

Modified Level II Streambed-Scour Analysis for Structure I-65-120-6016 Crossing Little Eagle Creek and I-65 in Marion County, Indiana

By ROBERT L. MILLER, BRET A. ROBINSON, and
DAVID C. VOELKER

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BRUCE BABBITT, Secretary

U.S. GEOLOGICAL SURVEY
Gordon P. Eaton, Director

For additional information, write to:
District Chief
U.S. Geological Survey
Water Resources Division
5957 Lakeside Boulevard
Indianapolis, IN 46278-1996

Copies of this report can be purchased from:
U.S. Geological Survey
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Federal Center
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CONVERSION FACTORS AND ABBREVIATIONS

	Multiply	By	To obtain
	inch (in.)	25.4	millimeter
	foot (ft)	0.3048	meter
	square foot (ft ²)	929.0	square centimeter
	feet per second (ft/s)	0.3048	meters per second
	cubic foot per second (ft ³ /s)	0.02832	cubic meter per second
	mile (mi)	1.609	kilometer
	square mile (mi ²)	2.590	square kilometer

Abbreviations used in this report:

D ₅₀	median diameter of bed material
Q100	100-year discharge
FEMA	Federal Emergency Management Agency
HEC	Hydraulic Engineering Circular
IDNR	Indiana Department of Natural Resources
INDOT	Indiana Department of Transportation
USGS	U. S. Geological Survey
WSPRO	Water Surface PROfile model

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ABSTRACT

Level II scour evaluations follow a process in which hydrologic, hydraulic, and sediment-transport data are evaluated to calculate the depth of scour that may result when a given discharge is routed through a bridge opening. The results of the modified Level II analysis for structure I-65-120-6016, Georgetown Road crossing Little Eagle Creek and I-65 in Marion County, Indiana, are presented. The site is in the city of Indianapolis in the northwestern part of Marion County. Scour depths were computed with the Water Surface PROfile model, version V050196, which incorporates the scour-calculation procedures outlined in Hydraulic Engineering Circular No. 18. Total scour depths at the piers were approximately 5.2 feet for the modeled discharge of 3,450 cubic feet per second and approximately 5.6 feet for the modeled discharge of 5,210 cubic feet per second.

INTRODUCTION

The U.S. Geological Survey (USGS), in cooperation with the Indiana Department of Transportation (INDOT), is conducting Level II scour analyses at a number of bridges throughout Indiana. This report describes the methods applied and the modeling results for bridge I-65-120-6016.

Background and Scope

Level I scour assessment is a process where a large number of bridges are studied as a group. Assessments usually are made by evaluating a combination of geomorphic, hydrologic, and bridge-characteristic data. The results help investigators determine which bridges appear to be most likely to experience streambed-scour problems and which bridges appear to be relatively immune to problems brought on by streambed scour (for example, bridges built on bedrock).

When applied correctly, Level I scour assessments provide an investigator with information to identify those bridges that appear to be relatively safe and those bridges that fall into higher risk categories.

Level II scour evaluations describe the process for an investigator to apply a model to a bridge site and calculate the potential depth of scour that may result from a given flood event. Level II analyses involve the application of basic hydrologic, hydraulic, and sediment-transport engineering concepts and may include an evaluation of flood history, channel hydraulic conditions (for example, water-surface profile analysis), and basic sediment-transport analyses such as scour calculations (Lagasse and others, 1995).

The methods and model outlined in Hydraulic Engineering Circular (HEC) No. 18 (Richardson and Davis, 1995) formulate the basis for Level II scour evaluations. Methods used in this study for Level II scour evaluations are a modification of the HEC-18 standards. These modifications were made to comply with the methodology requested by INDOT (Merril Dougherty, Indiana Department of Transportation, oral commun., 1996). Descriptions of the specific modifications are given in the "Evaluation Methods" section of this report.

This report presents the methods followed for modeling, special considerations for this study site, and the input for and the output from the Water Surface PROfile (WSPRO) model.

Site Description

The study site is located in the city of Indianapolis in the northwestern part of Marion County. The drainage area for the site is approximately 7.2 mi² (estimated using Hoggatt, 1975, and USGS 7.5-minute topographic data). The predominant land use in the basin is urban; in the immediate vicinity of the bridge, the land is also urban.

Within the immediate vicinity of the bridge, Little Eagle Creek has a channel-bed slope of approximately 0.0033 ft/ft. The channel-bed material is gravelly sandy silt-clay, and the channel banks consist of gravelly sandy silt-clay. At the time of the Level I site visit on June 17, 1994, the banks were observed to have 25 to 100 percent woody vegetative cover; the field report noted that the banks were experiencing fluvial erosion.

The Georgetown Road crossing of Little Eagle Creek and I-65 is a 474-ft-long, multi-lane bridge consisting of five spans supported by concrete and steel piers and sloping concrete spill-through abutments. Additional details describing conditions at the site are included in the Level I data base (Hopkins and Robinson, unpub. data, 1997). Photographs of the site, taken at the time of the Level I site visit, are archived at the USGS office in Indianapolis.

EVALUATION METHODS

The methods described in this section apply to a number of bridge sites in Indiana being evaluated for scour and outline the procedures requested by INDOT for these modified Level II scour analyses. The principal modification requested by INDOT was that the input data to the model come from or be estimated from existing data sources; no additional field data were collected. Actual methods used in the scour evaluation at this particular bridge site use the most applicable method possible, given the data available.

To determine drainage area, either published values found in Hoggatt (1975) or 7.5-minute topographic maps with Hoggatt's original drainage-area delineations were used. Where there are no published data, drainage-area segments measured from the maps produced by Hoggatt were either subtracted from downstream sites or added to upstream sites published by Hoggatt (1975).

In Indiana, flood discharges are coordinated by agreement among State and Federal agencies. At sites where flood discharges officially are coordinated among State and Federal agencies in Indiana, the coordinated 100-year discharge (Q100) was modeled. INDOT also provided an additional flood discharge for these coordinated sites in excess of the Q100 to be modeled.

If a flood discharge was not coordinated, the USGS examined Federal Emergency Management Agency (FEMA) studies for Q100 determinations. Where FEMA studies did not produce a Q100, the USGS contacted IDNR for an estimated Q100 in the vicinity of the site being studied. If IDNR did not have a Q100, data from nearby USGS streamflow-gaging stations were analyzed with nearby and similar drainage basins that have been coordinated. At sites having no coordinated discharge data, the two discharges used in the model were 1) the approximated Q100 and 2) a discharge equal to 1.7 times the approximated Q100.

Most of the cross-section and bridge-opening geometry data were taken from the bridge plans (Indiana State Highway Commission, 1969) provided by INDOT. Bridge plans are presumed to be representative of current conditions at the site. To determine the cross-section geometry, a line was drawn on the bridge plans parallel to the bridge stationing and approximately one bridge width from the bridge. For sites where the bridge plans did not extend far enough laterally for collection of all cross-section data required for WSPRO model analysis, additional data were collected from 7.5-minute topographic maps.

The roadway and embankment profile was taken from the bridge and highway plans for those sites where roadway overtopping was expected. The INDOT bridge plans and 7.5-minute topographic maps were used as a guide, based on the water-surface elevations calculated by the WSPRO model, to determine if roadway overtopping might occur.

Roughness values (*n*-values) for the main channel were estimated by viewing photographs archived from the Level I scour assessments. The *n*-values for the overbanks were assigned on the basis of the surface-cover data summarized in the Level I data base (Hopkins and Robinson, unpub. data, 1997). From those data, the following roughness values were assigned to the surface-cover categories: urban—0.050, suburban—0.035, row crop—0.045, pasture—0.035, brush—0.120, forest—0.100, and wetland (any area covered by standing water)—0.100. The *n*-values for the overbanks were adjusted if the Level I photographs provided sufficient detail to warrant an adjustment.

WSPRO version V050196 was used to model flow through the study site. Starting water-surface elevation was obtained with a slope-conveyance computation. The channel-bed slope in the immediate vicinity of the bridge was estimated from the 7.5-minute topographic map and was used as the slope of the energy grade line for this computation.

WSPRO version V050196 includes a field that allows the input of up to four scour-adjustment factors (K1 to K4). For this modeling, the default value for K4 (bed armoring) was chosen. For scour-adjustment factors K1 and K2 (pier-nose shape and angle of attack, respectively), input values were determined by evaluating the data archived in the Level I data base (Hopkins and Robinson, unpub. data, 1997). For the K3 factor (bed forms), a value of 1.1 was applied in all cases.

In some cases, piers set on the overbanks are constructed with footings that are higher in elevation than pier footings in the main channel. In these situations, if the channel position changes, the piers that were initially constructed on the overbank may become part of the main channel. Therefore, to evaluate total potential scour, the model results obtained for contraction scour and deepest local scour in the main channel were added and applied to all piers in the bridge opening. This methodology allowed for an evaluation of potential undermining of pier supports in the event that future channel movement placed overbank piers in the main channel.

Where bridge pairs have a continuous abutment or fill between the bridges that does not allow expansion of flow, the bridge pair was modeled as one bridge. Sites with discontinuous abutments, allowing expansion between the bridges, were modeled as two separate bridges. In those cases, a valley cross section was measured between the bridges and used as the approach section for the downstream bridge and as the exit section for the upstream bridge.

At sites with no embankment to function as a weir or at sites where the tailwater drowns out the embankment, a composite bridge and road section was used to compute flow. Those sites were computed with friction-loss equations rather than with a bridge routine.

Total scour is taken as the sum of local scour plus contraction scour. If the model predicted negative contraction scour (aggradation), the contraction-scour value was assumed to be zero in determining the total scour depth (table 1). This assumption was made so that a negative contraction scour would not mask the potentially detrimental effects of local scour at a pier. No abutment scour evaluations were made in this study.

Table 1. Cumulative scour depths for the modeled discharges at structure I-65-120-6016 crossing Little Eagle Creek and I-65 in Marion County, Indiana

[--, no value]

Pier number ¹	Stationing from bridge plans ²	Initial bed-elevation at pier (feet)	Main-channel contraction scour depth (feet)	Local scour depth (feet)	Worst-case total-scour depth ³ (feet)	Bottom elevation of pier (feet)	Worst-case bed elevation after scour ⁴ (feet)
Modeled discharge⁵ is 3,450 cubic feet per second							
1	48+54	775	0	5.2	5.2	762	750.6
2	49+70	770	0	5.2	5.2	766	750.6
3	50+85	767	0	5.2	5.2	760	750.6
4	51+98	759	0	5.2	5.2	752	750.6
Modeled discharge is 5,210 cubic feet per second							
1	48+54	775	0	5.6	5.6	762	750.2
2	49+70	770	0	5.6	5.6	766	750.2
3	50+85	767	0	5.6	5.6	760	750.2
4	51+98	759	0	5.6	5.6	752	750.2

¹Pier numbers were assigned from left to right as shown on the bridge plans.

²Stationing is the center line of the pier as determined from the bridge plans. Stationing from bridge plan, 48+54, represents a point 4,854 feet from an arbitrary starting location referenced on the bridge plans.

³Worst-case total-scour depths are generated by summing the calculated contraction-scour depth with the worst case of local scour.

⁴Worst-case bed elevation is computed by subtracting the worst-case total-scour depth from the lowest initial bed elevation in the bridge opening (755.8 feet).

⁵Coordinated discharge.

SPECIAL CONSIDERATIONS

Model runs indicate the water-surface elevation at the bridge is lower than the low-steel elevation for the modeled discharges. Therefore, there should be no pressure flow through the bridge opening for the discharges modeled. The modeling was applied only to the main channel of Little Eagle Creek, and an imaginary abutment was placed at stationing 50+40. The only pier wetted in these model runs was the pier located at stationing 51+98. Therefore, scour was only calculated at this one pier, but the results were applied to all piers (see table 1). Roughness values were estimated from the 7.5-minute topographic map and slope was computed from the elevation data shown on the bridge plans.

RESULTS

Scour depths were computed with a version of WSPRO (Larry Arneson, Federal Highway Administration, written commun., 1996) modified from Shearman (1990). This version of WSPRO includes scour calculations in the model output. Scour depths were calculated assuming an infinite depth of material that could erode and a homogeneous particle-size distribution. The results of the scour analysis are presented in table 1; a complete input file and output results are presented in the appendix.

REFERENCES

- Hoggatt, R.E., 1975, Drainage areas of Indiana streams: U.S. Geological Survey, Water Resources Division, 231 p.
- Indiana State Highway Commission, 1969, Bridge plans Interstate Route 65: Bridge File I-65-120-6016.
- Lagasse, P.F.; Schall, J.D.; Johnson, F.; Richardson, E.V.; and Chang, F., 1995, Stream stability at highway structures (2d ed.): Federal Highway Administration, Hydraulic Engineering Circular No. 20, Publication FHWA-IP-90-014, 144 p.
- Richardson, E.V., and Davis, S.R., 1995, Evaluating scour at bridges (3d ed.): Federal Highway Administration, Hydraulic Engineering Circular No. 18, Publication FHWA-IP-90-017, 204 p.
- Shearman, J.O., 1990, User's manual for WSPRO, a computer model for water-surface profile computations: Federal Highway Administration Publication FHWA-IP-89-027, 177 p.

APPENDIX

WSPRO INPUT FILE

```

T1          Coffman Rd. over Little Eagle Creek and I-65 in Marion Co.
T2          County: Marion                      Indianapolis West 111C
T3          05-30-97                            Robert L. Miller
SI          0
Q           3450      5210
SK          .0033      .0033
XS  EXIT    0        40
GR          4939 774  4961 770  4970 765
GR          5007 764  5032 756  5047 755  5087 755  5104 760  5118 765  5128 770
GR          5139 773  5177 773  5238 774  5312 775  5752 775  6593 780
N           .050      .038      050
SA          5007      5139
XS  FULLV   200 40
GR          4939 774  4961 770  4970 765
GR          5007 764  5032 756  5047 756  5087 756  5104 760  5118 765  5128 770
GR          5139 773  5177 773  5238 774  5312 775  5752 775  6593 780
N           .050      .038      050
SA          5007      5139
BR  BRDGE   200      785      40
GR          5040 789.9
GR          5041 775.5  5058 773.8  5069 769.6  5075 767.9  5108 765.8
GR          5111 764.6  5136 756.1  5162 755.8  5187 756.0  5215 764.8
GR          5233 773.9  5238 773.8  5276 787.7  5280 787.5  5281 789.2
GR          5040 789.9
N           .050      .038
SA          5111
PD 1        760      2      2
CD          3        37      2        788
DC  BRDGE   5115 5209 5223 5285 * 2
DP  BRDGE   5040 5281 2 * * 1.0 1.0 1.1
XS  APPR    437
GR          5094 773  5101 772  5113 771  5119 771  5129 772  5136 773
GR          5170 775  5181 774  5192 770  5194 769  5213 768
GR          5217 767  5232 760  5238 757  5253 756.6  5268 757  5279 760
GR          5288 765  5298 770  5309 775  5313 776  5595 780
N           .050      .038
SA          5201
EX
ER

```

WSPRO OUTPUT

```
***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Run Date & Time: 8/ 4/97 3:33 pm Version V050196
Input File: 6016.dat Output File: 6016.LST
```

```
*-----*
T1 COFFMAN RD. OVER LITTLE EAGLE CREEK AND I-65 IN MARION CO.
T2 COUNTY: MARION INDIANAPOLIS WEST 111C
T3 05-30-97 ROBERT L. MILLER
SI 0
Q 3450 5210
```

*** Processing Flow Data; Placing Information into Sequence 1 ***

```
SK .0033 .0033
```

```
***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English
```

```
*-----*
COFFMAN RD. OVER LITTLE EAGLE CREEK AND I-65 IN MARION CO.
COUNTY: MARION INDIANAPOLIS WEST 111C
05-30-97 ROBERT L. MILLER
```

```
*-----*
* Starting To Process Header Record EXIT *
*-----*
```

```
XS EXIT 0 40
GR 4939 774 4961 770 4970 765
GR 5007 764 5032 756 5047 755 5087 755 5104 760 5118 765
5128 770
GR 5139 773 5177 773 5238 774 5312 775 5752 775 6593 780
N .050 .038 050
SA 5007 5139
```

*** Completed Reading Data Associated With Header Record EXIT ***
 *** Storing X-Section Data In Temporary File As Record Number 1 ***

```
*** Data Summary For Header Record EXIT ***
SRD Location: 0. Cross-Section Skew: 40.0 Error Code 0
Valley Slope: .00000 Averaging Conveyance By Geometric Mean.
Energy Loss Coefficients -> Expansion: .50 Contraction: .00
```

X,Y-coordinates (16 pairs)

X	Y	X	Y	X	Y
4939.000	774.000	4961.000	770.000	4970.000	765.000
5007.000	764.000	5032.000	756.000	5047.000	755.000
5087.000	755.000	5104.000	760.000	5118.000	765.000
5128.000	770.000	5139.000	773.000	5177.000	773.000
5238.000	774.000	5312.000	775.000	5752.000	775.000
6593.000	780.000				

WSPRO OUTPUT

Minimum and Maximum X,Y-coordinates

```

Minimum X-Station:  4939.000  ( associated Y-Elevation:  774.000 )
Maximum X-Station:  6593.000  ( associated Y-Elevation:  780.000 )
Minimum Y-Elevation:  755.000  ( associated X-Station:  5087.000 )
Maximum Y-Elevation:  780.000  ( associated X-Station:  6593.000 )
    
```

X-coordinates & Horizontal Breakpoints Translated by Skew Angle

X Input	X Skewed	X Input	X Skewed	X Input	X Skewed
4939.000	4973.625	4961.000	4990.479	4970.000	4997.373
5007.000	5025.716	5032.000	5044.868	5047.000	5056.358
5087.000	5087.000	5104.000	5100.023	5118.000	5110.748
5128.000	5118.408	5139.000	5126.834	5177.000	5155.944
5238.000	5202.673	5312.000	5259.360	5752.000	5596.419
6593.000	6240.663				

Roughness Data (3 SubAreas)

SubArea	Roughness Coefficient	Horizontal Breakpoint
1	.050	---
	---	5025.716
2	.038	---
	---	5126.834
3	*****	---

```

*-----*
* Finished Processing Header Record EXIT *
*-----*
    
```

```

***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English
    
```

```

*-----*
COFFMAN RD. OVER LITTLE EAGLE CREEK AND I-65 IN MARION CO.
COUNTY: MARION INDIANAPOLIS WEST 111C
05-30-97 ROBERT L. MILLER
    
```

```

*-----*
* Starting To Process Header Record FULLV *
*-----*
    
```

```

XS FULLV 200 40
GR 4939 774 4961 770 4970 765
GR 5007 764 5032 756 5047 756 5087 756 5104 760 5118 765
5128 770
GR 5139 773 5177 773 5238 774 5312 775 5752 775 6593 780
N .050 .038 050
SA 5007 5139
    
```

WSPRO OUTPUT

*** Completed Reading Data Associated With Header Record FULLV ***
 *** Storing X-Section Data In Temporary File As Record Number 2 ***

*** Data Summary For Header Record FULLV ***

SRD Location: 200. Cross-Section Skew: 40.0 Error Code 0
 Valley Slope: .00000 Averaging Conveyance By Geometric Mean.
 Energy Loss Coefficients -> Expansion: .50 Contraction: .00

X,Y-coordinates (16 pairs)					
X	Y	X	Y	X	Y
4939.000	774.000	4961.000	770.000	4970.000	765.000
5007.000	764.000	5032.000	756.000	5047.000	756.000
5087.000	756.000	5104.000	760.000	5118.000	765.000
5128.000	770.000	5139.000	773.000	5177.000	773.000
5238.000	774.000	5312.000	775.000	5752.000	775.000
6593.000	780.000				

Minimum and Maximum X,Y-coordinates

Minimum X-Station: 4939.000 (associated Y-Elevation: 774.000)
 Maximum X-Station: 6593.000 (associated Y-Elevation: 780.000)
 Minimum Y-Elevation: 756.000 (associated X-Station: 5087.000)
 Maximum Y-Elevation: 780.000 (associated X-Station: 6593.000)

X-coordinates & Horizontal Breakpoints Translated by Skew Angle					
X Input	X Skewed	X Input	X Skewed	X Input	X Skewed
4939.000	4973.625	4961.000	4990.479	4970.000	4997.373
5007.000	5025.716	5032.000	5044.868	5047.000	5056.358
5087.000	5087.000	5104.000	5100.023	5118.000	5110.748
5128.000	5118.408	5139.000	5126.834	5177.000	5155.944
5238.000	5202.673	5312.000	5259.360	5752.000	5596.419
6593.000	6240.663				

Roughness Data (3 SubAreas)		
SubArea	Roughness Coefficient	Horizontal Breakpoint
1	.050	---
	---	5025.716
2	.038	---
	---	5126.834
3	*****	---

 * Finished Processing Header Record FULLV *

***** W S P R O *****
 Federal Highway Administration - U. S. Geological Survey

WSPRO OUTPUT

5238.000	5220.219	5276.000	5249.329	5280.000	5252.393
5281.000	5253.159	5040.000	5068.542		

```

Roughness Data ( 2 SubAreas )
      Roughness   Horizontal
SubArea Coefficient Breakpoint
-----
      1           .050         ---
      1           ---         5122.932
      2           .038         ---
-----
    
```

```

Discharge coefficient parameters
BRType BRWdth  EMBSS  EMBELv  UserCD
3       37.000  2.00   788.000 *****
    
```

```

Pressure flow elevations
      AVBCEL      PFElev
*****          785.000
    
```

```

Abutment Parameters
ABSLPL  ABSLPR  XTOELT  YTOELT  XTOERT  YTOERT
*****  *****  *****  *****  *****  *****
    
```

```

Pier/Pile Data ( 1 Group(s) )
Code Indicates Bridge Uses Piles
Group Elevation Gross Width Number
-----
      1       760.000      2.000      2
-----
    
```

```

*-----*
* Finished Processing Header Record BRDGE *
*-----*
    
```

```

***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English
*-----*
    
```

```

COFFMAN RD. OVER LITTLE EAGLE CREEK AND I-65 IN MARION CO.
COUNTY: MARION INDIANAPOLIS WEST 111C
05-30-97 ROBERT L. MILLER
    
```

```

DC BRDGE 5115 5209 5223 5285 * 2
DP BRDGE 5040 5281 2 * * 1.0 1.0 1.1
    
```

```

*-----*
* Starting To Process Header Record APPR *
*-----*
    
```

```

XS APPR 437
GR 5094 773 5101 772 5113 771 5119 771 5129 772 5136 773
    
```

WSPRO OUTPUT

```

GR          5170 775  5181 774  5192 770  5194 769  5213 768
GR          5217 767  5232 760  5238 757  5253 756.6  5268 757  5279 760
GR          5288 765  5298 770  5309 775  5313 776  5595 780
N           .050   .038
SA          5201
  
```

```

*** Completed Reading Data Associated With Header Record APPR ***
*** Storing X-Section Data In Temporary File As Record Number 4 ***
  
```

```

*** Data Summary For Header Record APPR ***
SRD Location:      437.  Cross-Section Skew:   .0  Error Code  0
Valley Slope:     .00000  Averaging Conveyance By Geometric Mean.
Energy Loss Coefficients -> Expansion:   .50  Contraction:   .00
  
```

X,Y-coordinates (22 pairs)

X	Y	X	Y	X	Y
5094.000	773.000	5101.000	772.000	5113.000	771.000
5119.000	771.000	5129.000	772.000	5136.000	773.000
5170.000	775.000	5181.000	774.000	5192.000	770.000
5194.000	769.000	5213.000	768.000	5217.000	767.000
5232.000	760.000	5238.000	757.000	5253.000	756.600
5268.000	757.000	5279.000	760.000	5288.000	765.000
5298.000	770.000	5309.000	775.000	5313.000	776.000
5595.000	780.000				

Minimum and Maximum X,Y-coordinates

```

Minimum X-Station:  5094.000  ( associated Y-Elevation:  773.000 )
Maximum X-Station:  5595.000  ( associated Y-Elevation:  780.000 )
Minimum Y-Elevation:  756.600  ( associated X-Station:  5253.000 )
Maximum Y-Elevation:  780.000  ( associated X-Station:  5595.000 )
  
```

Roughness Data (2 SubAreas)

SubArea	Roughness Coefficient	Horizontal Breakpoint
1	.050	---
	---	5201.000
2	.038	---

```

Bridge datum projection(s):  XREFLT  XREFRT  FDSTLT  FDSTRT
*****  *****  *****  *****
  
```

```

*-----*
* Finished Processing Header Record APPR *
*-----*
  
```

```

***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English
*-----*
  
```


WSPRO OUTPUT

KRATIO: .55

Section: APPR	763.344	2.047	3450.000	300.717	237.000	5224.834
Header Type: AS	765.391	1.394	11.473	33368.17	237.000	5285.020
SRD: 437.000	762.991	.684	.905	.0059	1.000	-.001

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

<<< The Following Data Reflect The "Constricted" Profile >>>

<<< Beginning Bridge/Culvert Hydraulic Computations >>>

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: BRDGE	762.948	.729	3450.000	503.897	200.000	5115.859
Header Type: BR	763.677	1.019	6.847	59744.12	200.000	5209.107
SRD: 200.000	760.671	.000	.519	*****	1.000	-.005

Specific Bridge Information	C	P/A	PFELEV	BLEN	XLAB	XRAB
Bridge Type 3	Flow Type 1					
Pier/Pile Code 1	1.0000	.012	785.000	*****	*****	*****

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: APPR	763.887	1.660	3450.000	333.969	200.000	5223.671
Header Type: AS	765.547	1.571	10.330	38761.66	200.000	5285.997
SRD: 437.000	762.991	.290	.787	.0059	1.000	-.015

Approach Section APPR	Flow Contraction Information
M(G) M(K) KQ	XLKQ XRKQ OTEL
.000 .000 38921.0	5208.419 5301.698 763.887

<<< End of Bridge Hydraulics Computations >>>

***** W S P R O *****
 Federal Highway Administration - U. S. Geological Survey
 Model for Water-Surface Profile Computations.
 Input Units: English / Output Units: English

-----*
 COFFMAN RD. OVER LITTLE EAGLE CREEK AND I-65 IN MARION CO.
 COUNTY: MARION INDIANAPOLIS WEST 111C
 05-30-97 ROBERT L. MILLER

WSEL	VHD	Q	AREA	SRDL	LEW
EGEL	HF	V	K	FLEN	REW
CRWS	HO	FR #	SF	ALPHA	ERR

WSPRO OUTPUT

```

-----
Section: EXIT      763.637   .909   5210.000   681.562 ***** 5008.133
Header Type: XS   764.546 *****   7.644   90667.70 ***** 5114.185
SRD:      .000   761.023 *****   .532   *****   1.000   *****

Section: FULLV    764.349   .855   5210.000   704.546   200.000 4994.098
Header Type: FV   765.204   .640   7.395   93608.65   200.000 5116.176
SRD:    200.000   761.694   .000   .544   .0032   1.006   .018
  
```

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

```

===125 FR# EXCEEDS FNTEST AT SECID "APPR ": TRIALS CONTINUED.
      FNTEST, FR#, WSEL, CRWS:   .80   .96   764.81   764.61
  
```

```

===110 WSEL NOT FOUND AT SECID "APPR ": REDUCED DELTAY.
      WSLIM1, WSLIM2, DELTAY:  764.61   780.00   .50
  
```

```

===115 WSEL NOT FOUND AT SECID "APPR ": USED WSMIN = CRWS.
      WSLIM1, WSLIM2, CRWS:   764.61   780.00   764.61
  
```

```

===135 CONVEYANCE RATIO OUTSIDE OF RECOMMENDED LIMITS AT SECID "APPR ".
      KRATIO:   .52
  
```

```

Section: APPR      764.816  2.725   5210.000   393.560   237.000 5221.681
Header Type: AS    767.541  1.405   13.238   48930.52   237.000 5287.668
SRD:    437.000   764.615   .935   .956   .0059   1.000   -.002
  
```

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

<<< The Following Data Reflect The "Constricted" Profile >>>
 <<< Beginning Bridge/Culvert Hydraulic Computations >>>

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: BRDGE	764.700	.922	5210.000	676.641	200.000	5110.751
Header Type: BR	765.622	1.077	7.700	90809.19	200.000	5214.681
SRD: 200.000	762.027	.000	.532	*****	1.000	.006

Specific Bridge Information	C	P/A	PFELEV	BLEN	XLAB	XRAB
Bridge Type 3 Flow Type 1	-----	-----	-----	-----	-----	-----
Pier/Pile Code 1	1.0000	.014	785.000	*****	*****	*****

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: APPR	765.387	2.263	5210.000	431.920	200.000	5220.456
Header Type: AS	767.650	1.757	12.062	55753.34	200.000	5288.774
SRD: 437.000	764.615	.279	.846	.0059	1.000	.014

WSPRO OUTPUT

```

Approach Section APPR Flow Contraction Information
M( G )   M( K )   KQ       XLKQ       XRKQ       OTEL
-----
      .000      .000  55586.8 5203.419 5307.313 765.387
-----
  
```

<<< End of Bridge Hydraulics Computations >>>

```

***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English
  
```

```

*-----*
COFFMAN RD. OVER LITTLE EAGLE CREEK AND I-65 IN MARION CO.
COUNTY: MARION                                INDIANAPOLIS WEST 111C
05-30-97                                       ROBERT L. MILLER
  
```

*** Live-Bed Contraction Scour Calculations for Header Record BRDGE ***

Constants and Input Variables

```

*-----*
Bed Material Transport Mode Factor (k1):   .64
Total Pier Width Value                    (Pw): 2.000
*-----*
  
```

#	Scour Depth	-- Flow --		-- Width --		--- X-Limits ---	
		Contract	Approach	Contract	Approach	Side	Contract Approach
1	-1.201	3450.000	3450.000	92.000	62.000	Left:	5115.000 5223.000
	Approach Channel Depth:		5.382	Right:	5209.000 5285.000
* Negative Scour Depth Encountered - Check If Variables Are Reasonable *							
2	-1.536	5210.000	5210.000	92.000	62.000	Left:	5115.000 5223.000
	Approach Channel Depth:		6.881	Right:	5209.000 5285.000
* Negative Scour Depth Encountered - Check If Variables Are Reasonable *							

```

***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English
  
```

```

*-----*
COFFMAN RD. OVER LITTLE EAGLE CREEK AND I-65 IN MARION CO.
COUNTY: MARION                                INDIANAPOLIS WEST 111C
05-30-97                                       ROBERT L. MILLER
  
```

*** Pier Scour Calculations for Header Record BRDGE ***

Constants and Input Variables

```

Pier Width: 2.000
*-----*
  
```

WSPRO OUTPUT

Pier Shape Factor (K1): 1.00
Flow Angle of Attack Factor (K2): 1.00
Bed Condition Factor (K3): 1.10
Bed Material Factor (K4): 1.00
Velocity Multiplier (VM): 1.00
Depth Multiplier (YM): 1.00

#	Scour	---- Localized Hydraulic Properties ----					-- X-Stations --	
	Depth	Flow	WSE	Depth	Velocity	Froude #	Left	Right
1	5.15	3450.000	763.238	7.438	7.652	.494	5040.000	5281.000
2	5.62	5210.000	764.979	9.179	8.778	.511	5040.000	5281.000

ER

***** Normal end of WSPRO execution. *****
***** Elapsed Time: 0 Minutes 3 Seconds *****