

Modified Level II Streambed-Scour Analysis for Structure I-74-70-4437 Crossing Eagle Creek in Marion County, Indiana

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CONVERSION FACTORS AND ABBREVIATIONS

Multiply	By	To obtain
inch (in.)	25.4	millimeter
foot (ft)	0.3048	meter
square foot (ft ²)	929.0	square centimeter
feet per second (ft/s)	0.3048	meters per second
cubic foot per second (ft ³ /s)	0.02832	cubic meter per second
mile (mi)	1.609	kilometer
square mile (mi ²)	2.590	square kilometer

ABBREVIATIONS used in this report:

D ₅₀	median diameter of bed material
Q100	100-year discharge
FEMA	Federal Emergency Management Agency
HEC	Hydraulic Engineering Circular
IDNR	Indiana Department of Natural Resources
INDOT	Indiana Department of Transportation
USGS	United States Geological Survey
WSPRO	Water Surface PROfile model

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By Bret A. Robinson, David C. Voelker, *and* Robert L. Miller

ABSTRACT

Level II scour evaluations follow a process in which hydrologic, hydraulic, and sediment-transport data are evaluated to calculate the depth of scour that may result when a given discharge is routed through a bridge opening. The results of the modified Level II analysis for structure I-74-70-4437 on Interstate 74 crossing Eagle Creek in Marion County, Indiana, are presented. The site is near the town of Clermont in the northwestern part of Marion County. Scour depths were computed with the Water Surface PROfile model, version V050196, which incorporates the scour-calculation procedures outlined in Hydraulic Engineering Circular No. 18. Total scour depths at the piers were approximately 16.0 feet for the modeled discharge of 22,700 cubic feet per second and approximately 31.7 feet for the modeled discharge of 31,600 cubic feet per second.

INTRODUCTION

The U.S. Geological Survey (USGS), in cooperation with the Indiana Department of Transportation (INDOT), is conducting Level II scour analyses at a number of bridges throughout Indiana. This report describes the methods applied and the modeling results for bridge I-74-70-4437.

Background and Scope

Level I scour assessment is a process where a large number of bridges are studied as a group. Assessments usually are made by evaluating a combination of geomorphic, hydrologic, and bridge-characteristic data. The results help investigators determine which bridges appear to be most likely to experience streambed-scour problems and which bridges appear to be relatively immune to problems brought on by streambed scour (for example, bridges built on bedrock).

When applied correctly, Level I scour assessments provide an investigator with information to identify those bridges that appear to be relatively safe and those bridges that fall into higher risk categories.

Level II scour evaluations describe the process for an investigator to apply a model to a bridge site and calculate the potential depth of scour that may result from a given flood event. Level II analyses involve the application of basic hydrologic, hydraulic, and sediment-transport engineering concepts and may include an evaluation of flood history, channel hydraulic conditions (for example, water-surface profile analysis), and basic sediment-transport analyses such as scour calculations (Lagasse and others, 1995).

The methods and model outlined in Hydraulic Engineering Circular (HEC) No. 18 (Richardson and Davis, 1995) formulate the basis for Level II scour evaluations. Methods used in this study for Level II scour evaluations are a modification of the HEC-18 standards. These modifications were made to comply with the methodology requested by INDOT (Merril Dougherty, Indiana Department of Transportation, oral commun., 1996). Descriptions of the specific modifications are given in the "Evaluation Methods" section of this report.

This report presents the methods followed for modeling, special considerations for this study site, and the input for and the output from the Water Surface PROfile (WSPRO) model.

Site Description

The study site is located near the town of Clermont in the northwestern part of Marion County. This site is immediately downstream from the Eagle Creek Reservoir Dam. The drainage area for the site is approximately 168 mi² (estimated using Hoggatt, 1975, and USGS 7.5-minute topographic data). The predominant land uses in the basin are suburban and agricultural; in the immediate vicinity of the bridge, the land is predominantly grassy with forest nearby.

Within the immediate vicinity of the bridge, Eagle Creek has a channel-bed slope of approximately 0.0012 ft/ft. The channel-bed material is cobbly sandy silt-clay, and the channel banks consist of gravelly sandy silt/clay. At the time of the Level I site visit on June 26, 1995, the banks were observed to have 0 to 50 percent woody vegetative cover; the field report noted that the banks were experiencing some fluvial erosion.

The Interstate 74 crossing of Eagle Creek is a 252-ft-long, multi-lane bridge consisting of five spans supported by concrete and steel piers and sloping earthen spill-through abutments. Additional details describing conditions at the site are included in the Level I data base (Hopkins and Robinson, unpub. data, 1997). Photographs of the site, taken at the time of the Level I site visit, are archived at the USGS office in Indianapolis.

EVALUATION METHODS

The methods described in this section apply to a number of bridge sites in Indiana being evaluated for scour and outline the procedures requested by INDOT for these modified Level II scour analyses. The principal modification requested by INDOT was that the input data to the model come from or be estimated from existing data sources; no additional field data were collected. Actual methods used in the scour evaluation at this particular bridge site use the most applicable method possible, given the data available.

To determine drainage area, either published values found in Hoggatt (1975) or 7.5-minute topographic maps with Hoggatt's original drainage-area delineations were used. Where there are no published data, drainage-area segments measured from the maps produced by Hoggatt were either subtracted from downstream sites or added to upstream sites published by Hoggatt (1975).

In Indiana, flood discharges are coordinated by agreement among State and Federal agencies. At sites where flood discharges officially are coordinated among State and Federal agencies in Indiana, the coordinated 100-year discharge (Q100) was modeled. INDOT also provided an additional flood discharge for these coordinated sites in excess of the Q100 to be modeled.

If a flood discharge was not coordinated, the USGS examined Federal Emergency Management Agency (FEMA) studies for Q100 determinations. Where FEMA studies did not produce a Q100, the USGS contacted IDNR for an estimated Q100 in the vicinity of the site being studied. If IDNR did not have a Q100, data from nearby USGS streamflow-gaging stations were analyzed with nearby and similar drainage basins that have been coordinated. At sites having no coordinated discharge data, the two discharges used in the model were 1) the approximated Q100 and 2) a discharge equal to 1.7 times the approximated Q100.

Most of the cross-section and bridge-opening geometry data were taken from the bridge plans (Indiana State Highway Commission, 1958) provided by INDOT. Bridge plans are presumed to be representative of current conditions at the site. To determine the cross-section geometry, a line was drawn on the bridge plans parallel to the bridge stationing and approximately one bridge width from the bridge. For sites where the bridge plans did not extend far enough laterally for collection of all cross-section data required for WSPRO model analysis, additional data were collected from 7.5-minute topographic maps.

The roadway and embankment profile was taken from the bridge and highway plans for those sites where roadway overtopping was expected. The INDOT bridge plans and 7.5-minute topographic maps were used as a guide, based on the water-surface elevations calculated by the WSPRO model, to determine if roadway overtopping might occur.

Roughness values (n -values) for the main channel were estimated by viewing photographs archived from the Level I scour assessments. The n -values for the overbanks were assigned on the basis of the surface-cover data summarized in the Level I data base (Hopkins and Robinson, unpub. data, 1997). From those data, the following roughness values were assigned to the surface-cover categories: urban—0.050, suburban—0.035, row crop—0.045, pasture—0.035, brush—0.120, forest—0.100, and wetland (any area covered by standing water)—0.100. The n -values for the overbanks were adjusted if the Level I photographs provided sufficient detail to warrant an adjustment.

WSPRO version V050196 was used to model flow through the study site. Starting water-surface elevation was obtained with a slope-conveyance computation. The channel-bed slope in the immediate vicinity of the bridge was estimated from the 7.5-minute topographic map and was used as the slope of the energy grade line for this computation.

WSPRO version V050196 includes a field that allows the input of up to four scour-adjustment factors (K1 to K4). For this modeling, the default value for K4 (bed armoring) was chosen. For scour-adjustment factors K1 and K2 (pier-nose shape and angle of attack, respectively), input values were determined by evaluating the data archived in the Level I data base (Hopkins and Robinson, unpub. data, 1997). For the K3 factor (bed forms), a value of 1.1 was applied in all cases.

In some cases, piers set on the overbanks are constructed with footings that are higher in elevation than pier footings in the main channel. In these situations, if the channel position changes, the piers that were initially constructed on the overbank may become part of the main channel. Therefore, to evaluate total potential scour, the model results obtained for contraction scour and deepest local scour in the main channel were added and applied to all piers in the bridge opening. This methodology allowed for an evaluation of potential undermining of pier supports in the event that future channel movement placed overbank piers in the main channel.

Where bridge pairs have a continuous abutment or fill between the bridges that does not allow expansion of flow, the bridge pair was modeled as one bridge. Sites with discontinuous abutments, allowing expansion between the bridges, were modeled as two separate bridges. In those cases, a valley cross section was measured between the bridges and used as the approach section for the downstream bridge and as the exit section for the upstream bridge.

At sites with no embankment to function as a weir or at sites where the tailwater drowns out the embankment, a composite bridge and road section was used to compute flow. Those sites were computed with friction-loss equations rather than with a bridge routine.

Total scour is taken as the sum of local scour plus contraction scour. If the model predicted negative contraction scour (aggradation), the contraction-scour value was assumed to be zero in determining the total scour depth (table 1). This assumption was made so that a negative contraction scour would not mask the potentially detrimental effects of local scour at a pier. No abutment scour evaluations were made in this study.

Table 1. Cumulative scour depths for the modeled discharges at structure I-74-70-4437 crossing Eagle Creek in Marion County, Indiana
 [--, no value]

Pier number ¹	Stationing from bridge plans ²	Initial bed-elevation at pier (feet)	Main-channel contraction scour depth (feet)	Local scour depth (feet)	Worst-case total-scour depth ³ (feet)	Bottom elevation of pier (feet)	Worst-case bed elevation after scour ⁴ (feet)
Modeled discharge⁵ is 22,700 cubic feet per second							
1	749+06	753	9.0	7.0	16.0	737.4	724.5
2	749+67	741	9.0	7.0	16.0	734.4	724.5
3	750+39	741	9.0	7.0	16.0	734.4	724.5
4	751+00	753	9.0	7.0	16.0	737.4	724.5
Modeled discharge is 31,600 cubic feet per second							
1	749+06	753	23.9	7.8	31.7	737.4	708.8
2	749+67	741	23.9	7.8	31.7	734.4	708.8
3	750+39	741	23.9	7.8	31.7	734.4	708.8
4	751+00	753	23.9	7.8	31.7	737.7	708.8

¹Pier numbers were assigned from left to right as shown on the bridge plans.

²Stationing is the center line of the pier as determined from the bridge plans. Stationing from bridge plan, 749+06, represents a point 74,906 feet from an arbitrary starting location referenced on the bridge plans.

³Worst-case total-scour depths are generated by summing the calculated contraction-scour depth with the worst case of local scour.

⁴Worst-case bed elevation is computed by subtracting the worst-case total-scour depth from the lowest initial bed elevation in the bridge opening (740.5 feet).

⁵Coordinated discharge.

SPECIAL CONSIDERATIONS

Model runs indicate the water-surface elevation at the bridge is lower than the low-steel elevation for the modeled discharges. Therefore, there should be no pressure flow through the bridge opening for the discharges modeled. This site has been modeled without consideration for how Eagle Creek Reservoir affects the hydraulics or streambed-scour conditions.

RESULTS

Scour depths were computed with a version of WSPRO (Larry Arneson, Federal Highway Administration, written commun., 1996) modified from Shearman (1990). This version of WSPRO includes scour calculations in the model output. Scour depths were calculated assuming an infinite depth of material that could erode and a homogeneous particle-size distribution. The results of the scour analysis are presented in table 1; a complete input file and output results are presented in the appendix.

REFERENCES

- Hoggatt, R.E., 1975, Drainage areas of Indiana streams: U.S. Geological Survey, Water Resources Division, 231 p.
- Indiana State Highway Commission, 1958, Bridge plans Interstate Route 74: Bridge File I-74-70-4437.
- Lagasse, P.F.; Schall, J.D.; Johnson, F.; Richardson, E.V.; and Chang, F., 1995, Stream stability at highway structures (2d ed.): Federal Highway Administration, Hydraulic Engineering Circular No. 20, Publication FHWA-IP-90-014, 144 p.
- Richardson, E.V., and Davis, S.R., 1995, Evaluating scour at bridges (3d ed.): Federal Highway Administration, Hydraulic Engineering Circular No. 18, Publication FHWA-IP-90-017, 204 p.
- Shearman, J.O., 1990, User's manual for WSPRO, a computer model for water-surface profile computations: Federal Highway Administration Publication FHWA-IP-89-027, 177 p.

APPENDIX

WSPRO INPUT FILE

T1 I-74 Over Eagle Creek I74-70-4437
T2 County: Marion Quad: Clermont 110D
T3 11-14-96 Bret A. Robinson
SI 0
Q 22700 31600
SK .0012 .0012
XS EXIT 0
GR 73337 790 73402 765 73773 760 74146 755 74882 751 74936 745
GR 74976 741.5 75027 740.5 75075 741.5 75082 743 75101 745
GR 75117 750 76050 760 76174 790
N .100 .034 .100
SA 74800 75150
XS FULLV 250
GR 73337 790 73402 765 73773 760 74146 755 74882 751 74936 745
GR 74976 741.5 75027 740.5 75075 741.5 75082 743 75101 745
GR 75117 750 76050 760 76174 790
N .100 .034 .100
SA 74800 75150
BR BRDGE 250 764.7 0
GR 74876 0764.7 74876 0764.4 74882 0764.3 74917 0749.1 74942 0746.6
GR 74956 0741.6 75005 0740.5 75056 0741.5 75069 0746.6 75092 0749.0
GR 75130 0764.3 75135 0764.3 75135 0764.8 74876 0764.7
N .034
PD 742 4 1
PD 753 4 2
PD 753 8 3
CD 3 125 2 764.2
DC 0 BRDGE 74906 75105 74890 75110 * 8
* LPierEdge RPierEdge PierWdth * * K1 K2 K3(1.1)
DP 74876 75135 2 * * 1 1 1.1
DP 74876 75135 2 * * 1 1 1.1
DP 74876 75135 2 * * 1 1 1.1
DP 74876 75135 2 * * 1 1 1.1
XS APPR 625
GR 72665 780 72848 775 73720 760 74332 755 74882 751 74936 745
GR 74976 741.5 75027 740.5 75075 741.5 75082 743 75091 745
GR 75117 750 75720 755 76105 760 76233 780
N .034
EX
ER

WSPRO OUTPUT

***** W S P R O *****

Federal Highway Administration - U. S. Geological Survey
 Model for Water-Surface Profile Computations.

Run Date & Time: 8/ 4/97 1:51 pm Version V050196
 Input File: 4437.dat Output File: 4437.LST

```
T1      I-74 OVER EAGLE CREEK          I74-70-4437
T2      COUNTY: MARION                 QUAD: CLERMONT 110D
T3      11-14-96                       BRET A. ROBINSON
SI      0
Q       22700 31600
```

*** Processing Flow Data; Placing Information into Sequence 1 ***

SK .0012 .0012

***** W S P R O *****

Federal Highway Administration - U. S. Geological Survey
 Model for Water-Surface Profile Computations.

Input Units: English / Output Units: English

```
I-74 OVER EAGLE CREEK          I74-70-4437
COUNTY: MARION                 QUAD: CLERMONT 110D
11-14-96                       BRET A. ROBINSON
```

 * Starting To Process Header Record EXIT *

```
XS  EXIT 0
GR   73337 790 73402 765 73773 760 74146 755 74882 751 74936 745
GR   74976 741.5 75027 740.5 75075 741.5 75082 743 75101 745
GR   75117 750 76050 760 76174 790
N    .100 .034 .100
SA   74800 75150
```

*** Completed Reading Data Associated With Header Record EXIT ***
 *** Storing X-Section Data In Temporary File As Record Number 1 ***

*** Data Summary For Header Record EXIT ***
 SRD Location: 0. Cross-Section Skew: .0 Error Code 0
 Valley Slope: .00000 Averaging Conveyance By Geometric Mean.
 Energy Loss Coefficients -> Expansion: .50 Contraction: .00

X,Y-coordinates (14 pairs)

X	Y	X	Y	X	Y
73337.000	790.000	73402.000	765.000	73773.000	760.000
74146.000	755.000	74882.000	751.000	74936.000	745.000
74976.000	741.500	75027.000	740.500	75075.000	741.500
75082.000	743.000	75101.000	745.000	75117.000	750.000
76050.000	760.000	76174.000	790.000		

WSPRO OUTPUT

Minimum and Maximum X,Y-coordinates

```

Minimum X-Station:  73337.000  ( associated Y-Elevation:  790.000 )
Maximum X-Station:  76174.000  ( associated Y-Elevation:  790.000 )
Minimum Y-Elevation:  740.500  ( associated X-Station:  75027.000 )
Maximum Y-Elevation:  790.000  ( associated X-Station:  73337.000 )
    
```

Roughness Data (3 SubAreas)

SubArea	Roughness Coefficient	Horizontal Breakpoint
1	.100	---
	---	*****
2	.034	---
	---	*****
3	.100	---

```

*-----*
*           Finished Processing Header Record EXIT           *
*-----*
    
```

```

***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English
    
```

```

*-----*
I-74 OVER EAGLE CREEK                I74-70-4437
COUNTY: MARION                       QUAD: CLERMONT 110D
11-14-96                               BRET A. ROBINSON
    
```

```

*-----*
*           Starting To Process Header Record FULLV         *
*-----*
    
```

```

XS  FULLV 250
GR   73337 790 73402 765 73773 760 74146 755 74882 751 74936 745
GR   74976 741.5 75027 740.5 75075 741.5 75082 743 75101 745
GR   75117 750 76050 760 76174 790
N     .100 .034 .100
SA   74800 75150
    
```

```

*** Completed Reading Data Associated With Header Record FULLV ***
*** Storing X-Section Data In Temporary File As Record Number 2 ***
    
```

```

*** Data Summary For Header Record FULLV ***
SRD Location:      250.  Cross-Section Skew:      .0  Error Code  0
Valley Slope:     .00000  Averaging Conveyance By Geometric Mean.
Energy Loss Coefficients -> Expansion:      .50  Contraction:      .00
    
```

X,Y-coordinates (14 pairs)

X	Y	X	Y	X	Y
73337.000	790.000	73402.000	765.000	73773.000	760.000
74146.000	755.000	74882.000	751.000	74936.000	745.000

WSPRO OUTPUT

```

74976.000      741.500      75027.000      740.500      75075.000      741.500
75082.000      743.000      75101.000      745.000      75117.000      750.000
76050.000      760.000      76174.000      790.000
-----

```

Minimum and Maximum X,Y-coordinates

```

Minimum X-Station:  73337.000  ( associated Y-Elevation:  790.000 )
Maximum X-Station:  76174.000  ( associated Y-Elevation:  790.000 )
Minimum Y-Elevation:  740.500  ( associated X-Station:  75027.000 )
Maximum Y-Elevation:  790.000  ( associated X-Station:  73337.000 )

```

Roughness Data (3 SubAreas)

SubArea	Roughness Coefficient	Horizontal Breakpoint
1	.100	---
	---	*****
2	.034	---
	---	*****
3	.100	---

```

*-----*
*      Finished Processing Header Record FULLV      *
*-----*

```

```

***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English

```

```

*-----*
I-74 OVER EAGLE CREEK                I74-70-4437
COUNTY: MARION                       QUAD: CLERMONT 110D
11-14-96                               BRET A. ROBINSON

```

```

*-----*
*      Starting To Process Header Record BRDGE      *
*-----*

```

```

BR  BRDGE 250 764.7 0
GR    74876 0764.7  74876 0764.4  74882 0764.3  74917 0749.1  74942
0746.6
GR    74956 0741.6  75005 0740.5  75056 0741.5  75069 0746.6  75092
0749.0
GR    75130 0764.3  75135 0764.3  75135 0764.8  74876 0764.7
N      .034
PD    742 4 1
PD    753 4 2
PD    753 8 3
CD    3 125 2 764.2

```

```

*** Completed Reading Data Associated With Header Record BRDGE ***
+++072 NOTICE: X-coordinate # 2 increased to eliminate vertical segment.
+++072 NOTICE: X-coordinate #13 increased to eliminate vertical segment.

```

WSPRO OUTPUT

*** Storing Bridge Data In Temporary File As Record Number 3 ***

*** Data Summary For Bridge Record BRDGE ***

SRD Location: 250. Cross-Section Skew: .0 Error Code 0
 Valley Slope: ***** Averaging Conveyance By Geometric Mean.
 Energy Loss Coefficients -> Expansion: .50 Contraction: .00

X,Y-coordinates (14 pairs)

X	Y	X	Y	X	Y
74876.000	764.700	74876.100	764.400	74882.000	764.300
74917.000	749.100	74942.000	746.600	74956.000	741.600
75005.000	740.500	75056.000	741.500	75069.000	746.600
75092.000	749.000	75130.000	764.300	75135.000	764.300
75135.100	764.800	74876.000	764.700		

Minimum and Maximum X,Y-coordinates

Minimum X-Station: 74876.000 (associated Y-Elevation: 764.700)
 Maximum X-Station: 75135.100 (associated Y-Elevation: 764.800)
 Minimum Y-Elevation: 740.500 (associated X-Station: 75005.000)
 Maximum Y-Elevation: 764.800 (associated X-Station: 75135.100)

Roughness Data (1 SubAreas)

SubArea	Roughness Coefficient	Horizontal Breakpoint
1	.034	---

Discharge coefficient parameters

BRType	BRwidth	EMBSS	EMBElv	UserCD
3	125.000	2.00	764.200	*****

Pressure flow elevations

AVBCEL	PFElev
*****	764.700

Abutment Parameters

ABSLPL	ABSLPR	XTOELT	YTOELT	XTOERT	YTOERT
*****	*****	*****	*****	*****	*****

Pier/Pile Data (3 Group(s))

Code Indicates Bridge Uses Piers

Group	Elevation	Gross Width	Number
1	742.000	4.000	1
2	753.000	4.000	2
3	753.000	8.000	3

* Finished Processing Header Record BRDGE *

WSPRO OUTPUT

***** W S P R O *****
 Federal Highway Administration - U. S. Geological Survey
 Model for Water-Surface Profile Computations.
 Input Units: English / Output Units: English

I-74 OVER EAGLE CREEK	I74-70-4437
COUNTY: MARION	QUAD: CLERMONT 110D
11-14-96	BRET A. ROBINSON

DC 0 BRDGE 74906 75105 74890 75110 * 8
 DP 74876 75135 2 * * 1 1 1.1
 DP 74876 75135 2 * * 1 1 1.1
 DP 74876 75135 2 * * 1 1 1.1
 DP 74876 75135 2 * * 1 1 1.1

* Starting To Process Header Record APPR *

XS APPR 625
 GR 72665 780 72848 775 73720 760 74332 755 74882 751 74936 745
 GR 74976 741.5 75027 740.5 75075 741.5 75082 743 75091 745
 GR 75117 750 75720 755 76105 760 76233 780
 N .034

*** Completed Reading Data Associated With Header Record APPR ***
 *** Storing X-Section Data In Temporary File As Record Number 4 ***

*** Data Summary For Header Record APPR ***
 SRD Location: 625. Cross-Section Skew: .0 Error Code 0
 Valley Slope: .00000 Averaging Conveyance By Geometric Mean.
 Energy Loss Coefficients -> Expansion: .50 Contraction: .00

X,Y-coordinates (15 pairs)

X	Y	X	Y	X	Y
72665.000	780.000	72848.000	775.000	73720.000	760.000
74332.000	755.000	74882.000	751.000	74936.000	745.000
74976.000	741.500	75027.000	740.500	75075.000	741.500
75082.000	743.000	75091.000	745.000	75117.000	750.000
75720.000	755.000	76105.000	760.000	76233.000	780.000

Minimum and Maximum X,Y-coordinates

Minimum X-Station: 72665.000 (associated Y-Elevation: 780.000)
 Maximum X-Station: 76233.000 (associated Y-Elevation: 780.000)
 Minimum Y-Elevation: 740.500 (associated X-Station: 75027.000)
 Maximum Y-Elevation: 780.000 (associated X-Station: 72665.000)

Roughness Data (1 SubAreas)

SubArea	Roughness Coefficient	Horizontal Breakpoint
-----	-----	-----

WSPRO OUTPUT

1 .034 ---

Bridge datum projection(s): XREFLT XREFRT FDSTLT FDSTRT

 * Finished Processing Header Record APPR *

***** W S P R O *****
 Federal Highway Administration - U. S. Geological Survey
 Model for Water-Surface Profile Computations.
 Input Units: English / Output Units: English

I-74 OVER EAGLE CREEK I74-70-4437
 COUNTY: MARION QUAD: CLERMONT 110D
 11-14-96 BRET A. ROBINSON

EX

=====
 * Summary of Boundary Condition Information *
 =====

#	Reach Discharge	Water Surface Elevation	Friction Slope	Flow Regime
1	22700.00	*****	.0012	Sub-Critical
2	31600.00	*****	.0012	Sub-Critical

=====
 * Beginning 2 Profile Calculation(s) *
 =====

***** W S P R O *****
 Federal Highway Administration - U. S. Geological Survey
 Model for Water-Surface Profile Computations.
 Input Units: English / Output Units: English

I-74 OVER EAGLE CREEK I74-70-4437
 COUNTY: MARION QUAD: CLERMONT 110D
 11-14-96 BRET A. ROBINSON

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: EXIT	755.125	.612	22700.000	5471.388	*****	74136.680
Header Type: XS	755.737	*****	4.149	654930.10	*****	75595.160
SRD: .000	750.541	*****	.571	*****	2.286	*****
Section: FULLV	755.475	.542	22700.000	5992.653	250.000	74110.550

WSPRO OUTPUT

```
Header Type: FV      756.017   .278      3.788  708470.10  250.000  75627.840
SRD:      250.000    750.541   .000      .524    .0011    2.429    .003
```

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

```
Section: APPR      756.190   .163  22700.000   7017.645  375.000  74186.340
Header Type: AS    756.353   .336    3.235  812811.60  375.000  75811.630
SRD:      625.000    751.706   .000      .274    .0009    1.000    .000
```

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

<<< The Following Data Reflect The "Constricted" Profile >>>

<<< Beginning Bridge/Culvert Hydraulic Computations >>>

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: BRDGE	754.747	2.520	22700.000	2070.282	250.000	74904.000
Header Type: BR	757.266	.481	10.965	420485.50	250.000	75106.270
SRD: 250.000	751.682	1.046	.701	*****	1.348	-.006

Specific Bridge Information	C	P/A	PFELEV	BLEN	XLAB	XRAB
Bridge Type 3	Flow Type 1					
Pier/Pile Code 0	.8615	.028	764.700	*****	*****	*****

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: APPR	757.887	.079	22700.000	10063.630	250.000	73978.590
Header Type: AS	757.966	.280	2.256	1306773.00	288.677	75942.330
SRD: 625.000	751.706	.424	.176	.0009	1.000	.017

Approach	Section	APPR	Flow	Contraction	Information
M(G)	M(K)	KQ	XLKQ	XRKQ	OTEL
.873	.401	778737.9	*****	*****	757.887

<<< End of Bridge Hydraulics Computations >>>

***** W S P R O *****
 Federal Highway Administration - U. S. Geological Survey
 Model for Water-Surface Profile Computations.
 Input Units: English / Output Units: English

I-74 OVER EAGLE CREEK	I74-70-4437
COUNTY: MARION	QUAD: CLERMONT 110D
11-14-96	BRET A. ROBINSON

WSPRO OUTPUT

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: EXIT	756.672	.707	31600.000	7928.978	*****	74021.260
Header Type: XS	757.379	*****	3.985	911933.80	*****	75739.520
SRD: .000	752.672	*****	.553	*****	2.863	*****
Section: FULLV	757.019	.634	31600.000	8535.311	250.000	73995.370
Header Type: FV	757.653	.280	3.702	976961.60	250.000	75771.890
SRD: 250.000	752.672	.000	.514	.0011	2.974	-.006

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

Section: APPR	757.794	.159	31600.000	9881.290	375.000	73990.010
Header Type: AS	757.953	.300	3.198	1275617.00	375.000	75935.140
SRD: 625.000	753.062	.000	.250	.0008	1.000	.000

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

<<< The Following Data Reflect The "Constricted" Profile >>>

<<< Beginning Bridge/Culvert Hydraulic Computations >>>

	WSEL	VHD	Q	AREA	SRDL	LEW	
	EGEL	HF	V	K	FLEN	REW	
	CRWS	HO	FR #	SF	ALPHA	ERR	
Section: BRDGE	755.917	4.531	31600.000	2310.182	250.000	74901.300	
Header Type: BR	760.448	.569	13.679	495141.40	250.000	75109.180	
SRD: 250.000	753.683	2.498	.903	*****	1.557	-.002	
Specific Bridge Information		C	P/A	PFELEV	BLEN	XLAB	XRAB
Bridge Type 3	Flow Type 1						
Pier/Pile Code	0	.8014	.029	764.700	*****	*****	*****

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: APPR	761.183	.051	31600.000	17522.970	250.000	73651.250
Header Type: AS	761.233	.221	1.803	2833095.00	300.595	76112.570
SRD: 625.000	753.062	.566	.119	.0008	1.000	.008

Approach Section APPR Flow Contraction Information						
M(G)	M(K)	KQ	XLKQ	XRKQ	OTEL	
.890	.609	1105968.0	*****	*****	761.183	

<<< End of Bridge Hydraulics Computations >>>

WSPRO OUTPUT

2 7.77 31600.000 756.483 15.983 15.672 .691 74876.000 75135.000

***** W S P R O *****
 Federal Highway Administration - U. S. Geological Survey
 Model for Water-Surface Profile Computations.
 Input Units: English / Output Units: English

I-74 OVER EAGLE CREEK I74-70-4437
 COUNTY: MARION QUAD: CLERMONT 110D
 11-14-96 BRET A. ROBINSON

*** Pier Scour Calculations for Header Record BRDGE ***

Constants and Input Variables

Pier Width: 2.000

```

*-----*
Pier Shape Factor                      (K1):    1.00
Flow Angle of Attack Factor        (K2):    1.00
Bed Condition Factor                (K3):    1.10
Bed Material Factor                 (K4):    1.00
Velocity Multiplier                 (VM):    1.00
Depth Multiplier                     (YM):    1.00
*-----*
  
```

#	Scour Depth	---- Localized Hydraulic Properties ----					-- X-Stations --	
		Flow	WSE	Depth	Velocity	Froude #	Left	Right
1	7.01	22700.000	755.171	14.671	12.659	.582	74876.000	75135.000
2	7.77	31600.000	756.483	15.983	15.672	.691	74876.000	75135.000

***** W S P R O *****
 Federal Highway Administration - U. S. Geological Survey
 Model for Water-Surface Profile Computations.
 Input Units: English / Output Units: English

I-74 OVER EAGLE CREEK I74-70-4437
 COUNTY: MARION QUAD: CLERMONT 110D
 11-14-96 BRET A. ROBINSON

*** Pier Scour Calculations for Header Record BRDGE ***

Constants and Input Variables

Pier Width: 2.000

```

*-----*
Pier Shape Factor                      (K1):    1.00
Flow Angle of Attack Factor        (K2):    1.00
Bed Condition Factor                (K3):    1.10
Bed Material Factor                 (K4):    1.00
*-----*
  
```

