

Modified Level II Streambed-Scour Analysis for Structure I-164-7-6973 Crossing Bluegrass Creek in Vanderburgh County, Indiana

By JOHN T. WILSON, BRET A. ROBINSON,
DAVID C. VOELKER, and ROBERT L. MILLER

Prepared in cooperation with the
INDIANA DEPARTMENT OF TRANSPORTATION

U.S. GEOLOGICAL SURVEY
Open-File Report 97-331



Indianapolis, Indiana

1997

**U.S. DEPARTMENT OF THE INTERIOR
BRUCE BABBITT, Secretary**

**U.S. GEOLOGICAL SURVEY
Gordon P. Eaton, Director**

**For additional information, write to:
District Chief
U.S. Geological Survey
Water Resources Division
5957 Lakeside Boulevard
Indianapolis, IN 46278-1996**

**Copies of this report can be purchased from:
U.S. Geological Survey
Branch of Information Services
Box 25286
Federal Center
Denver, CO 80225**

CONTENTS

Abstract	1
Introduction	1
Background and Scope	1
Site Description	2
Evaluation Methods	3
Special Considerations	6
Results	6
References	6
Appendix	7
Water Surface PROfile Model (WSPRO) Input File	8
Water Surface PROfile Model (WSPRO) Output	9

Tables

1. Cumulative scour depths for the modeled discharges at structure I-164-7-6973 crossing Bluegrass Creek in Vanderburgh County, Indiana	5
---	---

CONVERSION FACTORS AND ABBREVIATIONS

Multiply	By	To obtain
inch (in.)	25.4	millimeter
foot (ft)	0.3048	meter
square foot (ft ²)	929.0	square centimeter
feet per second (ft/s)	0.3048	meters per second
cubic foot per second (ft ³ /s)	0.02832	cubic meter per second
mile (mi)	1.609	kilometer
square mile (mi ²)	2.590	square kilometer

Abbreviations used in this report:

D ₅₀	median diameter of bed material
Q100	100-year discharge
FEMA	Federal Emergency Management Agency
HEC	Hydraulic Engineering Circular
IDNR	Indiana Department of Natural Resources
INDOT	Indiana Department of Transportation
USGS	U. S. Geological Survey
WSPRO	Water Surface PROFILE model

Modified Level II Streambed-Scour Analysis for Structure I-164-7-6973 Crossing Bluegrass Creek in Vanderburgh County, Indiana

By John T. Wilson, Bret A. Robinson, David C. Voelker, *and* Robert L. Miller

ABSTRACT

Level II scour evaluations follow a process in which hydrologic, hydraulic, and sediment-transport data are evaluated to calculate the depth of scour that may result when a given discharge is routed through a bridge opening. The results of the modified Level II analysis for structure I-164-7-6973 on Interstate 164 crossing Bluegrass Creek in Vanderburgh County, Indiana, are presented. The site is near the town of Daylight in the northeastern part of Vanderburgh County. Scour depths were computed with the Water Surface PROfile model, version V050196, which incorporates the scour-calculation procedures outlined in Hydraulic Engineering Circular No. 18. Total scour depths at the piers were approximately 13.1 feet for the modeled discharge of 4,440 cubic feet per second and approximately 18.7 feet for the modeled discharge of 6,220 cubic feet per second.

INTRODUCTION

The U.S. Geological Survey (USGS), in cooperation with the Indiana Department of Transportation (INDOT), is conducting Level II scour analyses at a number of bridges throughout Indiana. This report describes the methods applied and the modeling results for bridge I-164-7-6973.

Background and Scope

Level I scour assessment is a process where a large number of bridges are studied as a group. Assessments usually are made by evaluating a combination of geomorphic, hydrologic, and bridge-characteristic data. The results help investigators determine which bridges appear to be most likely to experience streambed-scour problems and which bridges appear to be relatively immune to problems brought on by streambed scour (for example, bridges built on bedrock).

When applied correctly, Level I scour assessments provide an investigator with information to identify those bridges that appear to be relatively safe and those bridges that fall into higher risk categories.

Level II scour evaluations describe the process for an investigator to apply a model to a bridge site and calculate the potential depth of scour that may result from a given flood event. Level II analyses involve the application of basic hydrologic, hydraulic, and sediment-transport engineering concepts and may include an evaluation of flood history, channel hydraulic conditions (for example, water-surface profile analysis), and basic sediment-transport analyses such as scour calculations (Lagasse and others, 1995).

The methods and model outlined in Hydraulic Engineering Circular (HEC) No. 18 (Richardson and Davis, 1995) formulate the basis for Level II scour evaluations. Methods used in this study for Level II scour evaluations are a modification of the HEC-18 standards. These modifications were made to comply with the methodology requested by INDOT (Merril Dougherty, Indiana Department of Transportation, oral commun., 1996). Descriptions of the specific modifications are given in the "Evaluation Methods" section of this report.

This report presents the methods followed for modeling, special considerations for this study site, and the input for and the output from the Water Surface PROfile (WSPRO) model.

Site Description

The study site is located near the town of Daylight in the northeastern part of Vanderburgh County. The drainage area for the site is approximately 35.3 mi² (Merril Dougherty, Indiana Department of Transportation, written commun., 1997). The predominant land use in the basin is agricultural; in the immediate vicinity of the bridge, the land is predominantly pasture land.

Within the immediate vicinity of the bridge, Bluegrass Creek has a channel-bed slope of approximately 0.00075 ft/ft. The channel-bed material is silt-clay, and the channel banks consist of silt-clay. At the time of the Level I site visit on March 4, 1992, the banks were observed to have 5 to 25 percent woody vegetative cover; the field report noted that the banks were experiencing fluvial erosion.

The Interstate 164 crossing of Bluegrass Creek is a 163-ft-long, multi-lane bridge consisting of three spans supported by concrete and steel piers and sloping riprap-covered spill-through abutments. Additional details describing conditions at the site are included in the Level I data base (Hopkins and Robinson, unpub. data, 1997). Photographs of the site, taken at the time of the Level I site visit, are archived at the USGS office in Indianapolis.

EVALUATION METHODS

The methods described in this section apply to a number of bridge sites in Indiana being evaluated for scour and outline the procedures requested by INDOT for these modified Level II scour analyses. The principal modification requested by INDOT was that the input data to the model come from or be estimated from existing data sources; no additional field data were collected. Actual methods used in the scour evaluation at this particular bridge site use the most applicable method possible, given the data available.

To determine drainage area, either published values found in Hoggatt (1975) or USGS 7.5-minute topographic maps with Hoggatt's original drainage-area delineations were used. Where there are no published data, drainage-area segments measured from the maps produced by Hoggatt were either subtracted from downstream sites or added to upstream sites published by Hoggatt (1975).

In Indiana, flood discharges are coordinated by agreement among State and Federal agencies. At sites where flood discharges officially are coordinated among State and Federal agencies in Indiana, the coordinated 100-year discharge (Q100) was modeled. INDOT also provided an additional flood discharge for these coordinated sites in excess of the Q100 to be modeled.

If a flood discharge was not coordinated, the USGS examined Federal Emergency Management Agency (FEMA) studies for Q100 determinations. Where FEMA studies did not produce a Q100, the USGS contacted IDNR for an estimated Q100 in the vicinity of the site being studied. If IDNR did not have a Q100, data from nearby USGS streamflow-gaging stations were analyzed with nearby and similar drainage basins that have been coordinated. At sites having no coordinated discharge data, the two discharges used in the model were 1) the approximated Q100 and 2) a discharge equal to 1.7 times the approximated Q100.

Most of the cross-section and bridge-opening geometry data were taken from the bridge plans (Indiana State Highway Commission, 1985) provided by INDOT. Bridge plans are presumed to be representative of current conditions at the site. To determine the cross-section geometry, a line was drawn on the bridge plans parallel to the bridge stationing and approximately one bridge width from the bridge. For sites where the bridge plans did not extend far enough laterally for collection of all cross-section data required for WSPRO model analysis, additional data were collected from 7.5-minute topographic maps.

The roadway and embankment profile was taken from the bridge and highway plans for those sites where roadway overtopping was expected. The INDOT bridge plans and 7.5-minute topographic maps were used as a guide, based on the water-surface elevations calculated by the WSPRO model, to determine if roadway overtopping might occur.

Roughness values (*n*-values) for the main channel were estimated by viewing photographs archived from the Level I scour assessments. The *n*-values for the overbanks were assigned on the basis of the surface-cover data summarized in the Level I data base (Hopkins and Robinson, unpub. data, 1997). From those data, the following roughness values were assigned to the surface-cover categories: urban—0.050, suburban—0.035, row crop—0.045, pasture—0.035, brush—0.120, forest—0.100, and wetland (any area covered by standing water)—0.100. The *n*-values for the overbanks were adjusted if the Level I photographs provided sufficient detail to warrant an adjustment.

WSPRO version V050196 was used to model flow through the study site. Starting water-surface elevation was obtained with a slope-conveyance computation. The channel-bed slope in the immediate vicinity of the bridge was estimated from the 7.5-minute topographic map and was used as the slope of the energy grade line for this computation.

WSPRO version V050196 includes a field that allows the input of up to four scour-adjustment factors (K1 to K4). For this modeling, the default value for K4 (bed armoring) was chosen. For scour-adjustment factors K1 and K2 (pier-nose shape and angle of attack, respectively), input values were determined by evaluating the data archived in the Level I data base (Hopkins and Robinson, unpub. data, 1997). For the K3 factor (bed forms), a value of 1.1 was applied in all cases.

In some cases, piers set on the overbanks are constructed with footings that are higher in elevation than pier footings in the main channel. In these situations, if the channel position changes, the piers that were initially constructed on the overbank may become part of the main channel. Therefore, to evaluate total potential scour, the model results obtained for contraction scour and deepest local scour in the main channel were added and applied to all piers in the bridge opening. This methodology allowed for an evaluation of potential undermining of pier supports in the event that future channel movement placed overbank piers in the main channel.

Where bridge pairs have a continuous abutment or fill between the bridges that does not allow expansion of flow, the bridge pair was modeled as one bridge. Sites with discontinuous abutments, allowing expansion between the bridges, were modeled as two separate bridges. In those cases, a valley cross section was measured between the bridges and used as the approach section for the downstream bridge and as the exit section for the upstream bridge.

At sites with no embankment to function as a weir or at sites where the tailwater drowns out the embankment, a composite bridge and road section was used to compute flow. Those sites were computed with friction-loss equations rather than with a bridge routine.

Total scour is taken as the sum of local scour plus contraction scour. If the model predicted negative contraction scour (aggradation), the contraction-scour value was assumed to be zero in determining the total scour depth (table 1). This assumption was made so that a negative contraction scour would not mask the potentially detrimental effects of local scour at a pier. No abutment scour evaluations were made in this study.

Table 1. Cumulative scour depths for the modeled discharges at structure I-164-7-6973 crossing Bluegrass Creek in Vanderburgh County, Indiana

Pier number ¹	Stationing from bridge plans ²	Initial bed-elevation at pier (feet)	Main-channel contraction scour depth (feet)	Local scour depth (feet)	Worst-case total-scour depth ³ (feet)	Bottom elevation of pier (feet)	Worst-case bed elevation after scour ⁴ (feet)
Modeled discharge⁵ is 4,440 cubic feet per second							
1	105+73	366	3.3	9.8	13.1	358.3	350.1
2	106+26	365	3.3	9.8	13.1	358.3	350.1
Modeled discharge is 6,220 cubic feet per second							
1	105+73	366	7.8	10.9	18.7	358.3	344.5
2	106+26	365	7.8	10.9	18.7	358.3	344.5

¹Pier numbers were assigned from left to right as shown on the bridge plans.

²Stationing is the center line of the pier as determined from the bridge plans. Stationing from bridge plan, 105+73, represents a point 10,573 feet from an arbitrary starting location referenced on the bridge plans.

³Worst-case total-scour depths are generated by summing the calculated contraction-scour depth with the worst case of local scour.

⁴Worst-case bed elevation is computed by subtracting the worst-case total-scour depth from the lowest initial bed elevation in the bridge opening (363.2 feet).

⁵Coordinated discharge.

SPECIAL CONSIDERATIONS

Model runs indicate the water-surface elevation at the bridge is lower than the low-steel elevation for the modeled discharges. Therefore, there should be no pressure flow through the bridge opening for the discharges modeled.

RESULTS

Scour depths were computed with a version of WSPRO (Larry Arneson, Federal Highway Administration, written commun., 1996) modified from Shearman (1990). This version of WSPRO includes scour calculations in the model output. Scour depths were calculated assuming an infinite depth of material that could erode and a homogeneous particle-size distribution. The results of the scour analysis are presented in table 1; a complete input file and output results are presented in the appendix.

REFERENCES

- Hoggatt, R.E., 1975, Drainage areas of Indiana streams: U.S. Geological Survey, Water Resources Division, 231 p.
- Indiana State Highway Commission, 1985, Bridge plans Interstate Route 164: Bridge File I-164-7-6973.
- Lagasse, P.F.; Schall, J.D.; Johnson, F.; Richardson, E.V.; and Chang, F., 1995, Stream stability at highway structures (2d ed.): Federal Highway Administration, Hydraulic Engineering Circular No. 20, Publication FHWA-IP-90-014, 144 p.
- Richardson, E.V., and Davis, S.R., 1995, Evaluating scour at bridges (3d ed.): Federal Highway Administration, Hydraulic Engineering Circular No. 18, Publication FHWA-IP-90-017, 204 p.
- Shearman, J.O., 1990, User's manual for WSPRO, a computer model for water-surface profile computations: Federal Highway Administration Publication FHWA-IP-89-027, 177 p.

APPENDIX

WSPRO INPUT FILE

T1 I-164 OVER BLUEGRASS CREEK I164-7-6973
T2 COUNTY: VANDERBURGH QUAD: DAYLIGHT 194C
T3 7-31-97 JOHN T. WILSON
Q 4440 6220
SK .00075 .00075
XS EXIT 0 23
GR 9750 380 10241 376 10537 374 10548 364 10576 364
GR 10590 374 10611 376 10621 378 10647 378 10656 376
GR 10774 376 10799 378 10848 380
N .045 .036 .045
SA 10537 10590
XS FULLV 155 23
BR BRDGE 155 389.0 23
* ***BRIDGE DECK SKEWED 23 DEGREES (FROM BRIDGE PLANS)***
GR 10522 389.0 10522 388.2 10525 388.2 10576 364.9 10582 364.8
GR 10584 363.9 10584 363.5 10587 363.2 10594 364.0 10610 364.9
GR 10624 364.9 10674 388.3 10677 388.2 10677 389.0 10522 389.0
N .036
PD 365.3 2.0 1
PD 365.8 2.0 2
PD 365.8 4.0 3
CD 3 140 3 391
DC 0 BRDGE 10548 10651 10612 10661 * 4
* ***DC LIMITS AT BRIDGE ARE LEW AND REW FOR Q1***
* BXL BXR PW * * K1 K2 K3
DP 10522 10677 2.0 * * 1 2.0 1.1
DP 10522 10677 2.0 * * 1 2.0 1.1
* ***DP CARDS USE WHOLE BRIDGE OPENING FOR BXL/BXR***
* ***NO PIERS IN MAIN CHANNEL***
XS APPR 450
* ***SECTION IS ADJUSTED FOR 23 DEGREES OF SKEW***
GR 9793 380 10189 376 10397 374 10413 372 10469 372 10554 374
GR 10612 374 10623 364 10648 364 10661 374 10706 376 10715 378
GR 10744 378 10761 378 10811 380 10811 385
N .045 .036 .045
SA 10612 10661
HP 2 BRDGE 377.7 * 377.7 4440
HP 2 BRDGE 378.8 * 378.8 6220
EX
ER

WSPRO OUTPUT

```
***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Run Date & Time: 8/ 4/97 2:13 pm Version V050196
Input File: bluegras.dat Output File: bluegras.LST
```

```
-----*
T1      I-164 OVER BLUEGRASS CREEK      I164-7-6973
T2      COUNTY: VANDERBURGH              QUAD: DAYLIGHT 194C
T3      7-31-97                          JOHN T. WILSON
Q       4440      6220
```

*** Processing Flow Data; Placing Information into Sequence 1 ***

SK .00075 .00075

```
***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English
```

```
-----*
I-164 OVER BLUEGRASS CREEK      I164-7-6973
COUNTY: VANDERBURGH              QUAD: DAYLIGHT 194C
7-31-97                          JOHN T. WILSON
```

```
-----*
* Starting To Process Header Record EXIT *
-----*
```

```
XS EXIT 0 23
GR      9750 380 10241 376 10537 374 10548 364 10576 364
GR      10590 374 10611 376 10621 378 10647 378 10656 376
GR      10774 376 10799 378 10848 380
N       .045      .036      .045
SA      10537      10590
```

*** Completed Reading Data Associated With Header Record EXIT ***
 *** Storing X-Section Data In Temporary File As Record Number 1 ***

```
*** Data Summary For Header Record EXIT ***
SRD Location:      0. Cross-Section Skew: 23.0 Error Code 0
Valley Slope:     .00000 Averaging Conveyance By Geometric Mean.
Energy Loss Coefficients -> Expansion: .50 Contraction: .00
```

X,Y-coordinates (13 pairs)					
X	Y	X	Y	X	Y
9750.000	380.000	10241.000	376.000	10537.000	374.000
10548.000	364.000	10576.000	364.000	10590.000	374.000
10611.000	376.000	10621.000	378.000	10647.000	378.000
10656.000	376.000	10774.000	376.000	10799.000	378.000
10848.000	380.000				

WSPRO OUTPUT

Minimum and Maximum X,Y-coordinates

```

Minimum X-Station:   9750.000 ( associated Y-Elevation:  380.000 )
Maximum X-Station:  10848.000 ( associated Y-Elevation:  380.000 )
Minimum Y-Elevation:  364.000 ( associated X-Station:  10576.000 )
Maximum Y-Elevation:  380.000 ( associated X-Station:   9750.000 )
    
```

X-coordinates & Horizontal Breakpoints Translated by Skew Angle

X Input	X Skewed	X Input	X Skewed	X Input	X Skewed
9750.000	9815.663	10241.000	10267.630	10537.000	10540.100
10548.000	10550.230	10576.000	10576.000	10590.000	10588.890
10611.000	10608.220	10621.000	10617.420	10647.000	10641.360
10656.000	10649.640	10774.000	10758.260	10799.000	10781.270
10848.000	10826.380				

Roughness Data (3 SubAreas)

SubArea	Roughness Coefficient	Horizontal Breakpoint
1	.045	---
	---	*****
2	.036	---
	---	*****
3	.045	---

```

*-----*
* Finished Processing Header Record EXIT *
*-----*
    
```

```

***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English
    
```

```

*-----*
I-164 OVER BLUEGRASS CREEK I164-7-6973
COUNTY: VANDERBURGH QUAD: DAYLIGHT 194C
7-31-97 JOHN T. WILSON
    
```

```

*-----*
* Starting To Process Header Record FULLV *
*-----*
    
```

XS FULLV 155 23

```

*** Completed Reading Data Associated With Header Record FULLV ***
*** No Roughness Data Input, Propagating From Previous Section ***
*** Storing X-Section Data In Temporary File As Record Number 2 ***
    
```

```

*** Data Summary For Header Record FULLV ***
SRD Location: 155. Cross-Section Skew: 23.0 Error Code 0
Valley Slope: .00000 Averaging Conveyance By Geometric Mean.
Energy Loss Coefficients -> Expansion: .50 Contraction: .00
    
```

WSPRO OUTPUT

X,Y-coordinates (13 pairs)					
X	Y	X	Y	X	Y
9750.000	380.000	10241.000	376.000	10537.000	374.000
10548.000	364.000	10576.000	364.000	10590.000	374.000
10611.000	376.000	10621.000	378.000	10647.000	378.000
10656.000	376.000	10774.000	376.000	10799.000	378.000
10848.000	380.000				

Minimum and Maximum X,Y-coordinates

```

Minimum X-Station:  9750.000  ( associated Y-Elevation:  380.000 )
Maximum X-Station: 10848.000  ( associated Y-Elevation:  380.000 )
Minimum Y-Elevation: 364.000  ( associated X-Station: 10576.000 )
Maximum Y-Elevation: 380.000  ( associated X-Station:  9750.000 )
    
```

X-coordinates & Horizontal Breakpoints Translated by Skew Angle

X Input	X Skewed	X Input	X Skewed	X Input	X Skewed
9750.000	9815.663	10241.000	10267.630	10537.000	10540.100
10548.000	10550.230	10576.000	10576.000	10590.000	10588.890
10611.000	10608.220	10621.000	10617.420	10647.000	10641.360
10656.000	10649.640	10774.000	10758.260	10799.000	10781.270
10848.000	10826.380				

Roughness Data (3 SubAreas)

SubArea	Roughness Coefficient	Horizontal Breakpoint
1	.045	---
	---	*****
2	.036	---
	---	*****
3	.045	---

```

*-----*
*           Finished Processing Header Record FULLV           *
*-----*
    
```

***** W S P R O *****

Federal Highway Administration - U. S. Geological Survey
 Model for Water-Surface Profile Computations.
 Input Units: English / Output Units: English

```

*-----*
I-164 OVER BLUEGRASS CREEK   I164-7-6973
COUNTY: VANDERBURGH         QUAD: DAYLIGHT 194C
7-31-97                      JOHN T. WILSON
    
```

```

*-----*
*           Starting To Process Header Record BRDGE           *
*-----*
    
```

WSPRO OUTPUT

```

BR   BRDGE  155   389.0   23
GR      10522 389.0  10522 388.2  10525 388.2  10576 364.9  10582 364.8
GR      10584 363.9  10584 363.5  10587 363.2  10594 364.0  10610 364.9
GR      10624 364.9  10674 388.3  10677 388.2  10677 389.0  10522 389.0
N      .036
PD      365.3   2.0   1
PD      365.8   2.0   2
PD      365.8   4.0   3
CD      3     140    3     391
    
```

```

***   Completed Reading Data Associated With Header Record BRDGE   ***
+++072 NOTICE: X-coordinate # 2 increased to eliminate vertical segment.
+++072 NOTICE: X-coordinate # 7 increased to eliminate vertical segment.
+++072 NOTICE: X-coordinate #14 increased to eliminate vertical segment.
***   Storing Bridge Data In Temporary File As Record Number 3   ***
    
```

```

***           Data Summary For Bridge Record BRDGE           ***
SRD Location:      155.   Cross-Section Skew:  23.0   Error Code  0
Valley Slope:  *****   Averaging Conveyance By Geometric Mean.
Energy Loss Coefficients ->   Expansion:   .50   Contraction:   .00
    
```

X,Y-coordinates (15 pairs)					
X	Y	X	Y	X	Y
-----	-----	-----	-----	-----	-----
10522.000	389.000	10522.100	388.200	10525.000	388.200
10576.000	364.900	10582.000	364.800	10584.000	363.900
10584.100	363.500	10587.000	363.200	10594.000	364.000
10610.000	364.900	10624.000	364.900	10674.000	388.300
10677.000	388.200	10677.100	389.000	10522.000	389.000
-----	-----	-----	-----	-----	-----

```

           Minimum and Maximum X,Y-coordinates
Minimum X-Station:  10522.000  ( associated Y-Elevation:  389.000 )
Maximum X-Station:  10677.100  ( associated Y-Elevation:  389.000 )
Minimum Y-Elevation:  363.200  ( associated X-Station:  10587.000 )
Maximum Y-Elevation:  389.000  ( associated X-Station:  10522.000 )
    
```

X-coordinates & Horizontal Breakpoints Translated by Skew Angle					
X Input	X Skewed	X Input	X Skewed	X Input	X Skewed
-----	-----	-----	-----	-----	-----
10522.000	10527.170	10522.100	10527.260	10525.000	10529.930
10576.000	10576.870	10582.000	10582.400	10584.000	10584.240
10584.100	10584.330	10587.000	10587.000	10594.000	10593.440
10610.000	10608.170	10624.000	10621.060	10674.000	10667.080
10677.000	10669.850	10677.100	10669.940	10522.000	10527.170
-----	-----	-----	-----	-----	-----

```

           Roughness Data ( 1 SubAreas )
           Roughness Horizontal
SubArea Coefficient Breakpoint
-----
           1           .036           ---
-----
    
```

WSPRO OUTPUT

Discharge coefficient parameters

BRType	BRWdth	EMBSS	EMBElv	UserCD
3	140.000	3.00	391.000	*****

Pressure flow elevations

AVBCEL	PFElev
*****	389.000

Abutment Parameters

ABSLPL	ABSLPR	XTOELT	YTOELT	XTOERT	YTOERT
*****	*****	*****	*****	*****	*****

Pier/Pile Data (3 Group(s))

Code Indicates Bridge Uses Piers

Group	Elevation	Gross Width	Number
1	365.300	2.000	1
2	365.800	2.000	2
3	365.800	4.000	3

* Finished Processing Header Record BRDGE *

***** W S P R O *****

Federal Highway Administration - U. S. Geological Survey

Model for Water-Surface Profile Computations.

Input Units: English / Output Units: English

I-164 OVER BLUEGRASS CREEK I164-7-6973

COUNTY: VANDERBURGH QUAD: DAYLIGHT 194C

7-31-97 JOHN T. WILSON

DC 0 BRDGE	10548	10651	10612	10661	* 4
DP	10522	10677	2.0	* * 1	2.0 1.1
DP	10522	10677	2.0	* * 1	2.0 1.1

* Starting To Process Header Record APPR *

XS	APPR	450							
GR		9793 380	10189 376	10397 374	10413 372	10469 372	10554 374		
GR		10612 374	10623 364	10648 364	10661 374	10706 376	10715 378		
GR		10744 378	10761 378	10811 380	10811 385				
N		.045	.036	.045					
SA		10612	10661						

*** Completed Reading Data Associated With Header Record APPR ***

+++072 NOTICE: X-coordinate #16 increased to eliminate vertical segment.

*** Storing X-Section Data In Temporary File As Record Number 4 ***

WSPRO OUTPUT

```

***          Data Summary For Header Record APPR          ***
SRD Location:      450.   Cross-Section Skew:   .0   Error Code   0
Valley Slope:     .00000   Averaging Conveyance By Geometric Mean.
Energy Loss Coefficients ->   Expansion:   .50   Contraction:   .00
    
```

X,Y-coordinates (16 pairs)					
X	Y	X	Y	X	Y
-----	-----	-----	-----	-----	-----
9793.000	380.000	10189.000	376.000	10397.000	374.000
10413.000	372.000	10469.000	372.000	10554.000	374.000
10612.000	374.000	10623.000	364.000	10648.000	364.000
10661.000	374.000	10706.000	376.000	10715.000	378.000
10744.000	378.000	10761.000	378.000	10811.000	380.000
10811.100	385.000				
-----	-----	-----	-----	-----	-----

```

Minimum and Maximum X,Y-coordinates
Minimum X-Station:  9793.000 ( associated Y-Elevation:  380.000 )
Maximum X-Station: 10811.100 ( associated Y-Elevation:  385.000 )
Minimum Y-Elevation: 364.000 ( associated X-Station: 10648.000 )
Maximum Y-Elevation: 385.000 ( associated X-Station: 10811.100 )
    
```

```

Roughness Data ( 3 SubAreas )
      Roughness   Horizontal
SubArea Coefficient Breakpoint
-----
      1          .045         ---
      ---          *****
      2          .036         ---
      ---          *****
      3          .045         ---
-----
    
```

```

Bridge datum projection(s): XREFLT XREFRT FDSTLT FDSTRT
*****
    
```

```

*-----*
* Finished Processing Header Record APPR *
*-----*
    
```

```

***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English
    
```

```

*-----*
I-164 OVER BLUEGRASS CREEK I164-7-6973
COUNTY: VANDERBURGH QUAD: DAYLIGHT 194C
7-31-97 JOHN T. WILSON
    
```

```

HP 2 BRDGE 377.7 * 377.7 4440
HP 2 BRDGE 378.8 * 378.8 6220
EX
    
```

WSPRO OUTPUT

```

*====*
*      Summary of Boundary Condition Information      *
*====*
  
```

#	Reach Discharge	Water Surface Elevation	Friction Slope	Flow Regime
1	4440.00	*****	.0007	Sub-Critical
2	6220.00	*****	.0007	Sub-Critical

```

*====*
*      Beginning 2 Profile Calculation(s)      *
*====*
  
```

```

***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English
  
```

```

-----*
I-164 OVER BLUEGRASS CREEK I164-7-6973
COUNTY: VANDERBURGH QUAD: DAYLIGHT 194C
7-31-97 JOHN T. WILSON
  
```

	WSEL EGEL CRWS	VHD HF HO	Q V FR #	AREA K SF	SRDL FLEN ALPHA	LEW REW ERR
Section: EXIT	377.431	.292	4440.000	1674.664	*****	10065.320
Header Type: XS	377.724	*****	2.651	161998.20	*****	10791.890
SRD: .000	372.111	*****	.493	*****	2.675	*****
Section: FULLV	377.576	.261	4440.000	1776.783	155.000	10047.560
Header Type: FV	377.837	.110	2.499	170718.80	155.000	10793.700
SRD: 155.000	372.111	.000	.458	.0007	2.686	.003

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

===135 CONVEYANCE RATIO OUTSIDE OF RECOMMENDED LIMITS AT SECID "APPR ".
KRATIO: 1.41

Section: APPR	377.877	.100	4440.000	2516.636	295.000	10003.190
Header Type: AS	377.977	.142	1.764	240046.20	295.000	10714.450
SRD: 450.000	373.705	.000	.238	.0005	2.069	-.002

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

<<< The Following Data Reflect The "Constricted" Profile >>>
<<< Beginning Bridge/Culvert Hydraulic Computations >>>

WSPRO OUTPUT

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: BRDGE	377.534	.349	4440.000	973.954	155.000	10548.350
Header Type: BR	377.883	.159	4.559	173325.40	155.000	10651.000
SRD: 155.000	370.639	.000	.271	*****	1.080	-.008

Specific Bridge Information	C	P/A	PFELEV	BLEN	XLAB	XRAB
Bridge Type 3 Flow Type 1						
Pier/Pile Code 0	.9621	.049	389.000	*****	*****	*****

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: APPR	378.037	.093	4440.000	2633.659	155.000	9987.326
Header Type: AS	378.130	.118	1.686	249520.20	177.564	10761.930
SRD: 450.000	373.705	.130	.234	.0005	2.112	-.005

Approach Section APPR Flow Contraction Information						
M(G)	M(K)	KQ	XLKQ	XRKQ	OTEL	
.855	.525	118648.7	*****	*****	378.037	

<<< End of Bridge Hydraulics Computations >>>

***** W S P R O *****
 Federal Highway Administration - U. S. Geological Survey
 Model for Water-Surface Profile Computations.
 Input Units: English / Output Units: English

I-164 OVER BLUEGRASS CREEK I164-7-6973
 COUNTY: VANDERBURGH QUAD: DAYLIGHT 194C
 7-31-97 JOHN T. WILSON

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: EXIT	378.392	.276	6220.000	2419.685	*****	9947.397
Header Type: XS	378.668	*****	2.571	227079.00	*****	10808.600
SRD: .000	375.777	*****	.443	*****	2.686	*****
Section: FULLV	378.537	.247	6220.000	2546.638	155.000	9929.524
Header Type: FV	378.784	.110	2.442	239037.00	155.000	10812.170
SRD: 155.000	375.777	.000	.414	.0007	2.661	.006

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

WSPRO OUTPUT

```

Section: APPR      378.818   .112   6220.000   3276.399   295.000   9910.013
Header Type: AS   378.931   .150     1.898   318418.10   295.000  10781.450
SRD:    450.000   375.161   .000     .244     .0005     2.006    -.004
  
```

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

<<< The Following Data Reflect The "Constricted" Profile >>>

<<< Beginning Bridge/Culvert Hydraulic Computations >>>

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: BRDGE	378.595	.581	6220.000	1085.344	155.000	10546.020
Header Type: BR	379.176	.394	5.731	201412.20	155.000	10653.260
SRD: 155.000	372.022	.115	.339	*****	1.138	.002

Specific Bridge Information	C	P/A	PFELEV	BLEN	XLAB	XRAB
Bridge Type 3	Flow Type 1					
Pier/Pile Code 0	.9376	.048	389.000	*****	*****	*****

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: APPR	379.551	.074	6220.000	3948.323	155.000	9837.461
Header Type: AS	379.625	.265	1.575	394817.70	180.861	10799.770
SRD: 450.000	375.161	.184	.189	.0005	1.907	.006

Approach Section APPR Flow Contraction Information						
M(G)	M(K)	KQ	XLKQ	XRKQ	OTEL	
.877	.830	67105.7	*****	*****	379.551	

<<< End of Bridge Hydraulics Computations >>>

***** W S P R O *****

Federal Highway Administration - U. S. Geological Survey

Model for Water-Surface Profile Computations.

Input Units: English / Output Units: English

I-164 OVER BLUEGRASS CREEK	I164-7-6973
COUNTY: VANDERBURGH	QUAD: DAYLIGHT 194C
7-31-97	JOHN T. WILSON

WSPRO OUTPUT

*** Beginning Velocity Distribution For Header Record BRDGE ***
 SRD Location: 155.000 Header Record Number 3

Water Surface Elevation: 377.700 Element # 1
 Flow: 4440.000 Velocity: 4.48 Hydraulic Depth: 9.588
 Cross-Section Area: 991.07 Conveyance: 177571.10
 Bank Stations -> Left: 10547.980 Right: 10651.350

X STA.	10548.0	10567.6	10573.4	10577.4	10581.0	10584.5
A(I)	87.7	59.6	50.0	45.9	46.4	
V(I)	2.53	3.73	4.44	4.84	4.79	
D(I)	4.48	10.28	12.41	12.85	13.32	

X STA.	10584.5	10587.4	10590.2	10593.2	10596.1	10599.2
A(I)	42.1	40.8	40.9	40.4	41.1	
V(I)	5.27	5.44	5.43	5.50	5.40	
D(I)	14.38	14.29	13.96	13.67	13.50	

X STA.	10599.2	10602.2	10605.3	10608.6	10611.9	10615.3
A(I)	40.6	41.3	42.0	42.0	43.5	
V(I)	5.47	5.37	5.28	5.29	5.11	
D(I)	13.32	13.15	12.97	12.82	12.80	

X STA.	10615.3	10618.7	10622.3	10626.3	10632.0	10651.4
A(I)	44.0	46.3	50.4	58.6	87.8	
V(I)	5.05	4.80	4.41	3.79	2.53	
D(I)	12.80	12.80	12.48	10.38	4.53	

***** W S P R O *****
 Federal Highway Administration - U. S. Geological Survey
 Model for Water-Surface Profile Computations.
 Input Units: English / Output Units: English

 I-164 OVER BLUEGRASS CREEK I164-7-6973
 COUNTY: VANDERBURGH QUAD: DAYLIGHT 194C
 7-31-97 JOHN T. WILSON

*** Beginning Velocity Distribution For Header Record BRDGE ***
 SRD Location: 155.000 Header Record Number 3

Water Surface Elevation: 378.800 Element # 1
 Flow: 6220.000 Velocity: 5.62 Hydraulic Depth: 10.242
 Cross-Section Area: 1107.39 Conveyance: 207095.10
 Bank Stations -> Left: 10545.580 Right: 10653.700

X STA.	10545.6	10566.3	10572.5	10576.8	10580.5	10584.0
A(I)	98.3	67.1	56.7	51.3	50.1	
V(I)	3.16	4.63	5.49	6.06	6.21	
D(I)	4.74	10.89	13.24	13.94	14.24	

X STA.	10584.0	10587.1	10590.0	10593.0	10596.1	10599.2
A(I)	47.8	45.4	45.6	45.7	45.1	
V(I)	6.51	6.85	6.82	6.81	6.89	
D(I)	15.44	15.42	15.08	14.78	14.59	

WSPRO OUTPUT

X STA.	10599.2	10602.3	10605.6	10608.9	10612.3	10615.7
A(I)	45.1	46.0	46.8	46.8	47.4	
V(I)	6.89	6.76	6.64	6.64	6.56	
D(I)	14.42	14.24	14.06	13.91	13.90	

X STA.	10615.7	10619.3	10623.0	10627.2	10633.3	10653.7
A(I)	50.1	51.7	55.3	67.3	97.7	
V(I)	6.20	6.02	5.62	4.62	3.18	
D(I)	13.90	13.90	13.34	10.99	4.78	

***** W S P R O *****

Federal Highway Administration - U. S. Geological Survey
 Model for Water-Surface Profile Computations.
 Input Units: English / Output Units: English

I-164 OVER BLUEGRASS CREEK I164-7-6973
 COUNTY: VANDERBURGH QUAD: DAYLIGHT 194C
 7-31-97 JOHN T. WILSON

*** Live-Bed Contraction Scour Calculations for Header Record BRDGE ***

Constants and Input Variables

 Bed Material Transport Mode Factor (k1): .64
 Total Pier Width Value (Pw): 4.000

#	Scour Depth	-- Flow -- Contract	-- Width -- Approach	-- X-Limits -- Contract	-- Approach
1	3.310	4440.000	1947.640	99.000	49.000
	Approach	Channel Depth:	11.588
2	7.811	6220.000	2115.898	99.000	49.000
	Approach	Channel Depth:	13.102

***** W S P R O *****

Federal Highway Administration - U. S. Geological Survey
 Model for Water-Surface Profile Computations.
 Input Units: English / Output Units: English

I-164 OVER BLUEGRASS CREEK I164-7-6973
 COUNTY: VANDERBURGH QUAD: DAYLIGHT 194C
 7-31-97 JOHN T. WILSON

*** Pier Scour Calculations for Header Record BRDGE ***

WSPRO OUTPUT

Constants and Input Variables

Pier Width: 2.000

```
*-----*
Pier Shape Factor          (K1):  1.00
Flow Angle of Attack Factor (K2):  2.00
Bed Condition Factor       (K3):  1.10
Bed Material Factor        (K4):  1.00
Velocity Multiplier        (VM):  1.00
Depth Multiplier           (YM):  1.00
*-----*
```

```
Scour  ---- Localized Hydraulic Properties ----  -- X-Stations --
#  Depth  Flow      WSE      Depth Velocity Froude #  Left      Right
-----
1   9.79  4440.000  377.663  14.463   5.520    .256  10522.000  10677.000
2  10.91  6220.000  378.779  15.579   6.940    .310  10522.000  10677.000
-----
```

***** W S P R O *****

Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English

I-164 OVER BLUEGRASS CREEK I164-7-6973
COUNTY: VANDERBURGH QUAD: DAYLIGHT 194C
7-31-97 JOHN T. WILSON

*** Pier Scour Calculations for Header Record BRDGE ***

Constants and Input Variables

Pier Width: 2.000

```
*-----*
Pier Shape Factor          (K1):  1.00
Flow Angle of Attack Factor (K2):  2.00
Bed Condition Factor       (K3):  1.10
Bed Material Factor        (K4):  1.00
Velocity Multiplier        (VM):  1.00
Depth Multiplier           (YM):  1.00
*-----*
```

```
Scour  ---- Localized Hydraulic Properties ----  -- X-Stations --
#  Depth  Flow      WSE      Depth Velocity Froude #  Left      Right
-----
1   9.79  4440.000  377.663  14.463   5.520    .256  10522.000  10677.000
2  10.91  6220.000  378.779  15.579   6.940    .310  10522.000  10677.000
-----
```

ER

***** Normal end of WSPRO execution. *****
***** Elapsed Time: 0 Minutes 53 Seconds *****