

Modified Level II Streambed-Scour Analysis for Structure (I-465)-431-49-4445 Crossing Lick Creek in Marion County, Indiana

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CONVERSION FACTORS AND ABBREVIATIONS

Multiply	By	To obtain
inch (in.)	25.4	millimeter
foot (ft)	0.3048	meter
square foot (ft ²)	929.0	square centimeter
feet per second (ft/s)	0.3048	meters per second
cubic foot per second (ft ³ /s)	0.02832	cubic meter per second
mile (mi)	1.609	kilometer
square mile (mi ²)	2.590	square kilometer

Abbreviations used in this report:

D ₅₀	median diameter of bed material
Q ₁₀₀	100-year discharge
FEMA	Federal Emergency Management Agency
HEC	Hydraulic Engineering Circular
IDNR	Indiana Department of Natural Resources
INDOT	Indiana Department of Transportation
USGS	U. S. Geological Survey
WSPRO	Water Surface PROfile model

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ABSTRACT

Level II scour evaluations follow a process in which hydrologic, hydraulic, and sediment-transport data are evaluated to calculate the depth of scour that may result when a given discharge is routed through a bridge opening. The results of the modified Level II analysis for structure (I-465)-431-49-4445 on State Route 431 crossing Interstate 465 and Lick Creek in Marion County, Indiana, are presented. The site is near the city of Edgewood in the southern part of Marion County. Scour depths were computed with the Water Surface PROfile model, version V050196, which incorporates the scour-calculation procedures outlined in Hydraulic Engineering Circular No. 18. Total scour depths at the piers were approximately 6.7 feet for the modeled discharge of 8,200 cubic feet per second and approximately 7.0 feet for the modeled discharge of 11,400 cubic feet per second.

INTRODUCTION

The U.S. Geological Survey (USGS), in cooperation with the Indiana Department of Transportation (INDOT), is conducting Level II scour analyses at a number of bridges throughout Indiana. This report describes the methods applied and the modeling results for bridge (I-465)-431-49-4445.

Background and Scope

Level I scour assessment is a process where a large number of bridges are studied as a group. Assessments usually are made by evaluating a combination of geomorphic, hydrologic, and bridge-characteristic data. The results help investigators determine which bridges appear to be most likely to experience streambed-scour problems and which bridges appear to be relatively immune to problems brought on by streambed scour (for example, bridges built on bedrock).

When applied correctly, Level I scour assessments provide an investigator with information to identify those bridges that appear to be relatively safe and those bridges that fall into higher risk categories.

Level II scour evaluations describe the process for an investigator to apply a model to a bridge site and calculate the potential depth of scour that may result from a given flood event. Level II analyses involve the application of basic hydrologic, hydraulic, and sediment-transport engineering concepts and may include an evaluation of flood history, channel hydraulic conditions (for example, water-surface profile analysis), and basic sediment-transport analyses such as scour calculations (Lagasse and others, 1995).

The methods and model outlined in Hydraulic Engineering Circular (HEC) No. 18 (Richardson and Davis, 1995) formulate the basis for Level II scour evaluations. Methods used in this study for Level II scour evaluations are a modification of the HEC-18 standards. These modifications were made to comply with the methodology requested by INDOT (Merril Dougherty, Indiana Department of Transportation, oral commun., 1996). Descriptions of the specific modifications are given in the "Evaluation Methods" section of this report.

This report presents the methods followed for modeling, special considerations for this study site, and the input for and the output from the Water Surface PROfile (WSPRO) model.

Site Description

The study site is located near the city of Edgewood in the southern part of Marion County. The drainage area for the site is approximately 20.4 mi² (Merril Dougherty, Indiana Department of Transportation, written commun., 1997). The predominant land use in the basin is urban; in the immediate vicinity of the bridge, the land is predominantly interstate right-of-way with the channelized waterway of Lick Creek.

Within the immediate vicinity of the bridge, Lick Creek has a channel-bed slope of approximately 0.0025 ft/ft. The channel-bed material is a cobble gravelly sand, and the channel banks consist of concrete. At the time of the Level I site visit on July 21, 1995, the banks were observed to have 0 to 25 percent woody vegetative cover; the field report noted that the banks were stable.

The State Route 431 crossing of Interstate 465 and Lick Creek is a 348-ft-long, multi-lane bridge consisting of six spans supported by concrete and steel piers and sloping concrete spill-through abutments. Parts of the southern three spans cross Lick Creek, the northern three spans cross Interstate 465. Additional details describing conditions at the site are included in the Level I data base (Hopkins and Robinson, unpub. data, 1997). Photographs of the site, taken at the time of the Level I site visit, are archived at the USGS office in Indianapolis.

EVALUATION METHODS

The methods described in this section apply to a number of bridge sites in Indiana being evaluated for scour and outline the procedures requested by INDOT for these modified Level II scour analyses. The principal modification requested by INDOT was that the input data to the model come from or be estimated from existing data sources; no additional field data were collected. Actual methods used in the scour evaluation at this particular bridge site use the most applicable method possible, given the data available.

To determine drainage area, either published values found in Hoggatt (1975) or USGS 7.5-minute topographic maps with Hoggatt's original drainage-area delineations were used. Where there are no published data, drainage-area segments measured from the maps produced by Hoggatt were either subtracted from downstream sites or added to upstream sites published by Hoggatt (1975).

In Indiana, flood discharges are coordinated by agreement among State and Federal agencies. At sites where flood discharges officially are coordinated among State and Federal agencies in Indiana, the coordinated 100-year discharge (Q100) was modeled. INDOT also provided an additional flood discharge for these coordinated sites in excess of the Q100 to be modeled.

If a flood discharge was not coordinated, the USGS examined Federal Emergency Management Agency (FEMA) studies for Q100 determinations. Where FEMA studies did not produce a Q100, the USGS contacted IDNR for an estimated Q100 in the vicinity of the site being studied. If IDNR did not have a Q100, data from nearby USGS streamflow-gaging stations were analyzed with nearby and similar drainage basins that have been coordinated. At sites having no coordinated discharge data, the two discharges used in the model were 1) the approximated Q100 and 2) a discharge equal to 1.7 times the approximated Q100.

Most of the cross-section and bridge-opening geometry data were taken from the bridge plans (Indiana State Highway Commission, 1959) provided by INDOT. Bridge plans are presumed to be representative of current conditions at the site. To determine the cross-section geometry, a line was drawn on the bridge plans parallel to the bridge stationing and approximately one bridge width from the bridge. For sites where the bridge plans did not extend far enough laterally for collection of all cross-section data required for WSPRO model analysis, additional data were collected from 7.5-minute topographic maps.

The roadway and embankment profile was taken from the bridge and highway plans for those sites where roadway overtopping was expected. The INDOT bridge plans and 7.5-minute topographic maps were used as a guide, based on the water-surface elevations calculated by the WSPRO model, to determine if roadway overtopping might occur.

Roughness values (*n*-values) for the main channel were estimated by viewing photographs archived from the Level I scour assessments. The *n*-values for the overbanks were assigned on the basis of the surface-cover data summarized in the Level I data base (Hopkins and Robinson, unpub. data, 1997). From those data, the following roughness values were assigned to the surface-cover categories: urban—0.050, suburban—0.035, row crop—0.045, pasture—0.035, brush—0.120, forest—0.100, and wetland (any area covered by standing water)—0.100. The *n*-values for the overbanks were adjusted if the Level I photographs provided sufficient detail to warrant an adjustment.

WSPRO version V050196 was used to model flow through the study site. Starting water-surface elevation was obtained with a slope-conveyance computation. The channel-bed slope in the immediate vicinity of the bridge was estimated from the 7.5-minute topographic map and was used as the slope of the energy grade line for this computation.

WSPRO version V050196 includes a field that allows the input of up to four scour-adjustment factors (K1 to K4). For this modeling, the default value for K4 (bed armoring) was chosen. For scour-adjustment factors K1 and K2 (pier-nose shape and angle of attack, respectively), input values were determined by evaluating the data archived in the Level I data base (Hopkins and Robinson, unpub. data, 1997). For the K3 factor (bed forms), a value of 1.1 was applied in all cases.

In some cases, piers set on the overbanks are constructed with footings that are higher in elevation than pier footings in the main channel. In these situations, if the channel position changes, the piers that were initially constructed on the overbank may become part of the main channel. Therefore, to evaluate total potential scour, the model results obtained for contraction scour and deepest local scour in the main channel were added and applied to all piers in the bridge opening. This methodology allowed for an evaluation of potential undermining of pier supports in the event that future channel movement placed overbank piers in the main channel.

Where bridge pairs have a continuous abutment or fill between the bridges that does not allow expansion of flow, the bridge pair was modeled as one bridge. Sites with discontinuous abutments, allowing expansion between the bridges, were modeled as two separate bridges. In those cases, a valley cross section was measured between the bridges and used as the approach section for the downstream bridge and as the exit section for the upstream bridge.

At sites with no embankment to function as a weir or at sites where the tailwater drowns out the embankment, a composite bridge and road section was used to compute flow. Those sites were computed with friction-loss equations rather than with a bridge routine.

Total scour is taken as the sum of local scour plus contraction scour. If the model predicted negative contraction scour (aggradation), the contraction-scour value was assumed to be zero in determining the total scour depth (table 1). This assumption was made so that a negative contraction scour would not mask the potentially detrimental effects of local scour at a pier. No abutment scour evaluations were made in this study.

Table 1. Cumulative scour depths for the modeled discharges at structure (I-465)-431-49-4445 crossing Lick Creek in Marion County, Indiana

Pier number ¹	Stationing from bridge plans ²	Initial bed-elevation at pier (feet)	Main-channel contraction scour depth (feet)	Local scour depth (feet)	Worst-case total-scour depth ³ (feet)	Bottom elevation of pier (feet)	Worst-case bed elevation after scour ⁴ (feet)
Modeled discharge⁵ is 8,200 cubic feet per second							
1	26+19	707	0	6.7	6.7	700.0	698.3
2	26+85	711	0	6.7	6.7	700.0	698.3
Modeled discharge is 11,400 cubic feet per second							
1	26+19	707	0	7.0	7.0	700.0	698.0
2	26+85	711	0	7.0	7.0	700.0	698.0

¹Pier numbers were assigned from left to right as shown on the bridge plans.

²Stationing is the center line of the pier as determined from the bridge plans. Stationing from bridge plan, 26+19, represents a point 2,619 feet from an arbitrary starting location referenced on the bridge plans.

³Worst-case total-scour depths are generated by summing the calculated contraction-scour depth with the worst case of local scour.

⁴Worst-case bed elevation is computed by subtracting the worst-case total-scour depth from the lowest initial bed elevation in the bridge opening (705.0 feet).

⁵Coordinated discharge.

SPECIAL CONSIDERATIONS

Model runs indicate the water-surface elevation at the bridge is lower than the low-steel elevation for the modeled discharges. Therefore, there should be no pressure flow through the bridge opening for the discharges modeled.

Lick Creek has concrete banks in the vicinity of the bridge. The bases of the southern two piers are protected by the concrete banks. The bridge was modeled and scour was computed as if this concrete protection did not exist. Only the part of the bridge crossing Lick Creek was modeled—the spans crossing Interstate 465 were above the modeled water-surface elevations and were not included.

RESULTS

Scour depths were computed with a version of WSPRO (Larry Arneson, Federal Highway Administration, written commun., 1996) modified from Shearman (1990). This version of WSPRO includes scour calculations in the model output. Scour depths were calculated assuming an infinite depth of material that could erode and a homogeneous particle-size distribution. The results of the scour analysis are presented in table 1; a complete input file and output results are presented in the appendix.

REFERENCES

- Hoggatt, R.E., 1975, Drainage areas of Indiana streams: U.S. Geological Survey, Water Resources Division, 231 p.
- Indiana State Highway Commission, 1959, Bridge plans State Route 431: Bridge File (I-465)-431-49-4445.
- Lagasse, P.F.; Schall, J.D.; Johnson, F.; Richardson, E.V.; and Chang, F., 1995, Stream stability at highway structures (2d ed.): Federal Highway Administration, Hydraulic Engineering Circular No. 20, Publication FHWA-IP-90-014, 144 p.
- Richardson, E.V., and Davis, S.R., 1995, Evaluating scour at bridges (3d ed.): Federal Highway Administration, Hydraulic Engineering Circular No. 18, Publication FHWA-IP-90-017, 204 p.
- Shearman, J.O., 1990, User's manual for WSPRO, a computer model for water-surface profile computations: Federal Highway Administration Publication FHWA-IP-89-027, 177 p.

APPENDIX

WSPRO INPUT FILE

```

T1      U.S. 431 OVER LICK CREEK      (I465)431-49-4445
T2      COUNTY: MARION                QUAD: MAYWOOD 124A
T3      7-29-97                      JOHN T. WILSON
Q        8200      11400
SK       .0025     .0025
XS      EXIT 0
*        ***CHANNEL IS STRAIGHT WITH CONCRETE SLOPEWALLS***
GR       2565 735.4 2565 719.4 2580 714.8 2606 713.8 2624 705.0
GR       2672 705.0 2690 713.5 2695 713.5 2736 733.0 2740 733.0
N        .035      .020      .035
SA              2606      2690
XS      FULLV 140
GR       2565 735.4 2565 719.4 2580 714.8 2606 713.8 2624 705.0
GR       2672 705.0 2690 713.5 2695 713.5 2736 733.0 2740 733.0
BR      BRDGE 140 735
*        ***BASE OF PIERS ARE SET IN CONCRETE SLOPEWALLS***
*        ***BRIDGE SECTION WAS USED AS A TEMPLATE FOR APPR/EXIT***
GR       2565 735.4 2565 719.4 2580 714.8 2606 713.8 2624 705.0
GR       2672 705.0 2690 713.5 2695 713.5 2736 733.0 2740 733.0
GR       2740 734.6 2565 735.4
N        .035      .020      .035
SA              2606      2690
PD       707.3      2.0      1
PD       711.0      2.0      2
PD       711.0      4.0      3
CD       3      76      2      737
DC 0 BRDGE 2578 2699 2606 2690 * 4
*        ***DC LIMITS AT BRIDGE ARE APPROX. LEW AND REW FOR Q1***
*        BXL      BXR      PW      *      *      K1      K2      K3
DP       2565.0 2740.0 2.0 * * 1 1 1.1
DP       2565.0 2740.0 2.0 * * 1 1 1.1
*        ***DP CARDS USE THE WHOLE BRIDGE OPENING FOR BXL/BXR***
XS      APPR 356
GR       2565 735.4 2565 719.4 2580 714.8 2606 713.8 2624 705.0
GR       2672 705.0 2690 713.5 2695 713.5 2736 733.0 2740 733.0
N        .035      .020      .035
SA              2606      2690
HP 2 BRDGE 715.6 * 715.6 8200
HP 2 BRDGE 718.1 * 718.1 11400
EX
ER

```

WSPRO OUTPUT

***** W S P R O *****

Federal Highway Administration - U. S. Geological Survey

Model for Water-Surface Profile Computations.

Run Date & Time: 7/31/97 8:27 am Version V050196

Input File: LICK.DAT Output File: LICK.LST

T1 U.S. 431 OVER LICK CREEK (I465)431-49-4445
T2 COUNTY: MARION QUAD: MAYWOOD 124A
T3 7-29-97 JOHN T. WILSON
Q 8200 11400

*** Processing Flow Data; Placing Information into Sequence 1 ***

SK .0025 .0025

***** W S P R O *****

Federal Highway Administration - U. S. Geological Survey

Model for Water-Surface Profile Computations.

Input Units: English / Output Units: English

U.S. 431 OVER LICK CREEK (I465)431-49-4445
COUNTY: MARION QUAD: MAYWOOD 124A
7-29-97 JOHN T. WILSON

* Starting To Process Header Record EXIT *

XS EXIT 0
GR 2565 735.4 2565 719.4 2580 714.8 2606 713.8 2624 705.0
GR 2672 705.0 2690 713.5 2695 713.5 2736 733.0 2740 733.0
N .035 .020 .035
SA 2606 2690

*** Completed Reading Data Associated With Header Record EXIT ***

+++072 NOTICE: X-coordinate # 2 increased to eliminate vertical segment.

*** Storing X-Section Data In Temporary File As Record Number 1 ***

*** Data Summary For Header Record EXIT ***

SRD Location: 0. Cross-Section Skew: .0 Error Code 0
Valley Slope: .00000 Averaging Conveyance By Geometric Mean.
Energy Loss Coefficients -> Expansion: .50 Contraction: .00

X,Y-coordinates (10 pairs)

X	Y	X	Y	X	Y
2565.000	735.400	2565.100	719.400	2580.000	714.800
2606.000	713.800	2624.000	705.000	2672.000	705.000
2690.000	713.500	2695.000	713.500	2736.000	733.000
2740.000	733.000				

WSPRO OUTPUT

Minimum and Maximum X,Y-coordinates

Minimum X-Station: 2565.000 (associated Y-Elevation: 735.400)
 Maximum X-Station: 2740.000 (associated Y-Elevation: 733.000)
 Minimum Y-Elevation: 705.000 (associated X-Station: 2672.000)
 Maximum Y-Elevation: 735.400 (associated X-Station: 2565.000)

Roughness Data (3 SubAreas)

SubArea	Roughness Coefficient	Horizontal Breakpoint
1	.035	---
	---	2606.000
2	.020	---
	---	2690.000
3	.035	---

 * Finished Processing Header Record EXIT *

***** W S P R O *****

Federal Highway Administration - U. S. Geological Survey
 Model for Water-Surface Profile Computations.
 Input Units: English / Output Units: English

U.S. 431 OVER LICK CREEK (I465)431-49-4445
 COUNTY: MARION QUAD: MAYWOOD 124A
 7-29-97 JOHN T. WILSON

 * Starting To Process Header Record FULLV *

XS FULLV 140
 GR 2565 735.4 2565 719.4 2580 714.8 2606 713.8 2624 705.0
 GR 2672 705.0 2690 713.5 2695 713.5 2736 733.0 2740 733.0

*** Completed Reading Data Associated With Header Record FULLV ***
 *** No Roughness Data Input, Propagating From Previous Section ***
 +++072 NOTICE: X-coordinate # 2 increased to eliminate vertical segment.
 *** Storing X-Section Data In Temporary File As Record Number 2 ***

*** Data Summary For Header Record FULLV ***
 SRD Location: 140. Cross-Section Skew: .0 Error Code 0
 Valley Slope: .00000 Averaging Conveyance By Geometric Mean.
 Energy Loss Coefficients -> Expansion: .50 Contraction: .00

WSPRO OUTPUT

X,Y-coordinates (10 pairs)					
X	Y	X	Y	X	Y
2565.000	735.400	2565.100	719.400	2580.000	714.800
2606.000	713.800	2624.000	705.000	2672.000	705.000
2690.000	713.500	2695.000	713.500	2736.000	733.000
2740.000	733.000				

Minimum and Maximum X,Y-coordinates
 Minimum X-Station: 2565.000 (associated Y-Elevation: 735.400)
 Maximum X-Station: 2740.000 (associated Y-Elevation: 733.000)
 Minimum Y-Elevation: 705.000 (associated X-Station: 2672.000)
 Maximum Y-Elevation: 735.400 (associated X-Station: 2565.000)

Roughness Data (3 SubAreas)		
SubArea	Roughness Coefficient	Horizontal Breakpoint
1	.035	---
	---	2606.000
2	.020	---
	---	2690.000
3	.035	---

 * Finished Processing Header Record FULLV *

***** W S P R O *****
 Federal Highway Administration - U. S. Geological Survey
 Model for Water-Surface Profile Computations.
 Input Units: English / Output Units: English

 U.S. 431 OVER LICK CREEK (I465)431-49-4445
 COUNTY: MARION QUAD: MAYWOOD 124A
 7-29-97 JOHN T. WILSON

 * Starting To Process Header Record BRDGE *

BR	BRDGE	140	735					
GR		2565	735.4	2565	719.4	2580	714.8	2606 713.8 2624 705.0
GR		2672	705.0	2690	713.5	2695	713.5	2736 733.0 2740 733.0
GR		2740	734.6	2565	735.4			
N		.035		.020		.035		
SA		2606		2690				
PD		707.3	2.0	1				
PD		711.0	2.0	2				
PD		711.0	4.0	3				
CD		3	76	2	737			

WSPRO OUTPUT

```

***    Completed Reading Data Associated With Header Record BRDGE    ***
+++072 NOTICE:  X-coordinate # 2 increased to eliminate vertical segment.
+++072 NOTICE:  X-coordinate #11 increased to eliminate vertical segment.
***    Storing Bridge Data In Temporary File As Record Number  3    ***

```

```

***                               Data Summary For Bridge Record BRDGE                               ***
SRD Location:      140.    Cross-Section Skew:      .0    Error Code    0
Valley Slope:     *****    Averaging Conveyance By Geometric Mean.
Energy Loss Coefficients ->    Expansion:      .50    Contraction:      .00

```

X,Y-coordinates (12 pairs)					
X	Y	X	Y	X	Y
2565.000	735.400	2565.100	719.400	2580.000	714.800
2606.000	713.800	2624.000	705.000	2672.000	705.000
2690.000	713.500	2695.000	713.500	2736.000	733.000
2740.000	733.000	2740.100	734.600	2565.000	735.400

```

Minimum and Maximum X,Y-coordinates
Minimum X-Station:      2565.000  ( associated Y-Elevation:  735.400 )
Maximum X-Station:      2740.100  ( associated Y-Elevation:  734.600 )
Minimum Y-Elevation:     705.000  ( associated X-Station:    2672.000 )
Maximum Y-Elevation:     735.400  ( associated X-Station:    2565.000 )

```

```

Roughness Data ( 3 SubAreas )
      Roughness    Horizontal
SubArea Coefficient Breakpoint
-----
      1          .035      ---
      ---          2606.000
      2          .020      ---
      ---          2690.000
      3          .035      ---
-----

```

```

Discharge coefficient parameters
BRType  BRWidth  EMBSS  EMBElv  UserCD
3        76.000   2.00   737.000  *****

```

```

Pressure flow elevations
      AVBCEL      PFElev
*****          735.000

```

```

Abutment Parameters
ABSLPL  ABSLPR  XTOELT  YTOELT  XTOERT  YTOERT
*****  *****  *****  *****  *****  *****

```


WSPRO OUTPUT

Pier/Pile Data (3 Group(s))
 Code Indicates Bridge Uses Piers
 Group Elevation Gross Width Number

1	707.300	2.000	1
2	711.000	2.000	2
3	711.000	4.000	3

* Finished Processing Header Record BRDGE *

***** W S P R O *****

Federal Highway Administration - U. S. Geological Survey
 Model for Water-Surface Profile Computations.
 Input Units: English / Output Units: English

U.S. 431 OVER LICK CREEK (I465)431-49-4445
 COUNTY: MARION QUAD: MAYWOOD 124A
 7-29-97 JOHN T. WILSON

DC 0 BRDGE	2578	2699	2606	2690	*	4
DP	2565.0	2740.0	2.0	*	*	1 1 1.1
DP	2565.0	2740.0	2.0	*	*	1 1 1.1

* Starting To Process Header Record APPR *

XS	APPR	356						
GR		2565	735.4	2565	719.4	2580	714.8	2606 713.8 2624 705.0
GR		2672	705.0	2690	713.5	2695	713.5	2736 733.0 2740 733.0
N		.035		.020		.035		
SA			2606		2690			

*** Completed Reading Data Associated With Header Record APPR ***

+++072 NOTICE: X-coordinate # 2 increased to eliminate vertical segment.

*** Storing X-Section Data In Temporary File As Record Number 4 ***

*** Data Summary For Header Record APPR ***

SRD Location: 356. Cross-Section Skew: .0 Error Code 0
 Valley Slope: .00000 Averaging Conveyance By Geometric Mean.
 Energy Loss Coefficients -> Expansion: .50 Contraction: .00

X,Y-coordinates (10 pairs)

X	Y	X	Y	X	Y
2565.000	735.400	2565.100	719.400	2580.000	714.800
2606.000	713.800	2624.000	705.000	2672.000	705.000
2690.000	713.500	2695.000	713.500	2736.000	733.000
2740.000	733.000				

WSPRO OUTPUT

Minimum and Maximum X,Y-coordinates

Minimum X-Station: 2565.000 (associated Y-Elevation: 735.400)
 Maximum X-Station: 2740.000 (associated Y-Elevation: 733.000)
 Minimum Y-Elevation: 705.000 (associated X-Station: 2672.000)
 Maximum Y-Elevation: 735.400 (associated X-Station: 2565.000)

Roughness Data (3 SubAreas)

SubArea	Roughness Coefficient	Horizontal Breakpoint
1	.035	---
	---	2606.000
2	.020	---
	---	2690.000
3	.035	---

Bridge datum projection(s): XREFLT XREFRT FDSTLT FDSTRT

 * Finished Processing Header Record APPR *

***** W S P R O *****

Federal Highway Administration - U. S. Geological Survey
 Model for Water-Surface Profile Computations.
 Input Units: English / Output Units: English

U.S. 431 OVER LICK CREEK (I465)431-49-4445
 COUNTY: MARION QUAD: MAYWOOD 124A
 7-29-97 JOHN T. WILSON

HP 2 BRDGE 715.6 * 715.6 8200
 HP 2 BRDGE 718.1 * 718.1 11400
 EX

=====

* Summary of Boundary Condition Information *

=====

#	Reach Discharge	Water Surface Elevation	Friction Slope	Flow Regime
1	8200.00	*****	.0025	Sub-Critical
2	11400.00	*****	.0025	Sub-Critical

=====

* Beginning 2 Profile Calculation(s) *

=====

WSPRO OUTPUT

***** W S P R O *****
 Federal Highway Administration - U. S. Geological Survey
 Model for Water-Surface Profile Computations.
 Input Units: English / Output Units: English

U.S. 431 OVER LICK CREEK (I465)431-49-4445
 COUNTY: MARION QUAD: MAYWOOD 124A
 7-29-97 JOHN T. WILSON

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: EXIT	714.087	2.827	8200.000	611.999	*****	2598.532
Header Type: XS	716.914	*****	13.399	163881.50	*****	2696.235
SRD: .000	713.495	*****	.950	*****	1.013	*****

==125 FR# EXCEEDS FNTEST AT SECID "FULLV": TRIALS CONTINUED.
 FNTEST, FR#, WSEL, CRWS: .80 .86 715.00 713.50

==110 WSEL NOT FOUND AT SECID "FULLV": REDUCED DELTAY.
 WSLIM1, WSLIM2, DELTAY: 713.50 735.40 .50

==115 WSEL NOT FOUND AT SECID "FULLV": USED WSMIN = CRWS.
 WSLIM1, WSLIM2, CRWS: 713.50 735.40 713.50

Section: FULLV	714.995	2.201	8200.000	711.867	140.000	2579.368
Header Type: FV	717.196	.286	11.519	200513.20	140.000	2698.144
SRD: 140.000	713.495	.000	.857	.0020	1.067	-.005

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

Section: APPR	715.656	1.847	8200.000	791.454	216.000	2577.229
Header Type: AS	717.503	.315	10.361	229837.90	216.000	2699.532
SRD: 356.000	713.495	.000	.755	.0015	1.106	-.009

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

<<< The Following Data Reflect The "Constricted" Profile >>>

<<< Beginning Bridge/Culvert Hydraulic Computations >>>

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: BRDGE	715.405	1.806	8200.000	760.943	140.000	2578.042
Header Type: BR	717.211	.271	10.776	218447.00	140.000	2699.005
SRD: 140.000	713.636	.019	.758	*****	1.000	-.011

Specific Bridge Information	C	P/A	PFELEV	BLEN	XLAB	XRAB
Bridge Type 3 Flow Type 1	-----	-----	-----	-----	-----	-----
Pier/Pile Code 0	1.0000	.033	735.000	*****	*****	*****

WSPRO OUTPUT

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: APPR	715.789	1.784	8200.000	807.865	140.000	2576.795
Header Type: AS	717.574	.188	10.150	236036.60	140.103	2699.813
SRD: 356.000	713.495	.178	.737	.0015	1.114	-.008

Approach Section APPR Flow Contraction Information					
M(G)	M(K)	KQ	XLKQ	XRKQ	OTEL
.029	.000	236484.1	2577.911	2698.934	715.789

<<< End of Bridge Hydraulics Computations >>>

***** W S P R O *****
 Federal Highway Administration - U. S. Geological Survey
 Model for Water-Surface Profile Computations.
 Input Units: English / Output Units: English

 U.S. 431 OVER LICK CREEK (I465)431-49-4445
 COUNTY: MARION QUAD: MAYWOOD 124A
 7-29-97 JOHN T. WILSON

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: EXIT	715.615	3.608	11400.000	786.443	*****	2577.362
Header Type: XS	719.222	*****	14.496	227954.80	*****	2699.446
SRD: .000	715.428	*****	1.058	*****	1.104	*****

===125 FR# EXCEEDS FNTEST AT SECID "FULLV": TRIALS CONTINUED.
 FNTEST, FR#, WSEL, CRWS: .80 .85 716.83 715.43

===110 WSEL NOT FOUND AT SECID "FULLV": REDUCED DELTAY.
 WSLIM1, WSLIM2, DELTAY: 715.43 735.40 .50

===115 WSEL NOT FOUND AT SECID "FULLV": USED WSMIN = CRWS.
 WSLIM1, WSLIM2, CRWS: 715.43 735.40 715.43

Section: FULLV	716.825	2.672	11400.000	938.147	140.000	2573.441
Header Type: FV	719.497	.278	12.152	286864.80	140.000	2701.991
SRD: 140.000	715.428	.000	.855	.0020	1.163	-.004

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

Section: APPR	717.515	2.279	11400.000	1028.088	216.000	2571.206
Header Type: AS	719.794	.303	11.089	323457.40	216.000	2703.441
SRD: 356.000	715.428	.000	.766	.0014	1.192	-.005

WSPRO OUTPUT

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

<<< The Following Data Reflect The "Constricted" Profile >>>

<<< Beginning Bridge/Culvert Hydraulic Computations >>>

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: BRDGE	717.949	1.714	11400.000	1085.945	140.000	2569.802
Header Type: BR	719.662	.238	10.498	347575.60	140.000	2704.353
SRD: 140.000	715.629	.210	.651	*****	1.000	.019

Specific Bridge Information	C	P/A	PFELEV	BLN	XLAB	XRAB
Bridge Type 3 Flow Type 1						
Pier/Pile Code 0	1.0000	.032	735.000	*****	*****	*****

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: APPR	717.836	2.123	11400.000	1070.888	140.000	2570.165
Header Type: AS	719.959	.159	10.645	341257.00	140.104	2704.118
SRD: 356.000	715.428	.132	.729	.0014	1.204	.006

Approach Section APPR Flow Contraction Information					
M(G)	M(K)	KQ	XLKQ	XRKQ	OTEL
.028	.000	340905.1	2569.761	2704.211	717.836

<<< End of Bridge Hydraulics Computations >>>

***** W S P R O *****

Federal Highway Administration - U. S. Geological Survey

Model for Water-Surface Profile Computations.

Input Units: English / Output Units: English

U.S. 431 OVER LICK CREEK	(I465)431-49-4445
COUNTY: MARION	QUAD: MAYWOOD 124A
7-29-97	JOHN T. WILSON

WSPRO OUTPUT

*** Beginning Velocity Distribution For Header Record BRDGE ***
 SRD Location: 140.000 Header Record Number 3

Water Surface Elevation: 715.600 Element # 1
 Flow: 8200.000 Velocity: 10.45 Hydraulic Depth: 6.431
 Cross-Section Area: 784.67 Conveyance: 227290.30
 Bank Stations -> Left: 2577.409 Right: 2699.415

X STA.	2577.4	2617.5	2622.5	2626.1	2629.4	2632.6
A(I)		87.6	43.4	37.7	34.7	34.2
V(I)		4.68	9.44	10.89	11.83	12.00
D(I)		2.19	8.63	10.45	10.60	10.60

X STA.	2632.6	2635.7	2638.8	2642.0	2645.0	2648.1
A(I)		33.4	32.8	33.1	32.7	32.7
V(I)		12.26	12.50	12.40	12.55	12.55
D(I)		10.60	10.60	10.60	10.60	10.60

X STA.	2648.1	2651.2	2654.2	2657.3	2660.5	2663.6
A(I)		32.4	32.4	32.8	33.2	33.4
V(I)		12.64	12.64	12.50	12.33	12.29
D(I)		10.60	10.60	10.60	10.60	10.60

X STA.	2663.6	2666.8	2670.1	2673.6	2678.6	2699.4
A(I)		34.1	34.6	36.3	43.3	69.9
V(I)		12.02	11.86	11.29	9.46	5.86
D(I)		10.60	10.60	10.43	8.67	3.36

***** W S P R O *****

Federal Highway Administration - U. S. Geological Survey
 Model for Water-Surface Profile Computations.
 Input Units: English / Output Units: English

U.S. 431 OVER LICK CREEK (I465)431-49-4445
 COUNTY: MARION QUAD: MAYWOOD 124A
 7-29-97 JOHN T. WILSON

*** Beginning Velocity Distribution For Header Record BRDGE ***
 SRD Location: 140.000 Header Record Number 3

Water Surface Elevation: 718.100 Element # 1
 Flow: 11400.000 Velocity: 10.30 Hydraulic Depth: 8.174
 Cross-Section Area: 1106.38 Conveyance: 356195.70
 Bank Stations -> Left: 2569.311 Right: 2704.672

X STA.	2569.3	2611.8	2618.9	2623.5	2627.1	2630.6
A(I)		149.6	63.3	53.9	46.7	46.1
V(I)		3.81	9.01	10.58	12.20	12.36
D(I)		3.52	8.88	11.74	13.08	13.10

X STA.	2630.6	2634.0	2637.4	2640.8	2644.2	2647.6
A(I)		45.1	44.3	44.6	44.1	44.1
V(I)		12.63	12.88	12.78	12.93	12.93
D(I)		13.10	13.10	13.10	13.10	13.10

WSPRO OUTPUT

X STA.	2647.6	2650.9	2654.3	2657.7	2661.0	2664.4
A(I)		44.0	44.0	44.5	44.1	43.9
V(I)		12.96	12.97	12.81	12.91	12.98
D(I)		13.10	13.10	13.10	13.10	13.10

X STA.	2664.4	2667.9	2671.4	2675.6	2681.2	2704.7
A(I)		45.8	46.4	51.4	57.0	103.6
V(I)		12.46	12.28	11.08	10.01	5.50
D(I)		13.10	13.10	12.37	10.07	4.42

***** W S P R O *****

Federal Highway Administration - U. S. Geological Survey

Model for Water-Surface Profile Computations.

Input Units: English / Output Units: English

U.S. 431 OVER LICK CREEK (I465)431-49-4445

COUNTY: MARION

QUAD: MAYWOOD 124A

7-29-97

JOHN T. WILSON

*** Live-Bed Contraction Scour Calculations for Header Record BRDGE ***

Constants and Input Variables

Bed Material Transport Mode Factor (k1): .64

Total Pier Width Value (Pw): 4.000

#	Scour Depth	-- Flow --		-- Width --		--- X-Limits ---	
		Contract	Approach	Contract	Approach	Side	Contract Approach
1	-1.625	8200.000	8091.780	117.000	84.000	Left: 2578.000	2606.000
 Approach Channel Depth:		8.936 Right:		2699.000	2690.000
* Negative Scour Depth Encountered - Check If Variables Are Reasonable *							
2	-1.806	11400.000	10973.390	117.000	84.000	Left: 2578.000	2606.000
 Approach Channel Depth:		10.983 Right:		2699.000	2690.000
* Negative Scour Depth Encountered - Check If Variables Are Reasonable *							

***** W S P R O *****

Federal Highway Administration - U. S. Geological Survey

Model for Water-Surface Profile Computations.

Input Units: English / Output Units: English

U.S. 431 OVER LICK CREEK (I465)431-49-4445

COUNTY: MARION

QUAD: MAYWOOD 124A

7-29-97

JOHN T. WILSON

*** Pier Scour Calculations for Header Record BRDGE ***

Constants and Input Variables

WSPRO OUTPUT

Pier Width: 2.000

```

*-----*
Pier Shape Factor          (K1): 1.00
Flow Angle of Attack Factor (K2): 1.00
Bed Condition Factor       (K3): 1.10
Bed Material Factor        (K4): 1.00
Velocity Multiplier        (VM): 1.00
Depth Multiplier           (YM): 1.00
*-----*

```

#	----- Localized Hydraulic Properties -----						-- X-Stations --	
	Scour Depth	Flow	WSE	Depth	Velocity	Froude #	Left	Right
1	6.70	8200.000	715.582	10.582	12.631	.684	2565.000	2740.000
2	6.98	11400.000	718.081	13.081	13.017	.634	2565.000	2740.000

```

***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English
*-----*
U.S. 431 OVER LICK CREEK      (I465)431-49-4445
COUNTY: MARION              QUAD: MAYWOOD 124A
7-29-97                      JOHN T. WILSON

```

*** Pier Scour Calculations for Header Record BRDGE ***

Constants and Input Variables

Pier Width: 2.000

```

*-----*
Pier Shape Factor          (K1): 1.00
Flow Angle of Attack Factor (K2): 1.00
Bed Condition Factor       (K3): 1.10
Bed Material Factor        (K4): 1.00
Velocity Multiplier        (VM): 1.00
Depth Multiplier           (YM): 1.00
*-----*

```

#	----- Localized Hydraulic Properties -----						-- X-Stations --	
	Scour Depth	Flow	WSE	Depth	Velocity	Froude #	Left	Right
1	6.70	8200.000	715.582	10.582	12.631	.684	2565.000	2740.000
2	6.98	11400.000	718.081	13.081	13.017	.634	2565.000	2740.000

ER

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***** Normal end of WSPRO execution. *****
***** Elapsed Time: 0 Minutes 56 Seconds *****

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