

Modified Level II Streambed-Scour Analysis for Structure I-64-11-5202 Crossing Flat Run Ditch in Posey County, Indiana

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CONVERSION FACTORS AND ABBREVIATIONS

Multiply	By	To obtain
inch (in.)	25.4	millimeter
foot (ft)	0.3048	meter
square foot (ft ²)	929.0	square centimeter
feet per second (ft/s)	0.3048	meters per second
cubic foot per second (ft ³ /s)	0.02832	cubic meter per second
mile (mi)	1.609	kilometer
square mile (mi ²)	2.590	square kilometer

Abbreviations used in this report:

D ₅₀	median diameter of bed material
Q ₁₀₀	100-year discharge
FEMA	Federal Emergency Management Agency
HEC	Hydraulic Engineering Circular
IDNR	Indiana Department of Natural Resources
INDOT	Indiana Department of Transportation
USGS	U. S. Geological Survey
WSPRO	Water Surface PROfile model

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ABSTRACT

Level II scour evaluations follow a process in which hydrologic, hydraulic, and sediment-transport data are evaluated to calculate the depth of scour that may result when a given discharge is routed through a bridge opening. The results of the modified Level II analysis for structure I-64-11-5202 on Interstate 64 crossing Flat Run Ditch in Posey County, Indiana, are presented. The site is near the town of Poseyville in the northeastern part of Posey County. Scour depths were computed with the Water Surface PROfile model, version V050196, which incorporates the scour-calculation procedures outlined in Hydraulic Engineering Circular No. 18. Total scour depths at the piers were approximately 7.3 feet for the modeled discharge of 1,630 cubic feet per second and approximately 13.5 feet for the modeled discharge of 2,780 cubic feet per second.

INTRODUCTION

The U.S. Geological Survey (USGS), in cooperation with the Indiana Department of Transportation (INDOT), is conducting Level II scour analyses at a number of bridges throughout Indiana. This report describes the methods applied and the modeling results for bridge I-64-11-5202.

Background and Scope

Level I scour assessment is a process where a large number of bridges are studied as a group. Assessments usually are made by evaluating a combination of geomorphic, hydrologic, and bridge-characteristic data. The results help investigators determine which bridges appear to be most likely to experience streambed-scour problems and which bridges appear to be relatively immune to problems brought on by streambed scour (for example, bridges built on bedrock).

When applied correctly, Level I scour assessments provide an investigator with information to identify those bridges that appear to be relatively safe and those bridges that fall into higher risk categories.

Level II scour evaluations describe the process for an investigator to apply a model to a bridge site and calculate the potential depth of scour that may result from a given flood event. Level II analyses involve the application of basic hydrologic, hydraulic, and sediment-transport engineering concepts and may include an evaluation of flood history, channel hydraulic conditions (for example, water-surface profile analysis), and basic sediment-transport analyses such as scour calculations (Lagasse and others, 1995).

The methods and model outlined in Hydraulic Engineering Circular (HEC) No. 18 (Richardson and Davis, 1995) formulate the basis for Level II scour evaluations. Methods used in this study for Level II scour evaluations are a modification of the HEC-18 standards. These modifications were made to comply with the methodology requested by INDOT (Merril Dougherty, Indiana Department of Transportation, oral commun., 1996). Descriptions of the specific modifications are given in the "Evaluation Methods" section of this report.

This report presents the methods followed for modeling, special considerations for this study site, and the input for and the output from the Water Surface PROfile (WSPRO) model.

Site Description

The study site is located near the town of Poseyville in the northeastern part of Posey County. The drainage area for the site is approximately 2.59 mi² (Merril Dougherty, Indiana Department of Transportation, written commun., 1997). The predominant land use in the basin is agricultural; in the immediate vicinity of the bridge, the land is predominantly row crop with some pasture land nearby.

Within the immediate vicinity of the bridge, Flat Run Ditch has a channel-bed slope of approximately 0.0028 ft/ft. The channel-bed material is silt-clay, and the channel banks consist of silt-clay. At the time of the Level I site visit on June 17, 1992, the banks were observed to have no woody vegetative cover; the field report noted that the banks were stable.

The Interstate 64 crossing of Flat Run Ditch is a 70-ft-long, multi-lane bridge consisting of three spans supported by concrete and steel piers and sloping concrete spill-through abutments. Additional details describing conditions at the site are included in the Level I data base (Hopkins and Robinson, unpub. data, 1997). Photographs of the site, taken at the time of the Level I site visit, are archived at the USGS office in Indianapolis.

EVALUATION METHODS

The methods described in this section apply to a number of bridge sites in Indiana being evaluated for scour and outline the procedures requested by INDOT for these modified Level II scour analyses. The principal modification requested by INDOT was that the input data to the model come from or be estimated from existing data sources; no additional field data were collected. Actual methods used in the scour evaluation at this particular bridge site use the most applicable method possible, given the data available.

To determine drainage area, either published values found in Hoggatt (1975) or USGS 7.5-minute topographic maps with Hoggatt's original drainage-area delineations were used. Where there are no published data, drainage-area segments measured from the maps produced by Hoggatt were either subtracted from downstream sites or added to upstream sites published by Hoggatt (1975).

In Indiana, flood discharges are coordinated by agreement among State and Federal agencies. At sites where flood discharges officially are coordinated among State and Federal agencies in Indiana, the coordinated 100-year discharge (Q100) was modeled. INDOT also provided an additional flood discharge for these coordinated sites in excess of the Q100 to be modeled.

If a flood discharge was not coordinated, the USGS examined Federal Emergency Management Agency (FEMA) studies for Q100 determinations. Where FEMA studies did not produce a Q100, the USGS contacted IDNR for an estimated Q100 in the vicinity of the site being studied. If IDNR did not have a Q100, data from nearby USGS streamflow-gaging stations were analyzed with nearby and similar drainage basins that have been coordinated. At sites having no coordinated discharge data, the two discharges used in the model were 1) the approximated Q100 and 2) a discharge equal to 1.7 times the approximated Q100.

Most of the cross-section and bridge-opening geometry data were taken from the bridge plans (Indiana State Highway Commission, 1965) provided by INDOT. Bridge plans are presumed to be representative of current conditions at the site. To determine the cross-section geometry, a line was drawn on the bridge plans parallel to the bridge stationing and approximately one bridge width from the bridge. For sites where the bridge plans did not extend far enough laterally for collection of all cross-section data required for WSPRO model analysis, additional data were collected from 7.5-minute topographic maps.

The roadway and embankment profile was taken from the bridge and highway plans for those sites where roadway overtopping was expected. The INDOT bridge plans and 7.5-minute topographic maps were used as a guide, based on the water-surface elevations calculated by the WSPRO model, to determine if roadway overtopping might occur.

Roughness values (n -values) for the main channel were estimated by viewing photographs archived from the Level I scour assessments. The n -values for the overbanks were assigned on the basis of the surface-cover data summarized in the Level I data base (Hopkins and Robinson, unpub. data, 1997). From those data, the following roughness values were assigned to the surface-cover categories: urban—0.050, suburban—0.035, row crop—0.045, pasture—0.035, brush—0.120, forest—0.100, and wetland (any area covered by standing water)—0.100. The n -values for the overbanks were adjusted if the Level I photographs provided sufficient detail to warrant an adjustment.

WSPRO version V050196 was used to model flow through the study site. Starting water-surface elevation was obtained with a slope-conveyance computation. The channel-bed slope in the immediate vicinity of the bridge was estimated from the 7.5-minute topographic map and was used as the slope of the energy grade line for this computation.

WSPRO version V050196 includes a field that allows the input of up to four scour-adjustment factors (K1 to K4). For this modeling, the default value for K4 (bed armoring) was chosen. For scour-adjustment factors K1 and K2 (pier-nose shape and angle of attack, respectively), input values were determined by evaluating the data archived in the Level I data base (Hopkins and Robinson, unpub. data, 1997). For the K3 factor (bed forms), a value of 1.1 was applied in all cases.

In some cases, piers set on the overbanks are constructed with footings that are higher in elevation than pier footings in the main channel. In these situations, if the channel position changes, the piers that were initially constructed on the overbank may become part of the main channel. Therefore, to evaluate total potential scour, the model results obtained for contraction scour and deepest local scour in the main channel were added and applied to all piers in the bridge opening. This methodology allowed for an evaluation of potential undermining of pier supports in the event that future channel movement placed overbank piers in the main channel.

Where bridge pairs have a continuous abutment or fill between the bridges that does not allow expansion of flow, the bridge pair was modeled as one bridge. Sites with discontinuous abutments, allowing expansion between the bridges, were modeled as two separate bridges. In those cases, a valley cross section was measured between the bridges and used as the approach section for the downstream bridge and as the exit section for the upstream bridge.

At sites with no embankment to function as a weir or at sites where the tailwater drowns out the embankment, a composite bridge and road section was used to compute flow. Those sites were computed with friction-loss equations rather than with a bridge routine.

Total scour is taken as the sum of local scour plus contraction scour. If the model predicted negative contraction scour (aggradation), the contraction-scour value was assumed to be zero in determining the total scour depth (table 1). This assumption was made so that a negative contraction scour would not mask the potentially detrimental effects of local scour at a pier. No abutment scour evaluations were made in this study.

Table 1. Cumulative scour depths for the modeled discharges at structure I-64-11-5202 crossing Flat Run Ditch in Posey County, Indiana

Pier number ¹	Stationing from bridge plans ²	Initial bed-elevation at pier (feet)	Main-channel contraction scour depth (feet)	Local scour depth (feet)	Worst-case total-scour depth ³ (feet)	Bottom elevation of pier (feet)	Worst-case bed elevation after scour ⁴ (feet)
Modeled discharge⁵ is 1,630 cubic feet per second							
1	634+17	406	1.9	5.4	7.3	398	393.4
2	634+45	406	1.9	5.4	7.3	398	393.4
Modeled discharge is 2,780 cubic feet per second							
1	634+17	406	7.2	6.3	13.5	398	387.2
2	634+45	406	7.2	6.3	13.5	398	387.2

¹Pier numbers were assigned from left to right as shown on the bridge plans.

²Stationing is the center line of the pier as determined from the bridge plans. Stationing from bridge plan, 634+17, represents a point 63,417 feet from an arbitrary starting location referenced on the bridge plans.

³Worst-case total-scour depths are generated by summing the calculated contraction-scour depth with the worst case of local scour.

⁴Worst-case bed elevation is computed by subtracting the worst-case total-scour depth from the lowest initial bed elevation in the bridge opening (400.7 feet).

⁵Not a coordinated discharge.

SPECIAL CONSIDERATIONS

Model runs indicate the water-surface elevation at the bridge is lower than the low-steel elevation for the modeled discharges. Therefore, there should be no pressure flow through the bridge opening for the discharges modeled.

RESULTS

Scour depths were computed with a version of WSPRO (Larry Arneson, Federal Highway Administration, written commun., 1996) modified from Shearman (1990). This version of WSPRO includes scour calculations in the model output. Scour depths were calculated assuming an infinite depth of material that could erode and a homogeneous particle-size distribution. The results of the scour analysis are presented in table 1; a complete input file and output results are presented in the appendix.

REFERENCES

- Hoggatt, R.E., 1975, Drainage areas of Indiana streams: U.S. Geological Survey, Water Resources Division, 231 p.
- Indiana State Highway Commission, 1965, Bridge plans Interstate Route 64: Bridge File I-64-11-5202.
- Lagasse, P.F.; Schall, J.D.; Johnson, F.; Richardson, E.V.; and Chang, F., 1995, Stream stability at highway structures (2d ed.): Federal Highway Administration, Hydraulic Engineering Circular No. 20, Publication FHWA-IP-90-014, 144 p.
- Richardson, E.V., and Davis, S.R., 1995, Evaluating scour at bridges (3d ed.): Federal Highway Administration, Hydraulic Engineering Circular No. 18, Publication FHWA-IP-90-017, 204 p.
- Shearman, J.O., 1990, User's manual for WSPRO, a computer model for water-surface profile computations: Federal Highway Administration Publication FHWA-IP-89-027, 177 p.

APPENDIX

WSPRO INPUT FILE

```

T1      I-64 OVER FLAT RUN DITCH      I64-11-5202
T2      COUNTY: POSEY                  QUAD: POSEYVILLE 192B
T3      8-01-97                        JOHN T. WILSON
Q        1630      2780
SK       .0028      .0028
XS      EXIT  0      11
GR       62880 420      63418 411  63433 411  63454 400.7  63458 400.7
GR       63478 410.5  63515 411  63550 412  63575 413  63596 414
GR       63611 415      63630 416  63648 417  63694 422
N        .035      .035      .035
SA              63433      63478
XS      FULLV  68      11
BR      BRDGE  68      414.9      11
*        ***BRIDGE DECK SKEWED 11 DEGREES (FROM BRIDGE PLANS)***
GR       63396 414.9  63396 414.8  63399 414.8  63417 406.4  63422 404.1
GR       63429 400.7  63434 400.7  63443 405.4  63461 414.6  63464 414.6
GR       63464 414.8  63396 414.9
N        .035
PD       406.4      2.0      1
PD       406.4      4.0      2
CD       3      122      2      417
DC 0 BRDGE  63407  63454  63378  63420  *  4
*        ***DC LIMITS AT BRIDGE ARE LEW AND REW FOR Q1***
*        BXL      BXR      PW      *  *  K1  K2  K3
DP       63396  63464  2.0  *  *  1  1  1.1
DP       63396  63464  2.0  *  *  1  1  1.1
*        ***DP CARDS USE WHOLE BRIDGE OPENING FOR BXL/BXR***
*        ***NO PIERS IN MAIN CHANNEL***
XS      APPR  258
*        ***SECTION IS ADJUSTED FOR 11 DEGREES OF SKEW***
GR       62887 420      63221 410      63311 410  63367 411  63378 410.5
GR       63397 400.9  63402 400.9  63420 410  63461 413  63487 414
GR       63511 415      63532 416      63690 430
N        .045      .035      .045
SA              63378      63420
HP 2 BRDGE  411.4  *  411.4  1630
HP 2 BRDGE  412.7  *  412.7  2780
EX
ER

```

WSPRO OUTPUT

```
***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Run Date & Time: 8/ 4/97 1:00 pm Version V050196
Input File: flatrun.dat Output File: flatrun.LST
```

```
*****
T1      I-64 OVER FLAT RUN DITCH      I64-11-5202
T2      COUNTY: POSEY                  QUAD: POSEYVILLE 192B
T3      8-01-97                        JOHN T. WILSON
Q        1630      2780
```

*** Processing Flow Data; Placing Information into Sequence 1 ***

SK .0028 .0028

```
***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English
```

```
*****
      I-64 OVER FLAT RUN DITCH      I64-11-5202
COUNTY: POSEY                  QUAD: POSEYVILLE 192B
      8-01-97                        JOHN T. WILSON
```

```
*****
* Starting To Process Header Record EXIT *
*****
```

```
XS  EXIT  0  11
GR   62880 420      63418 411  63433 411  63454 400.7  63458 400.7
GR   63478 410.5    63515 411  63550 412  63575 413    63596 414
GR   63611 415      63630 416  63648 417  63694 422
N     .035      .035      .035
SA           63433      63478
```

*** Completed Reading Data Associated With Header Record EXIT ***
 *** Storing X-Section Data In Temporary File As Record Number 1 ***

```
*** Data Summary For Header Record EXIT ***
SRD Location:      0. Cross-Section Skew: 11.0 Error Code 0
Valley Slope:     .00000 Averaging Conveyance By Geometric Mean.
Energy Loss Coefficients -> Expansion: .50 Contraction: .00
```

X,Y-coordinates (14 pairs)					
X	Y	X	Y	X	Y
62880.000	420.000	63418.000	411.000	63433.000	411.000
63454.000	400.700	63458.000	400.700	63478.000	410.500
63515.000	411.000	63550.000	412.000	63575.000	413.000
63596.000	414.000	63611.000	415.000	63630.000	416.000
63648.000	417.000	63694.000	422.000		

WSPRO OUTPUT

Minimum and Maximum X,Y-coordinates

Minimum X-Station: 62880.000 (associated Y-Elevation: 420.000)
 Maximum X-Station: 63694.000 (associated Y-Elevation: 422.000)
 Minimum Y-Elevation: 400.700 (associated X-Station: 63458.000)
 Maximum Y-Elevation: 422.000 (associated X-Station: 63694.000)

X-coordinates & Horizontal Breakpoints Translated by Skew Angle

X Input	X Skewed	X Input	X Skewed	X Input	X Skewed
62880.000	62890.620	63418.000	63418.730	63433.000	63433.460
63454.000	63454.070	63458.000	63458.000	63478.000	63477.630
63515.000	63513.950	63550.000	63548.310	63575.000	63572.850
63596.000	63593.460	63611.000	63608.190	63630.000	63626.840
63648.000	63644.510	63694.000	63689.660		

Roughness Data (3 SubAreas)

SubArea	Roughness Coefficient	Horizontal Breakpoint
1	.035	---
	---	*****
2	.035	---
	---	*****
3	.035	---

* Finished Processing Header Record EXIT *

***** W S P R O *****

Federal Highway Administration - U. S. Geological Survey
 Model for Water-Surface Profile Computations.
 Input Units: English / Output Units: English

I-64 OVER FLAT RUN DITCH I64-11-5202
 COUNTY: POSEY QUAD: POSEYVILLE 192B
 8-01-97 JOHN T. WILSON

* Starting To Process Header Record FULLV *

XS FULLV 68 11

*** Completed Reading Data Associated With Header Record FULLV ***
 *** No Roughness Data Input, Propagating From Previous Section ***
 *** Storing X-Section Data In Temporary File As Record Number 2 ***

WSPRO OUTPUT

```

***                               Data Summary For Header Record FULLV                               ***
SRD Location:      68.   Cross-Section Skew:  11.0   Error Code   0
Valley Slope:     .00000   Averaging Conveyance By Geometric Mean.
Energy Loss Coefficients ->   Expansion:   .50   Contraction:   .00
  
```

X,Y-coordinates (14 pairs)					
X	Y	X	Y	X	Y
62880.000	420.000	63418.000	411.000	63433.000	411.000
63454.000	400.700	63458.000	400.700	63478.000	410.500
63515.000	411.000	63550.000	412.000	63575.000	413.000
63596.000	414.000	63611.000	415.000	63630.000	416.000
63648.000	417.000	63694.000	422.000		

```

Minimum and Maximum X,Y-coordinates
Minimum X-Station:  62880.000  ( associated Y-Elevation:  420.000 )
Maximum X-Station:  63694.000  ( associated Y-Elevation:  422.000 )
Minimum Y-Elevation:  400.700  ( associated X-Station:  63458.000 )
Maximum Y-Elevation:  422.000  ( associated X-Station:  63694.000 )
  
```

X-coordinates & Horizontal Breakpoints Translated by Skew Angle					
X Input	X Skewed	X Input	X Skewed	X Input	X Skewed
62880.000	62890.620	63418.000	63418.730	63433.000	63433.460
63454.000	63454.070	63458.000	63458.000	63478.000	63477.630
63515.000	63513.950	63550.000	63548.310	63575.000	63572.850
63596.000	63593.460	63611.000	63608.190	63630.000	63626.840
63648.000	63644.510	63694.000	63689.660		

Roughness Data (3 SubAreas)		
SubArea	Roughness Coefficient	Horizontal Breakpoint
1	.035	---
	---	*****
2	.035	---
	---	*****
3	.035	---

```

*-----*
*      Finished Processing Header Record FULLV      *
*-----*
  
```

```

***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English
*-----*
  
```

```

I-64 OVER FLAT RUN DITCH      I64-11-5202
COUNTY: POSEY                QUAD: POSEYVILLE 192B
8-01-97                       JOHN T. WILSON
  
```

WSPRO OUTPUT

```
*-----*
*           Starting To Process Header Record BRDGE           *
*-----*
```

```
BR   BRDGE  68   414.9   11
GR      63396 414.9   63396 414.8   63399 414.8   63417 406.4   63422 404.1
GR      63429 400.7   63434 400.7   63443 405.4   63461 414.6   63464 414.6
GR      63464 414.8   63396 414.9
N      .035
PD      406.4   2.0    1
PD      406.4   4.0    2
CD      3      122    2      417
```

```
*** Completed Reading Data Associated With Header Record BRDGE ***
+++072 NOTICE: X-coordinate # 2 increased to eliminate vertical segment.
+++072 NOTICE: X-coordinate #11 increased to eliminate vertical segment.
*** Storing Bridge Data In Temporary File As Record Number 3 ***
```

```
*** Data Summary For Bridge Record BRDGE ***
SRD Location:      68. Cross-Section Skew: 11.0 Error Code 0
Valley Slope: ***** Averaging Conveyance By Geometric Mean.
Energy Loss Coefficients -> Expansion: .50 Contraction: .00
```

```

X,Y-coordinates (12 pairs)
      X      Y      X      Y      X      Y
-----
63396.000    414.900    63396.100    414.800    63399.000    414.800
63417.000    406.400    63422.000    404.100    63429.000    400.700
63434.000    400.700    63443.000    405.400    63461.000    414.600
63464.000    414.600    63464.100    414.800    63396.000    414.900
-----
```

```

Minimum and Maximum X,Y-coordinates
Minimum X-Station: 63396.000 ( associated Y-Elevation: 414.900 )
Maximum X-Station: 63464.100 ( associated Y-Elevation: 414.800 )
Minimum Y-Elevation: 400.700 ( associated X-Station: 63434.000 )
Maximum Y-Elevation: 414.900 ( associated X-Station: 63396.000 )
```

```

X-coordinates & Horizontal Breakpoints Translated by Skew Angle
      X Input      X Skewed      X Input      X Skewed      X Input      X Skewed
-----
63396.000    63396.700    63396.100    63396.800    63399.000    63399.640
63417.000    63417.310    63422.000    63422.220    63429.000    63429.090
63434.000    63434.000    63443.000    63442.840    63461.000    63460.500
63464.000    63463.450    63464.100    63463.550    63396.000    63396.700
-----
```

```

Roughness Data ( 1 SubAreas )
      Roughness Horizontal
SubArea Coefficient Breakpoint
-----
      1      .035      ---
-----
```


WSPRO OUTPUT

Discharge coefficient parameters
 BRType BRWidth EMBSS EMBElv UserCD
 3 122.000 2.00 417.000 *****

Pressure flow elevations
 AVBCEL PFElev
 ***** 414.900

Abutment Parameters
 ABSLPL ABSLPR XTOELT YTOELT XTOERT YTOERT

Pier/Pile Data (2 Group(s))
 Code Indicates Bridge Uses Piers
 Group Elevation Gross Width Number

 1 406.400 2.000 1
 2 406.400 4.000 2

 * Finished Processing Header Record BRDGE *

***** W S P R O *****
 Federal Highway Administration - U. S. Geological Survey
 Model for Water-Surface Profile Computations.
 Input Units: English / Output Units: English

 I-64 OVER FLAT RUN DITCH I64-11-5202
 COUNTY: POSEY QUAD: POSEYVILLE 192B
 8-01-97 JOHN T. WILSON
 DC 0 BRDGE 63407 63454 63378 63420 * 4
 DP 63396 63464 2.0 * * 1 1 1.1
 DP 63396 63464 2.0 * * 1 1 1.1

 * Starting To Process Header Record APPR *

XS APPR 258
 GR 62887 420 63221 410 63311 410 63367 411 63378 410.5
 GR 63397 400.9 63402 400.9 63420 410 63461 413 63487 414
 GR 63511 415 63532 416 63690 430
 N .045 .035 .045
 SA 63378 63420

*** Completed Reading Data Associated With Header Record APPR ***
 *** Storing X-Section Data In Temporary File As Record Number 4 ***

WSPRO OUTPUT

```

***                               Data Summary For Header Record APPR                               ***
SRD Location:      258.   Cross-Section Skew:      .0   Error Code      0
Valley Slope:     .00000   Averaging Conveyance By Geometric Mean.
Energy Loss Coefficients ->   Expansion:      .50   Contraction:      .00
  
```

X,Y-coordinates (13 pairs)					
X	Y	X	Y	X	Y
62887.000	420.000	63221.000	410.000	63311.000	410.000
63367.000	411.000	63378.000	410.500	63397.000	400.900
63402.000	400.900	63420.000	410.000	63461.000	413.000
63487.000	414.000	63511.000	415.000	63532.000	416.000
63690.000	430.000				

```

Minimum and Maximum X,Y-coordinates
Minimum X-Station:  62887.000  ( associated Y-Elevation:  420.000 )
Maximum X-Station:  63690.000  ( associated Y-Elevation:  430.000 )
Minimum Y-Elevation:  400.900  ( associated X-Station:  63402.000 )
Maximum Y-Elevation:  430.000  ( associated X-Station:  63690.000 )
  
```

```

Roughness Data ( 3 SubAreas )
      Roughness   Horizontal
SubArea Coefficient Breakpoint
-----
      1          .045      ---
      2          .035      ---
      3          .045      ---
  
```

```

Bridge datum projection(s):  XREFLT  XREFRT  FDSTLT  FDSTRT
*****
  
```

```

*-----*
*      Finished Processing Header Record APPR      *
*-----*
  
```

```

***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English
*-----*
  
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      I-64 OVER FLAT RUN DITCH      I64-11-5202
COUNTY: POSEY      QUAD: POSEYVILLE 192B
      8-01-97      JOHN T. WILSON
HP 2 BRDGE  411.4  *  411.4  1630
HP 2 BRDGE  412.7  *  412.7  2780
EX
  
```

```

=====
*      Summary of Boundary Condition Information      *
=====
  
```

WSPRO OUTPUT

#	Reach Discharge	Water Surface Elevation	Friction Slope	Flow Regime
1	1630.00	*****	.0028	Sub-Critical
2	2780.00	*****	.0028	Sub-Critical

```

*****
*           Beginning  2 Profile Calculation(s)           *
*****

```

```

***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English

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*-----*
I-64 OVER FLAT RUN DITCH      I64-11-5202
COUNTY: POSEY                QUAD: POSEYVILLE 192B
8-01-97                       JOHN T. WILSON

```

	WSEL EGEL CRWS	VHD HF HO	Q V FR #	AREA K SF	SRDL FLEN ALPHA	LEW REW ERR
Section: EXIT	410.767	.676	1630.000	249.542	*****	63433.480
Header Type: XS	411.443	*****	6.532	30785.70	*****	63497.750
SRD: .000	408.116	*****	.590	*****	1.019	*****
Section: FULLV	411.018	.610	1630.000	268.354	68.000	63416.930
Header Type: FV	411.628	.177	6.074	33064.78	68.000	63515.630
SRD: 68.000	408.116	.000	.670	.0026	1.064	.008

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

===135 CONVEYANCE RATIO OUTSIDE OF RECOMMENDED LIMITS AT SECID "APPR ".
KRATIO: 1.53

Section: APPR	411.676	.254	1630.000	572.535	190.000	63165.020
Header Type: AS	411.930	.302	2.847	50507.66	190.000	63442.910
SRD: 258.000	408.152	.000	.496	.0016	2.013	-.001

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

<<< The Following Data Reflect The "Constricted" Profile >>>
<<< Beginning Bridge/Culvert Hydraulic Computations >>>

WSPRO OUTPUT

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: BRDGE	411.053	.578	1630.000	267.838	68.000	63407.030
Header Type: BR	411.631	.186	6.086	33975.45	68.000	63454.060
SRD: 68.000	407.964	.002	.450	*****	1.003	.000

Specific Bridge Information	C	P/A	PFELEV	BLEN	XLAB	XRAB
Bridge Type 3 Flow Type 1						
Pier/Pile Code 0	.9985	.069	414.900	*****	*****	*****

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: APPR	411.908	.206	1630.000	638.082	68.000	63157.290
Header Type: AS	412.114	.132	2.555	56167.14	88.070	63446.070
SRD: 258.000	408.152	.351	.432	.0016	2.031	.001

Approach Section APPR Flow Contraction Information						
M(G)	M(K)	KQ	XLKQ	XRKQ	OTEL	
.831	.255	41819.1	*****	*****	411.908	

<<< End of Bridge Hydraulics Computations >>>

***** W S P R O *****

Federal Highway Administration - U. S. Geological Survey

Model for Water-Surface Profile Computations.

Input Units: English / Output Units: English

I-64 OVER FLAT RUN DITCH I64-11-5202

COUNTY: POSEY

QUAD: POSEYVILLE 192B

8-01-97

JOHN T. WILSON

===150 WARNING: SLOPE-CONVEYANCE CONVERGENCE FAILURE.

Used final trial values.

QCOMP, WSTRY: 2767. 412.30

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: EXIT	412.296	.830	2780.000	471.367	*****	63340.550
Header Type: XS	413.125	*****	5.898	52289.23	*****	63557.390
SRD: .000	410.057	*****	.873	*****	1.533	*****
Section: FULLV	412.649	.639	2780.000	553.281	68.000	63319.430
Header Type: FV	413.288	.167	5.025	60094.78	68.000	63566.220
SRD: 68.000	410.057	.000	.755	.0025	1.627	-.005

WSPRO OUTPUT

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

===135 CONVEYANCE RATIO OUTSIDE OF RECOMMENDED LIMITS AT SECID "APPR ".
KRATIO: 1.68

Section: APPR	413.334	.193	2780.000	1098.741	190.000	63109.630
Header Type: AS	413.528	.241	2.530	101257.90	190.000	63469.700
SRD: 258.000	411.275	.000	.356	.0013	1.940	-.001

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

<<< The Following Data Reflect The "Constricted" Profile >>>
<<< Beginning Bridge/Culvert Hydraulic Computations >>>

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: BRDGE	412.122	1.374	2780.000	320.456	68.000	63404.740
Header Type: BR	413.496	.250	8.675	43149.08	68.000	63456.150
SRD: 68.000	410.117	.117	.664	*****	1.174	-.010

Specific Bridge Information	C	P/A	PFELEV	BLEN	XLAB	XRAB
Bridge Type 3 Flow Type 1						
Pier/Pile Code 0	.9229	.071	414.900	*****	*****	*****

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: APPR	414.120	.114	2780.000	1399.927	68.000	63083.390
Header Type: AS	414.234	.131	1.986	133960.30	90.769	63489.880
SRD: 258.000	411.275	.613	.258	.0013	1.866	.018

Approach Section APPR Flow Contraction Information						
M(G)	M(K)	KQ	XLKQ	XRKQ	OTEL	
.851	.493	67526.6	*****	*****	414.120	

<<< End of Bridge Hydraulics Computations >>>

***** W S P R O *****

Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English

I-64 OVER FLAT RUN DITCH	I64-11-5202
COUNTY: POSEY	QUAD: POSEYVILLE 192B
8-01-97	JOHN T. WILSON

WSPRO OUTPUT

*** Beginning Velocity Distribution For Header Record BRDGE ***
 SRD Location: 68.000 Header Record Number 3

Water Surface Elevation: 411.400 Element # 1
 Flow: 1630.000 Velocity: 5.73 Hydraulic Depth: 5.870
 Cross-Section Area: 284.40 Conveyance: 36804.18
 Bank Stations -> Left: 63406.290 Right: 63454.740

X STA.	63406.3	63417.0	63420.1	63422.3	63424.1	63425.6
A(I)		26.6	17.7	15.6	14.1	12.9
V(I)		3.07	4.62	5.23	5.78	6.33
D(I)		2.49	5.69	6.92	7.88	8.68

X STA.	63425.6	63426.9	63428.1	63429.1	63430.1	63431.1
A(I)		12.1	11.7	11.3	10.7	10.7
V(I)		6.72	6.94	7.20	7.65	7.65
D(I)		9.35	9.95	10.50	10.70	10.70

X STA.	63431.1	63432.1	63433.1	63434.1	63435.3	63436.5
A(I)		10.7	10.7	10.8	11.7	12.0
V(I)		7.62	7.62	7.56	6.98	6.77
D(I)		10.70	10.70	10.70	10.34	9.72

X STA.	63436.5	63437.9	63439.6	63441.6	63444.6	63454.7
A(I)		12.7	13.8	14.9	17.5	26.3
V(I)		6.42	5.89	5.47	4.66	3.10
D(I)		9.03	8.22	7.24	5.94	2.59

***** W S P R O *****

Federal Highway Administration - U. S. Geological Survey

Model for Water-Surface Profile Computations.

Input Units: English / Output Units: English

I-64 OVER FLAT RUN DITCH I64-11-5202

COUNTY: POSEY

QUAD: POSEYVILLE 192B

8-01-97

JOHN T. WILSON

*** Beginning Velocity Distribution For Header Record BRDGE ***
 SRD Location: 68.000 Header Record Number 3

Water Surface Elevation: 412.700 Element # 1
 Flow: 2780.000 Velocity: 7.92 Hydraulic Depth: 6.524
 Cross-Section Area: 350.85 Conveyance: 48688.02
 Bank Stations -> Left: 63403.500 Right: 63457.280

X STA.	63403.5	63415.4	63418.8	63421.3	63423.3	63424.9
A(I)		32.8	21.8	19.2	17.4	15.8
V(I)		4.24	6.39	7.23	8.00	8.81
D(I)		2.77	6.33	7.70	8.74	9.62

X STA.	63424.9	63426.4	63427.7	63428.9	63430.0	63431.1
A(I)		15.0	14.7	13.9	13.1	13.2
V(I)		9.28	9.48	9.98	10.59	10.55
D(I)		10.37	11.04	11.65	12.00	12.00

WSPRO OUTPUT

X STA.	63431.1	63432.2	63433.3	63434.4	63435.6	63437.0
A(I)		13.2	13.2	13.4	14.3	14.7
V(I)		10.52	10.52	10.40	9.73	9.43
D(I)		12.00	12.00	11.96	11.47	10.79

X STA.	63437.0	63438.6	63440.5	63442.8	63446.0	63457.3
A(I)		15.6	17.4	18.3	21.6	32.4
V(I)		8.92	8.00	7.59	6.44	4.29
D(I)		10.02	9.12	8.03	6.59	2.88

```

***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English
*-----*
I-64 OVER FLAT RUN DITCH I64-11-5202
COUNTY: POSEY QUAD: POSEYVILLE 192B
8-01-97 JOHN T. WILSON
  
```

*** Live-Bed Contraction Scour Calculations for Header Record BRDGE ***

Constants and Input Variables

```

*-----*
Bed Material Transport Mode Factor (k1): .64
Total Pier Width Value (Pw): 4.000
*-----*
  
```

#	Scour		-- Flow --		-- Width --		--- X-Limits ---	
	Depth	Contract	Approach	Contract	Approach	Side	Contract	Approach
1	1.880	1630.000	1205.958	43.000	42.000	Left:	*****	*****
 Approach Channel Depth:		6.886	Right:	*****	*****	
2	7.249	2780.000	1372.024	43.000	42.000	Left:	*****	*****
 Approach Channel Depth:		9.099	Right:	*****	*****	

```

***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English
*-----*
I-64 OVER FLAT RUN DITCH I64-11-5202
COUNTY: POSEY QUAD: POSEYVILLE 192B
8-01-97 JOHN T. WILSON
  
```

*** Pier Scour Calculations for Header Record BRDGE ***

WSPRO OUTPUT

Constants and Input Variables

Pier Width: 2.000

```
*-----*
Pier Shape Factor          (K1):  1.00
Flow Angle of Attack Factor (K2):  1.00
Bed Condition Factor       (K3):  1.10
Bed Material Factor        (K4):  1.00
Velocity Multiplier        (VM):  1.00
Depth Multiplier           (YM):  1.00
*-----*
```

#	Scour Depth	Flow	WSE	Depth	Velocity	Froude #	Left	Right
1	5.40	1630.000	411.404	10.704	7.644	.412	63396.000	63464.000
2	6.30	2780.000	412.735	12.035	10.522	.535	63396.000	63464.000

***** W S P R O *****

Federal Highway Administration - U. S. Geological Survey

Model for Water-Surface Profile Computations.

Input Units: English / Output Units: English

I-64 OVER FLAT RUN DITCH I64-11-5202
COUNTY: POSEY QUAD: POSEYVILLE 192B
8-01-97 JOHN T. WILSON

*** Pier Scour Calculations for Header Record BRDGE ***

Constants and Input Variables

Pier Width: 2.000

```
*-----*
Pier Shape Factor          (K1):  1.00
Flow Angle of Attack Factor (K2):  1.00
Bed Condition Factor       (K3):  1.10
Bed Material Factor        (K4):  1.00
Velocity Multiplier        (VM):  1.00
Depth Multiplier           (YM):  1.00
*-----*
```

#	Scour Depth	Flow	WSE	Depth	Velocity	Froude #	Left	Right
1	5.40	1630.000	411.404	10.704	7.644	.412	63396.000	63464.000
2	6.30	2780.000	412.735	12.035	10.522	.535	63396.000	63464.000

ER

***** Normal end of WSPRO execution. *****
***** Elapsed Time: 0 Minutes 46 Seconds *****