OK RT

SCOUR ANALYSIS AND REPORTING FORM

Bridge Structure No. 55152270 Date 8/3/12 Initials RAT Region (A B C D)

Site _____ Location 3.7 mi E of Ext 224 on 127 St

Q_{100} = 3070 by: drainage area ratio____ flood freq. anal. ____ regional regression eq. X

Bridge discharge (Q_2) = 3070 (should be Q_{100} unless there is a relief bridge, road overflow, or bridge overtopping)

Analytical Procedure for Estimating Hydraulic Variables Needed to Apply Method

PGRM: "RegionA", "RegionB", "RegionC", or "RegionD"

Bridge Width = 59 ft. Flow angle at bridge = 10 ° Abut. Skew = 0 ° Effective Skew = 10 °

Width (W_2) iteration = _____ _____ _____ _____

Avg. flow depth at bridge, y_2 iteration = _____ _____ _____ _____

Corrected channel width at bridge Section = W_2 times cos of flow angle = 58.1 ft* $q_2 = Q_2/W_2$ = 52.8 ft^2/s

Bridge Vel, V_2 = 5.2 ft/s Final $y_2 = q_2/V_2$ = 10.3 ft Δh = 0.5 ft

Average main channel depth at approach section, $y_1 = \Delta h + y_2$ = 10.8 ft

** NOTE: repeat above calculations until y_2 changes by less than 0.2* *Effective pier width = L sin(q) + a cos(q)*

If y_2 is above LS, then account for Road Overflow using PRGM: RDOVREGA, RDOVREGB, RDOVREGC, or RDOVREGD,

0.6

Water Surface Elev. = 02.4 ft

Low Steel Elev. = 11.9 ft

n (Channel) = 0.035

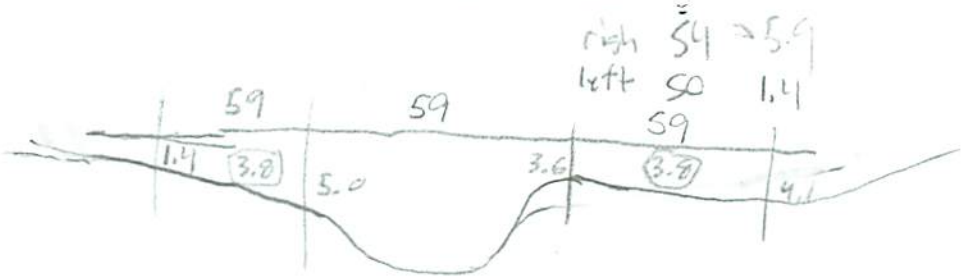
n (LOB) = 0.040

n (ROB) = 0.025

Pier Width = 1.35 ft

Pier Length = 1.35 ft

Piers for 100 yr = 2 ft



CONTRACTION SCOUR

PGRM: Contract

Width of main channel at approach section W_1 = 59 ft

Width of left overbank flow at approach, W_{lob} = 59 ft Average left overbank flow depth, y_{lob} = 3.8 ft

Width of right overbank flow at approach, W_{rob} = 59 ft Average right overbank flow depth, y_{rob} = 3.8 ft

Live Bed Contraction Scour (use if bed material is small cobbles or finer)

x = 4.48 From Figure 9 W_2 (effective) = 55.4 ft y_{cs} = 5.1 ft

PGRM: CWCSNEW

Clear Water Contraction Scour (use if bed material is larger than small cobbles)

Estimated bed material D_{50} = _____ ft Average approach velocity, $V_1 = Q_{100}/(y_1W_1)$ = _____ ft/s

Critical approach velocity, $Vc = 11.17y_1^{1/6}D_{50}^{1/3}$ = _____ ft/s

If $V_1<V_c$ and D_{50} >= 0.2 ft, use clear water equation below, otherwise use live bed scour equation above.

$D_{c50} = 0.0006(q_2/y_1^{7/6})^3$ = _____ ft If $D_{50} >= D_{c50}$, $\chi = 0.0$

Otherwise, $\chi = 0.122y_1[q_2/(D_{50}^{1/3}y_1^{7/6})]^{6/7} - y_1$ = _____ From Figure 10, y_{cs} = _____ ft

PGRM: Pier

PIER SCOUR CALCULATIONS

L/a ratio = 1 Correction factor for flow angle of attack (from Table 1), K2 = 1

Froude # at bridge = 0.29 Using pier width a on Figure 11, ξ = 6 Pier scour y_{ps} = 4.9 ft

PGRM: Abutment

ABUTMENT SCOUR CALCULATIONS

Average flow depth blocked by: left abutment, y_{aLT} = 3.8 ft right abutment, y_{aRT} = 3.8 ft

Shape coefficient K_1= 1.00 for vertical-wall, 0.82 for vertical-wall with wingwalls, 0.55 for spill-through

Using values for y_{aLT} and y_{aRT} on figure 12, ψ_{LT} = 12.9 and ψ_{RT} = 12.9

Left abutment scour, $y_{as} = \psi_{LT}(K_1/0.55)$ = 23.5 ft Right abutment scour $y_{as} = \psi_{RT}(K_1/0.55)$ = 23.5 ft

SCOUR ANALYSIS AND REPORTING FORM

Bridge Structure No. 55152270 Date 8/3/12 Initials RaT Region (A B C D)

Site _____ Location 3.7 mi E of Exit 224 on 127 St

Q_{500} = 5020 by: drainage area ratio ____ flood freq. anal. ____ regional regression eq. X

Bridge discharge (Q_2) = 4135 (should be Q_{500} unless there is a relief bridge, road overflow, or bridge overtopping)

Analytical Procedure for Estimating Hydraulic Variables Needed to Apply Method

PGRM: "RegionA", "RegionB", "RegionC", or "RegionD"

Bridge Width = 59 ft. Flow angle at bridge = 10 ° Abut. Skew = 0 ° Effective Skew = 10 °

Width (W_2) iteration = _____ _____ _____ _____

Avg. flow depth at bridge, y_2 iteration = _____ _____ _____ _____ _____

Corrected channel width at bridge Section = W_2 times cos of flow angle = 58.1 ft* $q_2 = Q_2/W_2$ = 71.2 ft^2/s

Bridge Vel, V_2 = 6 ft/s Final $y_2 = q_2/V_2$ = 11.9 ft Δh = 0.7 ft

Average main channel depth at approach section, $y_1 = \Delta h + y_2$ = 12.6 ft

** NOTE: repeat above calculations until y_2 changes by less than 0.2* *Effective pier width = L sin(q) + a cos(q)*

If y_2 is above LS, then account for Road Overflow using PRGM: RDOVREGA, RDOVREGB, RDOVREGC, or RDOVREGD,

Water Surface Elev. = 0-24 ft

Low Steel Elev. = ~~11.9~~ ft

n (Channel) = 0.035

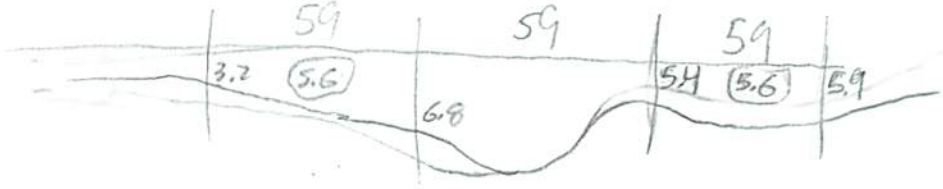
n (LOB) = 0.040

n (ROB) = 0.035

Pier Width = 1.35 ft

Pier Length = 1.35 ft

Piers for 500 yr = 2 ft



CONTRACTION SCOUR

PGRM: Contract

Width of main channel at approach section W_1 = 59 ft

Width of left overbank flow at approach, W_{lob} = 59 ft Average left overbank flow depth, y_{lob} = 5.6 ft

Width of right overbank flow at approach, W_{rob} = 59 ft Average right overbank flow depth, y_{rob} = 5.6 ft

Live Bed Contraction Scour (use if bed material is small cobbles or finer)

x = 7.33 From Figure 9 W_2 (effective) = 55.4 ft y_{cs} = 8.1 ft

PGRM: CWCSNEW

Clear Water Contraction Scour (use if bed material is larger than small cobbles)

Estimated bed material D_{50} = _____ ft Average approach velocity, $V_1 = Q_{500}/(y_1W_1)$ = _____ ft/s

Critical approach velocity, $Vc = 11.17 y_1^{1/6} D_{50}^{1/3}$ = _____ ft/s

If $V_1 < V_c$ and $D_{50} >= 0.2$ ft, use clear water equation below, otherwise use live bed scour equation above.

$D_{c50} = 0.0006(q_2/y_1^{7/6})^3$ = _____ ft If $D_{50} >= D_{c50}$, $\chi = 0.0$

Otherwise, $\chi = 0.122 y_1 [q_2/(D_{50}^{1/3} y_1^{7/6})]^{6/7} - y_1$ = _____ From Figure 10, y_{cs} = _____ ft

PGRM: Pier

PIER SCOUR CALCULATIONS

L/a ratio = _____ Correction factor for flow angle of attack (from Table 1), K2 = 1

Froude # at bridge = 0.31 Using pier width a on Figure 11, ξ = 6 Pier scour y_{ps} = 5 ft

PGRM: Abutment

ABUTMENT SCOUR CALCULATIONS

Average flow depth blocked by: left abutment, y_{aLT} = 5.6 ft right abutment, y_{aRT} = 5.6 ft

Shape coefficient K_1= 1.00 for vertical-wall, 0.82 for vertical-wall with wingwalls, 0.55 for spill-through

Using values for y_{aLT} and y_{aRT} on figure 12, ψ_{LT} = 16.1 and ψ_{RT} = 16.1

Left abutment scour, $y_{as} = \psi_{LT}(K_1/0.55)$ = 29.3 ft Right abutment scour $y_{as} = \psi_{RT}(K_1/0.55)$ = 29.3 ft

Route 127 St Stream Jorgenson River MRM _____ Date 8/3/12 Initials RaT

Bridge Structure No. 55152270 Location 3.7 mi E of Exit 224 on 127 St

GPS coordinates: N 45° 32' 36.91" W 96° 54' 49.41" taken from: USL abutment X centerline of ↑ MRM end _____

Datum of coordinates: WGS84 X NAD27 _____

Drainage area = 123.34 sq. mi.

The average bottom of the main channel was 16.3 ft below top of guardrail at a point 30 ft from left abutment.

Method used to determine flood flows: ____Freq. Anal. ____drainage area ratio X regional regression equations.

MISCELLANEOUS CONSIDERATIONS

Flows	Q_{100} = 3070			Q_{500} = 5020		
Estimated flow passing through bridge	3070			4135		
Estimated road overflow & overtopping	0			885		
Consideration	Yes	No	Possibly	Yes	No	Possibly
Chance of overtopping		X		X		
Chance of Pressure flow		X		X		
Armored appearance to channel		X			X	
Lateral instability of channel		X			X	

~~7/3~~ ~~8/26~~

2	219
5	630
10	1040
25	1730
50	2360
100	3070
500	5020

Riprap at abutments? ____Yes X No ____Marginal significant pier/abutment/contracte-

Evidence of past Scour? X Yes ____No ____Don't know

Debris Potential? ____High ____Med X Low

Does scour countermeasure(s) appear to have been designed?

Riprap	____Yes	X No	____Don't know	____NA
Spur Dike	____Yes	X No	____Don't know	____NA
Other _____	____Yes	X No	____Don't know	____NA

Bed Material Classification Based on Median Particle Size (D_{50})

Material	Silt/Clay X	Sand ____	Gravel ____	Cobbles ____	Boulders ____
Size range, in mm	<0.062	0.062-2.00	2.00-64	64-250	>250

Comments, Diagrams & orientation of digital photos

1). left OB
2). main channel
3). right OB
4). pier
5-6). left abutment
7-9). right abutment
10). left abutment
11). main channel

Summary of Results

	Q100	Q500
Bridge flow evaluated	3070	5020
Flow depth at left abutment (yaLT), in feet	3.8	5.6
Flow depth at right abutment (yaRT), in feet	3.8	5.6
Contraction scour depth (ycs), in feet	5.1	8.1
Pier scour depth (yps), in feet	4.4	5
Left abutment scour depth (yas), in feet	~~23.5~~ 23.5	29.3
Right abutment scour depth (yas), in feet	23.5	29.3
1Flow angle of attack	10	10

See Comments/Diagram for justification where required