were assigned on the basis of the surface-cover data summarized in the Level I data base (Hopkins and Robinson, unpub. data, 1997). From those data, the following roughness values were assigned to the surface-cover categories: urban—0.050, suburban—0.035, row crop—0.045, pasture—0.035, brush—0.120, forest—0.100, and wetland (any area covered by standing water)—0.100. The *n*-values for the overbanks were adjusted if the Level I photographs provided sufficient detail to warrant an adjustment.

WSPRO version V050196 was used to model flow through the study site. Starting water-surface elevation was obtained with a slope-conveyance computation. The channel-bed slope in the immediate vicinity of the bridge was estimated from the 7.5-minute topographic map and was used as the slope of the energy grade line for this computation.

WSPRO version V050196 includes a field that allows the input of up to four scour-adjustment factors (K1 to K4). For this modeling, the default value for K4 (bed armoring) was chosen. For scour-adjustment factors K1 and K2 (pier-nose shape and angle of attack, respectively), input values were determined by evaluating the data archived in the Level I data base (Hopkins and Robinson, unpub. data, 1997). For the K3 factor (bed forms), a value of 1.1 was applied in all cases.

In some cases, piers set on the overbanks are constructed with footings that are higher in elevation than pier footings in the main channel. In these situations, if the channel position changes, the piers that were initially constructed on the overbank may become part of the main channel. Therefore, to evaluate total potential scour, the model results obtained for contraction scour and deepest local scour in the main channel were added and applied to all piers in the bridge opening. This methodology allowed for an evaluation of potential undermining of pier supports in the event that future channel movement placed overbank piers in the main channel.

Where bridge pairs have a continuous abutment or fill between the bridges that does not allow expansion of flow, the bridge pair was modeled as one bridge. Sites with discontinuous abutments, allowing expansion between the bridges, were modeled as two separate bridges. In those cases, a valley cross section was measured between the bridges and used as the approach section for the downstream bridge and as the exit section for the upstream bridge.

At sites with no embankment to function as a weir or at sites where the tailwater drowns out the embankment, a composite bridge and road section was used to compute flow. Those sites were computed with friction-loss equations rather than with a bridge routine.

Total scour is taken as the sum of local scour plus contraction scour. If the model predicted negative contraction scour (aggradation), the contraction-scour value was assumed to be zero in determining the total scour depth (table 1). This assumption was made so that a negative contraction scour would not mask the potentially detrimental effects of local scour at a pier. No abutment scour evaluations were made in this study.

Table 1. Cumulative scour depths for the modeled discharges at structure I-65-81-5523 crossing Big Blue River in Shelby County, Indiana
[--, no value]

Pier number ¹	Stationing from bridge plans ²	Initial bed-elevation at pier (feet)	Main-channel contraction scour depth (feet)	Local scour depth (feet)	Worst-case total-scour depth ³ (feet)	Bottom elevation of pier (feet)	Worst-case bed elevation after scour ⁴ (feet)
Modeled discharge⁵ is 23,900 cubic feet per second							
1	80+33	669	8.5	22.3	30.8	663	634.6
2	81+22	665	8.5	22.3	30.8	660	634.6
3	82+14	665	8.5	22.3	30.8	660	634.6
4	83+05	669	8.5	22.3	30.8	663	634.6
Modeled discharge is 31,300 cubic feet per second							
1	80+33	669	11.6	24.1	35.7	663	629.7
2	81+22	665	11.6	24.1	35.7	660	629.7
3	82+14	665	11.6	24.1	35.7	660	629.7
4	83+05	669	11.6	24.1	35.7	663	629.7

¹Pier numbers were assigned from left to right as shown on the bridge plans.

²Stationing is the center line of the pier as determined from the bridge plans. Stationing from bridge plan, 80+33, represents a point 8,033 feet from an arbitrary starting location referenced on the bridge plans.

³Worst-case total-scour depths are generated by summing the calculated contraction-scour depth with the worst case of local scour.

⁴Worst-case bed elevation is computed by subtracting the worst-case total-scour depth from the lowest initial bed elevation in the bridge opening (665.4 feet).

⁵Coordinated discharge.

SPECIAL CONSIDERATIONS

Model runs indicate the water-surface elevation at the bridge is lower than the low-steel elevation for the modeled discharges. Therefore, there should be no pressure flow through the bridge opening for the discharges modeled.

RESULTS

Scour depths were computed with a version of WSPRO (Larry Arneson, Federal Highway Administration, written commun., 1996) modified from Shearman (1990). This version of WSPRO includes scour calculations in the model output. Scour depths were calculated assuming an infinite depth of material that could erode and a homogeneous particle-size distribution. The results of the scour analysis are presented in table 1; a complete input file and output results are presented in the appendix.

REFERENCES

- Hoggatt, R.E., 1975, Drainage areas of Indiana streams: U.S. Geological Survey, Water Resources Division, 231 p.
- Indiana State Highway Commission, 1968, Bridge plans Interstate Route 65: Bridge File I-65-81-5523.
- Lagasse, P.F.; Schall, J.D.; Johnson, F.; Richardson, E.V.; and Chang, F., 1995, Stream stability at highway structures (2d ed.): Federal Highway Administration, Hydraulic Engineering Circular No. 20, Publication FHWA-IP-90-014, 144 p.
- Richardson, E.V., and Davis, S.R., 1995, Evaluating scour at bridges (3d ed.): Federal Highway Administration, Hydraulic Engineering Circular No. 18, Publication FHWA-IP-90-017, 204 p.
- Shearman, J.O., 1990, User's manual for WSPRO, a computer model for water-surface profile computations: Federal Highway Administration Publication FHWA-IP-89-027, 177 p.

APPENDIX

WSPRO INPUT FILE

```

T1      I-65 over Big Blue River in Shelby Co.  I-65-81-5523
T2      County: Shelby                          Quad: Marietta 138A
T3      06-02-97                                Robert Miller
SI      0
Q        23900  31300
SK        .00077 .00077
XS      EXIT    0  52
GR        2768 690  2830 680  4450 680  5664 680  5878 670
GR        7920 670  7935 669
GR        7945 669  7954 670  7961 671  7966 672  7993 673
GR        8000 674  8037 674
GR        8121 674  8132 667  8148 666  8168 666  8212 667
GR        8222 672  8227 673
GR        8285 673  8303 672  8314 671  8321 671  8330 674
GR        8425 670  9027 670
GR        10616 680  10766 690
N          .100          .038          .100
SA          7887          8156
XS      FULLV   350 52
GR        2768 690  2830 680  4450 680  5664 680  5878 670
GR        7920 670  7935 669
GR        7945 669  7954 670  7961 671  7966 672  7993 673
GR        8000 674  8037 674
GR        8121 674  8132 667  8148 666  8168 666  8212 667
GR        8222 672  8227 673
GR        8285 673  8303 672  8314 671  8321 671  8330 674
GR        8425 670  9027 670
GR        10616 680  10766 690
N          .100          .038          .100
SA          7887          8156
BR      BRDGE   350  685.5  30
GR        7962 685.8  7963 684.3  7965 684.2  7990 672.7  8000 672.7
GR        8008 668.9  8102 668.8  8110 665.3  8229 665.4  8236 668.9
GR        8332 668.9  8341 672.8  8351 672.6  8374 683.7  8376 683.6
GR        8377 685.1  7962 685.8
N          .032
PD 1      665.4  6  1
PD 1      668.9  6  2
PD 1      668.9 12  3
CD        3  156  2  685
DC 0 BRDGE   8102 8236 7887 8156  *  12
DP      BRDGE   7962 8377  3.0  *  *  1.0  2.9  1.1
DP      BRDGE   7962 8377  3.0  *  *  1.0  2.9  1.1
DP      BRDGE   7962 8377  3.0  *  *  1.0  2.9  1.1
DP      BRDGE   7962 8377  3.0  *  *  1.0  2.9  1.1
XS      APPR    850
GR        6511 690  6573 680  7943 675  7980 675
GR        8021 672  8031 671
GR        8035 671  8053 672  8067 671  8076 670  8084 669
GR        8099 668  8108 667
GR        8111 666  8125 665  8131 665  8133 666  8134 667
GR        8143 672  8146 673
GR        8165 673  8206 672  8211 671  8214 671  8223 672
GR        8233 673  8249 673

```

WSPRO INPUT FILE

GR	8278	673	8295	673	8300	672	8309	672	8338	673
GR	8380	670								
GR	9961	680	10075	690						
N		.100		.038			.100			
SA			7887			8156				
EX										
ER										

WSPRO OUTPUT

***** W S P R O *****

Federal Highway Administration - U. S. Geological Survey

Model for Water-Surface Profile Computations.

Run Date & Time: 8/ 4/97 3:16 pm Version V050196

Input File: 5523.dat Output File: 5523.LST

T1 I-65 OVER BIG BLUE RIVER IN SHELBY CO. I-65-81-5523
T2 COUNTY: SHELBY QUAD: MARIETTA 138A
T3 06-02-97 ROBERT MILLER
SI 0
Q 23900 31300

*** Processing Flow Data; Placing Information into Sequence 1 ***

SK .00077 .00077

***** W S P R O *****

Federal Highway Administration - U. S. Geological Survey

Model for Water-Surface Profile Computations.

Input Units: English / Output Units: English

I-65 OVER BIG BLUE RIVER IN SHELBY CO. I-65-81-5523
COUNTY: SHELBY QUAD: MARIETTA 138A
06-02-97 ROBERT MILLER

* Starting To Process Header Record EXIT *

XS EXIT 0 52
GR 2768 690 2830 680 4450 680 5664 680 5878 670
GR 7920 670 7935 669
GR 7945 669 7954 670 7961 671 7966 672 7993 673
GR 8000 674 8037 674
GR 8121 674 8132 667 8148 666 8168 666 8212 667
GR 8222 672 8227 673
GR 8285 673 8303 672 8314 671 8321 671 8330 674
GR 8425 670 9027 670
GR 10616 680 10766 690
N .100 .038 .100
SA 7887 8156

*** Completed Reading Data Associated With Header Record EXIT ***

*** Storing X-Section Data In Temporary File As Record Number 1 ***

*** Data Summary For Header Record EXIT ***

SRD Location: 0. Cross-Section Skew: 52.0 Error Code 0

Valley Slope: .00000 Averaging Conveyance By Geometric Mean.

Energy Loss Coefficients -> Expansion: .50 Contraction: .00

X,Y-coordinates (30 pairs)

X	Y	X	Y	X	Y
2768.000	690.000	2830.000	680.000	4450.000	680.000

WSPRO OUTPUT

5664.000	680.000	5878.000	670.000	7920.000	670.000
7935.000	669.000	7945.000	669.000	7954.000	670.000
7961.000	671.000	7966.000	672.000	7993.000	673.000
8000.000	674.000	8037.000	674.000	8121.000	674.000
8132.000	667.000	8148.000	666.000	8168.000	666.000
8212.000	667.000	8222.000	672.000	8227.000	673.000
8285.000	673.000	8303.000	672.000	8314.000	671.000
8321.000	671.000	8330.000	674.000	8425.000	670.000
9027.000	670.000	10616.000	680.000	10766.000	690.000

Minimum and Maximum X,Y-coordinates

Minimum X-Station: 2768.000 (associated Y-Elevation: 690.000)
 Maximum X-Station: 10766.000 (associated Y-Elevation: 690.000)
 Minimum Y-Elevation: 666.000 (associated X-Station: 8168.000)
 Maximum Y-Elevation: 690.000 (associated X-Station: 2768.000)

X-coordinates & Horizontal Breakpoints Translated by Skew Angle

X Input	X Skewed	X Input	X Skewed	X Input	X Skewed
2768.000	4843.428	2830.000	4881.599	4450.000	5878.971
5664.000	6626.384	5878.000	6758.135	7920.000	8015.316
7935.000	8024.551	7945.000	8030.708	7954.000	8036.249
7961.000	8040.558	7966.000	8043.636	7993.000	8060.259
8000.000	8064.569	8037.000	8087.348	8121.000	8139.064
8132.000	8145.836	8148.000	8155.687	8168.000	8168.000
8212.000	8195.089	8222.000	8201.246	8227.000	8204.324
8285.000	8240.032	8303.000	8251.114	8314.000	8257.887
8321.000	8262.196	8330.000	8267.737	8425.000	8326.225
9027.000	8696.854	10616.000	9675.140	10766.000	9767.488

Roughness Data (3 SubAreas)

SubArea	Roughness Coefficient	Horizontal Breakpoint
1	.100	---
	---	7994.999
2	.038	---
	---	8160.612
3	.100	---

* Finished Processing Header Record EXIT *

***** W S P R O *****

Federal Highway Administration - U. S. Geological Survey

Model for Water-Surface Profile Computations.

Input Units: English / Output Units: English

I-65 OVER BIG BLUE RIVER IN SHELBY CO. I-65-81-5523

COUNTY: SHELBY

QUAD: MARIETTA 138A

WSPRO OUTPUT

06-02-97

ROBERT MILLER

```
*-----*
*           Starting To Process Header Record FULLV           *
*-----*
```

```
XS    FULLV    350 52
GR      2768 690 2830 680 4450 680 5664 680 5878 670
GR      7920 670 7935 669
GR      7945 669 7954 670 7961 671 7966 672 7993 673
GR      8000 674 8037 674
GR      8121 674 8132 667 8148 666 8168 666 8212 667
GR      8222 672 8227 673
GR      8285 673 8303 672 8314 671 8321 671 8330 674
GR      8425 670 9027 670
GR      10616 680 10766 690
N          .100          .038          .100
SA              7887          8156
```

```
*** Completed Reading Data Associated With Header Record FULLV ***
*** Storing X-Section Data In Temporary File As Record Number 2 ***
```

```
*** Data Summary For Header Record FULLV ***
SRD Location:      350. Cross-Section Skew: 52.0 Error Code 0
Valley Slope:      .00000 Averaging Conveyance By Geometric Mean.
Energy Loss Coefficients -> Expansion: .50 Contraction: .00
```

```

X,Y-coordinates (30 pairs)
      X          Y          X          Y          X          Y
-----
2768.000    690.000    2830.000    680.000    4450.000    680.000
5664.000    680.000    5878.000    670.000    7920.000    670.000
7935.000    669.000    7945.000    669.000    7954.000    670.000
7961.000    671.000    7966.000    672.000    7993.000    673.000
8000.000    674.000    8037.000    674.000    8121.000    674.000
8132.000    667.000    8148.000    666.000    8168.000    666.000
8212.000    667.000    8222.000    672.000    8227.000    673.000
8285.000    673.000    8303.000    672.000    8314.000    671.000
8321.000    671.000    8330.000    674.000    8425.000    670.000
9027.000    670.000    10616.000    680.000    10766.000    690.000
-----
```

```

Minimum and Maximum X,Y-coordinates
Minimum X-Station: 2768.000 ( associated Y-Elevation: 690.000 )
Maximum X-Station: 10766.000 ( associated Y-Elevation: 690.000 )
Minimum Y-Elevation: 666.000 ( associated X-Station: 8168.000 )
Maximum Y-Elevation: 690.000 ( associated X-Station: 2768.000 )
```

```

X-coordinates & Horizontal Breakpoints Translated by Skew Angle
X Input    X Skewed    X Input    X Skewed    X Input    X Skewed
-----
2768.000    4843.428    2830.000    4881.599    4450.000    5878.971
5664.000    6626.384    5878.000    6758.135    7920.000    8015.316
7935.000    8024.551    7945.000    8030.708    7954.000    8036.249
```


WSPRO OUTPUT

7961.000	8040.558	7966.000	8043.636	7993.000	8060.259
8000.000	8064.569	8037.000	8087.348	8121.000	8139.064
8132.000	8145.836	8148.000	8155.687	8168.000	8168.000
8212.000	8195.089	8222.000	8201.246	8227.000	8204.324
8285.000	8240.032	8303.000	8251.114	8314.000	8257.887
8321.000	8262.196	8330.000	8267.737	8425.000	8326.225
9027.000	8696.854	10616.000	9675.140	10766.000	9767.488

```

      Roughness Data ( 3 SubAreas )
      Roughness Horizontal
SubArea Coefficient Breakpoint
-----
      1          .100          ---
      ---          7994.999
      2          .038          ---
      ---          8160.612
      3          .100          ---
      -----
  
```

```

*-----*
*      Finished Processing Header Record FULLV      *
*-----*
  
```

```

***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English
  
```

```

*-----*
I-65 OVER BIG BLUE RIVER IN SHELBY CO. I-65-81-5523
COUNTY: SHELBY QUAD: MARIETTA 138A
06-02-97 ROBERT MILLER
  
```

```

*-----*
*      Starting To Process Header Record BRDGE      *
*-----*
  
```

```

BR BRDGE 350 685.5 30
GR 7962 685.8 7963 684.3 7965 684.2 7990 672.7 8000 672.7
GR 8008 668.9 8102 668.8 8110 665.3 8229 665.4 8236 668.9
GR 8332 668.9 8341 672.8 8351 672.6 8374 683.7 8376 683.6
GR 8377 685.1 7962 685.8
N .032
PD 1 665.4 6 1
PD 1 668.9 6 2
PD 1 668.9 12 3
CD 3 156 2 685
  
```

```

*** Completed Reading Data Associated With Header Record BRDGE ***
*** Storing Bridge Data In Temporary File As Record Number 3 ***
  
```

```

*** Data Summary For Bridge Record BRDGE ***
SRD Location: 350. Cross-Section Skew: 30.0 Error Code 0
Valley Slope: ***** Averaging Conveyance By Geometric Mean.
  
```

WSPRO OUTPUT

Energy Loss Coefficients -> Expansion: .50 Contraction: .00

X,Y-coordinates (17 pairs)					
X	Y	X	Y	X	Y
7962.000	685.800	7963.000	684.300	7965.000	684.200
7990.000	672.700	8000.000	672.700	8008.000	668.900
8102.000	668.800	8110.000	665.300	8229.000	665.400
8236.000	668.900	8332.000	668.900	8341.000	672.800
8351.000	672.600	8374.000	683.700	8376.000	683.600
8377.000	685.100	7962.000	685.800		

Minimum and Maximum X,Y-coordinates
 Minimum X-Station: 7962.000 (associated Y-Elevation: 685.800)
 Maximum X-Station: 8377.000 (associated Y-Elevation: 685.100)
 Minimum Y-Elevation: 665.300 (associated X-Station: 8110.000)
 Maximum Y-Elevation: 685.800 (associated X-Station: 7962.000)

X-coordinates & Horizontal Breakpoints Translated by Skew Angle					
X Input	X Skewed	X Input	X Skewed	X Input	X Skewed
7962.000	7981.828	7963.000	7982.694	7965.000	7984.426
7990.000	8006.077	8000.000	8014.737	8008.000	8021.666
8102.000	8103.072	8110.000	8110.000	8229.000	8213.057
8236.000	8219.119	8332.000	8302.258	8341.000	8310.052
8351.000	8318.712	8374.000	8338.631	8376.000	8340.362
8377.000	8341.229	7962.000	7981.828		

Roughness Data (1 SubAreas)

SubArea	Roughness Coefficient	Horizontal Breakpoint
1	.032	---

Discharge coefficient parameters

BRType	BRWidth	EMBSS	EMBElv	UserCD
3	156.000	2.00	685.000	*****

Pressure flow elevations

AVBCEL	PFElev
*****	685.500

Abutment Parameters

ABSLPL	ABSLPR	XTOELT	YTOELT	XTOERT	YTOERT
*****	*****	*****	*****	*****	*****

Pier/Pile Data (3 Group(s))
 Code Indicates Bridge Uses Piles

Group	Elevation	Gross Width	Number
-----	-----	-----	-----

WSPRO OUTPUT

1	665.400	6.000	1
2	668.900	6.000	2
3	668.900	12.000	3

* Finished Processing Header Record BRDGE *

***** W S P R O *****

Federal Highway Administration - U. S. Geological Survey

Model for Water-Surface Profile Computations.

Input Units: English / Output Units: English

-----*

I-65 OVER BIG BLUE RIVER IN SHELBY CO. I-65-81-5523

COUNTY: SHELBY QUAD: MARIETTA 138A

06-02-97 ROBERT MILLER

DC 0	BRDGE	8102	8236	7887	8156	*	12
DP	BRDGE	7962	8377	3.0	*	*	1.0 2.9 1.1
DP	BRDGE	7962	8377	3.0	*	*	1.0 2.9 1.1
DP	BRDGE	7962	8377	3.0	*	*	1.0 2.9 1.1
DP	BRDGE	7962	8377	3.0	*	*	1.0 2.9 1.1

* Starting To Process Header Record APPR *

XS	APPR	850					
GR		6511 690	6573 680	7943 675	7980 675		
GR		8021 672	8031 671				
GR		8035 671	8053 672	8067 671	8076 670	8084 669	
GR		8099 668	8108 667				
GR		8111 666	8125 665	8131 665	8133 666	8134 667	
GR		8143 672	8146 673				
GR		8165 673	8206 672	8211 671	8214 671	8223 672	
GR		8233 673	8249 673				
GR		8278 673	8295 673	8300 672	8309 672	8338 673	
GR		8380 670					
GR		9961 680	10075 690				
N		.100	.038	.100			
SA			7887	8156			

*** Completed Reading Data Associated With Header Record APPR ***

*** Storing X-Section Data In Temporary File As Record Number 4 ***

*** Data Summary For Header Record APPR ***

SRD Location: 850. Cross-Section Skew: .0 Error Code 0

Valley Slope: .00000 Averaging Conveyance By Geometric Mean.

Energy Loss Coefficients -> Expansion: .50 Contraction: .00

X,Y-coordinates (35 pairs)					
X	Y	X	Y	X	Y
6511.000	690.000	6573.000	680.000	7943.000	675.000

WSPRO OUTPUT

7980.000	675.000	8021.000	672.000	8031.000	671.000
8035.000	671.000	8053.000	672.000	8067.000	671.000
8076.000	670.000	8084.000	669.000	8099.000	668.000
8108.000	667.000	8111.000	666.000	8125.000	665.000
8131.000	665.000	8133.000	666.000	8134.000	667.000
8143.000	672.000	8146.000	673.000	8165.000	673.000
8206.000	672.000	8211.000	671.000	8214.000	671.000
8223.000	672.000	8233.000	673.000	8249.000	673.000
8278.000	673.000	8295.000	673.000	8300.000	672.000
8309.000	672.000	8338.000	673.000	8380.000	670.000
9961.000	680.000	10075.000	690.000		

Minimum and Maximum X,Y-coordinates

Minimum X-Station: 6511.000 (associated Y-Elevation: 690.000)
 Maximum X-Station: 10075.000 (associated Y-Elevation: 690.000)
 Minimum Y-Elevation: 665.000 (associated X-Station: 8131.000)
 Maximum Y-Elevation: 690.000 (associated X-Station: 6511.000)

Roughness Data (3 SubAreas)

SubArea	Roughness Coefficient	Horizontal Breakpoint
1	.100	---
	---	7887.000
2	.038	---
	---	8156.000
3	.100	---

Bridge datum projection(s): XREFLT XREFRT FDSTLT FDSTRT

 * Finished Processing Header Record APPR *

***** W S P R O *****

Federal Highway Administration - U. S. Geological Survey

Model for Water-Surface Profile Computations.

Input Units: English / Output Units: English

I-65 OVER BIG BLUE RIVER IN SHELBY CO. I-65-81-5523
 COUNTY: SHELBY QUAD: MARIETTA 138A
 06-02-97 ROBERT MILLER

EX

=====
 * Summary of Boundary Condition Information *
 =====

#	Reach Discharge	Water Surface Elevation	Friction Slope	Flow Regime
--	-----	-----	-----	-----

WSPRO OUTPUT

```

1      23900.00      *****      .0008      Sub-Critical
2      31300.00      *****      .0008      Sub-Critical
- - - - -

```

```

*=====
*      Beginning  2 Profile Calculation(s)      *
*=====

```

```

***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English

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-----*
I-65 OVER BIG BLUE RIVER IN SHELBY CO. I-65-81-5523
COUNTY: SHELBY QUAD: MARIETTA 138A
06-02-97 ROBERT MILLER

```

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: EXIT	675.476	.031	23900.000	19203.210	*****	5760.818
Header Type: XS	675.506	*****	1.245	860843.00	*****	9897.100
SRD: .000	671.742	*****	.115	*****	1.269	*****
Section: FULLV	675.742	.028	23900.000	20309.580	350.000	5755.127
Header Type: FV	675.769	.247	1.177	939780.50	350.000	9939.356
SRD: 350.000	671.742	.000	.107	.0007	1.290	.016

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

===135 CONVEYANCE RATIO OUTSIDE OF RECOMMENDED LIMITS AT SECID "APPR ".
KRATIO: .29

Section: APPR	676.536	.672	23900.000	5758.271	500.000	7522.088
Header Type: AS	677.208	1.103	4.151	275468.60	500.000	9413.372
SRD: 850.000	675.219	.322	.664	.0022	2.509	.013

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

<<< The Following Data Reflect The "Constricted" Profile >>>
<<< Beginning Bridge/Culvert Hydraulic Computations >>>

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: BRDGE	675.878	1.750	23900.000	2886.801	350.000	7983.090
Header Type: BR	677.629	.762	8.279	518626.60	350.000	8357.793
SRD: 350.000	673.235	1.359	.674	*****	1.642	-.003

Specific Bridge Information C P/A PFELEV BLEN XLAB XRAB

WSPRO OUTPUT

```
Bridge Type 3   Flow Type 1 -----
Pier/Pile Code 1          .7805  .036  685.500  *****  *****  *****
-----
```

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: APPR	678.972	.200	23900.000	11646.310	344.000	6854.728
Header Type: AS	679.172	1.160	2.052	636589.50	395.886	9798.441
SRD: 850.000	675.219	.384	.318	.0022	3.059	-.015

```
Approach Section APPR Flow Contraction Information
M( G )   M( K )       KQ       XLKQ       XRKQ       OTEL
-----
      .802      .523  304903.8  8023.071  8397.787  678.972
-----
```

<<< End of Bridge Hydraulics Computations >>>

```
***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English
*-----*
```

```
I-65 OVER BIG BLUE RIVER IN SHELBY CO. I-65-81-5523
COUNTY: SHELBY QUAD: MARIETTA 138A
06-02-97 ROBERT MILLER
```

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: EXIT	676.336	.039	31300.000	22828.420	*****	5742.408
Header Type: XS	676.375	*****	1.371	1127479.00	*****	10033.800
SRD: .000	672.095	*****	.121	*****	1.333	*****
Section: FULLLV	676.594	.036	31300.000	23940.170	350.000	5736.894
Header Type: FV	676.630	.251	1.307	1213661.00	350.000	10074.750
SRD: 350.000	672.095	.000	.114	.0007	1.350	.004

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

```
===135 CONVEYANCE RATIO OUTSIDE OF RECOMMENDED LIMITS AT SECID "APPR ".
KRATIO: .31
```

Section: APPR	677.326	.759	31300.000	7387.297	500.000	7305.616
Header Type: AS	678.085	1.084	4.237	372319.40	500.000	9538.277
SRD: 850.000	675.834	.361	.677	.0022	2.717	.010

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

<<< The Following Data Reflect The "Constricted" Profile >>>

WSPRO OUTPUT

<<< Beginning Bridge/Culvert Hydraulic Computations >>>

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: BRDGE	676.605	2.566	31300.000	3160.216	350.000	7981.511
Header Type: BR	679.171	.754	9.904	599464.20	350.000	8359.299
SRD: 350.000	674.290	2.044	.783	*****	1.682	.004

Specific Bridge Information	C	P/A	PFELEV	BLFN	XLAB	XRAB
Bridge Type 3 Flow Type 1						
Pier/Pile Code 1	.7711	.036	685.500	*****	*****	*****

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: APPR	680.485	.172	31300.000	16545.860	344.000	6569.995
Header Type: AS	680.657	1.001	1.892	981265.80	402.249	9966.526
SRD: 850.000	675.834	.485	.266	.0022	3.097	.018

Approach Section APPR Flow Contraction Information						
M(G)	M(K)	KQ	XLKQ	XRKQ	OTEL	
.831	.587	403484.4	8018.313	8396.087	680.485	

<<< End of Bridge Hydraulics Computations >>>

***** W S P R O *****

Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English

I-65 OVER BIG BLUE RIVER IN SHELBY CO. I-65-81-5523
COUNTY: SHELBY QUAD: MARIETTA 138A
06-02-97 ROBERT MILLER

*** Live-Bed Contraction Scour Calculations for Header Record BRDGE ***

Constants and Input Variables

Bed Material Transport Mode Factor (k1): .64
Total Pier Width Value (Pw): 12.000

Scour	-- Flow --	-- Width --	--- X-Limits ---
# Depth	Contract Approach	Contract Approach	Side Contract Approach

WSPRO OUTPUT

```

1   8.517 13949.150  9841.987 122.000 269.000 Left: 8102.000 7887.000
..... Approach Channel Depth:   6.913 ..... Right: 8236.000 8156.000
2  11.557 17700.250 11629.050 122.000 269.000 Left: 8102.000 7887.000
..... Approach Channel Depth:   8.426 ..... Right: 8236.000 8156.000
--

```

***** W S P R O *****

Federal Highway Administration - U. S. Geological Survey
 Model for Water-Surface Profile Computations.
 Input Units: English / Output Units: English

I-65 OVER BIG BLUE RIVER IN SHELBY CO. I-65-81-5523
 COUNTY: SHELBY QUAD: MARIETTA 138A
 06-02-97 ROBERT MILLER

*** Pier Scour Calculations for Header Record BRDGE ***

Constants and Input Variables

Pier Width: 3.000

Pier Shape Factor (K1): 1.00
 Flow Angle of Attack Factor (K2): 2.90
 Bed Condition Factor (K3): 1.10
 Bed Material Factor (K4): 1.00
 Velocity Multiplier (VM): 1.00
 Depth Multiplier (YM): 1.00

#	Scour Depth	---- Localized Hydraulic Properties ----					-- X-Stations --	
		Flow	WSE	Depth	Velocity	Froude #	Left	Right
1	22.29	23900.000	676.262	10.962	9.330	.497	7962.000	8377.000
2	24.14	31300.000	677.090	11.790	10.969	.563	7962.000	8377.000

***** W S P R O *****

Federal Highway Administration - U. S. Geological Survey
 Model for Water-Surface Profile Computations.
 Input Units: English / Output Units: English

I-65 OVER BIG BLUE RIVER IN SHELBY CO. I-65-81-5523
 COUNTY: SHELBY QUAD: MARIETTA 138A
 06-02-97 ROBERT MILLER

*** Pier Scour Calculations for Header Record BRDGE ***

Constants and Input Variables

Pier Width: 3.000

Pier Shape Factor (K1): 1.00
 Flow Angle of Attack Factor (K2): 2.90

WSPRO OUTPUT

Bed Condition Factor (K3): 1.10
 Bed Material Factor (K4): 1.00
 Velocity Multiplier (VM): 1.00
 Depth Multiplier (YM): 1.00

#	----- Localized Hydraulic Properties -----						-- X-Stations --	
	Scour Depth	Flow	WSE	Depth	Velocity	Froude #	Left	Right
1	22.29	23900.000	676.262	10.962	9.330	.497	7962.000	8377.000
2	24.14	31300.000	677.090	11.790	10.969	.563	7962.000	8377.000

***** W S P R O *****

Federal Highway Administration - U. S. Geological Survey
 Model for Water-Surface Profile Computations.
 Input Units: English / Output Units: English

I-65 OVER BIG BLUE RIVER IN SHELBY CO. I-65-81-5523
 COUNTY: SHELBY QUAD: MARIETTA 138A
 06-02-97 ROBERT MILLER

*** Pier Scour Calculations for Header Record BRDGE ***

Constants and Input Variables

Pier Width: 3.000

Pier Shape Factor (K1): 1.00
 Flow Angle of Attack Factor (K2): 2.90
 Bed Condition Factor (K3): 1.10
 Bed Material Factor (K4): 1.00
 Velocity Multiplier (VM): 1.00
 Depth Multiplier (YM): 1.00

#	----- Localized Hydraulic Properties -----						-- X-Stations --	
	Scour Depth	Flow	WSE	Depth	Velocity	Froude #	Left	Right
1	22.29	23900.000	676.262	10.962	9.330	.497	7962.000	8377.000
2	24.14	31300.000	677.090	11.790	10.969	.563	7962.000	8377.000

***** W S P R O *****

Federal Highway Administration - U. S. Geological Survey
 Model for Water-Surface Profile Computations.
 Input Units: English / Output Units: English

I-65 OVER BIG BLUE RIVER IN SHELBY CO. I-65-81-5523
 COUNTY: SHELBY QUAD: MARIETTA 138A
 06-02-97 ROBERT MILLER

WSPRO OUTPUT

*** Pier Scour Calculations for Header Record BRDGE ***

Constants and Input Variables

Pier Width: 3.000

```
*-----*
Pier Shape Factor           (K1): 1.00
Flow Angle of Attack Factor (K2): 2.90
Bed Condition Factor        (K3): 1.10
Bed Material Factor         (K4): 1.00
Velocity Multiplier         (VM): 1.00
Depth Multiplier            (YM): 1.00
*-----*
```

#	Scour Depth	---- Localized Hydraulic Properties ----					-- X-Stations --	
		Flow	WSE	Depth	Velocity	Froude #	Left	Right
1	22.29	23900.000	676.262	10.962	9.330	.497	7962.000	8377.000
2	24.14	31300.000	677.090	11.790	10.969	.563	7962.000	8377.000

ER

```
***** Normal end of WSPRO execution. *****
***** Elapsed Time: 0 Minutes 6 Seconds *****
```