

SCOUR ANALYSIS AND REPORTING FORM

Bridge Structure No. 50183196 Date 6/20/12 Initials Lat Region (A B C D) (D)
 Site _____ Location Russell St + Big Sioux River (SD38E)
 Q₁₀₀ = _____ by: drainage area ratio _____ flood freq. anal. _____ regional regression eq. _____
 Bridge discharge (Q₂) = _____ (should be Q₁₀₀ unless there is a relief bridge, road overflow, or bridge overtopping)

Analytical Procedure for Estimating Hydraulic Variables Needed to Apply Method

Bridge Width = _____ ft. Flow angle at bridge = _____ ° Abut. Skew = _____ ° Effective Skew = _____ °
 Width (W₂) iteration = _____
 Avg. flow depth at bridge, y₂ iteration = _____
 Corrected channel width at bridge Section = W₂ times cos of flow angle = _____ ft* q₂ = Q₂/W₂ = _____ ft²/s
 Bridge Vel, V₂ = _____ ft/s Final y₂ = q₂/V₂ = _____ ft Δh = _____ ft
 Average main channel depth at approach section, y₁ = Δh + y₂ = _____ ft
 * NOTE: repeat above calculations until y₂ changes by less than 0.2 Effective pier width = L sin(a) + a cos(a)
 If y₂ is above LS, then account for Road Overflow using PRGM: RDOVREGA, RDOVREGB, RDOVREGC, or RDOVREGD.

Water Surface Elev. = _____ ft
 Low Steel Elev. = _____ ft
 n (Channel) = _____
 n (LOB) = _____
 n (ROB) = _____
 Pier Width = _____ ft
 Pier Length = _____ ft
 # Piers for 100 yr = _____ ft

see 501831965 for calculations.

CONTRACTION SCOUR

Width of main channel at approach section W₁ = _____ ft
 Width of left overbank flow at approach, W_{lob} = _____ ft Average left overbank flow depth, y_{lob} = _____ ft
 Width of right overbank flow at approach, W_{rob} = _____ ft Average right overbank flow depth, y_{rob} = _____ ft

Live Bed Contraction Scour (use if bed material is small cobbles or finer)

x = _____ From Figure 9 W₂ (effective) = _____ ft y_{cs} = _____ ft

Clear Water Contraction Scour (use if bed material is larger than small cobbles)

Estimated bed material D₅₀ = _____ ft Average approach velocity, V₁ = Q₁₀₀/(y₁W₁) = _____ ft/s

Critical approach velocity, V_c = 11.17y₁^{1/6}D₅₀^{1/3} = _____ ft/s

If V₁ < V_c and D₅₀ >= 0.2 ft, use clear water equation below, otherwise use live bed scour equation above.

D_{c50} = 0.0006(q₂/y₁^{7/6})³ = _____ ft If D₅₀ >= D_{c50}, χ = 0.0

Otherwise, χ = 0.122y₁[q₂/(D₅₀^{1/3}y₁^{7/6})]^{6/7} - y₁ = _____ From Figure 10, y_{cs} = _____ ft

PIER SCOUR CALCULATIONS

L/a ratio = _____ Correction factor for flow angle of attack (from Table 1), K₂ = _____
 Froude # at bridge = _____ Using pier width a on Figure 11, ξ = _____ Pier scour y_{ps} = _____ ft

ABUTMENT SCOUR CALCULATIONS

Average flow depth blocked by: left abutment, y_{aLT} = _____ ft right abutment, y_{aRT} = _____ ft
 Shape coefficient K₁ = 1.00 for vertical-wall, 0.82 for vertical-wall with wingwalls, 0.55 for spill-through
 Using values for y_{aLT} and y_{aRT} on figure 12, ψ_{LT} = _____ and ψ_{RT} = _____
 Left abutment scour, y_{as} = ψ_{LT}(K₁/0.55) = _____ ft Right abutment scour y_{as} = ψ_{RT}(K₁/0.55) = _____ ft

PGRM: "RegionA", "RegionB", "RegionC", or "RegionD"

PGRM: Contract

PGRM: CWCNEW

PGRM: Pier

PGRM: Abutment

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 Pier Width = _____ ft
 Pier Length = _____ ft
 # Piers for 500 yr = _____ ft

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PGRM: Contract

PGRM: CWCNEW

PGRM: Pie

PGRM: Abutment

43,56575

96.76513

Handwritten notes at the bottom of the page, including the word "open" and other illegible scribbles.

(SD38E)

Route Russell St Stream Big Sioux River MRM _____ Date 6/20/12 Initials RAT
Bridge Structure No. 50183196 Location Big Sioux River & Russell St on (SD38E)
GPS coordinates: _____ taken from: USL abutment _____ centerline of \uparrow MRM end _____
Datum of coordinates: WGS84 _____ NAD27 _____

Drainage area = _____ sq. mi.
The average bottom of the main channel was _____ ft below top of guardrail at a point _____ ft from left abutment.
Method used to determine flood flows: _____ Freq. Anal. _____ drainage area ratio _____ regional regression equations.

MISCELLANEOUS CONSIDERATIONS

Flows	Q ₁₀₀ =			Q ₅₀₀ =		
Estimated flow passing through bridge						
Estimated road overflow & overtopping						
Consideration	Yes	No	Possibly	Yes	No	Possibly
Chance of overtopping						
Chance of Pressure flow						
Armored appearance to channel						
Lateral instability of channel						

3130.07
6118
2 4770
5 11500
10 17800
25 27600
50 36500
100 44600
500 74300

Riprap at abutments? Yes No Marginal *mostly on right abutment*
Evidence of past Scour? Yes No Don't know *contraction/pier*
Debris Potential? High Med Low

Does scour countermeasure(s) appear to have been designed?
Riprap Yes No Don't know NA *rose quartz on right abutment*
Spur Dike Yes No Don't know NA
Other _____ Yes No Don't know NA

Bed Material Classification Based on Median Particle Size (D₅₀)

Material Silt/Clay Sand _____ Gravel _____ Cobbles _____ Boulders _____
Size range, in mm <0.062 0.062-2.00 2.00-64 64-250 >250

Comments, Diagrams & orientation of digital photos

see 5018196 for calculations & pictures.

Summary of Results

	Q100	Q500
Bridge flow evaluated		
Flow depth at left abutment (yaLT), in feet		
Flow depth at right abutment (yaRT), in feet		
Contraction scour depth (y _{cs}), in feet		
Pier scour depth (y _{ps}), in feet		
Left abutment scour depth (y _{as}), in feet		
Right abutment scour depth (y _{as}), in feet		
Flow angle of attack		

See Comments/Diagram for justification where required