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SCOUR ANALYSIS AND REPORTING FORM

Bridge Structure No. 63152070 Date 10-11-16 Initials RRL Region (A B C D)

Site _____ Location 1.2 E from Parker

Q_{100} = 11400 by: drainage area ratio ✓ flood freq. anal. ____ regional regression eq. ____

Bridge discharge (Q_2) = 10125 (should be Q_{100} unless there is a relief bridge, road overflow, or bridge overtopping)

Analytical Procedure for Estimating Hydraulic Variables Needed to Apply Method

PGRM: "RegionA", "RegionB", "RegionC", or "RegionD"

Bridge Width = 228 ft. Flow angle at bridge = 20 ° Abut. Skew = 0 ° Effective Skew = 20 °

Width (W_2) iteration = 228 _____ _____ _____

Avg. flow depth at bridge, y_2 iteration = 9.7 _____ _____ _____

Corrected channel width at bridge Section = W_2 times cos of flow angle = 214.25 ft* $q_2 = Q_2/W_2$ = 47.3 ft^2/s

Bridge Vel, V_2 = 4.9 ft/s Final $y_2 = q_2/V_2$ = 9.7 ft Δh = 0.5 ft

Average main channel depth at approach section, $y_1 = \Delta h + y_2$ = 10.2 ft

** NOTE: repeat above calculations until y_2 changes by less than 0.2* *Effective pier width = L sin(q) + a cos(q)*

If y_2 is above LS, then account for Road Overflow using PRGM: RDOVREGA, RDOVREGB, RDOVREGC, or RDOVREGD,

Water Surface Elev. = 1.6 ft

Low Steel Elev. = 9.7 ft

n (Channel) = 0.03

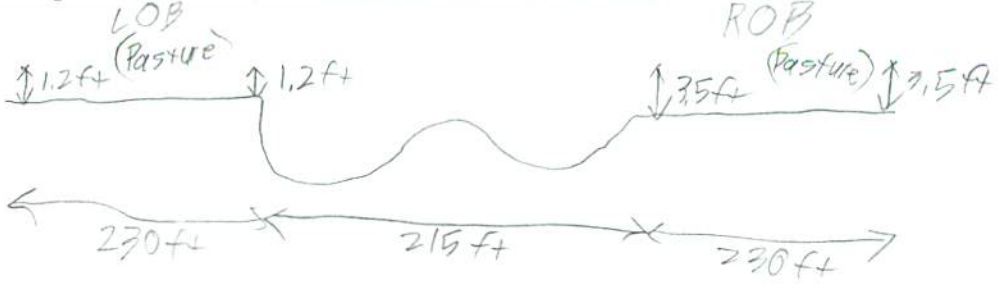
n (LOB) = 0.033

n (ROB) = 0.033

Pier Width = 2 ft

Pier Length = 2 ft

Piers for 100 yr = 3 ft



CONTRACTION SCOUR

PGRM: Contract

Width of main channel at approach section W_1 = 215 ft

Width of left overbank flow at approach, W_{lob} = 230 ft Average left overbank flow depth, y_{lob} = 1.2 ft

Width of right overbank flow at approach, W_{rob} = 230 ft Average right overbank flow depth, y_{rob} = 7.5 ft

Live Bed Contraction Scour (use if bed material is small cobbles or finer)

x = 2.34 From Figure 9 W_2 (effective) = 208.3 ft y_{cs} = 2.9 ft

PGRM: CWCSNEW

Clear Water Contraction Scour (use if bed material is larger than small cobbles)

Estimated bed material D_{50} = _____ ft Average approach velocity, $V_1 = Q_{100}/(y_1W_1)$ = _____ ft/s

Critical approach velocity, $Vc = 11.52y_1^{1/6}D_{50}^{1/3}$ = _____ ft/s

If $V_1 < V_c$ and $D_{50} >= 0.2$ ft, use clear water equation below, otherwise use live bed scour equation above.

$D_{c50} = 0.0006(q_2/y_1^{7/6})^3$ = _____ ft If $D_{50} >= D_{c50}$, $\chi = 0.0$

Otherwise, $\chi = 0.122y_1[q_2/(D_{50}^{1/3}y_1^{7/6})]^{6/7} - y_1$ = _____ From Figure 10, y_{cs} = _____ ft

PGRM: Pier

PIER SCOUR CALCULATIONS

L/a ratio = 1 Correction factor for flow angle of attack (from Table 1), K2 = 1

Froude # at bridge = 0.28 Using pier width a on Figure 11, ξ = 8 Pier scour y_{ps} = 6.6 ft

PGRM: Abutment

ABUTMENT SCOUR CALCULATIONS

Average flow depth blocked by: left abutment, y_{aLT} = 1.2 ft right abutment, y_{aRT} = 3.5 ft

Shape coefficient K_1= 1.00 for vertical-wall, 0.82 for vertical-wall with wingwalls, 0.55 for spill-through

Using values for y_{aLT} and y_{aRT} on figure 12, ψ_{LT} = 5.1 and ψ_{RT} = 12.4

Left abutment scour, $y_{as} = \psi_{LT}(K_1/0.55)$ = 5.1 ft Right abutment scour $y_{as} = \psi_{RT}(K_1/0.55)$ = 12.4 ft

SCOUR ANALYSIS AND REPORTING FORM

Bridge Structure No. 63152070 Date 10-11-10 Initials RRL Region (A B C D)

Site _____ Location 1.2 E From Parker

Q_{500} = 19900 by: drainage area ratio ✓ flood freq. anal. ____ regional regression eq. ____

Bridge discharge (Q_2) = 10125 (should be Q_{500} unless there is a relief bridge, road overflow, or bridge overtopping)

Analytical Procedure for Estimating Hydraulic Variables Needed to Apply Method

PGRM: "RegionA", "RegionB", "RegionC", or "RegionD"

Bridge Width = 228 ft. Flow angle at bridge = 20 ° Abut. Skew = 0 ° Effective Skew = ~~15~~ 20 CW °

Width (W_2) iteration = 228

Avg. flow depth at bridge, y_2 iteration = 9.7

Corrected channel width at bridge Section = W_2 times cos of flow angle = 214.25 ft* $q_2 = Q_2/W_2$ = 47.3 ft^2/s

Bridge Vel, V_2 = ~~4.9~~ 7.9 4.9 ft/s Final $y_2 = q_2/V_2$ = ~~9.7~~ ~~4.7~~ ~~5.9~~ ft Δh = ~~0.5~~ ~~1.3~~ 0.5 ft

Average main channel depth at approach section, $y_1 = \Delta h + y_2$ = ~~10.2~~ ~~7.2~~ 10.2 ft

** NOTE: repeat above calculations until y_2 changes by less than 0.2* *Effective pier width = L sin(q) + a cos(q)*

If y_2 is above LS, then account for Road Overflow using PRGM: RDOVREGA, RDOVREGB, RDOVREGC, or RDOVREGD.

See 100 yr. Diag.

Water Surface Elev. = 1.6 ft

Low Steel Elev. = 9.7 ft

n (Channel) = 0.03

n (LOB) = 0.033

n (ROB) = 0.033

Pier Width = 2 ft

Pier Length = 2 ft

Piers for 500 yr = 3 ft

CONTRACTION SCOUR

PGRM: Contract

Width of main channel at approach section W_1 = 215 ft

Width of left overbank flow at approach, W_{lob} = 230 ft Average left overbank flow depth, y_{lob} = 1.2 ft

Width of right overbank flow at approach, W_{rob} = 230 ft Average right overbank flow depth, y_{rob} = 3.5 ft

Live Bed Contraction Scour (use if bed material is small cobbles or finer)

x = ~~2.24~~ ~~2.77~~ CW 2.34 From Figure 9 W_2 (effective) = 208.3 ft y_{cs} = ~~2.9~~ ~~3.3~~ CW 2.9 ft

PGRM: CWCSNEW

Clear Water Contraction Scour (use if bed material is larger than small cobbles)

Estimated bed material D_{50} = _____ ft Average approach velocity, $V_1 = Q_{500}/(y_1 W_1)$ = _____ ft/s

Critical approach velocity, $Vc = 11.52 y_1^{1/6} D_{50}^{1/3}$ = _____ ft/s

If $V_1 < V_c$ and $D_{50} >= 0.2$ ft, use clear water equation below, otherwise use live bed scour equation above.

$D_{c50} = 0.0006(q_2/y_1^{7/6})^3$ = _____ ft If $D_{50} >= D_{c50}$, $\chi = 0.0$

Otherwise, $\chi = 0.122 y_1 [q_2/(D_{50}^{1/3} y_1^{7/6})]^{6/7} - y_1$ = _____ From Figure 10, y_{cs} = _____ ft

PGRM: Pier

PIER SCOUR CALCULATIONS

L/a ratio = 1 Correction factor for flow angle of attack (from Table 1), K2 = 1

Froude # at bridge = 0.28 ~~0.57~~ CW 0.28 Using pier width a on Figure 11, ξ = 8 Pier scour y_{ps} = ~~6.6~~ ~~7.5~~ CW 6.6 ft

PGRM: Abutment

ABUTMENT SCOUR CALCULATIONS

Average flow depth blocked by: left abutment, y_{aLT} = 1.2 ft right abutment, y_{aRT} = 3.5 ft

Shape coefficient K_1= 1.00 for vertical-wall, 0.82 for vertical-wall with wingwalls, 0.55 for spill-through

Using values for y_{aLT} and y_{aRT} on figure 12, ψ_{LT} = 5.1 and ψ_{RT} = 12.4

Left abutment scour, $y_{as} = \psi_{LT}(K_1/0.55)$ = 5.1 ft Right abutment scour $y_{as} = \psi_{RT}(K_1/0.55)$ = 12.4 ft

Route 275th St Stream W. Fork Vermillion Riv MRM _____ Date 10-11-10 Initials RRL

Bridge Structure No. 63152070 Location 1.2 E from Parker

GPS coordinates: N43° 24.172' W97° 05.950' taken from: USL abutment ✓ centerline of ⇑ MRM end _____

Datum of coordinates: WGS84 _____ NAD27 _____

Drainage area = 398.89 sq. mi.

The average bottom of the main channel was 15.3 ft below top of guardrail at a point 64 ft from left abutment.

Method used to determine flood flows: ___Freq. Anal. ✓ drainage area ratio ___regional regression equations.

MISCELLANEOUS CONSIDERATIONS

Flows	Q_{100} = 11400			Q_{500} = 19900		
Estimated flow passing through bridge	10125			10125		
Estimated road overflow & overtopping	1275			9775		
Consideration	Yes	No	Possibly	Yes	No	Possibly
Chance of overtopping	X			X		
Chance of Pressure flow	X			X		
Armored appearance to channel		X			X	
Lateral instability of channel	X			X		

Riprap at abutments? ___Yes ___No X Marginal

Evidence of past Scour? X Yes ___No ___Don't know

Debris Potential? ___High ___Med X Low

Does scour countermeasure(s) appear to have been designed?

Riprap	X Yes	___No	___Don't know	___NA
Spur Dike	___Yes	X No	___Don't know	___NA
Other _____	___Yes	X No	___Don't know	___NA

Bed Material Classification Based on Median Particle Size (D_{50})

Material	Silt/Clay X	Sand	Gravel	Cobbles	Boulders
Size range, in mm	<0.062	0.062-2.00	2.00-64	64-250	>250

Comments, Diagrams & orientation of digital photos

1- Bridge Deck
2- Upstream
3- Downstream
4- Left Overbank
5- Right Overbank
6- Left Abutment
7- Right Abutment
8- Piers

Summary of Results

	Q100	Q500
Bridge flow evaluated	~~1275~~ 1012.5	~~9775~~ 10125
Flow depth at left abutment (yaLT), in feet	1.7	1.2
Flow depth at right abutment (yaRT), in feet	3.5	3.5
Contraction scour depth (ycs), in feet	2.9	2.9
Pier scour depth (yps), in feet	6.6	6.6
Left abutment scour depth (yas), in feet	5.1	5.1
Right abutment scour depth (yas), in feet	12.4	12.4
1Flow angle of attack	20	20

See Comments/Diagram for justification where required