

Modified Level II Streambed-Scour Analysis for Structure I-64-108-5658 Crossing Indian Creek in Harrison County, Indiana

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CONVERSION FACTORS AND ABBREVIATIONS

Multiply	By	To obtain
inch (in.)	25.4	millimeter
foot (ft)	0.3048	meter
square foot (ft ²)	929.0	square centimeter
feet per second (ft/s)	0.3048	meters per second
cubic foot per second (ft ³ /s)	0.02832	cubic meter per second
mile (mi)	1.609	kilometer
square mile (mi ²)	2.590	square kilometer

Abbreviations used in this report:

D ₅₀	median diameter of bed material
Q100	100-year discharge
FEMA	Federal Emergency Management Agency
HEC	Hydraulic Engineering Circular
IDNR	Indiana Department of Natural Resources
INDOT	Indiana Department of Transportation
USGS	U. S. Geological Survey
WSPRO	Water Surface PROfile model

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By Robert L. Miller, Bret A. Robinson, *and* David C. Voelker

ABSTRACT

Level II scour evaluations follow a process in which hydrologic, hydraulic, and sediment-transport data are evaluated to calculate the depth of scour that may result when a given discharge is routed through a bridge opening. The results of the modified Level II analysis for structure I-64-108-5658 on Interstate 64 crossing Indian Creek in Harrison County, Indiana, are presented. The site is near the town of Corydon in the central part of Harrison County. Scour depths were computed with the Water Surface PROfile model, version V050196, which incorporates the scour-calculation procedures outlined in Hydraulic Engineering Circular No. 18. Total scour depths at the piers were approximately 6.6 feet for the modeled discharge of 26,000 cubic feet per second and approximately 8.0 feet for the modeled discharge of 34,000 cubic feet per second.

INTRODUCTION

The U.S. Geological Survey (USGS), in cooperation with the Indiana Department of Transportation (INDOT), is conducting Level II scour analyses at a number of bridges throughout Indiana. This report describes the methods applied and the modeling results for bridge I-64-108-5658.

Background and Scope

Level I scour assessment is a process where a large number of bridges are studied as a group. Assessments usually are made by evaluating a combination of geomorphic, hydrologic, and bridge-characteristic data. The results help investigators determine which bridges appear to be most likely to experience streambed-scour problems and which bridges appear to be relatively immune to problems brought on by streambed scour (for example, bridges built on bedrock).

When applied correctly, Level I scour assessments provide an investigator with information to identify those bridges that appear to be relatively safe and those bridges that fall into higher risk categories.

Level II scour evaluations describe the process for an investigator to apply a model to a bridge site and calculate the potential depth of scour that may result from a given flood event. Level II analyses involve the application of basic hydrologic, hydraulic, and sediment-transport engineering concepts and may include an evaluation of flood history, channel hydraulic conditions (for example, water-surface profile analysis), and basic sediment-transport analyses such as scour calculations (Lagasse and others, 1995).

The methods and model outlined in Hydraulic Engineering Circular (HEC) No. 18 (Richardson and Davis, 1995) formulate the basis for Level II scour evaluations. Methods used in this study for Level II scour evaluations are a modification of the HEC-18 standards. These modifications were made to comply with the methodology requested by INDOT (Merril Dougherty, Indiana Department of Transportation, oral commun., 1996). Descriptions of the specific modifications are given in the "Evaluation Methods" section of this report.

This report presents the methods followed for modeling, special considerations for this study site, and the input for and the output from the Water Surface PROfile (WSPRO) model.

Site Description

The study site is located near the town of Corydon in the central part of Harrison County. The drainage area for the site is approximately 144 mi² (estimated using Hoggatt, 1975, and USGS 7.5-minute topographic data). The predominant land use in the basin is agricultural; in the immediate vicinity of the bridge, the land is predominantly agricultural with some wooded land nearby.

Within the immediate vicinity of the bridge, Indian Creek has a channel-bed slope of approximately 0.000638 ft/ft. The channel-bed material is gravelly sandy silt-clay, and the channel banks consist of sandy silt-clay and bedrock. At the time of the Level I site visit on March 15, 1994, the banks were observed to have 0 to 75 percent woody vegetative cover; the field report noted that the banks were experiencing fluvial erosion.

The Interstate 64 crossing of Indian Creek is a 372-ft-long, multi-lane bridge consisting of four spans supported by concrete and steel piers and sloping spill-through abutments. Additional details describing conditions at the site are included in the Level I data base (Hopkins and Robinson, unpub. data, 1997). Photographs of the site, taken at the time of the Level I site visit, are archived at the USGS office in Indianapolis.

EVALUATION METHODS

The methods described in this section apply to a number of bridge sites in Indiana being evaluated for scour and outline the procedures requested by INDOT for these modified Level II scour analyses. The principal modification requested by INDOT was that the input data to the model come from or be estimated from existing data sources; no additional field data were collected. Actual methods used in the scour evaluation at this particular bridge site use the most applicable method possible, given the data available.

To determine drainage area, either published values found in Hoggatt (1975) or 7.5-minute topographic maps with Hoggatt's original drainage-area delineations were used. Where there are no published data, drainage-area segments measured from the maps produced by Hoggatt were either subtracted from downstream sites or added to upstream sites published by Hoggatt (1975).

In Indiana, flood discharges are coordinated by agreement among State and Federal agencies. At sites where flood discharges officially are coordinated among State and Federal agencies in Indiana, the coordinated 100-year discharge (Q100) was modeled. INDOT also provided an additional flood discharge for these coordinated sites in excess of the Q100 to be modeled.

If a flood discharge was not coordinated, the USGS examined Federal Emergency Management Agency (FEMA) studies for Q100 determinations. Where FEMA studies did not produce a Q100, the USGS contacted IDNR for an estimated Q100 in the vicinity of the site being studied. If IDNR did not have a Q100, data from nearby USGS streamflow-gaging stations were analyzed with nearby and similar drainage basins that have been coordinated. At sites having no coordinated discharge data, the two discharges used in the model were 1) the approximated Q100 and 2) a discharge equal to 1.7 times the approximated Q100.

Most of the cross-section and bridge-opening geometry data were taken from the bridge plans (Indiana State Highway Commission, 1969) provided by INDOT. Bridge plans are presumed to be representative of current conditions at the site. To determine the cross-section geometry, a line was drawn on the bridge plans parallel to the bridge stationing and approximately one bridge width from the bridge. For sites where the bridge plans did not extend far enough laterally for collection of all cross-section data required for WSPRO model analysis, additional data were collected from 7.5-minute topographic maps.

The roadway and embankment profile was taken from the bridge and highway plans for those sites where roadway overtopping was expected. The INDOT bridge plans and 7.5-minute topographic maps were used as a guide, based on the water-surface elevations calculated by the WSPRO model, to determine if roadway overtopping might occur.

Roughness values (*n*-values) for the main channel were estimated by viewing photographs archived from the Level I scour assessments. The *n*-values for the overbanks were assigned on the basis of the surface-cover data summarized in the Level I data base (Hopkins and Robinson, unpub. data, 1997). From those data, the following roughness values were assigned to the surface-cover categories: urban—0.050, suburban—0.035, row crop—0.045, pasture—0.035, brush—0.120, forest—0.100, and wetland (any area covered by standing water)—0.100. The *n*-values for the overbanks were adjusted if the Level I photographs provided sufficient detail to warrant an adjustment.

WSPRO version V050196 was used to model flow through the study site. Starting water-surface elevation was obtained with a slope-conveyance computation. The channel-bed slope in the immediate vicinity of the bridge was estimated from the 7.5-minute topographic map and was used as the slope of the energy grade line for this computation.

WSPRO version V050196 includes a field that allows the input of up to four scour-adjustment factors (K1 to K4). For this modeling, the default value for K4 (bed armoring) was chosen. For scour-adjustment factors K1 and K2 (pier-nose shape and angle of attack, respectively), input values were determined by evaluating the data archived in the Level I data base (Hopkins and Robinson, unpub. data, 1997). For the K3 factor (bed forms), a value of 1.1 was applied in all cases.

In some cases, piers set on the overbanks are constructed with footings that are higher in elevation than pier footings in the main channel. In these situations, if the channel position changes, the piers that were initially constructed on the overbank may become part of the main channel. Therefore, to evaluate total potential scour, the model results obtained for contraction scour and deepest local scour in the main channel were added and applied to all piers in the bridge opening. This methodology allowed for an evaluation of potential undermining of pier supports in the event that future channel movement placed overbank piers in the main channel.

Where bridge pairs have a continuous abutment or fill between the bridges that does not allow expansion of flow, the bridge pair was modeled as one bridge. Sites with discontinuous abutments, allowing expansion between the bridges, were modeled as two separate bridges. In those cases, a valley cross section was measured between the bridges and used as the approach section for the downstream bridge and as the exit section for the upstream bridge.

At sites with no embankment to function as a weir or at sites where the tailwater drowns out the embankment, a composite bridge and road section was used to compute flow. Those sites were computed with friction-loss equations rather than with a bridge routine.

Total scour is taken as the sum of local scour plus contraction scour. If the model predicted negative contraction scour (aggradation), the contraction-scour value was assumed to be zero in determining the total scour depth (table 1). This assumption was made so that a negative contraction scour would not mask the potentially detrimental effects of local scour at a pier. No abutment scour evaluations were made in this study.

Table 1. Cumulative scour depths for the modeled discharges at structure I-64-108-5658 crossing Indian Creek in Harrison County, Indiana

[--, no value]

Pier number ¹	Stationing from bridge plans ²	Initial bed-elevation at pier (feet)	Main-channel contraction scour depth (feet)	Local scour depth (feet)	Worst-case total-scour depth ³ (feet)	Bottom elevation of pier (feet)	Worst-case bed elevation after scour ⁴ (feet)
Modeled discharge⁵ is 26,000 cubic feet per second							
1	405+90	578	0.5	6.1	6.6	558.0	548.7
2	406+90	566	0.5	6.1	6.6	552.2	548.7
3	408+20	555	0.5	6.1	6.6	552.1	548.7
Modeled discharge is 34,000 cubic feet per second							
1	405+90	578	1.5	6.5	8.0	558.0	547.3
2	405+90	566	1.5	6.5	8.0	552.2	547.3
3	408+20	555	1.5	6.5	8.0	552.1	547.3

¹Pier numbers were assigned from left to right as shown on the bridge plans.

²Stationing is the center line of the pier as determined from the bridge plans. Stationing from bridge plan, 405+90, represents a point 40,590 feet from an arbitrary starting location referenced on the bridge plans.

³Worst-case total-scour depths are generated by summing the calculated contraction-scour depth with the worst case of local scour.

⁴Worst-case bed elevation is computed by subtracting the worst-case total-scour depth from the lowest initial bed elevation in the bridge opening (555.3 feet).

⁵Coordinated discharge.

SPECIAL CONSIDERATIONS

Model runs indicate the water-surface elevation at the bridge is lower than the low-steel elevation for the modeled discharges. Therefore, there should be no pressure flow through the bridge opening for the discharges modeled.

RESULTS

Scour depths were computed with a version of WSPRO (Larry Arneson, Federal Highway Administration, written commun., 1996) modified from Shearman (1990). This version of WSPRO includes scour calculations in the model output. Scour depths were calculated assuming an infinite depth of material that could erode and a homogeneous particle-size distribution. The results of the scour analysis are presented in table 1; a complete input file and output results are presented in the appendix.

REFERENCES

- Hoggatt, R.E., 1975, Drainage areas of Indiana streams: U.S. Geological Survey, Water Resources Division, 231 p.
- Indiana State Highway Commission, 1969, Bridge plans Interstate Route 64: Bridge File I-64-108-5658.
- Lagasse, P.F.; Schall, J.D.; Johnson, F.; Richardson, E.V.; and Chang, F., 1995, Stream stability at highway structures (2d ed.): Federal Highway Administration, Hydraulic Engineering Circular No. 20, Publication FHWA-IP-90-014, 144 p.
- Richardson, E.V., and Davis, S.R., 1995, Evaluating scour at bridges (3d ed.): Federal Highway Administration, Hydraulic Engineering Circular No. 18, Publication FHWA-IP-90-017, 204 p.
- Shearman, J.O., 1990, User's manual for WSPRO, a computer model for water-surface profile computations: Federal Highway Administration Publication FHWA-IP-89-027, 177 p.

APPENDIX

WSPRO INPUT FILE

```

T1          I-64 OVER INDIAN CREEK          I-64-108-5658
T2          COUNTY HARRISON                 QUAD CORYDON EAST
T3          11-18-96                       R L Miller
SI          0
Q           26000 34000
SK          .000638 .000638
XS  EXIT   0 48
GR          40184 590 40185 580 40725 570
GR          40749 565 40757 560 40766 559 40778 558 40791 557
GR          40811 556 40821 556 40837 557 40851 559 40854 560
GR          40875 580 40891 590 40911 605 40920 615 41323 630
N           .045 .032 .100
SA          40725 40920
XS  FULLV  379 48
GR          40184 590 40185 580 40725 570
GR          40749 565 40757 560 40766 559 40778 558 40791 557
GR          40811 556 40821 556 40837 557 40851 559 40854 560
GR          40875 580 40891 590 40911 605 40920 615 41323 630
N           .045 .032 .100
SA          40725 40920
BR  BRDGE  379 592 24
GR          40552 0593.0 40553 0592.4 40556 0592.5 40586 0578.1
GR          40623 0578.1 40648 0566.4 40776 0566.1 40781 0564.9
GR          40788 0559.9 40830 0554.6 40849 0554.9 40860 0556.1
GR          40876 0558.8 40879 0560.1 40895 0570.3 40911 0584.1
GR          40918 0590.5 40923 0590.9 40924 0591.2 40926 0591.3
GR          40927 0592.7 40926 0592.7 40810 0592.1
GR          40692 0591.5 40589 0591.2 40588 0593.1 40552 0593.0
N           .032
PD          556 2 1
PD          574 2 2
PD          574 4 3
PD          578 4 4
PD          578 6 5
CD          3 153 2 590
DC  BRDGE  40776 40935 40812 41001 * 6
DP          40552 40927 2 * * 1.0 1.0 1.1
DP          40552 40927 2 * * 1.0 1.0 1.1
DP          40552 40927 2 * * 1.0 1.0 1.1
XS  APPR   911
GR          38429 650 38502 640 38609 620 38702 610 39122 610
GR          39185 610 39406 600 39787 590 40197 580 40812 572
GR          40850 570 40863 565 40869 560 40874 557 40892 556
GR          40917 556 40925 557 40937 560 40946 565 40949 570
GR          40989 610 40995 615 41001 620 41048 620
GR          41318 630 41429 630 41659 630 41899 640
N           .045 .032 .100
SA          40812 41001
EX
ER

```

WSPRO OUTPUT

```
***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Run Date & Time: 8/ 6/97 8:05 am Version V050196
Input File: 5658.dat Output File: 5658.LST
```

```
-----*
T1          I-64 OVER INDIAN CREEK          I-64-108-5658
T2          COUNTY HARRISON                QUAD CORYDON EAST
T3          11-18-96                       R L MILLER
SI          0
Q           26000 34000
```

*** Processing Flow Data; Placing Information into Sequence 1 ***

```
SK          .000638 .000638
```

```
***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English
```

```
-----*
I-64 OVER INDIAN CREEK          I-64-108-5658
COUNTY HARRISON                QUAD CORYDON EAST
11-18-96                       R L MILLER
```

```
-----*
*           Starting To Process Header Record EXIT           *
-----*
```

```
XS  EXIT    0   48
GR   40184 590 40185 580 40725 570
GR   40749 565 40757 560 40766 559 40778 558 40791 557
GR   40811 556 40821 556 40837 557 40851 559 40854 560
GR   40875 580 40891 590 40911 605 40920 615 41323 630
N     .045 .032 .100
SA   40725 40920
```

*** Completed Reading Data Associated With Header Record EXIT ***
 *** Storing X-Section Data In Temporary File As Record Number 1 ***

```
*** Data Summary For Header Record EXIT ***
SRD Location:      0. Cross-Section Skew: 48.0 Error Code 0
Valley Slope:     .00000 Averaging Conveyance By Geometric Mean.
Energy Loss Coefficients -> Expansion: .50 Contraction: .00
```

X,Y-coordinates (18 pairs)					
X	Y	X	Y	X	Y
40184.000	590.000	40185.000	580.000	40725.000	570.000
40749.000	565.000	40757.000	560.000	40766.000	559.000
40778.000	558.000	40791.000	557.000	40811.000	556.000
40821.000	556.000	40837.000	557.000	40851.000	559.000
40854.000	560.000	40875.000	580.000	40891.000	590.000
40911.000	605.000	40920.000	615.000	41323.000	630.000

WSPRO OUTPUT

Minimum and Maximum X,Y-coordinates

```

Minimum X-Station:  40184.000  ( associated Y-Elevation:  590.000 )
Maximum X-Station:  41323.000  ( associated Y-Elevation:  630.000 )
Minimum Y-Elevation:  556.000  ( associated X-Station:  40821.000 )
Maximum Y-Elevation:  630.000  ( associated X-Station:  41323.000 )
    
```

X-coordinates & Horizontal Breakpoints Translated by Skew Angle

X Input	X Skewed	X Input	X Skewed	X Input	X Skewed
40184.000	40394.770	40185.000	40395.430	40725.000	40756.760
40749.000	40772.820	40757.000	40778.180	40766.000	40784.200
40778.000	40792.230	40791.000	40800.930	40811.000	40814.310
40821.000	40821.000	40837.000	40831.710	40851.000	40841.070
40854.000	40843.080	40875.000	40857.130	40891.000	40867.840
40911.000	40881.220	40920.000	40887.240	41323.000	41156.900

Roughness Data (3 SubAreas)

SubArea	Roughness Coefficient	Horizontal Breakpoint
1	.045	---
	---	*****
2	.032	---
	---	*****
3	.100	---

```

*-----*
* Finished Processing Header Record EXIT *
*-----*
    
```

```

***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English
    
```

```

*-----*
I-64 OVER INDIAN CREEK          I-64-108-5658
COUNTY HARRISON                QUAD CORYDON EAST
11-18-96                        R L MILLER
    
```

```

*-----*
* Starting To Process Header Record FULLV *
*-----*
    
```

```

XS  FULLV  379 48
GR   40184 590 40185 580 40725 570
GR   40749 565 40757 560 40766 559 40778 558 40791 557
GR   40811 556 40821 556 40837 557 40851 559 40854 560
GR   40875 580 40891 590 40911 605 40920 615 41323 630
N      .045 .032 .100
SA      40725 40920
    
```

WSPRO OUTPUT

*** Completed Reading Data Associated With Header Record FULLV ***
 *** Storing X-Section Data In Temporary File As Record Number 2 ***

*** Data Summary For Header Record FULLV ***

SRD Location: 379. Cross-Section Skew: 48.0 Error Code 0
 Valley Slope: .00000 Averaging Conveyance By Geometric Mean.
 Energy Loss Coefficients -> Expansion: .50 Contraction: .00

X,Y-coordinates (18 pairs)					
X	Y	X	Y	X	Y
40184.000	590.000	40185.000	580.000	40725.000	570.000
40749.000	565.000	40757.000	560.000	40766.000	559.000
40778.000	558.000	40791.000	557.000	40811.000	556.000
40821.000	556.000	40837.000	557.000	40851.000	559.000
40854.000	560.000	40875.000	580.000	40891.000	590.000
40911.000	605.000	40920.000	615.000	41323.000	630.000

Minimum and Maximum X,Y-coordinates

Minimum X-Station: 40184.000 (associated Y-Elevation: 590.000)
 Maximum X-Station: 41323.000 (associated Y-Elevation: 630.000)
 Minimum Y-Elevation: 556.000 (associated X-Station: 40821.000)
 Maximum Y-Elevation: 630.000 (associated X-Station: 41323.000)

X-coordinates & Horizontal Breakpoints Translated by Skew Angle

X Input	X Skewed	X Input	X Skewed	X Input	X Skewed
40184.000	40394.770	40185.000	40395.430	40725.000	40756.760
40749.000	40772.820	40757.000	40778.180	40766.000	40784.200
40778.000	40792.230	40791.000	40800.930	40811.000	40814.310
40821.000	40821.000	40837.000	40831.710	40851.000	40841.070
40854.000	40843.080	40875.000	40857.130	40891.000	40867.840
40911.000	40881.220	40920.000	40887.240	41323.000	41156.900

Roughness Data (3 SubAreas)

SubArea	Roughness Coefficient	Horizontal Breakpoint
1	.045	---
	---	*****
2	.032	---
	---	*****
3	.100	---

 * Finished Processing Header Record FULLV *

***** W S P R O *****
 Federal Highway Administration - U. S. Geological Survey

WSPRO OUTPUT

Model for Water-Surface Profile Computations.
 Input Units: English / Output Units: English

I-64 OVER INDIAN CREEK	I-64-108-5658
COUNTY HARRISON	QUAD CORYDON EAST
11-18-96	R L MILLER

* Starting To Process Header Record BRDGE *

BR	BRDGE	379	592	24				
GR		40552	0593.0	40553	0592.4	40556	0592.5	40586 0578.1
GR		40623	0578.1	40648	0566.4	40776	0566.1	40781 0564.9
GR		40788	0559.9	40830	0554.6	40849	0554.9	40860 0556.1
GR		40876	0558.8	40879	0560.1	40895	0570.3	40911 0584.1
GR		40918	0590.5	40923	0590.9	40924	0591.2	40926 0591.3
GR		40927	0592.7	40926	0592.7	40810	0592.1	
GR		40692	0591.5	40589	0591.2	40588	0593.1	40552 0593.0
N			.032					
PD		556	2 1					
PD		574	2 2					
PD		574	4 3					
PD		578	4 4					
PD		578	6 5					
CD		3	153 2	590				

*** Completed Reading Data Associated With Header Record BRDGE ***

*** Storing Bridge Data In Temporary File As Record Number 3 ***

*** Data Summary For Bridge Record BRDGE ***

SRD Location: 379. Cross-Section Skew: 24.0 Error Code 0

Valley Slope: ***** Averaging Conveyance By Geometric Mean.

Energy Loss Coefficients -> Expansion: .50 Contraction: .00

X,Y-coordinates (27 pairs)					
X	Y	X	Y	X	Y
40552.000	593.000	40553.000	592.400	40556.000	592.500
40586.000	578.100	40623.000	578.100	40648.000	566.400
40776.000	566.100	40781.000	564.900	40788.000	559.900
40830.000	554.600	40849.000	554.900	40860.000	556.100
40876.000	558.800	40879.000	560.100	40895.000	570.300
40911.000	584.100	40918.000	590.500	40923.000	590.900
40924.000	591.200	40926.000	591.300	40927.000	592.700
40926.000	592.700	40810.000	592.100	40692.000	591.500
40589.000	591.200	40588.000	593.100	40552.000	593.000

Minimum and Maximum X,Y-coordinates

Minimum X-Station: 40552.000 (associated Y-Elevation: 593.000)

Maximum X-Station: 40927.000 (associated Y-Elevation: 592.700)

Minimum Y-Elevation: 554.600 (associated X-Station: 40830.000)

Maximum Y-Elevation: 593.100 (associated X-Station: 40588.000)

WSPRO OUTPUT

X-coordinates & Horizontal Breakpoints Translated by Skew Angle

X Input	X Skewed	X Input	X Skewed	X Input	X Skewed
40552.000	40576.040	40553.000	40576.950	40556.000	40579.690
40586.000	40607.090	40623.000	40640.890	40648.000	40663.730
40776.000	40780.670	40781.000	40785.230	40788.000	40791.630
40830.000	40830.000	40849.000	40847.360	40860.000	40857.410
40876.000	40872.020	40879.000	40874.770	40895.000	40889.380
40911.000	40904.000	40918.000	40910.390	40923.000	40914.960
40924.000	40915.880	40926.000	40917.700	40927.000	40918.610
40926.000	40917.700	40810.000	40811.730	40692.000	40703.930
40589.000	40609.840	40588.000	40608.920	40552.000	40576.040

Roughness Data (1 SubAreas)

SubArea	Roughness Coefficient	Horizontal Breakpoint
1	.032	---

Discharge coefficient parameters

BRType	BRWidth	EMBSS	EMBELv	UserCD
3	153.000	2.00	590.000	*****

Pressure flow elevations

AVBCEL	PFElev
*****	592.000

Abutment Parameters

ABSLPL	ABSLPR	XTOELT	YTOELT	XTOERT	YTOERT
*****	*****	*****	*****	*****	*****

Pier/Pile Data (5 Group(s))

Code Indicates Bridge Uses Piers

Group	Elevation	Gross Width	Number
1	556.000	2.000	1
2	574.000	2.000	2
3	574.000	4.000	3
4	578.000	4.000	4
5	578.000	6.000	5

* Finished Processing Header Record BRDGE *

***** W S P R O *****

Federal Highway Administration - U. S. Geological Survey

Model for Water-Surface Profile Computations.

Input Units: English / Output Units: English

WSPRO OUTPUT

	I-64 OVER INDIAN CREEK		I-64-108-5658		
	COUNTY HARRISON		QUAD CORYDON EAST		
	11-18-96		R L MILLER		
DC	BRDGE	40776	40935	40812	41001 * 6
DP		40552	40927	2 * *	1.0 1.0 1.1
DP		40552	40927	2 * *	1.0 1.0 1.1
DP		40552	40927	2 * *	1.0 1.0 1.1

* Starting To Process Header Record APPR *

XS	APPR	911				
GR		38429	650	38502	640	38609 620 38702 610 39122 610
GR		39185	610	39406	600	39787 590 40197 580 40812 572
GR		40850	570	40863	565	40869 560 40874 557 40892 556
GR		40917	556	40925	557	40937 560 40946 565 40949 570
GR		40989	610	40995	615	41001 620 41048 620
GR		41318	630	41429	630	41659 630 41899 640
N		.045	.032	.100		
SA		40812	41001			

*** Completed Reading Data Associated With Header Record APPR ***

*** Storing X-Section Data In Temporary File As Record Number 4 ***

*** Data Summary For Header Record APPR ***

SRD Location: 911. Cross-Section Skew: .0 Error Code 0

Valley Slope: .00000 Averaging Conveyance By Geometric Mean.

Energy Loss Coefficients -> Expansion: .50 Contraction: .00

X,Y-coordinates (28 pairs)						
X	Y	X	Y	X	X	Y
38429.000	650.000	38502.000	640.000	38609.000		620.000
38702.000	610.000	39122.000	610.000	39185.000		610.000
39406.000	600.000	39787.000	590.000	40197.000		580.000
40812.000	572.000	40850.000	570.000	40863.000		565.000
40869.000	560.000	40874.000	557.000	40892.000		556.000
40917.000	556.000	40925.000	557.000	40937.000		560.000
40946.000	565.000	40949.000	570.000	40989.000		610.000
40995.000	615.000	41001.000	620.000	41048.000		620.000
41318.000	630.000	41429.000	630.000	41659.000		630.000
41899.000	640.000					

Minimum and Maximum X,Y-coordinates

Minimum X-Station: 38429.000 (associated Y-Elevation: 650.000)

Maximum X-Station: 41899.000 (associated Y-Elevation: 640.000)

Minimum Y-Elevation: 556.000 (associated X-Station: 40917.000)

Maximum Y-Elevation: 650.000 (associated X-Station: 38429.000)

Roughness Data (3 SubAreas)

Roughness Horizontal

WSPRO OUTPUT

SubArea	Coefficient	Breakpoint
1	.045	---
	---	*****
2	.032	---
	---	*****
3	.100	---

Bridge datum projection(s): XREFLT XREFRT FDSTLT FDSTRT

* Finished Processing Header Record APPR *

***** W S P R O *****

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I-64 OVER INDIAN CREEK I-64-108-5658
 COUNTY HARRISON QUAD CORYDON EAST
 11-18-96 R L MILLER

EX

=====

* Summary of Boundary Condition Information *

=====

#	Reach Discharge	Water Surface Elevation	Friction Slope	Flow Regime
1	26000.00	*****	.0006	Sub-Critical
2	34000.00	*****	.0006	Sub-Critical

=====

* Beginning 2 Profile Calculation(s) *

=====

***** W S P R O *****

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I-64 OVER INDIAN CREEK I-64-108-5658
 COUNTY HARRISON QUAD CORYDON EAST
 11-18-96 R L MILLER

WSEL	VHD	Q	AREA	SRDL	LEW
EGEL	HF	V	K	FLEN	REW
CRWS	HO	FR #	SF	ALPHA	ERR

WSPRO OUTPUT

```

-----
Section: EXIT      579.061  .769  26000.000  4916.594  *****  40235.730
Header Type: XS   579.829  *****  5.288  1028468.00  *****  40874.010
SRD:      .000    570.309  *****  .447      *****  1.767      *****
  
```

```

Section: FULLV    579.341  .721  26000.000  5097.945  379.000  40220.570
Header Type: FV   580.063  .234  5.100  1065521.00  379.000  40874.310
SRD:    379.000  570.309  .000  .430  .0006  1.783  .000
  
```

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

```

Section: APPR     579.645  .927  26000.000  4625.172  532.000  40224.310
Header Type: AS   580.571  .396  5.621  852491.10  532.000  40958.640
SRD:    911.000  573.645  .103  .542  .0007  1.886  .010
  
```

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

<<< The Following Data Reflect The "Constricted" Profile >>>
 <<< Beginning Bridge/Culvert Hydraulic Computations >>>

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: BRDGE	579.602	.539	26000.000	4513.172	379.000	40582.870
Header Type: BR	580.141	.313	5.761	1186785.00	379.000	40905.790
SRD: 379.000	569.624	.000	.278	*****	1.045	.016

Specific Bridge Information	C	P/A	PFELEV	BLEN	XLAB	XRAB
Bridge Type 3 Flow Type 1						
Pier/Pile Code 0	.9782	.014	592.000	*****	*****	*****

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: APPR	579.700	.913	26000.000	4665.594	379.000	40220.090
Header Type: AS	580.612	.384	5.573	859553.10	388.663	40958.700
SRD: 911.000	573.645	.086	.537	.0007	1.890	.001

Approach Section APPR Flow Contraction Information						
M(G)	M(K)	KQ	XLKQ	XRKQ	OTEL	
.561	.074	795581.9	*****	*****	579.700	

<<< End of Bridge Hydraulics Computations >>>

***** W S P R O *****
 Federal Highway Administration - U. S. Geological Survey
 Model for Water-Surface Profile Computations.

WSPRO OUTPUT

Input Units: English / Output Units: English

I-64 OVER INDIAN CREEK COUNTY HARRISON 11-18-96	I-64-108-5658 QUAD CORYDON EAST R L MILLER
---	--

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: EXIT	581.139	.797	34000.000	6327.500	*****	40184.890
Header Type: XS	581.935	*****	5.373	1345932.00	*****	40876.820
SRD: .000	573.350	*****	.417	*****	1.774	*****
Section: FULLV	581.424	.743	34000.000	6524.533	379.000	40184.860
Header Type: FV	582.167	.233	5.211	1395928.00	379.000	40877.280
SRD: 379.000	573.350	.000	.397	.0006	1.760	-.002

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

Section: APPR	581.736	.874	34000.000	6277.332	532.000	40125.810
Header Type: AS	582.610	.377	5.416	1169744.00	532.000	40960.730
SRD: 911.000	576.496	.065	.482	.0007	1.915	.001

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

<<< The Following Data Reflect The "Constricted" Profile >>>
<<< Beginning Bridge/Culvert Hydraulic Computations >>>

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: BRDGE	581.481	.773	34000.000	5125.701	379.000	40578.960
Header Type: BR	582.254	.318	6.633	1446494.00	379.000	40907.960
SRD: 379.000	571.023	.000	.315	*****	1.130	-.002

Specific Bridge Information	C	P/A	PFELEV	BLEN	XLAB	XRAB
Bridge Type 3 Flow Type 1						
Pier/Pile Code 0	.9407	.014	592.000	*****	*****	*****

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: APPR	581.876	.841	34000.000	6394.078	379.000	40120.100
Header Type: AS	582.717	.370	5.317	1193558.00	389.731	40960.880
SRD: 911.000	576.496	.096	.470	.0007	1.913	.012

Approach Section	APPR	Flow Contraction Information			
M(G)	M(K)	KQ	XLKQ	XRKQ	OTEL

WSPRO OUTPUT

.606 .142 1022652.0 ***** ***** 581.876

<<< End of Bridge Hydraulics Computations >>>

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I-64 OVER INDIAN CREEK I-64-108-5658
 COUNTY HARRISON QUAD CORYDON EAST
 11-18-96 R L MILLER

*** Live-Bed Contraction Scour Calculations for Header Record BRDGE ***

Constants and Input Variables

 Bed Material Transport Mode Factor (k1): .64
 Total Pier Width Value (Pw): 6.000

#	Scour Depth	-- Flow --		-- Width --		--- X-Limits ---	
		Contract	Approach	Contract	Approach	Side	Contract Approach
1	.489	18204.910	20409.290	153.000	189.000	Left:	*****
	Approach Channel Depth:		12.629	Right:	*****
2	1.489	22446.440	23426.140	153.000	189.000	Left:	*****
	Approach Channel Depth:		14.331	Right:	*****

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I-64 OVER INDIAN CREEK I-64-108-5658
 COUNTY HARRISON QUAD CORYDON EAST
 11-18-96 R L MILLER

*** Pier Scour Calculations for Header Record BRDGE ***

Constants and Input Variables

Pier Width: 2.000

Pier Shape Factor	(K1):	1.00
Flow Angle of Attack Factor	(K2):	1.00
Bed Condition Factor	(K3):	1.10
Bed Material Factor	(K4):	1.00
Velocity Multiplier	(VM):	1.00
Depth Multiplier	(YM):	1.00

WSPRO OUTPUT

Pier Width: 2.000

```
*-----*
Pier Shape Factor          (K1):  1.00
Flow Angle of Attack Factor (K2):  1.00
Bed Condition Factor       (K3):  1.10
Bed Material Factor        (K4):  1.00
Velocity Multiplier        (VM):  1.00
Depth Multiplier           (YM):  1.00
*-----*
```

```
Scour  ---- Localized Hydraulic Properties ----  -- X-Stations --
#  Depth  Flow      WSE    Depth Velocity Froude #  Left    Right
-----
1   6.10  26000.000  579.688  25.088   7.769   .273  40552.000  40927.000
2   6.51  34000.000  581.577  26.977   8.828   .300  40552.000  40927.000
-----
```

ER

```
***** Normal end of WSPRO execution. *****
***** Elapsed Time:  0 Minutes  6 Seconds *****
```