

Modified Level II Streambed-Scour Analysis for Structure I-74-36-4949 Crossing Little Sugar Creek in Montgomery County, Indiana

By DAVID C. VOELKER, ROBERT L. MILLER,
and BRET A. ROBINSON

Prepared in cooperation with the
INDIANA DEPARTMENT OF TRANSPORTATION

U.S. GEOLOGICAL SURVEY
Open-File Report 97-325



Indianapolis, Indiana

1997

U.S. DEPARTMENT OF THE INTERIOR
BRUCE BABBITT, Secretary

U.S. GEOLOGICAL SURVEY
Gordon P. Eaton, Director

For additional information, write to:
District Chief
U.S. Geological Survey
Water Resources Division
5957 Lakeside Boulevard
Indianapolis, IN 46278-1996

Copies of this report can be purchased from:
U.S. Geological Survey
Branch of Information Services
Box 25286
Federal Center
Denver, CO 80225

CONTENTS

Abstract	1
Introduction	1
Background and Scope	1
Site Description	2
Evaluation Methods	3
Special Considerations	6
Results	6
References	6
Appendix	7
Water Surface PROfile Model (WSPRO) Input File	8
Water Surface PROfile Model (WSPRO) Output	9
 Tables	
1. Cumulative scour depths for the modeled discharges at structure I-74-36-4949 crossing Little Sugar Creek in Montgomery County, Indiana	5

CONVERSION FACTORS AND ABBREVIATIONS

Multiply	By	To obtain
inch (in.)	25.4	millimeter
foot (ft)	0.3048	meter
square foot (ft ²)	929.0	square centimeter
feet per second (ft/s)	0.3048	meters per second
cubic foot per second (ft ³ /s)	0.02832	cubic meter per second
mile (mi)	1.609	kilometer
square mile (mi ²)	2.590	square kilometer

Abbreviations used in this report:

D ₅₀	median diameter of bed material
Q100	100-year discharge
FEMA	Federal Emergency Management Agency
HEC	Hydraulic Engineering Circular
IDNR	Indiana Department of Natural Resources
INDOT	Indiana Department of Transportation
USGS	U. S. Geological Survey
WSPRO	Water Surface PROfile model

Modified Level II Streambed-Scour Analysis for Structure I-74-36-4949 Crossing Little Sugar Creek in Montgomery County, Indiana

By David C. Voelker, Robert L. Miller, *and* Bret A. Robinson

ABSTRACT

Level II scour evaluations follow a process in which hydrologic, hydraulic, and sediment-transport data are evaluated to calculate the depth of scour that may result when a given discharge is routed through a bridge opening. The results of the modified Level II analysis for structure I-74-36-4949 on Interstate 74 crossing Little Sugar Creek in Montgomery County, Indiana, are presented. The site is near the town of Smartsburg in the east-central part of Montgomery County. Scour depths were computed with the Water Surface PROfile model, version V050196, which incorporates the scour-calculation procedures outlined in Hydraulic Engineering Circular No. 18. Total scour depths at the piers were approximately 5.1 feet for the modeled discharge of 6,100 cubic feet per second and approximately 5.8 feet for the modeled discharge of 10,400 cubic feet per second.

INTRODUCTION

The U.S. Geological Survey (USGS), in cooperation with the Indiana Department of Transportation (INDOT), is conducting Level II scour analyses at a number of bridges throughout Indiana. This report describes the methods applied and the modeling results for bridge I-74-36-4949.

Background and Scope

Level I scour assessment is a process where a large number of bridges are studied as a group. Assessments usually are made by evaluating a combination of geomorphic, hydrologic, and bridge-characteristic data. The results help investigators determine which bridges appear to be most likely to experience streambed-scour problems and which bridges appear to be relatively immune to problems brought on by streambed scour (for example, bridges built on bedrock).

When applied correctly, Level I scour assessments provide an investigator with information to identify those bridges that appear to be relatively safe and those bridges that fall into higher risk categories.

Level II scour evaluations describe the process for an investigator to apply a model to a bridge site and calculate the potential depth of scour that may result from a given flood event. Level II analyses involve the application of basic hydrologic, hydraulic, and sediment-transport engineering concepts and may include an evaluation of flood history, channel hydraulic conditions (for example, water-surface profile analysis), and basic sediment-transport analyses such as scour calculations (Lagasse and others, 1995).

The methods and model outlined in Hydraulic Engineering Circular (HEC) No. 18 (Richardson and Davis, 1995) formulate the basis for Level II scour evaluations. Methods used in this study for Level II scour evaluations are a modification of the HEC-18 standards. These modifications were made to comply with the methodology requested by INDOT (Merril Dougherty, Indiana Department of Transportation, oral commun., 1996). Descriptions of the specific modifications are given in the "Evaluation Methods" section of this report.

This report presents the methods followed for modeling, special considerations for this study site, and the input for and the output from the Water Surface PROfile (WSPRO) model.

Site Description

The study site is located near the town of Smartsburg in the east-central part of Montgomery County. The drainage area for the site is approximately 40.2 mi² (estimated using Hoggatt, 1975, and USGS 7.5-minute topographic data). The predominant land use in the basin is agricultural; in the immediate vicinity of the bridge, the land is predominantly forest with some agricultural land nearby.

Within the immediate vicinity of the bridge, Little Sugar Creek has a channel-bed slope of approximately 0.0021 ft/ft. The channel-bed material is sand, and the channel banks consist of silt-clay. At the time of the Level I site visit on June 25, 1991, the banks were observed to have 0 to 60 percent woody vegetative cover; the field report noted that the banks were experiencing fluvial erosion.

The Interstate 74 crossing of Little Sugar Creek is a 135-ft-long, multi-lane bridge consisting of three spans supported by concrete and steel piers and sloping spill-through abutments. Additional details describing conditions at the site are included in the Level I data base (Hopkins and Robinson, unpub. data, 1997). Photographs of the site, taken at the time of the Level I site visit, are archived at the USGS office in Indianapolis.

EVALUATION METHODS

The methods described in this section apply to a number of bridge sites in Indiana being evaluated for scour and outline the procedures requested by INDOT for these modified Level II scour analyses. The principal modification requested by INDOT was that the input data to the model come from or be estimated from existing data sources; no additional field data were collected. Actual methods used in the scour evaluation at this particular bridge site use the most applicable method possible, given the data available.

To determine drainage area, either published values found in Hoggatt (1975) or 7.5-minute topographic maps with Hoggatt's original drainage-area delineations were used. Where there are no published data, drainage-area segments measured from the maps produced by Hoggatt were either subtracted from downstream sites or added to upstream sites published by Hoggatt (1975).

In Indiana, flood discharges are coordinated by agreement among State and Federal agencies. At sites where flood discharges officially are coordinated among State and Federal agencies in Indiana, the coordinated 100-year discharge (Q100) was modeled. INDOT also provided an additional flood discharge for these coordinated sites in excess of the Q100 to be modeled.

If a flood discharge was not coordinated, the USGS examined Federal Emergency Management Agency (FEMA) studies for Q100 determinations. Where FEMA studies did not produce a Q100, the USGS contacted IDNR for an estimated Q100 in the vicinity of the site being studied. If IDNR did not have a Q100, data from nearby USGS streamflow-gaging stations were analyzed with nearby and similar drainage basins that have been coordinated. At sites having no coordinated discharge data, the two discharges used in the model were 1) the approximated Q100 and 2) a discharge equal to 1.7 times the approximated Q100.

Most of the cross-section and bridge-opening geometry data were taken from the bridge plans (Indiana State Highway Commission, 1961) provided by INDOT. Bridge plans are presumed to be representative of current conditions at the site. To determine the cross-section geometry, a line was drawn on the bridge plans parallel to the bridge stationing and approximately one bridge width from the bridge. For sites where the bridge plans did not extend far enough laterally for collection of all cross-section data required for WSPRO model analysis, additional data were collected from 7.5-minute topographic maps.

The roadway and embankment profile was taken from the bridge and highway plans for those sites where roadway overtopping was expected. The INDOT bridge plans and 7.5-minute topographic maps were used as a guide, based on the water-surface elevations calculated by the WSPRO model, to determine if roadway overtopping might occur.

Roughness values (*n*-values) for the main channel were estimated by viewing photographs archived from the Level I scour assessments. The *n*-values for the overbanks were assigned on the basis of the surface-cover data summarized in the Level I data base (Hopkins and Robinson, unpub. data, 1997). From those data, the following roughness values were assigned to the surface-cover categories: urban—0.050, suburban—0.035, row crop—0.045, pasture—0.035, brush—0.120, forest—0.100, and wetland (any area covered by standing water)—0.100. The *n*-values for the overbanks were adjusted if the Level I photographs provided sufficient detail to warrant an adjustment.

WSPRO version V050196 was used to model flow through the study site. Starting water-surface elevation was obtained with a slope-conveyance computation. The channel-bed slope in the immediate vicinity of the bridge was estimated from the 7.5-minute topographic map and was used as the slope of the energy grade line for this computation.

WSPRO version V050196 includes a field that allows the input of up to four scour-adjustment factors (K1 to K4). For this modeling, the default value for K4 (bed armoring) was chosen. For scour-adjustment factors K1 and K2 (pier-nose shape and angle of attack, respectively), input values were determined by evaluating the data archived in the Level I data base (Hopkins and Robinson, unpub. data, 1997). For the K3 factor (bed forms), a value of 1.1 was applied in all cases.

In some cases, piers set on the overbanks are constructed with footings that are higher in elevation than pier footings in the main channel. In these situations, if the channel position changes, the piers that were initially constructed on the overbank may become part of the main channel. Therefore, to evaluate total potential scour, the model results obtained for contraction scour and deepest local scour in the main channel were added and applied to all piers in the bridge opening. This methodology allowed for an evaluation of potential undermining of pier supports in the event that future channel movement placed overbank piers in the main channel.

Where bridge pairs have a continuous abutment or fill between the bridges that does not allow expansion of flow, the bridge pair was modeled as one bridge. Sites with discontinuous abutments, allowing expansion between the bridges, were modeled as two separate bridges. In those cases, a valley cross section was measured between the bridges and used as the approach section for the downstream bridge and as the exit section for the upstream bridge.

At sites with no embankment to function as a weir or at sites where the tailwater drowns out the embankment, a composite bridge and road section was used to compute flow. Those sites were computed with friction-loss equations rather than with a bridge routine.

Total scour is taken as the sum of local scour plus contraction scour. If the model predicted negative contraction scour (aggradation), the contraction-scour value was assumed to be zero in determining the total scour depth (table 1). This assumption was made so that a negative contraction scour would not mask the potentially detrimental effects of local scour at a pier. No abutment scour evaluations were made in this study.

Table 1. Cumulative scour depths for the modeled discharges at structure I-74-36-4949 crossing Little Sugar Creek in Montgomery County, Indiana

Pier number ¹	Stationing from bridge plans ²	Initial bed-elevation at pier (feet)	Main-channel contraction scour depth (feet)	Local scour depth (feet)	Worst-case total-scour depth ³ (feet)	Bottom elevation of pier (feet)	Worst-case bed elevation after scour ⁴ (feet)
Modeled discharge⁵ is 6,100 cubic feet per second							
1	104+29	763.5	0.2	4.9	5.1	758.2	758.4
2	104+74	763.5	.2	4.9	5.1	758.0	758.4
Modeled discharge is 10,400 cubic feet per second							
1	104+29	763.5	0.2	5.6	5.8	758.2	757.7
2	104+74	763.5	.2	5.6	5.8	758.0	757.7

¹Pier numbers were assigned from left to right as shown on the bridge plans.

²Stationing is the center line of the pier as determined from the bridge plans. Stationing from bridge plan, 104+29, represents a point 10,429 feet from an arbitrary starting location referenced on the bridge plans.

³Worst-case total-scour depths are generated by summing the calculated contraction-scour depth with the worst case of local scour.

⁴Worst-case bed elevation is computed by subtracting the worst-case total-scour depth from the lowest initial bed elevation in the bridge opening (763.5 feet).

⁵Not a coordinated discharge.

SPECIAL CONSIDERATIONS

Model runs indicate the water-surface elevation at the bridge is lower than the low-steel elevation for the modeled discharges. Therefore, there should be no pressure flow through the bridge opening for the discharges modeled.

RESULTS

Scour depths were computed with a version of WSPRO (Larry Arneson, Federal Highway Administration, written commun., 1996) modified from Shearman (1990). This version of WSPRO includes scour calculations in the model output. Scour depths were calculated assuming an infinite depth of material that could erode and a homogeneous particle-size distribution. The results of the scour analysis are presented in table 1; a complete input file and output results are presented in the appendix.

REFERENCES

- Hoggatt, R.E., 1975, Drainage areas of Indiana streams: U.S. Geological Survey, Water Resources Division, 231 p.
- Indiana State Highway Commission, 1961, Bridge plans Interstate Route 74: Bridge File I-74-36-4949.
- Lagasse, P.F.; Schall, J.D.; Johnson, F.; Richardson, E.V.; and Chang, F., 1995, Stream stability at highway structures (2d ed.): Federal Highway Administration, Hydraulic Engineering Circular No. 20, Publication FHWA-IP-90-014, 144 p.
- Richardson, E.V., and Davis, S.R., 1995, Evaluating scour at bridges (3d ed.): Federal Highway Administration, Hydraulic Engineering Circular No. 18, Publication FHWA-IP-90-017, 204 p.
- Shearman, J.O., 1990, User's manual for WSPRO, a computer model for water-surface profile computations: Federal Highway Administration Publication FHWA-IP-89-027, 177 p.

APPENDIX

WSPRO INPUT FILE

```

T1      I-74 over Little Sugar Creek      I-74-36-4949
T2      County: Montgomery                 Quad: Darlington
T3      07-24-97                          D. C. Voelker
SI      0
Q       6100      10400
SK      .0021     .0021
XS      EXIT      0      0
GR      9448 790  9769 790  10000 780  10305 771  10407 770  10467 769.6
GR      10483 763.5  10549 763.5  10562 769.1  10580 769  10610 768
GR      10733 768  10805 769  10809 770  10823 775  10839 780  10855 785
GR      10861 787  11382 790
N       1.0      .034      1.0
SA      10467      10562
XS      FULLV     135  0
GR      9448 790  9769 790  10000 780  10305 771  10407 770  10467 769.6
GR      10483 763.5  10549 763.5  10562 769.1  10580 769  10610 768
GR      10733 768  10805 769  10809 770  10823 775  10839 780  10855 785
GR      10861 787  10932 770  11146 780  11382 790
N       1.0      .034      1.0
SA      10467      10562
BR      BRDGE     135 778.0  0
GR      10380 0778.5  10380 0776.0  10406 0763.5  10497 0763.5  10522 0775.3
GR      10522 0778.0  10380 0778.5
N       .035
PD      763.5  4      2
CD      3      135      2 775
DC 0 BRDGE 10406 10497 10343 10437
DP      10380 10522 2 * * 1.0 1.0 1.1
DP      10380 10522 2 * * 1.0 1.0 1.1
XS      APPR     388  0
GR      9526 800  9900 780  10316 772  10343 771.6  10359 763.5  10426 763.5
GR      10437 768  10486 768  10623 769  10770 770  10775 772  10780 773
GR      10787 776  10802 777  10807 780  10818 785  10822 787
GR      10851 790  11096 790
N       1.0      .034      1.0
SA      10343      10437
EX
ER

```

WSPRO OUTPUT

***** W S P R O *****

Federal Highway Administration - U. S. Geological Survey

Model for Water-Surface Profile Computations.

Run Date & Time: 8/ 6/97 0:41 pm Version V050196

Input File: 4949.dat Output File: 4949.LST

T1	I-74 OVER LITTLE SUGAR CREEK	I-74-36-4949
T2	COUNTY: MONTGOMERY	QUAD: DARLINGTON
T3	07-24-97	D. C. VOELKER
SI	0	
Q	6100 10400	

*** Processing Flow Data; Placing Information into Sequence 1 ***

SK .0021 .0021

***** W S P R O *****

Federal Highway Administration - U. S. Geological Survey

Model for Water-Surface Profile Computations.

Input Units: English / Output Units: English

I-74 OVER LITTLE SUGAR CREEK	I-74-36-4949
COUNTY: MONTGOMERY	QUAD: DARLINGTON
07-24-97	D. C. VOELKER

* Starting To Process Header Record EXIT *

XS	EXIT	0	0						
GR		9448	790	9769	790	10000	780	10305	771
GR		10483	763.5	10549	763.5	10562	769.1	10580	769
GR		10733	768	10805	769	10809	770	10823	775
GR		10861	787	11382	790				
N		1.0	.034	1.0					
SA		10467	10562						

*** Completed Reading Data Associated With Header Record EXIT ***

*** Storing X-Section Data In Temporary File As Record Number 1 ***

*** Data Summary For Header Record EXIT ***

SRD Location: 0. Cross-Section Skew: .0 Error Code 0
Valley Slope: .00000 Averaging Conveyance By Geometric Mean.
Energy Loss Coefficients -> Expansion: .50 Contraction: .00

X,Y-coordinates (19 pairs)

X	Y	X	Y	X	Y
9448.000	790.000	9769.000	790.000	10000.000	780.000
10305.000	771.000	10407.000	770.000	10467.000	769.600
10483.000	763.500	10549.000	763.500	10562.000	769.100
10580.000	769.000	10610.000	768.000	10733.000	768.000
10805.000	769.000	10809.000	770.000	10823.000	775.000

WSPRO OUTPUT

10839.000	780.000	10855.000	785.000	10861.000	787.000
11382.000	790.000				

Minimum and Maximum X,Y-coordinates

Minimum X-Station: 9448.000 (associated Y-Elevation: 790.000)
 Maximum X-Station: 11382.000 (associated Y-Elevation: 790.000)
 Minimum Y-Elevation: 763.500 (associated X-Station: 10549.000)
 Maximum Y-Elevation: 790.000 (associated X-Station: 9448.000)

Roughness Data (3 SubAreas)

SubArea	Roughness Coefficient	Horizontal Breakpoint
1	1.000	---
	---	*****
2	.034	---
	---	*****
3	1.000	---

* Finished Processing Header Record EXIT *

***** W S P R O *****

Federal Highway Administration - U. S. Geological Survey
 Model for Water-Surface Profile Computations.
 Input Units: English / Output Units: English

I-74 OVER LITTLE SUGAR CREEK	I-74-36-4949
COUNTY: MONTGOMERY	QUAD: DARLINGTON
07-24-97	D. C. VOELKER

* Starting To Process Header Record FULLV *

XS	FULLV	135	0					
GR	9448	790	9769	790	10000	780	10305	771 10407 770 10467 769.6
GR	10483	763.5	10549	763.5	10562	769.1	10580	769 10610 768
GR	10733	768	10805	769	10809	770	10823	775 10839 780 10855 785
GR	10861	787	10932	770	11146	780	11382	790
N	1.0	.034	1.0					
SA	10467	10562						

*** Completed Reading Data Associated With Header Record FULLV ***

*** Storing X-Section Data In Temporary File As Record Number 2 ***

*** Data Summary For Header Record FULLV ***

SRD Location: 135. Cross-Section Skew: .0 Error Code 0

Valley Slope: .00000 Averaging Conveyance By Geometric Mean.

Energy Loss Coefficients -> Expansion: .50 Contraction: .00

WSPRO OUTPUT

X,Y-coordinates (21 pairs)					
X	Y	X	Y	X	Y
9448.000	790.000	9769.000	790.000	10000.000	780.000
10305.000	771.000	10407.000	770.000	10467.000	769.600
10483.000	763.500	10549.000	763.500	10562.000	769.100
10580.000	769.000	10610.000	768.000	10733.000	768.000
10805.000	769.000	10809.000	770.000	10823.000	775.000
10839.000	780.000	10855.000	785.000	10861.000	787.000
10932.000	770.000	11146.000	780.000	11382.000	790.000

Minimum and Maximum X,Y-coordinates
 Minimum X-Station: 9448.000 (associated Y-Elevation: 790.000)
 Maximum X-Station: 11382.000 (associated Y-Elevation: 790.000)
 Minimum Y-Elevation: 763.500 (associated X-Station: 10549.000)
 Maximum Y-Elevation: 790.000 (associated X-Station: 9448.000)

Roughness Data (3 SubAreas)		
SubArea	Roughness Coefficient	Horizontal Breakpoint
1	1.000	---
	---	*****
2	.034	---
	---	*****
3	1.000	---

* Finished Processing Header Record FULLV *

***** W S P R O *****

Federal Highway Administration - U. S. Geological Survey

Model for Water-Surface Profile Computations.

Input Units: English / Output Units: English

I-74 OVER LITTLE SUGAR CREEK I-74-36-4949

COUNTY: MONTGOMERY QUAD: DARLINGTON

07-24-97 D. C. VOELKER

* Starting To Process Header Record BRDGE *

BR BRDGE 135 778.0 0

GR 10380 0778.5 10380 0776.0 10406 0763.5 10497 0763.5 10522

0775.3

GR 10522 0778.0 10380 0778.5

N .035

PD 763.5 4 2

CD 3 135 2 775

WSPRO OUTPUT

```

*** Completed Reading Data Associated With Header Record BRDGE ***
+++072 NOTICE: X-coordinate # 2 increased to eliminate vertical segment.
+++072 NOTICE: X-coordinate # 6 increased to eliminate vertical segment.
*** Storing Bridge Data In Temporary File As Record Number 3 ***

```

```

*** Data Summary For Bridge Record BRDGE ***
SRD Location: 135. Cross-Section Skew: .0 Error Code 0
Valley Slope: ***** Averaging Conveyance By Geometric Mean.
Energy Loss Coefficients -> Expansion: .50 Contraction: .00

```

X,Y-coordinates (7 pairs)					
X	Y	X	Y	X	Y
10380.000	778.500	10380.100	776.000	10406.000	763.500
10497.000	763.500	10522.000	775.300	10522.100	778.000
10380.000	778.500				

```

Minimum and Maximum X,Y-coordinates
Minimum X-Station: 10380.000 ( associated Y-Elevation: 778.500 )
Maximum X-Station: 10522.100 ( associated Y-Elevation: 778.000 )
Minimum Y-Elevation: 763.500 ( associated X-Station: 10497.000 )
Maximum Y-Elevation: 778.500 ( associated X-Station: 10380.000 )

```

```

Roughness Data ( 1 SubAreas )
Roughness Horizontal
SubArea Coefficient Breakpoint
-----
1 .035 ---
-----

```

```

Discharge coefficient parameters
BRTYPE BRWidth EMBSS EMBELv UserCD
3 135.000 2.00 775.000 *****

```

```

Pressure flow elevations
AVBCEL PFElev
***** 778.000

```

```

Abutment Parameters
ABSLPL ABSLPR XTOELT YTOELT XTOERT YTOERT
*****

```

```

Pier/Pile Data ( 1 Group(s) )
Code Indicates Bridge Uses Piers
Group Elevation Gross Width Number
-----
1 763.500 4.000 2
-----

```

```

*-----*
* Finished Processing Header Record BRDGE *
*-----*

```


WSPRO OUTPUT

***** W S P R O *****

Federal Highway Administration - U. S. Geological Survey

Model for Water-Surface Profile Computations.

Input Units: English / Output Units: English

I-74 OVER LITTLE SUGAR CREEK

I-74-36-4949

COUNTY: MONTGOMERY

QUAD: DARLINGTON

07-24-97

D. C. VOELKER

DC 0 BRDGE 10406 10497 10343 10437
DP 10380 10522 2 * * 1.0 1.0 1.1
DP 10380 10522 2 * * 1.0 1.0 1.1

* Starting To Process Header Record APPR *

XS APPR 388 0
GR 9526 800 9900 780 10316 772 10343 771.6 10359 763.5 10426
763.5
GR 10437 768 10486 768 10623 769 10770 770 10775 772 10780 773
GR 10787 776 10802 777 10807 780 10818 785 10822 787
GR 10851 790 11096 790
N 1.0 .034 1.0
SA 10343 10437

*** Completed Reading Data Associated With Header Record APPR ***
*** Storing X-Section Data In Temporary File As Record Number 4 ***

*** Data Summary For Header Record APPR ***

SRD Location: 388. Cross-Section Skew: .0 Error Code 0

Valley Slope: .00000 Averaging Conveyance By Geometric Mean.

Energy Loss Coefficients -> Expansion: .50 Contraction: .00

X,Y-coordinates (19 pairs)

X	Y	X	Y	X	Y
9526.000	800.000	9900.000	780.000	10316.000	772.000
10343.000	771.600	10359.000	763.500	10426.000	763.500
10437.000	768.000	10486.000	768.000	10623.000	769.000
10770.000	770.000	10775.000	772.000	10780.000	773.000
10787.000	776.000	10802.000	777.000	10807.000	780.000
10818.000	785.000	10822.000	787.000	10851.000	790.000
11096.000	790.000				

Minimum and Maximum X,Y-coordinates

Minimum X-Station: 9526.000 (associated Y-Elevation: 800.000)

Maximum X-Station: 11096.000 (associated Y-Elevation: 790.000)

Minimum Y-Elevation: 763.500 (associated X-Station: 10426.000)

Maximum Y-Elevation: 800.000 (associated X-Station: 9526.000)

Roughness Data (3 SubAreas)

Roughness Horizontal

WSPRO OUTPUT

SubArea	Coefficient	Breakpoint
1	1.000	---
	---	*****
2	.034	---
	---	*****
3	1.000	---

Bridge datum projection(s): XREFLT XREFRT FDSTLT FDSTRT

* Finished Processing Header Record APPR *

***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English

I-74 OVER LITTLE SUGAR CREEK I-74-36-4949
COUNTY: MONTGOMERY QUAD: DARLINGTON
07-24-97 D. C. VOELKER

EX

=====

* Summary of Boundary Condition Information *

=====

#	Reach Discharge	Water Surface Elevation	Friction Slope	Flow Regime
1	6100.00	*****	.0021	Sub-Critical
2	10400.00	*****	.0021	Sub-Critical

=====

* Beginning 2 Profile Calculation(s) *

=====

***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English

I-74 OVER LITTLE SUGAR CREEK I-74-36-4949
COUNTY: MONTGOMERY QUAD: DARLINGTON
07-24-97 D. C. VOELKER

WSEL	VHD	Q	AREA	SRDL	LEW
EGEL	HF	V	K	FLEN	REW
CRWS	HO	FR #	SF	ALPHA	ERR

WSPRO OUTPUT

```

-----
Section: EXIT      772.315   .923   6100.000   2115.145 ***** 10260.430
Header Type: XS    773.238 *****      2.884  133053.80 ***** 10815.480
SRD:      .000     769.506 *****      .696      *****      7.133      *****

```

```

Section: FULLV     772.656   .842   6100.000   2396.601   135.000 10248.880
Header Type: FV     773.498   .264      2.545  143079.40   135.000 10988.840
SRD:    135.000     769.506   .000      .668      .0020      8.357      -.004

```

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

```

Section: APPR      773.161   .772   6100.000   2334.171   253.000 10255.620
Header Type: AS     773.933   .427      2.613  154162.10   253.000 10780.380
SRD:    388.000     769.463   .000      .589      .0017      7.265      .008

```

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

<<< The Following Data Reflect The "Constricted" Profile >>>
 <<< Beginning Bridge/Culvert Hydraulic Computations >>>

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: BRDGE	772.958	.535	6100.000	1048.058	135.000	10386.400
Header Type: BR	773.492	.255	5.820	174536.40	135.000	10517.040
SRD: 135.000	768.623	.000	.365	*****	1.015	.006

Specific Bridge Information	C	P/A	PFELEV	BLEN	XLAB	XRAB
Bridge Type 3 Flow Type 1						
Pier/Pile Code 0	.9926	.036	778.000	*****	*****	*****

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: APPR	773.120	.780	6100.000	2312.818	118.000	10257.740
Header Type: AS	773.900	.223	2.637	152909.30	135.932	10780.280
SRD: 388.000	769.463	.183	.593	.0017	7.207	-.004

Approach Section APPR Flow Contraction Information					
M(G)	M(K)	KQ	XLKQ	XRKQ	OTEL
.753	.033	147999.0	*****	*****	773.120

<<< End of Bridge Hydraulics Computations >>>

***** W S P R O *****
 Federal Highway Administration - U. S. Geological Survey
 Model for Water-Surface Profile Computations.

WSPRO OUTPUT

Input Units: English / Output Units: English

I-74 OVER LITTLE SUGAR CREEK
COUNTY: MONTGOMERY
07-24-97

I-74-36-4949
QUAD: DARLINGTON
D. C. VOELKER

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: EXIT	775.147	1.355	10400.000	3833.807	*****	10164.470
Header Type: XS	776.502	*****	2.713	226788.50	*****	10823.470
SRD: .000	771.650	*****	.682	*****	*****	*****
Section: FULLV	775.526	1.248	10400.000	4476.767	135.000	10151.630
Header Type: FV	776.774	.267	2.323	241302.10	135.000	11050.250
SRD: 135.000	771.650	.000	.674	.0020	*****	.005

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

Section: APPR	776.031	1.189	10400.000	4063.949	253.000	10106.390
Header Type: AS	777.220	.448	2.559	253264.40	253.000	10787.460
SRD: 388.000	771.746	.000	.631	.0018	*****	-.002

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

<<< The Following Data Reflect The "Constricted" Profile >>>

<<< Beginning Bridge/Culvert Hydraulic Computations >>>

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: BRDGE	775.949	.820	10400.000	1457.172	135.000	10380.210
Header Type: BR	776.769	.268	7.137	284285.80	135.000	10522.020
SRD: 135.000	770.675	.000	.399	*****	1.035	.010

Specific Bridge Information	C	P/A	PFELEV	BLEN	XLAB	XRAB
Bridge Type 3	Flow Type 1					
Pier/Pile Code 0	.9832	.034	778.000	*****	*****	*****

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: APPR	776.008	1.194	10400.000	4048.315	118.000	10107.580
Header Type: AS	777.202	.236	2.569	252394.30	132.316	10787.120
SRD: 388.000	771.746	.194	.633	.0018	*****	-.006

Approach Section APPR Flow Contraction Information

M(G) M(K) KQ XLKQ XRKQ OTEL

WSPRO OUTPUT

.793 .051 239796.0 ***** 776.008

<<< End of Bridge Hydraulics Computations >>>

***** W S P R O *****

Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English

I-74 OVER LITTLE SUGAR CREEK	I-74-36-4949
COUNTY: MONTGOMERY	QUAD: DARLINGTON
07-24-97	D. C. VOELKER

*** Live-Bed Contraction Scour Calculations for Header Record BRDGE ***

Constants and Input Variables

Bed Material Transport Mode Factor (k1):	.64
Total Pier Width Value (Pw):	4.000

#	Scour Depth	-- Flow --		-- Width --		--- X-Limits ---	
		Contract	Approach	Contract	Approach	Side	Contract Approach
1	.221	5711.367	5877.314	87.000	94.000	Left:	*****
	Approach Channel Depth:		8.668	Right:	*****
2	.215	9451.665	9803.486	87.000	94.000	Left:	*****
	Approach Channel Depth:		11.555	Right:	*****

--

***** W S P R O *****

Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English

I-74 OVER LITTLE SUGAR CREEK	I-74-36-4949
COUNTY: MONTGOMERY	QUAD: DARLINGTON
07-24-97	D. C. VOELKER

*** Pier Scour Calculations for Header Record BRDGE ***

Constants and Input Variables

Pier Width: 2.000

Pier Shape Factor	(K1):	1.00
Flow Angle of Attack Factor	(K2):	1.00
Bed Condition Factor	(K3):	1.10
Bed Material Factor	(K4):	1.00
Velocity Multiplier	(VM):	1.00
Depth Multiplier	(YM):	1.00

WSPRO OUTPUT

```

*-----*

```

#	---- Localized Hydraulic Properties ----						-- X-Stations --	
	Scour Depth	Flow	WSE	Depth	Velocity	Froude #	Left	Right
1	4.93	6100.000	773.140	9.640	6.384	.362	10380.000	10522.000
2	5.63	10400.000	776.143	12.643	7.973	.395	10380.000	10522.000

```

-----

```

```

***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English

```

```

-----*
I-74 OVER LITTLE SUGAR CREEK          I-74-36-4949
COUNTY: MONTGOMERY                   QUAD: DARLINGTON
07-24-97                             D. C. VOELKER

```

*** Pier Scour Calculations for Header Record BRDGE ***

Constants and Input Variables

Pier Width: 2.000

```

*-----*
Pier Shape Factor          (K1): 1.00
Flow Angle of Attack Factor (K2): 1.00
Bed Condition Factor       (K3): 1.10
Bed Material Factor        (K4): 1.00
Velocity Multiplier        (VM): 1.00
Depth Multiplier           (YM): 1.00
*-----*

```

```

-----

```

#	---- Localized Hydraulic Properties ----						-- X-Stations --	
	Scour Depth	Flow	WSE	Depth	Velocity	Froude #	Left	Right
1	4.93	6100.000	773.140	9.640	6.384	.362	10380.000	10522.000
2	5.63	10400.000	776.143	12.643	7.973	.395	10380.000	10522.000

```

-----

```

ER

```

***** Normal end of WSPRO execution. *****
***** Elapsed Time: 0 Minutes 3 Seconds *****

```