

Modified Level II Streambed-Scour Analysis for Structure I-465-158-4458 Crossing State Ditch in Marion County, Indiana

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INDIANA DEPARTMENT OF TRANSPORTATION

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CONTENTS

Abstract	1
Introduction	1
Background and Scope	1
Site Description	2
Evaluation Methods	3
Special Considerations	5
Results	6
References	6
Appendix	7
Water Surface PROfile Model (WSPRO) Input File	8
Water Surface PROfile Model (WSPRO) Output	10

Tables

1. Cumulative scour depths for the modeled discharges at structure I-465-158-4458 crossing State Ditch in Marion County, Indiana	5
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CONVERSION FACTORS AND ABBREVIATIONS

Multiply	By	To obtain
inch (in.)	25.4	millimeter
foot (ft)	0.3048	meter
square foot (ft ²)	929.0	square centimeter
feet per second (ft/s)	0.3048	meters per second
cubic foot per second (ft ³ /s)	0.02832	cubic meter per second
mile (mi)	1.609	kilometer
square mile (mi ²)	2.590	square kilometer

Abbreviations used in this report:

D ₅₀	median diameter of bed material
Q100	100-year discharge
FEMA	Federal Emergency Management Agency
HEC	Hydraulic Engineering Circular
IDNR	Indiana Department of Natural Resources
INDOT	Indiana Department of Transportation
USGS	U. S. Geological Survey
WSPRO	Water Surface PROfile model

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By David C. Voelker, Robert L. Miller, *and* Bret A. Robinson

ABSTRACT

Level II scour evaluations follow a process in which hydrologic, hydraulic, and sediment-transport data are evaluated to calculate the depth of scour that may result when a given discharge is routed through a bridge opening. The results of the modified Level II analysis for structure I-465-158-4458 on Interstate 465 crossing State Ditch in Marion County, Indiana, are presented. The site is in the city of Indianapolis in the southwestern part of Marion County. Scour depths were computed with the Water Surface PROfile model, version V050196, which incorporates the scour-calculation procedures outlined in Hydraulic Engineering Circular No. 18. Total scour depths at the piers were approximately 28.9 feet for the modeled discharge of 4,300 cubic feet per second and approximately 38.6 feet for the modeled discharge of 6,450 cubic feet per second.

INTRODUCTION

The U.S. Geological Survey (USGS), in cooperation with the Indiana Department of Transportation (INDOT), is conducting Level II scour analyses at a number of bridges throughout Indiana. This report describes the methods applied and the modeling results for bridge I-465-158-4458.

Background and Scope

Level I scour assessment is a process where a large number of bridges are studied as a group. Assessments usually are made by evaluating a combination of geomorphic, hydrologic, and bridge-characteristic data. The results help investigators determine which bridges appear to be most likely to experience streambed-scour problems and which bridges appear to be relatively immune to problems brought on by streambed scour (for example, bridges built on bedrock).

When applied correctly, Level I scour assessments provide an investigator with information to identify those bridges that appear to be relatively safe and those bridges that fall into higher risk categories.

Level II scour evaluations describe the process for an investigator to apply a model to a bridge site and calculate the potential depth of scour that may result from a given flood event. Level II analyses involve the application of basic hydrologic, hydraulic, and sediment-transport engineering concepts and may include an evaluation of flood history, channel hydraulic conditions (for example, water-surface profile analysis), and basic sediment-transport analyses such as scour calculations (Lagasse and others, 1995).

The methods and model outlined in Hydraulic Engineering Circular (HEC) No. 18 (Richardson and Davis, 1995) formulate the basis for Level II scour evaluations. Methods used in this study for Level II scour evaluations are a modification of the HEC-18 standards. These modifications were made to comply with the methodology requested by INDOT (Merril Dougherty, Indiana Department of Transportation, oral commun., 1996). Descriptions of the specific modifications are given in the "Evaluation Methods" section of this report.

This report presents the methods followed for modeling, special considerations for this study site, and the input for and the output from the Water Surface PROfile (WSPRO) model.

Site Description

The study site is located in the city of Indianapolis in the southwestern part of Marion County. The drainage area for the site is approximately 9.5 mi² (estimated using Hoggatt, 1975, and USGS 7.5-minute topographic data). The predominant land use in the basin is urban; in the immediate vicinity of the bridge, the land is predominantly suburban with some pasture land nearby.

Within the immediate vicinity of the bridge, State Ditch has a channel-bed slope of approximately 0.004 ft/ft. The channel-bed material is gravelly sandy silt-clay, and the channel banks consist of gravelly sandy silt-clay. At the time of the Level I site visit on May 23, 1995, the banks were observed to have 26 to 75 percent woody vegetative cover; the field report noted that the banks were experiencing fluvial erosion.

The Interstate 465 crossing of State Ditch is a 116-ft-long, multi-lane bridge consisting of three spans supported by concrete and steel piers and sloping concrete spill-through abutments. Additional details describing conditions at the site are included in the Level I data base (Hopkins and Robinson, unpub. data, 1997). Photographs of the site, taken at the time of the Level I site visit, are archived at the USGS office in Indianapolis.

EVALUATION METHODS

The methods described in this section apply to a number of bridge sites in Indiana being evaluated for scour and outline the procedures requested by INDOT for these modified Level II scour analyses. The principal modification requested by INDOT was that the input data to the model come from or be estimated from existing data sources; no additional field data were collected. Actual methods used in the scour evaluation at this particular bridge site use the most applicable method possible, given the data available.

To determine drainage area, either published values found in Hoggatt (1975) or 7.5-minute topographic maps with Hoggatt's original drainage-area delineations were used. Where there are no published data, drainage-area segments measured from the maps produced by Hoggatt were either subtracted from downstream sites or added to upstream sites published by Hoggatt (1975).

In Indiana, flood discharges are coordinated by agreement among State and Federal agencies. At sites where flood discharges officially are coordinated among State and Federal agencies in Indiana, the coordinated 100-year discharge (Q100) was modeled. INDOT also provided an additional flood discharge for these coordinated sites in excess of the Q100 to be modeled.

If a flood discharge was not coordinated, the USGS examined Federal Emergency Management Agency (FEMA) studies for Q100 determinations. Where FEMA studies did not produce a Q100, the USGS contacted IDNR for an estimated Q100 in the vicinity of the site being studied. If IDNR did not have a Q100, data from nearby USGS streamflow-gaging stations were analyzed with nearby and similar drainage basins that have been coordinated. At sites having no coordinated discharge data, the two discharges used in the model were 1) the approximated Q100 and 2) a discharge equal to 1.7 times the approximated Q100.

Most of the cross-section and bridge-opening geometry data were taken from the bridge plans (Indiana State Highway Commission, 1959) provided by INDOT. Bridge plans are presumed to be representative of current conditions at the site. To determine the cross-section geometry, a line was drawn on the bridge plans parallel to the bridge stationing and approximately one bridge width from the bridge. For sites where the bridge plans did not extend far enough laterally for collection of all cross-section data required for WSPRO model analysis, additional data were collected from 7.5-minute topographic maps.

The roadway and embankment profile was taken from the bridge and highway plans for those sites where roadway overtopping was expected. The INDOT bridge plans and 7.5-minute topographic maps were used as a guide, based on the water-surface elevations calculated by the WSPRO model, to determine if roadway overtopping might occur.

Roughness values (*n*-values) for the main channel were estimated by viewing photographs archived from the Level I scour assessments. The *n*-values for the overbanks were assigned on the basis of the surface-cover data summarized in the Level I data base (Hopkins and Robinson, unpub. data, 1997). From those data, the following roughness values were assigned to the surface-cover categories: urban—0.050, suburban—0.035, row crop—0.045, pasture—0.035, brush—0.120, forest—0.100, and wetland (any area covered by standing water)—0.100. The *n*-values for the overbanks were adjusted if the Level I photographs provided sufficient detail to warrant an adjustment.

WSPRO version V050196 was used to model flow through the study site. Starting water-surface elevation was obtained with a slope-conveyance computation. The channel-bed slope in the immediate vicinity of the bridge was estimated from the 7.5-minute topographic map and was used as the slope of the energy grade line for this computation.

WSPRO version V050196 includes a field that allows the input of up to four scour-adjustment factors (K1 to K4). For this modeling, the default value for K4 (bed armoring) was chosen. For scour-adjustment factors K1 and K2 (pier-nose shape and angle of attack, respectively), input values were determined by evaluating the data archived in the Level I data base (Hopkins and Robinson, unpub. data, 1997). For the K3 factor (bed forms), a value of 1.1 was applied in all cases.

In some cases, piers set on the overbanks are constructed with footings that are higher in elevation than pier footings in the main channel. In these situations, if the channel position changes, the piers that were initially constructed on the overbank may become part of the main channel. Therefore, to evaluate total potential scour, the model results obtained for contraction scour and deepest local scour in the main channel were added and applied to all piers in the bridge opening. This methodology allowed for an evaluation of potential undermining of pier supports in the event that future channel movement placed overbank piers in the main channel.

Where bridge pairs have a continuous abutment or fill between the bridges that does not allow expansion of flow, the bridge pair was modeled as one bridge. Sites with discontinuous abutments, allowing expansion between the bridges, were modeled as two separate bridges. In those cases, a valley cross section was measured between the bridges and used as the approach section for the downstream bridge and as the exit section for the upstream bridge.

At sites with no embankment to function as a weir or at sites where the tailwater drowns out the embankment, a composite bridge and road section was used to compute flow. Those sites were computed with friction-loss equations rather than with a bridge routine.

Total scour is taken as the sum of local scour plus contraction scour. If the model predicted negative contraction scour (aggradation), the contraction-scour value was assumed to be zero in determining the total scour depth (table 1). This assumption was made so that a negative contraction scour would not mask the potentially detrimental effects of local scour at a pier. No abutment scour evaluations were made in this study.

Table 1. Cumulative scour depths for the modeled discharges at structure I-465-158-4458 crossing State Ditch in Marion County, Indiana

Pier number ¹	Stationing from bridge plans ²	Initial bed-elevation at pier (feet)	Main-channel contraction scour depth (feet)	Local scour depth (feet)	Worst-case total-scour depth ³ (feet)	Bottom elevation of pier (feet)	Worst-case bed elevation after scour ⁴ (feet)
Modeled discharge⁵ is 4,300 cubic feet per second							
1	182+33	661	15.3	13.6	28.9	655	631.2
2	182+82	661	15.3	13.6	28.9	655	631.2
Modeled discharge is 6,450 cubic feet per second							
1	182+33	661	24.4	14.2	38.6	655	621.5
2	182+82	661	24.4	14.2	38.6	655	621.5

¹Pier numbers were assigned from left to right as shown on the bridge plans.

²Stationing is the center line of the pier as determined from the bridge plans. Stationing from bridge plan, 182+33, represents a point 18,233 feet from an arbitrary starting location referenced on the bridge plans.

³Worst-case total-scour depths are generated by summing the calculated contraction-scour depth with the worst case of local scour.

⁴Worst-case bed elevation is computed by subtracting the worst-case total-scour depth from the lowest initial bed elevation at the bridge opening (660.1 feet).

⁵Coordinated discharge.

SPECIAL CONSIDERATIONS

Model runs indicate the water-surface elevation at the bridge is lower than the low-steel elevation for the modeled discharges. Therefore, there should be no pressure flow through the bridge opening for the discharges modeled.

This site is unique because of the presence of the Thompson Road bridge 45 ft downstream from the I-465-158-4458 bridge structure. The Thompson Road bridge opening is much smaller than the Interstate bridge opening and creates a major constriction in the downstream channel. The site was modeled, starting with a valley section downstream from this constriction.

A composite section was developed to represent the Thompson Road bridge, and the flow was routed through this constriction and the I-465-158-4458 bridge opening. The model calculated initial water-surface elevations based on the valley cross section. From those calculations, it was determined that critical flow occurred at the Thompson Road bridge. The model calculated a drop in water surface of more than 8 ft from the constricted opening of the Thompson Road bridge to the valley cross section. This flow solution does not appear to be unreasonable, based on the channel geometry provided on the bridge plans. Caution should be exercised, however, in using the results of this model because of the complex channel geometry and stream hydraulics in the study reach.

RESULTS

Scour depths were computed with a version of WSPRO (Larry Arneson, Federal Highway Administration, written commun., 1996) modified from Shearman (1990). This version of WSPRO includes scour calculations in the model output. Scour depths were calculated assuming an infinite depth of material that could erode and a homogeneous particle-size distribution. The results of the scour analysis are presented in table 1; a complete input file and output results are presented in the appendix.

REFERENCES

- Hoggatt, R.E., 1975, Drainage areas of Indiana streams: U.S. Geological Survey, Water Resources Division, 231 p.
- Indiana State Highway Commission, 1959, Bridge plans Interstate Route 465: Bridge File I-465-158-4458.
- Lagasse, P.F.; Schall, J.D.; Johnson, F.; Richardson, E.V.; and Chang, F., 1995, Stream stability at highway structures (2d ed.): Federal Highway Administration, Hydraulic Engineering Circular No. 20, Publication FHWA-IP-90-014, 144 p.
- Richardson, E.V., and Davis, S.R., 1995, Evaluating scour at bridges (3d ed.): Federal Highway Administration, Hydraulic Engineering Circular No. 18, Publication FHWA-IP-90-017, 204 p.
- Shearman, J.O., 1990, User's manual for WSPRO, a computer model for water-surface profile computations: Federal Highway Administration Publication FHWA-IP-89-027, 177 p.

APPENDIX

WSPRO INPUT FILE

```

T1          I-465 over State Ditch          I-465-158-4458
T2          County: Marion                  Quad: Maywood, IND 124A
T3          8-12-97                        David C Voelker
SI          0
J1          0.05 * * *
*          Q-100      Q-500
Q          4300      6450
*          WS                      669.65
*          SLOPE
SK          .004      .004
*          SRD SKEW (EK) (CK) (VSLOPE)
XS  VALL -206
GR          18234 675 18237 674 18242 673 18246 672 18256 671 18276 670
GR          18289 669 18304 668 18361 667 18378 666 18381 665 18384 664
GR          18387 663 18389 662 18390 661 18392 655 18399 654 18405 655
GR          18407 659 18414 660 18416 661 18418 662 18420 663 18422 664
GR          18424 665 18509 680 18556 675 18569 670 18637 665 18733 660
GR          19385 660 19475 665 19644 665 21182 670 21219 675
*          N1          N2          N3
N          .050          .035          .050
*          SA          SA
SA          18390          18414
XS  EXIT  0
GR          18205 677.4 18254 673 18273 671.6 18284 671.6 18284 657.4
GR          18292 656.6 18303 656.4 18314 657.8 18323 660.0 18323 671.6
GR          18337 671.6 18340 670 18381 677.4
*          N1          N2          N3
N          .050          .035          .050
*          SA          SA
SA          18283          18323
*          SRD SKEW (EK) (CK) (VSLOPE)
XS  FULLV 45
GR          18205 677.4 18254 673 18273 671.6 18284 671.6 18284 657.4
GR          18292 656.6 18303 656.4 18314 657.8 18323 660.0 18323 671.6
GR          18337 671.6 18340 670 18381 677.4
*          N1          N2          N3
N          .050          .035          .050
*          SA          SA
SA          18283          18323
*          SRD LSEL SKEW (EK) (CK) (USERCD)
BR  BRDGE 45      675.5      15
GR          18202 0675.9 18233 0661.3 18256 0660.2 18281 0661.2 18314 0676.0
GR          18310 0676.0 18306 0675.9 18291 0675.4 18287 0675.0 18285 0674.8
GR          18283 0674.6 18281 0674.5 18279 0674.9 18276 0675.1 18272 0675.4
GR          18268 0675.7 18265 0675.8 18260 0676.0 18257 0676.0 18254 0676.0
GR          18249 0675.9 18245 0675.7 18242 0675.4 18240 0675.2 18237 0674.9
GR          18234 0674.5 18232 0674.5 18230 0674.7 18228 0675.0 18225 0675.2
GR          18222 0675.4 18216 0675.7 18213 0675.8 18208 0675.9 18203 0675.9
GR          18202 0675.9
*          N1
N          .036
*  BLANK=
*  PIERS,
*  1=PILES PELV PDDTH

```

WSPRO INPUT FILE

```
PD 0      661.6   4.0   1
*          BRTYPE BRWDTH EMBSS EMBELV WWANGL WWWID ENTRND
CD         3      140   2    676
* X=0 FOR LIVE BED OR 1 FOR CLEARWATER
*          BXL   BXR   AXL   AXR
*
*                               X=K1 OR D50
*                               PW
DC 0 BRDGE  18202   18314  18185  18238  *  4.0
DP  BRDGE  18202   18314   2    *    *   1.0  2.49
DP  BRDGE  18202   18314   2    *    *   1.0  2.49
*          SRD SKEW (EK) (CK) (VSLOPE)
XS  APPR   325
GR    17846 685  17885 680  17939 675  17995 670  18036 667  18046 666
GR    18049 665  18052 664  18084 664  18088 665  18100 666  18109 667
GR    18179 667  18190 660.5  18240 660.5  18254 667  18323 667  18462 666
GR    18686 665  18795 670
N          .035      .06          .036          .06          .035
SA          18100      18175      18250      18290
EX
ER
```

WSPRO OUTPUT

***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Run Date & Time: 8/12/97 2:44 pm Version V050196
Input File: 4458.dat Output File: 4458.LST

-----*
T1 I-465 OVER STATE DITCH I-465-158-4458
T2 COUNTY: MARION QUAD: MAYWOOD, IND 124A
T3 8-12-97 DAVID C VOELKER
SI 0
J1 0.05 * * *

-----*

Computational Control (J1) Parameters

-----*
Trial WSE Stepping Increment (DeltaY): .05
Allowable Elevation Tolerance (yTol): .02
Allowable Discharge Tolerance (qTol): .02
Maximum Froude Number Test Value (FNTTest): .80
Friction-Loss Computation Method (IHFNOJ): 0
-----*

Q 4300 6450

*** Processing Flow Data; Placing Information into Sequence 1 ***

SK .004 .004

***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English

-----*

I-465 OVER STATE DITCH I-465-158-4458
COUNTY: MARION QUAD: MAYWOOD, IND 124A
8-12-97 DAVID C VOELKER

-----*
* Starting To Process Header Record VALL *
-----*

XS VALL -206
GR 18234 675 18237 674 18242 673 18246 672 18256 671 18276 670
GR 18289 669 18304 668 18361 667 18378 666 18381 665 18384 664
GR 18387 663 18389 662 18390 661 18392 655 18399 654 18405 655
GR 18407 659 18414 660 18416 661 18418 662 18420 663 18422 664
GR 18424 665 18509 680 18556 675 18569 670 18637 665 18733 660
GR 19385 660 19475 665 19644 665 21182 670 21219 675
N .050 .035 .050
SA 18390 18414

*** Completed Reading Data Associated With Header Record VALL ***
*** Storing X-Section Data In Temporary File As Record Number 1 ***

WSPRO OUTPUT

```

***          Data Summary For Header Record VALL          ***
SRD Location:   -206.   Cross-Section Skew:   .0   Error Code   0
Valley Slope:  .00000   Averaging Conveyance By Geometric Mean.
Energy Loss Coefficients ->   Expansion:   .50   Contraction:   .00
    
```

X,Y-coordinates (35 pairs)					
X	Y	X	Y	X	Y
18234.000	675.000	18237.000	674.000	18242.000	673.000
18246.000	672.000	18256.000	671.000	18276.000	670.000
18289.000	669.000	18304.000	668.000	18361.000	667.000
18378.000	666.000	18381.000	665.000	18384.000	664.000
18387.000	663.000	18389.000	662.000	18390.000	661.000
18392.000	655.000	18399.000	654.000	18405.000	655.000
18407.000	659.000	18414.000	660.000	18416.000	661.000
18418.000	662.000	18420.000	663.000	18422.000	664.000
18424.000	665.000	18509.000	680.000	18556.000	675.000
18569.000	670.000	18637.000	665.000	18733.000	660.000
19385.000	660.000	19475.000	665.000	19644.000	665.000
21182.000	670.000	21219.000	675.000		

Minimum and Maximum X,Y-coordinates

```

Minimum X-Station:  18234.000   ( associated Y-Elevation:  675.000 )
Maximum X-Station:  21219.000   ( associated Y-Elevation:  675.000 )
Minimum Y-Elevation:  654.000   ( associated X-Station:  18399.000 )
Maximum Y-Elevation:  680.000   ( associated X-Station:  18509.000 )
    
```

Roughness Data (3 SubAreas)

SubArea	Roughness Coefficient	Horizontal Breakpoint
1	.050	---
	---	*****
2	.035	---
	---	*****
3	.050	---

```

*-----*
*           Finished Processing Header Record VALL           *
*-----*
    
```

```

***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English
    
```

```

*-----*
I-465 OVER STATE DITCH                I-465-158-4458
COUNTY: MARION                        QUAD: MAYWOOD, IND 124A
8-12-97                                DAVID C VOELKER
    
```

```

*-----*
*           Starting To Process Header Record EXIT           *
    
```

WSPRO OUTPUT

```

XS   EXIT   0
GR      18205 677.4  18254 673  18273 671.6  18284 671.6  18284 657.4
GR      18292 656.6  18303 656.4  18314 657.8  18323 660.0  18323 671.6
GR      18337 671.6  18340 670  18381 677.4
N        .050      .035      .050
SA              18283      18323
    
```

```

***   Completed Reading Data Associated With Header Record EXIT   ***
+++072 NOTICE: X-coordinate # 5 increased to eliminate vertical segment.
+++072 NOTICE: X-coordinate #10 increased to eliminate vertical segment.
***   Storing X-Section Data In Temporary File As Record Number 2   ***
    
```

```

***           Data Summary For Header Record EXIT           ***
SRD Location:          0.   Cross-Section Skew:      .0   Error Code   0
Valley Slope:    .00000   Averaging Conveyance By Geometric Mean.
Energy Loss Coefficients ->   Expansion:    .50   Contraction:    .00
    
```

X,Y-coordinates (13 pairs)

X	Y	X	Y	X	Y
18205.000	677.400	18254.000	673.000	18273.000	671.600
18284.000	671.600	18284.100	657.400	18292.000	656.600
18303.000	656.400	18314.000	657.800	18323.000	660.000
18323.100	671.600	18337.000	671.600	18340.000	670.000
18381.000	677.400				

Minimum and Maximum X,Y-coordinates

```

Minimum X-Station:  18205.000  ( associated Y-Elevation:  677.400 )
Maximum X-Station:  18381.000  ( associated Y-Elevation:  677.400 )
Minimum Y-Elevation:  656.400  ( associated X-Station:  18303.000 )
Maximum Y-Elevation:  677.400  ( associated X-Station:  18205.000 )
    
```

Roughness Data (3 SubAreas)

SubArea	Roughness Coefficient	Horizontal Breakpoint
1	.050	---
	---	*****
2	.035	---
	---	*****
3	.050	---

* Finished Processing Header Record EXIT *

***** W S P R O *****

Federal Highway Administration - U. S. Geological Survey
 Model for Water-Surface Profile Computations.
 Input Units: English / Output Units: English

WSPRO OUTPUT

```

*-----*
I-465 OVER STATE DITCH                I-465-158-4458
COUNTY: MARION                        QUAD: MAYWOOD, IND 124A
8-12-97                                DAVID C VOELKER
  
```

```

*-----*
*           Starting To Process Header Record FULLV           *
*-----*
  
```

```

XS  FULLV 45
GR   18205 677.4 18254 673 18273 671.6 18284 671.6 18284 657.4
GR   18292 656.6 18303 656.4 18314 657.8 18323 660.0 18323 671.6
GR   18337 671.6 18340 670 18381 677.4
N    .050      .035      .050
SA           18283      18323
  
```

```

*** Completed Reading Data Associated With Header Record FULLV ***
+++072 NOTICE: X-coordinate # 5 increased to eliminate vertical segment.
+++072 NOTICE: X-coordinate #10 increased to eliminate vertical segment.
*** Storing X-Section Data In Temporary File As Record Number 3 ***
  
```

```

*** Data Summary For Header Record FULLV ***
SRD Location:      45.  Cross-Section Skew:      .0  Error Code  0
Valley Slope:     .00000  Averaging Conveyance By Geometric Mean.
Energy Loss Coefficients -> Expansion:      .50  Contraction:      .00
  
```

```

                X,Y-coordinates (13 pairs)
      X           Y           X           Y           X           Y
-----
18205.000      677.400      18254.000      673.000      18273.000      671.600
18284.000      671.600      18284.100      657.400      18292.000      656.600
18303.000      656.400      18314.000      657.800      18323.000      660.000
18323.100      671.600      18337.000      671.600      18340.000      670.000
18381.000      677.400
-----
  
```

```

                Minimum and Maximum X,Y-coordinates
Minimum X-Station: 18205.000 ( associated Y-Elevation: 677.400 )
Maximum X-Station: 18381.000 ( associated Y-Elevation: 677.400 )
Minimum Y-Elevation: 656.400 ( associated X-Station: 18303.000 )
Maximum Y-Elevation: 677.400 ( associated X-Station: 18205.000 )
  
```

```

                Roughness Data ( 3 SubAreas )
                Roughness Horizontal
                SubArea Coefficient Breakpoint
-----
                1          .050      ---
                ---          *****
                2          .035      ---
                ---          *****
                3          .050      ---
-----
  
```

```

*-----*
  
```

WSPRO OUTPUT

* Finished Processing Header Record FULLV *

***** W S P R O *****
 Federal Highway Administration - U. S. Geological Survey
 Model for Water-Surface Profile Computations.
 Input Units: English / Output Units: English

I-465 OVER STATE DITCH I-465-158-4458
 COUNTY: MARION QUAD: MAYWOOD, IND 124A
 8-12-97 DAVID C VOELKER

* Starting To Process Header Record BRDGE *

BR	BRDGE	45	675.5	15						
GR		18202	0675.9	18233	0661.3	18256	0660.2	18281	0661.2	18314
0676.0										
GR		18310	0676.0	18306	0675.9	18291	0675.4	18287	0675.0	18285
0674.8										
GR		18283	0674.6	18281	0674.5	18279	0674.9	18276	0675.1	18272
0675.4										
GR		18268	0675.7	18265	0675.8	18260	0676.0	18257	0676.0	18254
0676.0										
GR		18249	0675.9	18245	0675.7	18242	0675.4	18240	0675.2	18237
0674.9										
GR		18234	0674.5	18232	0674.5	18230	0674.7	18228	0675.0	18225
0675.2										
GR		18222	0675.4	18216	0675.7	18213	0675.8	18208	0675.9	18203
0675.9										
GR		18202	0675.9							
N			.036							
PD 0		661.6	4.0	1						
CD		3	140	2	676					

*** Completed Reading Data Associated With Header Record BRDGE ***
 *** Storing Bridge Data In Temporary File As Record Number 4 ***

*** Data Summary For Bridge Record BRDGE ***
 SRD Location: 45. Cross-Section Skew: 15.0 Error Code 0
 Valley Slope: ***** Averaging Conveyance By Geometric Mean.
 Energy Loss Coefficients -> Expansion: .50 Contraction: .00

X,Y-coordinates (36 pairs)

X	Y	X	Y	X	Y
18202.000	675.900	18233.000	661.300	18256.000	660.200
18281.000	661.200	18314.000	676.000	18310.000	676.000
18306.000	675.900	18291.000	675.400	18287.000	675.000
18285.000	674.800	18283.000	674.600	18281.000	674.500
18279.000	674.900	18276.000	675.100	18272.000	675.400
18268.000	675.700	18265.000	675.800	18260.000	676.000
18257.000	676.000	18254.000	676.000	18249.000	675.900

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18245.000	675.700	18242.000	675.400	18240.000	675.200
18237.000	674.900	18234.000	674.500	18232.000	674.500
18230.000	674.700	18228.000	675.000	18225.000	675.200
18222.000	675.400	18216.000	675.700	18213.000	675.800
18208.000	675.900	18203.000	675.900	18202.000	675.900

Minimum and Maximum X,Y-coordinates

Minimum X-Station: 18202.000 (associated Y-Elevation: 675.900)
 Maximum X-Station: 18314.000 (associated Y-Elevation: 676.000)
 Minimum Y-Elevation: 660.200 (associated X-Station: 18256.000)
 Maximum Y-Elevation: 676.000 (associated X-Station: 18314.000)

X-coordinates & Horizontal Breakpoints Translated by Skew Angle

X Input	X Skewed	X Input	X Skewed	X Input	X Skewed
18202.000	18203.840	18233.000	18233.780	18256.000	18256.000
18281.000	18280.150	18314.000	18312.020	18310.000	18308.160
18306.000	18304.300	18291.000	18289.810	18287.000	18285.940
18285.000	18284.010	18283.000	18282.080	18281.000	18280.150
18279.000	18278.220	18276.000	18275.320	18272.000	18271.460
18268.000	18267.590	18265.000	18264.690	18260.000	18259.860
18257.000	18256.970	18254.000	18254.070	18249.000	18249.240
18245.000	18245.380	18242.000	18242.480	18240.000	18240.540
18237.000	18237.650	18234.000	18234.750	18232.000	18232.820
18230.000	18230.890	18228.000	18228.950	18225.000	18226.060
18222.000	18223.160	18216.000	18217.360	18213.000	18214.460
18208.000	18209.630	18203.000	18204.810	18202.000	18203.840

Roughness Data (1 SubAreas)

SubArea	Roughness Coefficient	Horizontal Breakpoint
1	.036	---

Discharge coefficient parameters

BRType	BRWidth	EMBSS	EMBElv	UserCD
3	140.000	2.00	676.000	*****

Pressure flow elevations

AVBCEL	PFElev
*****	675.500

Abutment Parameters

ABSLPL	ABSLPR	XTOELT	YTOELT	XTOERT	YTOERT
*****	*****	*****	*****	*****	*****

Pier/Pile Data (1 Group(s))

Code Indicates Bridge Uses Piers

Group	Elevation	Gross Width	Number
-----	-----	-----	-----

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Maximum X-Station: 18795.000 (associated Y-Elevation: 670.000)
 Minimum Y-Elevation: 660.500 (associated X-Station: 18240.000)
 Maximum Y-Elevation: 685.000 (associated X-Station: 17846.000)

SubArea	Roughness Coefficient	Horizontal Breakpoint
1	.035	---
	---	*****
2	.060	---
	---	*****
3	.036	---
	---	*****
4	.060	---
	---	*****
5	.035	---

Bridge datum projection(s): XREFLT XREFRT FDSTLT FDSTRT

 * Finished Processing Header Record APPR *

***** W S P R O *****
 Federal Highway Administration - U. S. Geological Survey
 Model for Water-Surface Profile Computations.
 Input Units: English / Output Units: English

 I-465 OVER STATE DITCH I-465-158-4458
 COUNTY: MARION QUAD: MAYWOOD, IND 124A
 8-12-97 DAVID C VOELKER

EX

=====
 * Summary of Boundary Condition Information *
 =====

#	Reach Discharge	Water Surface Elevation	Friction Slope	Flow Regime
1	4300.00	*****	.0040	Sub-Critical
2	6450.00	*****	.0040	Sub-Critical

=====
 * Beginning 2 Profile Calculation(s) *
 =====

***** W S P R O *****
 Federal Highway Administration - U. S. Geological Survey

WSPRO OUTPUT

Model for Water-Surface Profile Computations.
 Input Units: English / Output Units: English

I-465 OVER STATE DITCH
 COUNTY: MARION
 8-12-97

I-465-158-4458
 QUAD: MAYWOOD, IND 124A
 DAVID C VOELKER

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: VALL	661.825	.246	4300.000	1384.116	*****	18389.180
Header Type: XS	662.071	*****	3.107	67948.19	*****	19417.850
SRD: -206.000	661.395	*****	.516	*****	1.642	*****

===110 WSEL NOT FOUND AT SECID "EXIT ": REDUCED DELTAY.
 WSLIM1, WSLIM2, DELTAY: 664.57 677.40 .03

===115 WSEL NOT FOUND AT SECID "EXIT ": USED WSMIN = CRWS.
 WSLIM1, WSLIM2, CRWS: 664.57 677.40 664.57

===130 CRITICAL WATER-SURFACE ELEVATION A _ S _ S _ U _ M _ E _ D !!!!!
 ENERGY EQUATION N _ O _ T _ B _ A _ L _ A _ N _ C _ E _ D AT SECID "EXIT ".
 WSBEQ, WSEND, CRWS: 664.57 677.40 664.57

Section: EXIT	664.566	3.621	4300.000	281.886	206.000	18284.050
Header Type: XS	668.187	*****	15.254	39790.75	206.000	18323.040
SRD: .000	664.566	*****	1.001	.0012	1.001	*****

Section: FULLV	666.115	2.456	4300.000	342.321	45.000	18284.040
Header Type: FV	668.572	.389	12.561	53798.84	45.000	18323.050
SRD: 45.000	664.566	.000	.748	.0086	1.001	-.004

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

===135 CONVEYANCE RATIO OUTSIDE OF RECOMMENDED LIMITS AT SECID "APPR ".
 KRATIO: 4.27

Section: APPR	668.921	.069	4300.000	2398.769	280.000	18009.750
Header Type: AS	668.990	.418	1.793	229983.90	280.000	18771.480
SRD: 325.000	666.500	.000	.209	.0015	1.381	.000

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

<<< The Following Data Reflect The "Constricted" Profile >>>
 <<< Beginning Bridge/Culvert Hydraulic Computations >>>

===210 QUESTIONABLE CRITICAL-FLOW SOLUTION AT SECID "BRDGE".
 Q, CRWS: 4300.00 666.84

WSEL	VHD	Q	AREA	SRDL	LEW
EGEL	HF	V	K	FLEN	REW
CRWS	HO	FR #	SF	ALPHA	ERR

WSPRO OUTPUT

```

-----
Section: BRDGE      666.839  2.626  4300.000  361.520  45.000 18221.240
Header Type: BR    669.465  *****  11.894  42649.25  45.000 18293.570
SRD:      45.000    666.839  *****  1.025    *****  1.194  *****
  
```

```

-----
Specific Bridge Information  C      P/A  PFELEV  BLEN  XLAB  XRAB
Bridge Type 3  Flow Type 1 -----
Pier/Pile Code 0          .9153  .058  675.500  *****  *****  *****
-----
  
```

```

-----
                WSEL  VHD      Q      AREA  SRDL      LEW
                EGEL  HF      V      K      FLEN      REW
                CRWS  HO      FR #    SF      ALPHA     ERR
-----
Section: APPR      671.321  .018  4300.000  4308.106  140.000 17980.200
Header Type: AS    671.340  .164  .998  544521.50  192.654 18795.000
SRD:      325.000    666.500  1.711  .083  .0015  1.187  .015
  
```

```

-----
Approach Section APPR Flow Contraction Information
M( G )  M( K )  KQ      XLKQ  XRKQ  OTEL
-----
        .909  .850  81331.3 ***** ***** 671.321
-----
  
```

<<< End of Bridge Hydraulics Computations >>>

```

***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English
  
```

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I-465 OVER STATE DITCH                I-465-158-4458
COUNTY: MARION                        QUAD: MAYWOOD, IND 124A
8-12-97                                DAVID C VOELKER
  
```

```

-----
                WSEL  VHD      Q      AREA  SRDL      LEW
                EGEL  HF      V      K      FLEN      REW
                CRWS  HO      FR #    SF      ALPHA     ERR
-----
Section: VALL      662.395  .275  6450.000  1817.359  ***** 18388.210
Header Type: XS    662.670  *****  3.549  101948.80  ***** 19428.110
SRD:      -206.000    661.722  *****  .483    *****  1.405  *****
  
```

```

===110 WSEL NOT FOUND AT SECID "EXIT ": REDUCED DELTAY.
      WSLIM1, WSLIM2, DELTAY:  666.81  677.40  .03
  
```

```

===115 WSEL NOT FOUND AT SECID "EXIT ": USED WSMIN = CRWS.
      WSLIM1, WSLIM2, CRWS:  666.81  677.40  666.81
  
```

```

===130 CRITICAL WATER-SURFACE ELEVATION A _ S _ S _ U _ M _ E _ D  !!!!!
      ENERGY EQUATION N_O_T  B_A_L_A_N_C_E_D  AT SECID "EXIT ".
      WSBEQ, WSEND, CRWS:  666.81  677.40  666.81
  
```

WSPRO OUTPUT

```

Section: EXIT      666.812  4.744  6450.000   369.515  206.000 18284.030
Header Type: XS   671.556  *****   17.455  60517.55  206.000 18323.060
SRD:      .000    666.812  *****   1.001    .0017   1.001   *****
  
```

```

Section: FULLV    668.562  3.380  6450.000   437.816   45.000 18284.020
Header Type: FV   671.942  .395   14.732  78396.63   45.000 18323.070
SRD:      45.000  666.812  .000     .776     .0088   1.001   -.008
  
```

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

===135 CONVEYANCE RATIO OUTSIDE OF RECOMMENDED LIMITS AT SECID "APPR ".
 KRATIO: 8.68

```

Section: APPR     672.130  .030  6450.000  4970.763  280.000 17971.140
Header Type: AS   672.160  .218   1.298  680215.60  280.000 18795.000
SRD:      325.000  667.261  .000   .100   .0008   1.159   .000
  
```

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

<<< The Following Data Reflect The "Constricted" Profile >>>
 <<< Beginning Bridge/Culvert Hydraulic Computations >>>

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: BRDGE	670.855	1.752	6450.000	687.181	45.000	18212.710
Header Type: BR	672.607	.338	9.386	106793.10	45.000	18302.530
SRD: 45.000	668.512	.722	.677	*****	1.279	.018

```

Specific Bridge Information      C      P/A      PFELEV      BLEN      XLAB      XRAB
Bridge Type 3      Flow Type 1
Pier/Pile Code 0      .8843  .054  675.500  *****  *****  *****
  
```

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: APPR	673.294	.021	6450.000	5937.574	140.000	17958.100
Header Type: AS	673.315	.103	1.086	898493.10	203.396	18795.000
SRD: 325.000	667.261	.597	.077	.0008	1.135	.001

```

Approach Section APPR      Flow Contraction Information
M( G )      M( K )      KQ      XLKQ      XRKQ      OTEL
-----
.903      .898      91636.7  *****  *****  673.294
  
```

<<< End of Bridge Hydraulic Computations >>>

***** W S P R O *****

WSPRO OUTPUT

Federal Highway Administration - U. S. Geological Survey
 Model for Water-Surface Profile Computations.
 Input Units: English / Output Units: English

-----*

I-465 OVER STATE DITCH	I-465-158-4458
COUNTY: MARION	QUAD: MAYWOOD, IND 124A
8-12-97	DAVID C VOELKER

*** Live-Bed Contraction Scour Calculations for Header Record BRDGE ***

Constants and Input Variables

-----*

Bed Material Transport Mode Factor (k1):	.64
Total Pier Width Value (Pw):	4.000

-----*

#	Scour Depth	-- Flow --		-- Width --		--- X-Limits ---	
		Contract	Approach	Contract	Approach	Side	Contract Approach
1	15.298	4300.000	886.114	108.000	53.000	Left:	*****
	Approach Channel Depth:		10.682	Right:	*****
2	24.408	6450.000	1068.489	108.000	53.000	Left:	*****
	Approach Channel Depth:		12.655	Right:	*****

***** W S P R O *****

Federal Highway Administration - U. S. Geological Survey
 Model for Water-Surface Profile Computations.
 Input Units: English / Output Units: English

-----*

I-465 OVER STATE DITCH	I-465-158-4458
COUNTY: MARION	QUAD: MAYWOOD, IND 124A
8-12-97	DAVID C VOELKER

*** Pier Scour Calculations for Header Record BRDGE ***

Constants and Input Variables

Pier Width: 2.000

-----*

Pier Shape Factor (K1):	1.00
Flow Angle of Attack Factor (K2):	2.49
Bed Condition Factor (K3):	1.00
Bed Material Factor (K4):	1.00
Velocity Multiplier (VM):	1.00
Depth Multiplier (YM):	1.00

-----*

#	Scour Depth	---- Localized Hydraulic Properties ----					-- X-Stations --	
		Flow	WSE	Depth	Velocity	Froude #	Left	Right
1	13.59	4300.000	668.550	8.350	10.547	.643	18202.000	18314.000
2	14.20	6450.000	671.452	11.252	10.647	.559	18202.000	18314.000

WSPRO OUTPUT

```

***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English

```

```

-----*
I-465 OVER STATE DITCH                I-465-158-4458
COUNTY: MARION                        QUAD: MAYWOOD, IND 124A
8-12-97                                DAVID C VOELKER

```

*** Pier Scour Calculations for Header Record BRDGE ***

Constants and Input Variables

Pier Width: 2.000

```

-----*
Pier Shape Factor          (K1):  1.00
Flow Angle of Attack Factor (K2):  2.49
Bed Condition Factor       (K3):  1.00
Bed Material Factor        (K4):  1.00
Velocity Multiplier        (VM):  1.00
Depth Multiplier           (YM):  1.00
-----*

```

#	Scour Depth	----- Localized Hydraulic Properties -----					-- X-Stations --	
		Flow	WSE	Depth	Velocity	Froude #	Left	Right
1	13.59	4300.000	668.550	8.350	10.547	.643	18202.000	18314.000
2	14.20	6450.000	671.452	11.252	10.647	.559	18202.000	18314.000

ER

```

***** Normal end of WSPRO execution. *****
***** Elapsed Time: 0 Minutes 13 Seconds *****

```