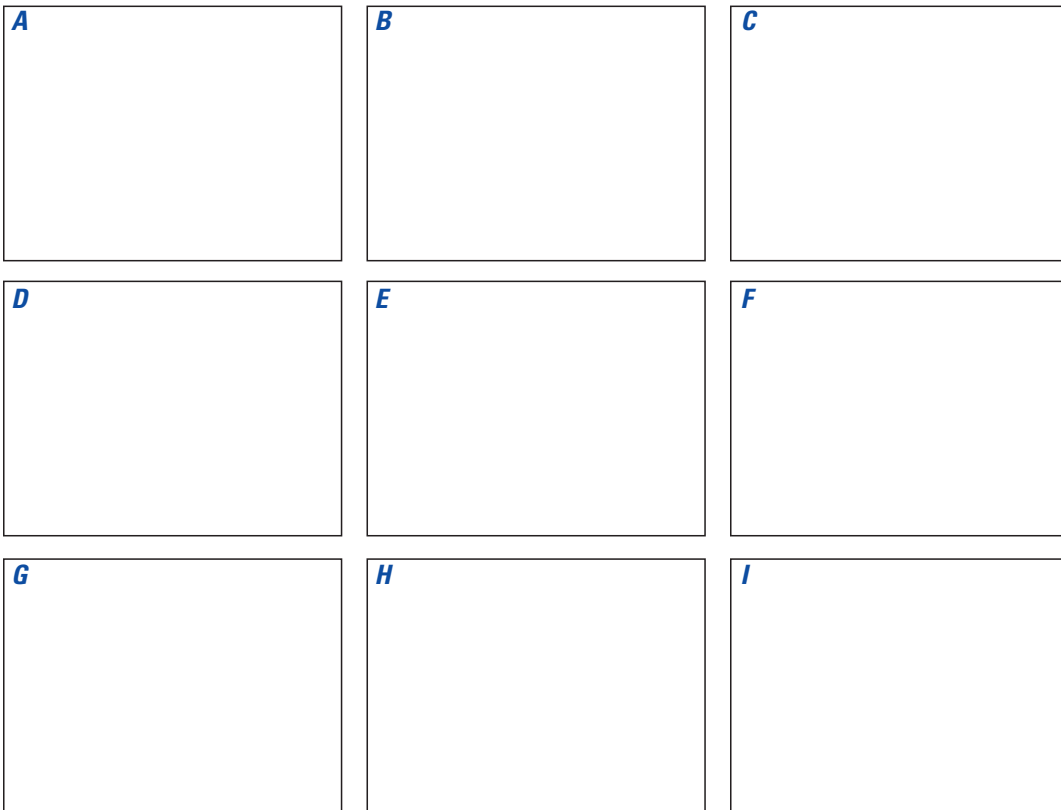


Prepared in cooperation with the Colorado Department of Transportation

Paleoflood Investigations to Improve Peak-Streamflow Regional-Regression Equations for Natural Streamflow in Eastern Colorado, 2015



Scientific Investigations Report 2016–5099



Cover. *A*, Arikaree River at Haigler, Nebr.; *B*, Taylor Arroyo below Rock Crossing near Thatcher, Colo.; *C*, Frijole Creek near Alfalfa, Colo.; *D*, Arikaree River at Haigler, Nebr.; *E*, Taylor Arroyo below Rock Crossing near Thatcher, Colo.; *F*, Little Fountain Creek near Fountain, Colo.; *G*, Frijole Creek near Alfalfa, Colo.; *H*, Frijole Creek near Alfalfa, Colo.; *I*, Big Arroyo near Thatcher, Colo. All cover photographs by Tessa Harden, USGS.

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By Michael S. Kohn, Michael R. Stevens, Tessa M. Harden, Jeanne E. Godaire,
Ralph E. Klinger, and Amanullah Mommandi

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Conversion Factors

Inch/Pound to International System of Units

Multiply	By	To obtain
Length		
inch (in.)	2.54	centimeter (cm)
inch (in.)	25.4	millimeter (mm)
foot (ft)	0.3048	meter (m)
mile (mi)	1.609	kilometer (km)
Area		
square mile (mi ²)	259.0	hectare (ha)
square mile (mi ²)	2.590	square kilometer (km ²)
Volume		
cubic foot (ft ³)	28.32	cubic decimeter (dm ³)
cubic foot (ft ³)	0.02832	cubic meter (m ³)
Flow rate		
cubic foot per second (ft ³ /s)	0.02832	cubic meter per second (m ³ /s)

Vertical coordinate information is referenced to the North American Vertical Datum of 1988 (NAVD 88).

Horizontal coordinate information is referenced to the North American Datum of 1983 (NAD 83).

Elevation, as used in this report, refers to distance above the vertical datum.

Water year in this report is defined as the period from October 1st of one year through September 30th of the following year and is named for the year of the ending date.

Abbreviations and Acronyms

adjR ²	adjusted coefficient of determination
AEP	annual exceedance-probability
AEPD	annual exceedance-probability discharge
CDWR	Colorado Division of Water Resources
DAR	drainage-area ratio
EMA	Expected Moments Algorithm
GLS	Generalized-Least Squares
HEC-RAS	Hydrologic Engineering Center River Analysis System
Mallow's C_p	measure of the total squared error for a subset model containing the number (n) of independent variables
MGB	multiple Grubbs-Beck test
NDNR	Nebraska Department of Natural Resources
NWIS	National Water Information System
OLS	Ordinary-Least Squares
PILF	Potentially Influential Low Flood
PRESS	Predicted Residual Sum of Squares
pseudo R^2	pseudo coefficient of determination
PSRRE	peak-streamflow regional-regression equations
R ²	coefficient of determination
SACGUI	slope-area computation graphical user interface
SEP	standard error of prediction
SME	standard model error
USGS	U.S. Geological Survey
WLS	Weighted-Least Squares

Paleoflood Investigations to Improve Peak-Streamflow Regional-Regression Equations for Natural Streamflow in Eastern Colorado, 2015

By Michael S. Kohn,¹ Michael R. Stevens,¹ Tessa M. Harden,² Jeanne E. Godaire,³ Ralph E. Klinger,³ and Amanullah Mommandi⁴

Abstract

The U.S. Geological Survey (USGS), in cooperation with the Colorado Department of Transportation, developed regional-regression equations for estimating the 50-, 20-, 10-, 4-, 2-, 1-, 0.5-, 0.2-percent annual exceedance-probability discharge (AEPD) for natural streamflow in eastern Colorado. A total of 188 streamgages, consisting of 6,536 years of record and a mean of approximately 35 years of record per streamgage, were used to develop the peak-streamflow regional-regression equations. The estimated AEPDs for each streamgage were computed using the USGS software program PeakFQ. The AEPDs were determined using systematic data through water year 2013. Based on previous studies conducted in Colorado and neighboring States and on the availability of data, 72 characteristics (57 basin and 15 climatic characteristics) were evaluated as candidate explanatory variables in the regression analysis. Paleoflood and non-exceedance bound ages were established based on reconnaissance-level methods. Multiple lines of evidence were used at each streamgage to arrive at a conclusion (age estimate) to add a higher degree of certainty to reconnaissance-level estimates. Paleoflood or non-exceedance bound evidence was documented at 41 streamgages, and 3 streamgages had previously collected paleoflood data. To determine the peak discharge of a paleoflood or non-exceedance bound, two different hydraulic models were used.

The mean standard error of prediction (*SEP*) for all 8 AEPDs was reduced approximately 25 percent compared to the previous flood-frequency study. For paleoflood data to be effective in reducing the *SEP* in eastern Colorado, a larger ratio than 44 of 188 (23 percent) streamgages would need paleoflood data and that paleoflood data would need to increase the record length by more than 25 years for the 1-percent AEPD. The greatest reduction in *SEP* for the peak-streamflow regional-regression equations was observed when additional new basin characteristics were included in the peak-streamflow regional-regression

equations and when eastern Colorado was divided into two separate hydrologic regions. To make further reductions in the uncertainties of the peak-streamflow regional-regression equations in the Foothills and Plains hydrologic regions, additional streamgages or crest-stage gages are needed to collect peak-streamflow data on natural streams in eastern Colorado.

Generalized-Least Squares regression was used to compute the final peak-streamflow regional-regression equations for peak-streamflow. Dividing eastern Colorado into two new individual regions at -104° longitude resulted in peak-streamflow regional-regression equations with the smallest *SEP*. The new hydrologic region located between -104° longitude and the Kansas-Nebraska State line will be designated the Plains hydrologic region and the hydrologic region comprising the rest of eastern Colorado located west of the -104° longitude and east of the Rocky Mountains and below 7,500 feet in the South Platte River Basin and below 9,000 feet in the Arkansas River Basin will be designated the Foothills hydrologic region.

Introduction

The U.S. Geological Survey (USGS), in cooperation with the Colorado Department of Transportation (CDOT), developed peak-streamflow regional-regression equations (PSRREs) for estimating the 50-, 20-, 10-, 4-, 2-, 1-, 0.5-, 0.2-percent annual exceedance-probability discharge (AEPD) for natural streamflow in eastern Colorado. The PSRREs in the Plains hydrologic region in eastern Colorado have the largest uncertainty of any hydrologic region in Colorado (Vaill, 1999; Capesius and Stephens, 2009). Reliable peak-streamflow information is critical for the proper design of stream-related infrastructure, such as bridges and dams, and flood-plain inundation maps. At gaged sites, where sufficient long-term streamflow data have been collected, statistics can be obtained from available publications, by an analysis of available data in the USGS National Water Information System (NWIS) database (<http://dx.doi.org/10.5066/F7P55KJN>), or other sources of flood information. However, estimates also are needed at ungaged sites where no site-specific streamflow data are available. The use of PSRREs with expressions of predictive uncertainty generally represents a reliable and cost-effective means for

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estimating peak streamflow at ungaged sites. The PSRREs are a common tool used to estimate streamflow statistics at ungaged sites across the Nation. The PSRREs are based on statistical relations between (1) peak streamflow from systematic, historic, censored, and paleoflood data at streamgages and (2) basin and climatic characteristics, for which data are typically readily available using a geographic information system (GIS).

Purpose and Scope

The purpose of this report is to present an updated set of PSRREs for estimating AEPDs for use in eastern Colorado. The PSRREs relate AEPDs to drainage basin size, topography, hydrology, and climatology. This report presents two sets of PSRREs to estimate 8 AEPD statistics that have probabilities of 50, 20, 10, 4, 2, 1, 0.5, and 0.2 percent, which are equivalent to annual flood-frequency recurrence intervals of 2, 5, 10, 25, 50, 100, 200, and 500 years, respectively. Hereafter, in this report, these statistics are denoted as $Q_{50\%}$ (in percent [%]), $Q_{20\%}$, $Q_{10\%}$, $Q_{4\%}$, $Q_{2\%}$, $Q_{1\%}$, $Q_{0.5\%}$, and $Q_{0.2\%}$, respectively. The procedure to develop PSRREs included Generalized-Least Squares (GLS) multilinear regression based on base-10 logarithmic transformations of all included variables. The PSRREs were developed both with and without paleoflood data to identify the effect paleoflood data have on the uncertainty of the PSRREs.

Annual peak-discharge data from streamgages with a record of at least 10 years were compiled from the USGS (U.S. Geological Survey, 2013b), Colorado Division of Water Resources (CDWR) (Colorado Division of Water Resources, 2013), and Nebraska Department of Natural Resources (NDNR) (Nebraska Department of Natural Resources, 2013) through water year 2013 as well as paleoflood data. A water year is the 12-month period from October 1 through September 30 designated by the calendar year in which it ends.

The limitations and accuracy of the PSRREs are presented in this report. The study area was extended 100 miles outside Colorado for the purpose of PSRRE development because the hydrology is not affected by State boundaries; however, the PSRREs are only applicable in Colorado. Also, it should be noted that the PSRREs presented in this report are only applicable to natural streamflow with drainage areas between 0.26 and 3,560 square miles (mi²). To clarify, the PSRREs are based on analysis of peak-streamflow data at streams relatively unaffected by anthropogenic activities such as storage, regulation, and diversion or return streamflows from a municipality or mining operation, or urban development in a basin. Kircher and others (1985) provide the most quantitative description of natural streamflow as streamflow from drainage basins relatively unaffected by urban development or water-management activities such as substantial reservoir storage, streamflow diversions, or return streamflows of previously diverted streamflow. Further, those authors defined natural streamflow as streamflow having less than about 10 percent of the mean-annual streamflow volume at the streamgage affected by anthropogenic activity. The definition by Kircher and others (1985) was used in Capesius and Stephens (2009) and this report.

Description of the Study Area

Colorado has a diverse landscape and climate and includes the headwaters of the major river basins of the Colorado, North Platte, South Platte, and Arkansas Rivers and the Rio Grande. The physiographic and hydrologic differences are discussed below.

Colorado can be described by three major physiographic provinces, which trend north to south across the State (Fenneman, 1931). The Great Plains Province, in the eastern 40 percent of the State, consists mostly of grasslands with scattered hills, bluffs, shallow river valleys, and some cultivated areas. The Southern Rocky Mountains Province, west of the Great Plains, includes most of central Colorado from north to south and is characterized by mountain ranges and inter-mountain valleys. The Colorado Plateau Province is in western Colorado between the Utah State line to the west and the Rocky Mountains to the east. The landscape is distinguished by mesas, plateaus, and eroded canyon terrain that includes much of the western quarter of Colorado from north to south. More detailed descriptions of the major physiographic provinces can be found in Fenneman (1931) and Capesius and Stephens (2009).

The five hydrologic regions of Colorado were defined on the basis of the physiographic and climatic characteristics that were used to develop best-fit PSRREs for previous flood-frequency studies (McCain and Jarrett (1976); Kircher and others, 1985; Vaill, 1999; Capesius and Stephens, 2009) (fig. 1). For this report “...a hydrologic region is qualitatively defined as a region of similar hydrology and climatology. The Mountain hydrologic region is identified as that region of central Colorado above about 7,500 feet in elevation located between the Colorado-Wyoming State line and the Rio Grande Basin. The Mountain hydrologic region encompasses the headwaters of most major river basins in Colorado where the annual peak streamflow generally is produced by snowmelt runoff. The Northwest hydrologic region is defined as the northwestern part of Colorado below 7,500 feet and encompassing substantial areas of the Yampa, White, and Gunnison River Basins. The Rio Grande hydrologic region ranges in elevation from about 5,000 feet near the Colorado-New Mexico State line to more than 14,000 feet in the northern parts and encompasses the Rio Grande Basin. The Southwest hydrologic region is defined as the region located south of the Gunnison River Basin and west of the Rio Grande Basin and encompasses the Dolores and San Juan River Basins. The Plains hydrologic region is east of the Rocky Mountains and below 7,500 feet in the South Platte River Basin and below 9,000 feet in the Arkansas River Basin” (Capesius and Stephens, 2009, p. 4). Because hydrology is not affected by state boundaries, the hydrologic region boundaries were extended 100 miles into all States surrounding Colorado to include more streamgages and improve statistical robustness for development of PSRREs. As a result, the study area includes parts of Kansas, Nebraska, New Mexico, Oklahoma, and Wyoming along with the Plains hydrologic region in Colorado.

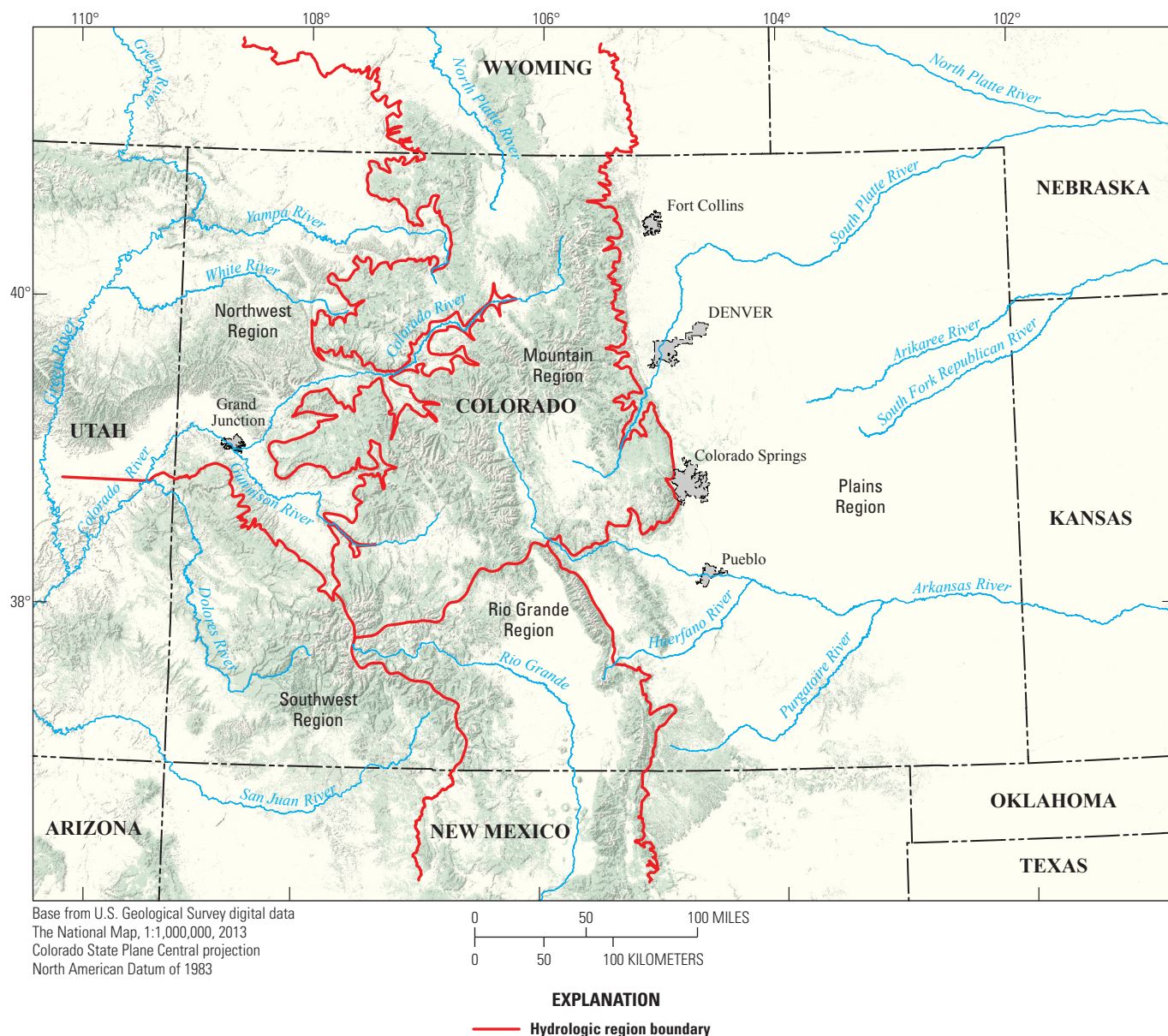


Figure 1. Boundaries of the hydrologic regions from the previous flood-frequency study in Colorado (modified from Capesius and Stephens, 2009).

Previous Studies and Background Information

Previous studies computed PSRREs in Colorado—Patterson (1964, 1965), Patterson and Somers (1966), Matthai (1968), Hedman and others (1972), McCain and Jarrett (1976), Kircher and others (1985), Livingston and Minges (1987), Vaill (1999), and Capesius and Stephens (2009). Fewer studies have developed regional-regression equations for mean-monthly streamflow, as was done by Kircher and others (1985) and Capesius and Stephens (2009). The hydrologic regions used in this report were delineated by McCain and Jarrett (1976) and were incorporated as the regional framework in Kircher and others (1985) and Capesius and Stephens (2009). Kircher and others (1985) developed regional-regression equations for mean-monthly streamflow in western Colorado for

data collected through 1983. Capesius and Stephens (2009) published Statewide peak and non-peak (with the exception of the Plains hydrologic region) statistics (including mean-monthly streamflow) using USGS streamflow data from the beginning of the period of record at each streamgage through water years 2006 and 2007, respectively. In Capesius and Stephens (2009), the PSRREs were developed using USGS systematic and historic data through water year 2006, and the error associated with the PSRREs was characterized using the standard error of prediction (*SEP*, in percent), the pseudo-coefficient of determination (pseudo R^2 , dimensionless), and the standard model error (*SME*, in percent). Systematic peaks are observed during the collection of the systematic streamgage record, and historic streamflow peaks are observed outside the streamgage record.

The *SEP* describes the sum of the model error and sampling error. The *SEP* is the square root of the mean GLS variance of prediction (Tasker and Stedinger, 1989; Eng and others, 2009). The pseudo R^2 value is a measure of the percentage of the variation explained by the basin characteristics (explanatory variables) included in the model and is calculated on the basis of the degrees of freedom in the regression (Eash and others, 2013). Griffis and Stedinger (2007) describe how the pseudo R^2 is more appropriate than the traditional R^2 or adj R^2 in measuring the true variation explained by the explanatory variables in the GLS model. *SME* measures only the error of the model and does not include sampling error regression (Eash and others, 2013). *SME* is the square root of the regression model error variance (Tasker and Stedinger, 1989).

Methods for Data Development for Streamgages

The development of PSRREs in eastern Colorado consists of five steps:

1. Selection of streamgages with natural streamflow conditions for inclusion in multilinear regression analysis,
2. Inclusion of paleoflood investigations conducted at a select number of streamgages where paleoflood evidence exists to provide additional streamflow data to supplement systematic data,
3. Flood-frequency analysis to compute AEPDs for all streamgages using systematic, historic, censored, and paleoflood data if available,
4. Determination of basin and climate characteristics for all the streamgages, and
5. Development of the PSRREs for eastern Colorado.

These steps are further described in the sections that follow.

Streamgage Selection

The selection of streamgages used for this report was effected by those streamgages selected by Kircher and others (1985), Vaill (1999), Capesius and Stephens (2009), and the authors' knowledge of hydrologic systems in Colorado. A comprehensive list of all USGS streamgages in the Plains hydrologic region and within 100 miles of the Colorado State line adjacent to the Plains hydrologic region was acquired from the NWIS Mapper (USGS, 2013a). From the comprehensive list of candidate streamgages, those streamgages with at least 10 years of streamflow record identified as representative of natural streamflow conditions were selected for this study.

Subsequently, multiple streamgages on the same stream course were evaluated for data independence as judged by the drainage-area ratio (DAR) and the proximity of the basin centroids. Standardized distance is a measure of the normalized, or unit-less distance, between the centroids of two basins and

DAR is used to determine if the size of two basins, when one basin is contained in the other, is sufficiently different that the precipitation events that generate the annual maximum floods in each basin are likely to be different (Veilleux, 2009). Additional information on DAR and basin centroid proximity is found in Asquith and others (2006) and Veilleux (2009). If the DAR was less than 5.0 and the standardized distance was less than 0.5, the streamgages were determined to be redundant (Veilleux, 2009; Gotvald and others, 2012; Eash and others, 2013; Southard and Veilleux, 2014). In such cases, the streamgage with the longer record was selected. Excluding redundant streamgages on the basis of relative DAR and basin centroid location helps to ensure the independence of the streamflow information among streamgages. This exclusion process serves the purpose of removing redundant data or hydrologic information from the analysis. At the completion of the selection process, 37 streamgages were excluded from analysis because of redundancy, and a total of 188 streamgages, consisting of 6,536 years of record and a mean of approximately 35 years of record per streamgage, were used to develop the PSRREs. A map showing the location of the streamgages is provided in figure 2, and each of the 188 streamgages, ancillary information, and basin and climatic characteristics are listed in table 1. The "home page" of each USGS streamgage, which provides authoritative streamgage names and other identifying characteristics, can be accessed online at http://waterdata.usgs.gov/nwis/nwisman/?site_no=STREAMGAGE&agency_cd=USGS, where the word *STREAMGAGE* is replaced by the eight-digit USGS streamgage number from table 1.

Paleoflood Investigations

Most conventional estimates for the frequency of large floods are based on extrapolations from streamgage records, commonly with record lengths shorter than 100 years (Harden and others, 2011). However, this can lead to great uncertainty in AEPDs of 1 percent or less (100-year flood discharge or larger) (fig. 3A) (Blainey and others, 2002; Harden and others, 2011; O'Connor and others, 2014). If a streamgage record contains an exceptionally large flood in the systematic record, the flood-frequency analysis can assign that flood an unrealistically short return period. This is especially true for streamgages with short records. Therefore, estimates of floods with a return period greater than about twice the record can incorporate large uncertainties (fig. 3A) (Harden and others, 2011; O'Connor and others, 2014). Of the 41 streamgages where new paleoflood data were collected in this study, all had streamgage records of less than 100 years and 23 streamgages had records of less than 25 years. The addition of paleoflood data to conventional streamgage data have been shown to increase the accuracy in discharge uncertainty estimates of low probability floods (fig. 3B) (Stedinger and Cohn, 1986; Blainey and others, 2002; Hosman and others, 2003). Even when the paleoflood age was not precisely known (Hosking and Wallis, 1986), the uncertainty estimates of low probability floods were reduced by up to 99 percent (Harden and others 2011; O'Connor and others, 2014).

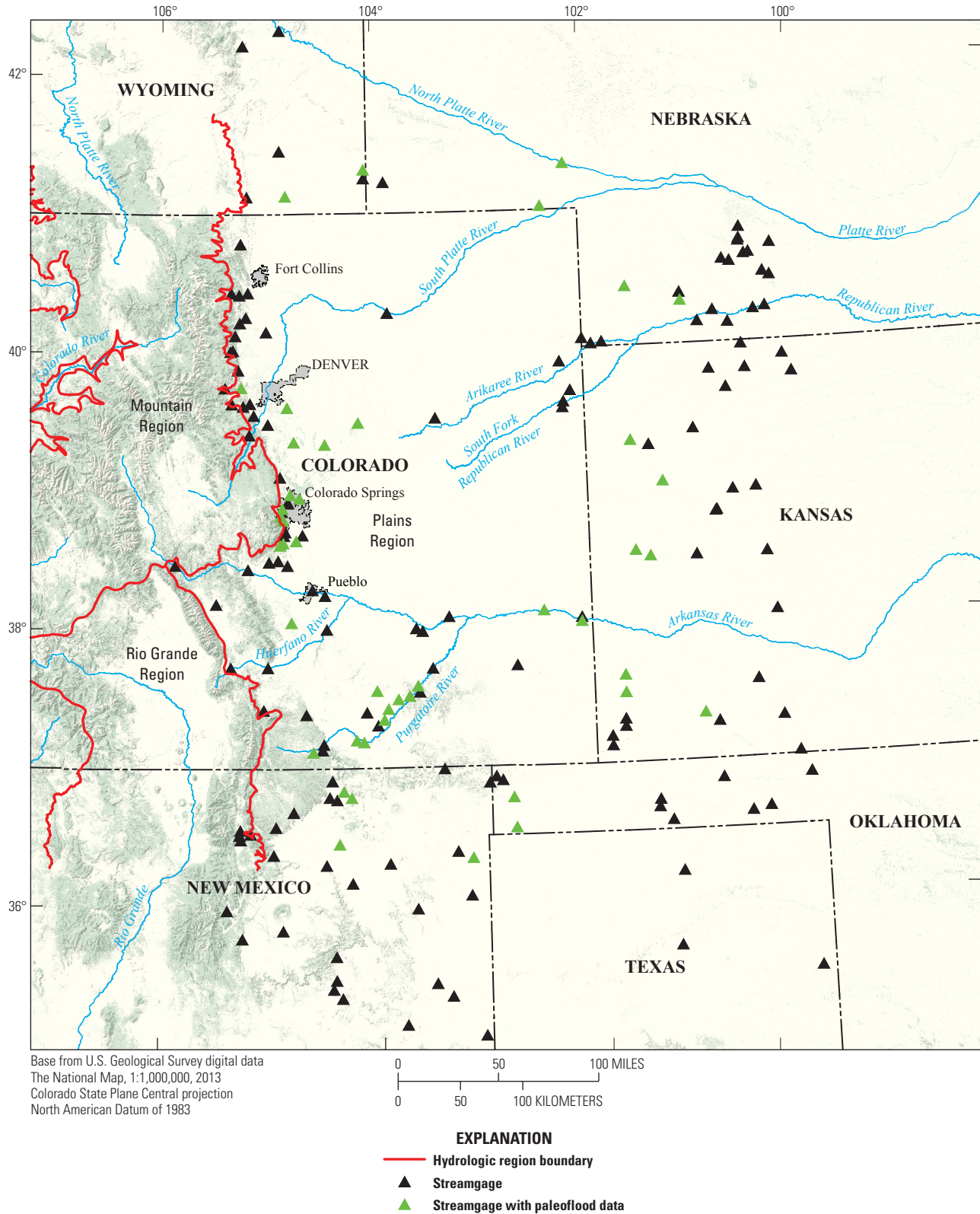


Figure 2. Location of the 188 streamgages used to develop the peak-streamflow regional-regression equations, of which 44 streamgages have paleoflood or non-exceedance bound data.

Table 1. Summary of the streamgages used in the regression analysis of natural streams in eastern Colorado, 2015.

[USGS, U.S. Geological Survey; CDWR, Colorado Division of Water Resources; NDNR, Nebraska Department of Natural Resources; A, drainage area; mi², square miles; C, amount of soil consisting of clay in basin; %, percent; E_{out} , elevation of basin outlet; ft, feet; P_{100} , mean basin 6-hour, 100-year precipitation; in., inches; S, mean basin slope; Wyo., Wyoming; Nebr., Nebraska; Colo., Colorado; Kans., Kansas; Okla., Oklahoma; N. Mex., New Mexico; Tex., Texas; NA, not applicable]

USGS streamgage number	Streamgage name	Latitude (decimal degrees)	Longitude (decimal degrees)	USGS peaks	CDWR peaks	NDNR peaks	Paleo- flood data	A (mi ²)	C (%)	E_{out} (ft)	P_{100} (in.)	S (%)
06655000	Cottonwood Creek at Wendover, Wyo.	42.3258	-104.8764	24	NA	NA	No	1,960	17.3	4,450	2.59	13.6
06668040	Rabbit Creek near Wheatland, Wyo.	42.2158	-105.2291	20	NA	NA	No	1.30	20.6	5,650	2.45	14.9
06675300	Horse Creek Tributary near Little Bear, Wyo.	41.4558	-104.8839	21	NA	NA	No	8.16	12.2	6,240	2.81	7.78
06687000	Blue Creek near Lewellen, Nebr.	41.3369	-102.1747	63	NA	18	Yes	898	5.20	3,310	3.88	5.53
06707000	North Fork South Platte River at South Platte, Colo.	39.4089	-105.1758	72	NA	NA	No	476	17.5	6,091	2.56	31.5
06708500	Deer Creek near Littleton, Colo.	39.5489	-105.1336	17	NA	NA	No	26.2	18.0	5,700	3.33	29.8
06709500	Plum Creek near Louviers, Colo.	39.4844	-105.0029	43	NA	NA	No	303	14.4	5,585	3.56	18.8
06710385	Bear Creek above Evergreen, Colo.	39.6332	-105.3369	30	NA	NA	No	103	16.0	7,080	2.55	31.8
06710990	Parmalee Gulch at Mouth, at Indian Hills, Colo.	39.6158	-105.2322	13	NA	NA	No	43.6	17.0	6,740	3.21	25.6
06711000	Turkey Creek near Morrison, Colo.	39.6355	-105.1686	28	NA	NA	No	50.2	18.9	5,718	3.13	24.9
06712000	Cherry Creek near Franktown, Colo.	39.3558	-104.7633	74	NA	NA	Yes	168	17.9	6,150	3.72	7.22
06712500	Cherry Creek near Melvin, Colo.	39.6050	-104.8225	30	NA	NA	Yes	360	17.4	5,608	3.64	8.34
06718550	North Clear Creek above Mouth near Black Hawk, Colo.	39.7489	-105.3997	19	NA	NA	No	60.0	17.0	6,910	2.46	30.5
06719500	Clear Creek near Golden, Colo.	39.7505	-105.2489	62	NA	NA	Yes	392	13.4	5,735	2.38	40.6
06724000	Saint Vrain Creek at Lyons, Colo.	40.2180	-105.2600	108	14	NA	No	216	12.6	5,292	3.24	33.7
06724500	Left Hand Creek near Boulder, Colo.	40.1255	-105.3039	16	NA	NA	No	52.0	12.6	5,710	3.23	36.4
06726900	Bummers Gulch near El Vado, Colo.	40.0117	-105.3486	12	NA	NA	No	3.90	14.0	6,270	3.27	24.3
06727500	Fourmile Creek at Orodell, Colo.	40.0187	-105.3262	22	NA	NA	No	24.2	12.6	5,750	3.12	36.7
06730300	Coal Creek near Plainview, Colo.	39.8778	-105.2772	23	20	NA	No	15.2	18.3	6,540	3.29	36.3
06730500	Boulder Creek at Mouth near Longmont, Colo.	40.1522	-105.0150	62	NA	NA	No	448	18.6	4,860	3.26	20.2
06736000	North Fork Big Thompson River at Drake, Colo.	40.4333	-105.3389	29	22	NA	No	85.1	12.5	6,170	3.63	37.4
06736650	Cedar Creek at Cedar Cove, Colo.	40.4189	-105.2653	14	NA	NA	No	18.9	16.8	5,600	4.89	33.0
06739500	Buckhorn Creek near Masonville, Colo.	40.4344	-105.1844	29	NA	NA	No	134	14.7	5,125	4.45	29.5
06742000	Little Thompson River near Berthoud, Colo.	40.2572	-105.2047	17	NA	NA	No	100	15.7	5,220	3.86	32.8
06751490	North Fork Cache La Poudre River at Livermore, Colo.	40.7875	-105.2522	26	NA	NA	No	538	18.3	5,715	3.03	19.2
06755000	South Crow Creek near Hecla, Wyo.	41.1264	-105.1944	36	NA	NA	No	13.5	16.1	7,130	3.02	10.2
06755960	Crow Creek at 19th Street, at Cheyenne, Wyo.	41.1311	-104.8286	23	NA	NA	Yes	257	15.7	6,050	2.93	9.80
06758200	Kiowa Creek at Kiowa, Colo.	39.3372	-104.4755	10	NA	NA	Yes	115	15.0	6,350	3.79	7.00
06758700	Middle Bijou Creek Tributary near Deer Trail, Colo.	39.4925	-104.1633	10	NA	NA	Yes	2.70	21.7	5,630	3.98	9.31
06759100	Bijou Creek near Fort Morgan, Colo.	40.2828	-103.8755	11	NA	NA	No	1,383	25.3	4,302	3.99	4.71
06761900	Lodgepole Creek Tributary near Pine Bluffs, Wyo.	41.2564	-104.0811	22	NA	NA	No	0.60	13.7	5,300	3.38	1.79
06762500	Lodgepole Creek at Bushnell, Nebr.	41.2278	-103.8931	72	NA	NA	No	1,216	14.1	4,843	3.15	4.14
06762600	Lodgepole Creek Tributary Number 2 near Albin, Wyo.	41.3197	-104.0808	25	NA	NA	Yes	7.60	15.7	5,330	3.44	1.99
06763500	Lodgepole Creek at Ralton, Nebr.	41.0333	-102.4050	30	NA	7	Yes	3,307	16.7	3,590	3.72	3.25
06767200	North Fork Plum Creek Tributary near Farnam, Nebr.	40.7050	-100.2404	27	NA	NA	No	1.83	22.1	2,737	4.76	7.39
06821300	North Fork Arikaree Tributary near Shaw, Colo.	39.5200	-103.4436	11	NA	NA	No	6.80	25.1	5,230	4.24	2.65
06821400	North Fork Black Wolf Creek near Vernon, Colo.	39.9067	-102.2694	11	NA	NA	No	19.8	21.4	3,680	4.37	2.23
06821500	Arikaree River at Haigler, Nebr.	40.0292	-101.9675	82	NA	NA	No	1,700	18.8	3,251	4.27	3.01
06823000	North Fork Republican River at Colorado-Nebraska State Line	40.0694	-102.0513	83	NA	NA	No	2,370	10.9	3,336	4.05	3.03

Table 1. Summary of the streamgages used in the regression analysis of natural streams in eastern Colorado, 2015.—Continued

[USGS, U.S. Geological Survey; CDWR, Colorado Division of Water Resources; NDNR, Nebraska Department of Natural Resources; A, drainage area; mi², square miles; C, amount of soil consisting of clay in basin; %, percent; E_{out} , elevation of basin outlet; ft, feet; P_{100} , mean basin 6-hour, 100-year precipitation; in., inches; S, mean basin slope; Wyo., Wyoming; Nebr., Nebraska; Colo., Colorado; Kans., Kansas; Okla., Oklahoma; N. Mex., New Mexico; Tex., Texas; NA, not applicable]

USGS streamgage number	Streamgage name	Latitude (decimal degrees)	Longitude (decimal degrees)	USGS peaks	CDWR peaks	NDNR peaks	Paleo- flood data	A (mi ²)	C (%)	E_{out} (ft)	P_{100} (in.)	S (%)
06823500	Buffalo Creek near Haigler, Nebr.	40.0394	-101.8667	73	NA	NA	No	177	6.59	3,189	4.26	4.32
06825000	South Fork Republican River near Idalia, Colo.	39.6164	-102.2427	32	NA	NA	No	1,412	21.7	3,680	4.56	2.77
06825500	Landsman Creek near Hale, Colo.	39.5755	-102.2521	26	NA	NA	No	269	23.7	3,720	4.63	2.98
06826900	Sand Creek near Hale, Colo.	39.6972	-102.1774	11	NA	NA	No	14.6	21.3	3,750	4.56	2.09
06829700	Thompson Canyon near Trenton, Nebr.	40.1622	-100.9590	13	NA	NA	No	9.10	22.0	2,670	4.52	10.6
06831500	Frenchman Creek near Imperial, Nebr.	40.4317	-101.6269	31	NA	NA	Yes	1,050	17.8	3,130	4.28	2.06
06835000	Stinking Water Creek near Palisade, Nebr.	40.3731	-101.1169	23	NA	NA	No	1,500	15.0	2,741	4.46	3.54
06835100	Bobtail Creek near Palisade, Nebr.	40.3147	-101.1116	13	NA	NA	Yes	30.2	22.0	2,920	4.38	8.38
06836000	Blackwood Creek near Culbertson, Nebr.	40.2361	-100.8111	42	NA	NA	No	320	20.7	2,555	4.44	8.91
06836500	Driftwood Creek near McCook, Nebr.	40.1458	-100.6732	33	NA	NA	No	361	21.7	2,503	4.61	6.30
06838200	Coon Creek at Indianola, Nebr.	40.2342	-100.4274	37	NA	NA	No	69.0	21.8	2,370	4.65	6.74
06838550	Dry Creek at Bartley, Nebr.	40.2506	-100.3176	23	NA	NA	No	42.0	21.8	2,320	4.75	6.26
06839200	Elkhorn Canyon near Maywood, Nebr.	40.6028	-100.7010	27	NA	NA	No	6.70	21.6	2,780	4.52	19.0
06839600	Frazier Creek near Maywood, Nebr.	40.5847	-100.6296	19	NA	NA	No	11.3	21.9	2,722	4.56	13.6
06839850	Fox Creek north of Curtis, Nebr.	40.8264	-100.5240	19	NA	NA	No	13.8	21.6	2,760	4.54	18.6
06839900	Fox Creek above Cut Canyon near Curtis, Nebr.	40.7445	-100.5315	28	NA	NA	No	31.8	21.6	2,640	4.55	18.8
06839950	Cut Canyon near Curtis, Nebr.	40.7275	-100.5365	28	NA	NA	No	25.6	21.7	2,625	4.53	21.9
06840000	Fox Creek at Curtis, Nebr.	40.6347	-100.4903	34	NA	NA	No	74.5	21.7	2,520	4.55	18.7
06840500	Dry Creek near Curtis, Nebr.	40.6425	-100.4450	20	NA	NA	No	21.6	22.1	2,548	4.63	13.7
06841000	Medicine Creek above Harry Strunk Lake, Nebr.	40.5006	-100.3228	21	NA	NA	No	770	16.9	2,381	4.54	11.6
06841500	Mitchell Creek above Harry Strunk Lake, Nebr.	40.4719	-100.2575	25	NA	NA	No	52.0	22.6	2,377	4.72	8.06
06844700	South Fork Sappa Creek near Brewster, Kans.	39.2853	-101.4660	22	NA	NA	No	85.7	20.1	3,440	4.77	1.57
06844800	South Fork Sappa Creek Tributary near Goodland, Kans.	39.3206	-101.6329	33	NA	NA	Yes	20.1	19.7	3,590	4.74	1.00
06844900	South Fork Sappa Creek near Achilles, Kans.	39.6769	-100.7221	50	NA	NA	No	446	20.5	2,722	4.85	2.61
06845000	Sappa Creek near Oberlin, Kans.	39.8131	-100.5343	45	NA	NA	No	1,063	20.8	2,538	4.83	3.16
06845100	Long Branch draw near Norcatur, Kans.	39.9017	-100.1790	53	NA	NA	No	31.7	21.9	2,440	4.84	4.73
06846200	Beaver Creek Tributary near Ludell, Kans.	39.8147	-100.8724	33	NA	NA	No	10.2	21.3	2,840	4.71	5.08
06846500	Beaver Creek at Cedar Bluffs, Kans.	39.9850	-100.5601	68	NA	NA	No	1,620	21.7	2,520	4.73	3.77
06847600	Prairie Dog Creek Tributary at Colby, Kans.	39.3911	-101.0457	56	NA	NA	No	7.53	19.7	3,130	4.92	1.59
06847900	Prairie Dog Creek above Keith Sebelius Lake, Kans.	39.7697	-100.1007	52	NA	NA	No	590	21.2	2,335	4.88	3.70
06858500	North Fork Smoky Hill River near McAllaster, Kans.	39.0179	-101.3479	33	NA	NA	Yes	696	22.5	3,070	4.63	2.58
06859500	Ladder Creek below Chalk Creek near Scott City, Kans.	38.7889	-100.8699	32	NA	NA	No	1,460	23.4	2,638	4.57	2.13
06860000	Smoky Hill River at Elkader, Kans.	38.7947	-100.8585	75	NA	NA	No	3,560	23.0	2,623	4.61	2.69
06860300	South Branch Hackberry Creek near Orion, Kans.	38.9417	-100.7032	12	NA	NA	No	49.6	20.2	2,840	4.85	1.55
06860500	Hackberry Creek near Gove, Kans.	38.9536	-100.4882	39	NA	NA	No	426	21.3	2,593	4.91	2.43
07093775	Badger Creek, Lower Station, near Howard, Colo.	38.4672	-105.8600	23	NA	NA	No	211	21.0	6,780	2.71	22.5
07095000	Grape Creek near Westcliffe, Colo.	38.1861	-105.4836	69	17	NA	No	321	16.2	7,690	3.42	19.3
07096500	Fournmile Creek near Canon City, Colo.	38.4364	-105.1914	32	NA	NA	No	436	19.1	5,254	3.19	26.0
07099060	Beaver Creek above Highway 115 near Penrose, Colo.	38.4892	-104.9975	23	NA	NA	No	138	17.3	5,660	3.37	34.9

Table 1. Summary of the streamgages used in the regression analysis of natural streams in eastern Colorado, 2015.—Continued

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USGS streamgage number	Streamgage name	Latitude (decimal degrees)	Longitude (decimal degrees)	USGS peaks	CDWR peaks	NDNR peaks	Paleo- flood data	A (mi ²)	C (%)	E_{out} (ft)	P_{100} (in.)	S (%)
07099080	Red Creek below Sullivan Park at Fort Carson, Colo.	38.4997	-104.9133	13	NA	NA	No	26.6	21.4	5,783	3.94	18.5
07099215	Turkey Creek near Fountain, Colo.	38.6117	-104.8947	30	NA	NA	Yes	13.0	22.5	6,420	3.97	44.5
07099220	Little Turkey Creek near Fountain, Colo.	38.6272	-104.8664	10	NA	NA	Yes	9.59	22.4	6,395	4.02	39.5
07099230	Turkey Creek above Teller Reservoir near Stone City, Colo.	38.4650	-104.8272	35	NA	NA	No	62.2	21.8	5,520	3.98	30.9
07103700	Fountain Creek near Colorado Springs, Colo.	38.8547	-104.8780	56	NA	NA	No	102	11.6	6,110	3.34	37.5
07103703	Camp Creek at Garden of the Gods, Colo.	38.8769	-104.8728	22	NA	NA	Yes	9.38	14.1	6,310	3.84	43.1
07103747	Monument Creek at Palmer Lake, Colo.	39.1026	-104.8923	20	NA	NA	No	25.7	9.87	6,950	3.62	32.1
07103960	Kettle Creek above U.S. Air Force Academy, Colo.	38.9761	-104.7986	13	NA	NA	Yes	16.2	16.1	6,620	3.92	7.92
07103977	Cottonwood Creek at Cowpoke Road at Colorado Springs, Colo.	38.9511	-104.7136	11	NA	NA	Yes	5.91	14.9	6,875	3.97	6.09
07104000	Monument Creek at Pikeview, Colo.	38.9178	-104.8186	49	NA	NA	No	203	13.7	6,203	3.82	16.2
07104050	North Rockrimmon Creek above Delmonico Drive at Colorado Springs, Colo.	38.9155	-104.8258	12	NA	NA	No	1.83	11.0	6,240	3.97	13.3
07105000	Bear Creek near Colorado Springs, Colo.	38.8225	-104.8886	22	NA	NA	Yes	6.89	12.6	6,520	3.66	48.6
07105490	Cheyenne Creek at Evans Avenue at Colorado Springs, Colo.	38.7905	-104.8641	22	NA	NA	Yes	21.7	11.1	6,280	3.49	48.7
07105900	Jimmy Camp Creek at Fountain, Colo.	38.6844	-104.6886	39	NA	NA	No	65.4	34.5	5,530	4.22	6.40
07105920	Little Fountain Creek above Keaton Reservoir, near Fort Carson, Colo.	38.6817	-104.8586	13	NA	NA	No	11.0	21.9	6,430	3.78	48.2
07105940	Little Fountain Creek near Fountain, Colo.	38.6431	-104.7467	12	NA	NA	Yes	26.9	27.9	5,566	4.00	24.0
07105945	Rock Creek above Fort Carson Reservation, Colo.	38.7075	-104.8466	35	NA	NA	No	6.80	22.8	6,390	3.91	48.2
07106500	Fountain Creek at Pueblo, Colo.	38.2878	-104.6011	74	NA	NA	No	925	22.4	4,705	3.89	7.49
07107500	St. Charles River at Burnt Mill, Colo.	38.0517	-104.7936	27	NA	NA	Yes	171	22.8	5,350	4.12	28.4
07108900	St. Charles River at Vineland, Colo.	38.2456	-104.4864	36	NA	NA	No	474	24.9	4,582	3.78	11.6
07111000	Huerfano River at Manzanares Crossing, near Redwing, Colo.	37.7278	-105.3514	58	18	NA	No	73.0	17.2	8,270	3.15	31.8
07112500	Huerfano River at Badito, Colo.	37.7272	-105.0125	22	6	NA	No	532	19.6	6,415	3.28	20.8
07114000	Cucharas River at Boyd Ranch near La Veta, Colo.	37.4200	-105.0528	47	18	NA	No	53.1	20.1	7,781	3.37	35.3
07116000	Huerfano River below Huerfano Valley Dam near Undercliffe, Colo.	38.0014	-104.4725	28	NA	NA	No	1,673	23.9	4,886	3.34	11.7
07118000	Apishapa River near Aguilar, Colo.	37.3864	-104.6658	12	NA	NA	No	140	24.3	6,408	3.42	26.8
07120620	Big Arroyo near Thatcher, Colo.	37.5548	-104.0212	30	NA	NA	Yes	15.6	26.0	5,288	3.82	6.14
07121500	Timpas Creek at Mouth near Swink, Colo.	38.0031	-103.6561	51	NA	NA	No	511	24.9	4,120	3.79	3.86
07122400	Crooked Arroyo near Swink, Colo.	37.9822	-103.5983	26	18	NA	No	108	25.9	4,100	3.94	2.14
07123675	Horse Creek near Las Animas, Colo.	38.0850	-103.3538	29	NA	NA	No	1,403	21.6	3,975	3.99	2.01
07124300	Long Canyon Creek near Madrid, Colo.	37.1147	-104.6053	18	NA	NA	Yes	100	36.8	6,259	3.53	25.4
07124500	Purgatoire River at Trinidad, Colo.	37.1709	-104.5092	70	NA	NA	No	749	30.5	5,980	3.35	28.0
07125100	Frijole Creek near Alfalfa, Colo.	37.1997	-104.2110	15	NA	NA	Yes	79.1	27.3	5,420	3.63	10.0
07125500	San Francisco Creek near Alfalfa, Colo.	37.1849	-104.1453	15	NA	NA	Yes	162	28.9	5,320	3.78	13.4
07126100	Luning Arroyo near Model, Colo.	37.3045	-104.0155	16	NA	NA	No	84.8	25.6	5,150	3.71	2.83
07126140	Van Bremer Arroyo near Tyrone, Colo.	37.3995	-104.1158	31	NA	NA	No	119	22.9	5,310	3.69	2.07
07126200	Van Bremer Arroyo near Model, Colo.	37.3456	-103.9580	48	NA	NA	Yes	161	23.3	4,960	3.72	4.81
07126325	Taylor Arroyo below Rock Crossing, near Thatcher, Colo.	37.4242	-103.9202	31	NA	NA	Yes	48.7	25.5	4,982	3.83	4.43
07126390	Lockwood Canyon Creek near Thatcher, Colo.	37.4928	-103.8280	30	NA	NA	Yes	48.6	23.6	4,785	3.89	4.59

Table 1. Summary of the streamgages used in the regression analysis of natural streams in eastern Colorado, 2015.—Continued

[USGS, U.S. Geological Survey; CDWR, Colorado Division of Water Resources; NDNR, Nebraska Department of Natural Resources; A, drainage area; mi², square miles; C, amount of soil consisting of clay in basin; %, percent; E_{out} , elevation of basin outlet; ft, feet; P_{100} , mean basin 6-hour, 100-year precipitation; in., inches; S, mean basin slope; Wyo., Wyoming; Nebr., Nebraska; Colo., Colorado; Kans., Kansas; Okla., Oklahoma; N. Mex., New Mexico; Tex., Texas; NA, not applicable]

USGS streamgage number	Streamgage name	Latitude (decimal degrees)	Longitude (decimal degrees)	USGS peaks	CDWR peaks	NDNR peaks	Paleo- flood data	A (mi ²)	C (%)	E_{out} (ft)	P_{100} (in.)	S (%)
07126415	Red Rock Canyon Creek at Mouth near Thatcher, Colo.	37.5153	-103.7255	31	NA	NA	Yes	48.7	18.1	4,510	3.98	11.1
07126470	Chacuaco Creek at Mouth near Timpas, Colo.	37.5439	-103.6322	16	NA	NA	No	421	19.9	4,410	4.30	8.55
07126480	Bent Canyon Creek at Mouth near Timpas, Colo.	37.5892	-103.6483	29	NA	NA	Yes	56.1	20.5	4,402	3.98	11.2
07126500	Purgatoire River at Ninemile Dam, near Higbee, Colo.	37.7147	-103.5111	71	17	NA	No	2,875	25.2	4,241	3.77	8.93
07133200	Clay Creek Tributary near Deora, Colo.	37.7242	-102.7405	11	NA	NA	No	2.40	22.5	4,285	4.82	5.05
07134100	Big Sandy Creek near Lamar, Colo.	38.1142	-102.4838	34	NA	NA	Yes	3,216	21.4	3,545	4.33	2.57
07134990	Wild Horse Creek above Holly, Colo.	38.0570	-102.1385	18	NA	NA	No	175	23.9	3,405	4.58	1.62
07135000	Two Butte Creek near Holly, Colo.	38.0278	-102.1391	10	NA	NA	Yes	817	21.6	3,415	4.87	3.57
07138600	White Woman Creek Tributary near Selkirk, Kans.	38.5250	-101.6216	39	NA	NA	Yes	25.2	26.0	3,510	4.45	1.16
07138650	White Woman Creek near Leoti, Kans.	38.4811	-101.4882	20	NA	NA	Yes	500	24.1	3,320	4.50	1.36
07138800	Lion Creek Tributary near Modoc, Kans.	38.4800	-101.0629	21	NA	NA	No	7.00	26.5	3,080	4.55	0.48
07140600	Pawnee River Tributary near Kalvesta, Kans.	38.0617	-100.3504	33	NA	NA	No	6.89	33.7	2,650	5.01	1.14
07141400	South Fork Walnut Creek Tributary near Dighton, Kans.	38.4828	-100.4154	21	NA	NA	No	0.81	28.6	2,700	5.10	1.57
07153500	Dry Cimarron River near Guy, N. Mex.	36.9875	-103.4241	33	NA	NA	No	500	21.7	4,900	4.10	13.5
07154400	Carrizozo Creek near Kenton, Okla.	36.8833	-103.0183	61	NA	NA	No	112	20.2	4,380	4.44	11.8
07154500	Cimarron River near Kenton, Okla.	36.9267	-102.9591	63	NA	NA	No	1,208	21.0	4,262	4.29	13.2
07154650	Tesesquite Creek near Kenton, Okla.	36.8978	-102.9016	22	NA	NA	No	24.9	16.7	4,250	5.07	12.3
07155100	Cold Springs Creek near Wheelless, Okla.	36.7722	-102.8049	18	NA	NA	Yes	10.6	28.7	4,450	4.99	1.86
07155590	Cimarron River near Elkhart, Kans.	37.1220	-101.8979	41	NA	NA	No	2,927	19.6	3,377	4.65	5.38
07155900	North Fork Cimarron River Tributary near Elkhart, Kans.	37.1909	-101.8988	33	NA	NA	No	57.1	26.7	3,490	4.59	0.58
07156000	North Fork Cimarron River Tributary near Richfield, Kans.	37.3113	-101.7736	21	NA	NA	No	103	24.9	3,350	4.58	0.81
07156010	North Fork Cimarron River at Richfield, Kans.	37.2584	-101.7754	15	NA	NA	No	411	21.4	3,344	4.78	0.91
07156100	Sand Arroyo Creek near Johnson, Kans.	37.5015	-101.7616	15	NA	NA	Yes	584	19.7	3,328	5.02	1.57
07156220	Bear Creek near Johnson, Kans.	37.6285	-101.7616	32	NA	NA	Yes	813	27.0	3,292	4.88	1.51
07156600	Cimarron River Tributary near Moscow, Kans.	37.3363	-101.0504	33	NA	NA	Yes	21.5	23.8	2,830	4.68	1.71
07156700	Cimarron River Tributary near Satanta, Kans.	37.2709	-100.9271	49	NA	NA	No	2.41	22.0	2,740	4.97	1.92
07157100	Crooked Creek near Copeland, Kans.	37.5653	-100.5546	33	NA	NA	No	44.0	29.6	2,695	4.97	1.14
07157400	Crooked Creek Tributary at Meade, Kans.	37.2964	-100.3399	33	NA	NA	No	6.57	30.3	2,450	5.48	4.12
07157500	Crooked Creek near Englewood, Kans.	37.0325	-100.2113	36	NA	NA	No	1,157	28.4	2,164	5.36	2.08
07157550	West Fork Creek near Knowles, Okla.	36.8750	-100.1226	22	NA	NA	No	4.44	20.2	2,280	5.60	6.27
07199000	Canadian River near Hebron, N. Mex.	36.7872	-104.4622	40	NA	NA	No	199	37.5	6,248	3.62	23.6
07201000	Raton Creek at Raton, N. Mex.	36.9058	-104.4361	54	NA	NA	No	14.6	32.6	6,640	3.94	23.4
07201200	Chicorica Creek Tributary near Raton, N. Mex.	36.8308	-104.3333	31	NA	NA	Yes	5.20	31.4	6,480	3.88	16.3
07201450	Green Mountain Arroyo near Raton, N. Mex.	36.7842	-104.2640	12	NA	NA	Yes	18.2	30.6	6,499	3.84	10.4
07202000	Chicorica Creek near Hebron, N. Mex.	36.7703	-104.3964	10	NA	NA	No	381	33.3	6,200	3.88	13.5
07203000	Vermejo River near Dawson, N. Mex.	36.6810	-104.7864	85	NA	NA	No	295	35.0	6,360	3.33	24.7
07203600	Rio Del Plano Tributary near Taylor Springs, N. Mex.	36.4498	-104.3767	12	NA	NA	Yes	6.71	32.6	6,148	3.64	2.03
07204000	Moreno Creek at Eagle Nest, N. Mex.	36.5539	-105.2680	72	NA	NA	No	73.8	23.9	8,197	3.26	26.3
07204500	Cieneguilla Creek near Eagle Nest, N. Mex.	36.4852	-105.2654	72	NA	NA	No	56.0	29.1	8,200	2.98	19.5
07205000	Sixmile Creek near Eagle Nest, N. Mex.	36.5185	-105.2752	76	NA	NA	No	10.5	24.1	8,195	3.13	26.9

Table 1. Summary of the streamgages used in the regression analysis of natural streams in eastern Colorado, 2015.—Continued

[USGS, U.S. Geological Survey; CDWR, Colorado Division of Water Resources; NDNR, Nebraska Department of Natural Resources; A, drainage area; mi², square miles; C, amount of soil consisting of clay in basin; %, percent; E_{out} , elevation of basin outlet; ft, feet; P_{100} , mean basin 6-hour, 100-year precipitation; in., inches; S, mean basin slope; Wyo., Wyoming; Nebr., Nebraska; Colo., Colorado; Kans., Kansas; Okla., Oklahoma; N. Mex., New Mexico; Tex., Texas; NA, not applicable]

USGS streamgage number	Streamgage name	Latitude (decimal degrees)	Longitude (decimal degrees)	USGS peaks	CDWR peaks	NDNR peaks	Paleo- flood data	A (mi ²)	C (%)	E_{out} (ft)	P_{100} (in.)	S (%)
07206400	Clear Creek near Ute Park, N. Mex.	36.5264	-105.1750	48	NA	NA	No	7.44	20.7	7,860	3.27	48.1
07207500	Ponil Creek near Cimarron, N. Mex.	36.5737	-104.9468	62	NA	NA	No	171	33.4	6,630	3.30	27.3
07208500	Rayado Creek near Cimarron, N. Mex.	36.3723	-104.9693	91	NA	NA	No	60.1	29.1	6,720	3.46	24.1
07211500	Canadian River near Taylor Springs, N. Mex.	36.2976	-104.4955	73	NA	NA	No	2,850	33.5	5,640	3.49	15.4
07213700	Canadian River Tributary near Mills, N. Mex.	36.1667	-104.2636	13	NA	NA	No	4.20	31.7	5,908	3.95	5.34
07214800	Rio La Casa near Cleveland, N. Mex.	35.9742	-105.3892	14	NA	NA	No	23.0	25.3	7,635	4.13	33.5
07220000	Sapello River at Sapello, N. Mex.	35.7698	-105.2520	17	NA	NA	No	132	32.4	6,910	3.94	24.1
07220900	Dog Creek near Shoemaker, N. Mex.	35.8261	-104.8881	57	NA	NA	No	18.4	30.1	6,300	3.64	6.00
07221600	Lagartija Creek Tributary near Sanchez, N. Mex.	35.6386	-104.4156	24	NA	NA	No	1.16	23.7	4,605	3.85	13.6
07222300	Trementina Creek at Trementina, N. Mex.	35.4672	-104.4167	52	NA	NA	No	63.9	23.2	4,580	3.96	17.0
07222500	Conchas River at Variadero, N. Mex.	35.4028	-104.4436	60	NA	NA	No	523	27.0	4,430	3.75	7.64
07222800	Garita Creek Tributary near Variadero, N. Mex.	35.3358	-104.3639	39	NA	NA	No	12.0	23.1	4,290	4.02	4.04
07225300	Bluewater Creek near Tucumcari, N. Mex.	35.1419	-103.7928	40	NA	NA	No	15.2	21.6	4,100	4.41	6.12
07225500	Ute Creek near Gladstone, N. Mex.	36.3059	-103.9255	12	NA	NA	No	269	26.5	5,800	4.07	6.16
07226200	Bueyeros Creek at Bueyeros, N. Mex.	35.9797	-103.6856	34	NA	NA	No	34.0	26.7	4,540	4.27	11.1
07226500	Ute Creek near Logan, N. Mex.	35.4385	-103.5258	72	NA	NA	No	2,060	24.1	3,820	4.17	5.34
07227100	Revuelto Creek near Logan, N. Mex.	35.3444	-103.3896	54	NA	NA	No	786	25.7	3,660	4.52	5.52
07227150	Arroyo Del Puerto near Endee, N. Mex.	35.0590	-103.1022	23	NA	NA	No	25.0	30.4	3,920	4.82	5.92
07227200	Tramperos Creek near Stead, N. Mex.	36.0708	-103.2033	47	NA	NA	No	556	25.9	4,481	4.41	5.93
07227295	Sandy Arroyo Tributary near Clayton, N. Mex.	36.3889	-103.3186	43	NA	NA	No	1.20	25.6	5,110	4.55	2.01
07227300	Sand Draw near Clayton, N. Mex.	36.3422	-103.1842	18	NA	NA	Yes	42.0	27.8	4,800	4.53	2.36
07227920	Dixon Creek near Borger, Tex.	35.6648	-101.3510	15	NA	NA	No	134	26.2	2,836	4.90	5.67
07232500	Beaver River near Guymon, Okla.	36.7214	-101.4896	57	NA	NA	No	2,033	26.8	2,971	4.57	2.88
07232550	South Fork Tributary near Guymon, Okla.	36.6684	-101.4988	22	NA	NA	No	0.26	38.5	3,120	4.70	0.67
07232650	Aqua Frio Creek near Felt, Okla.	36.5540	-102.7866	12	NA	NA	Yes	33.9	29.7	4,410	4.86	1.09
07232900	Coldwater Creek near Guymon, Okla.	36.5720	-101.3815	30	NA	NA	No	1,903	29.9	2,871	4.78	1.52
07233500	Palo Duro Creek near Spearman, Tex.	36.2023	-101.3060	49	NA	NA	No	625	35.4	2,962	4.75	1.11
07233850	Sharp Creek Tributary near Turpin, Okla.	36.8639	-100.9129	12	NA	NA	No	1.00	24.0	2,760	5.36	0.40
07234050	North Fork Clear Creek Tributary near Balko, Okla.	36.6170	-100.6643	22	NA	NA	No	4.30	30.9	2,770	5.19	1.96
07234100	Clear Creek near Elmwood, Okla.	36.6450	-100.5024	24	NA	NA	No	170	32.2	2,541	5.17	2.55
07301410	Sweetwater Creek near Kelton, Tex.	35.4731	-100.1210	52	NA	NA	No	297	18.6	2,230	5.33	3.42
NA	Raton Creek above Starkville, Colo.	37.1306	-104.5214	0	10	NA	No	54.7	35.9	6,240	3.77	28.0

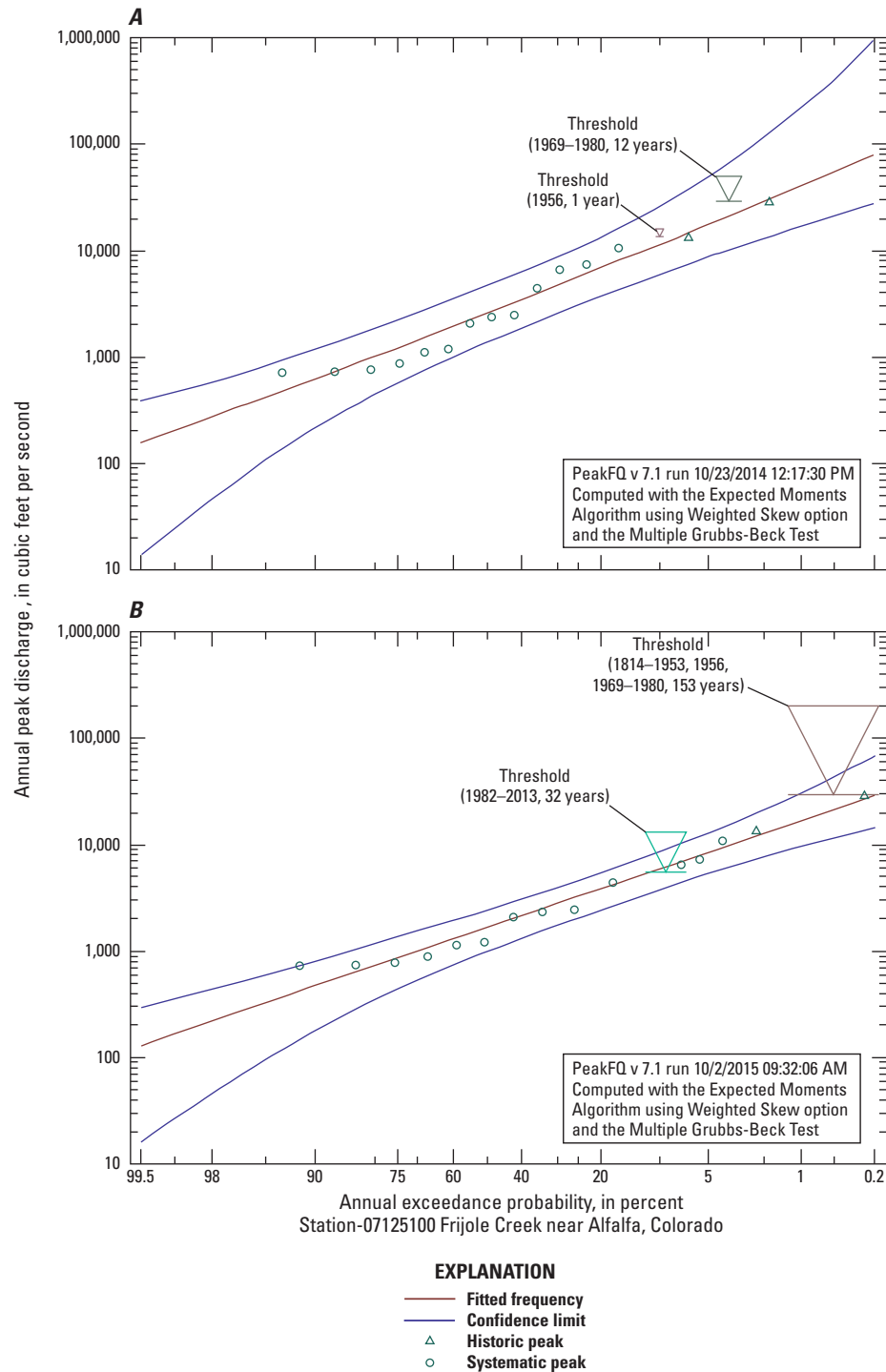


Figure 3. Flood-frequency analyses for 07125100 Frijole Creek near Alfalfa, Colo., using the *A*, 13-year streamgage record only and *B*, streamgage record with one paleoflood and one non-exceedance bound; the flood-frequency curve and its 95-percent confidence limits are reduced with the addition of the paleoflood data; where threshold is the years for which the historic or paleoflood data is applicable.

Paleoflood hydrology has been used extensively to document large magnitude floods to improve flood-frequency estimates (England and others, 2010; Harden and others, 2011). Paleoflood hydrology (Kochel and Baker, 1982) is the study of previously unrecorded floods using slackwater deposits and other paleostage indicators (fig. 4) (Baker, 1987) to reconstruct the magnitude of the flood and geomorphic, stratigraphic, and (or) botanic evidence to reconstruct the timing of the flood. Paleoflood hydrology has a long history in a wide variety of settings throughout the world (Costa, 1986; Patton, 1987; Wohl and others, 1994; Enzel and others, 1994; Maas and Macklin, 2002; Heine, 2004; Sheffer and others, 2008; Harden and others, 2011).

Paleoflood Slackwater Deposits

One widely used technique in the study of paleofloods uses fine-grained sedimentology records that accumulate in backwater areas (slackwater) to construct a history of past floods (Patton and others, 1979; Kochel and Baker, 1988). This approach has been developed during the last several decades and is now a widely used method for quantifying unrecorded floods (Baker, 1987; Ely and others, 1993; O'Connor and others, 1994; Sheffer and others, 2008; Harden and others, 2011). This technique can be extremely useful in

characterizing the frequency of large floods; however, slackwater sediments often times do not represent a complete and continuous record of floods at a particular site. Paleoflood studies can also provide a long-term perspective that places historical large-magnitude floods into temporal context and assists in reconciliation of conflicting information (Boner and Stermitz, 1967; Helley and LaMarche, 1973).

Other Paleostage Indicators

Another aspect of paleoflood hydrology is the concept of a paleohydrologic bound (non-exceedance bound/perception threshold), defined as a time interval during which a particular discharge has not been exceeded (Levish, 2002). Reconstructing the discharge and magnitude of individual paleofloods can be difficult to interpret in alluvial streams and rivers because of potentially complex stratigraphy and variable preservation (House and others, 2002). Additionally, individual paleofloods typically represent a minimum discharge value; whereas, non-exceedance bounds are a maximum value. Non-exceedance bound data are especially helpful in flood-frequency analyses because there is no constraint on the flood-frequency extrapolation at low probabilities using paleofloods alone (Levish, 2002). Considering these limitations, the concept of the non-exceedance bound was developed by the Bureau of

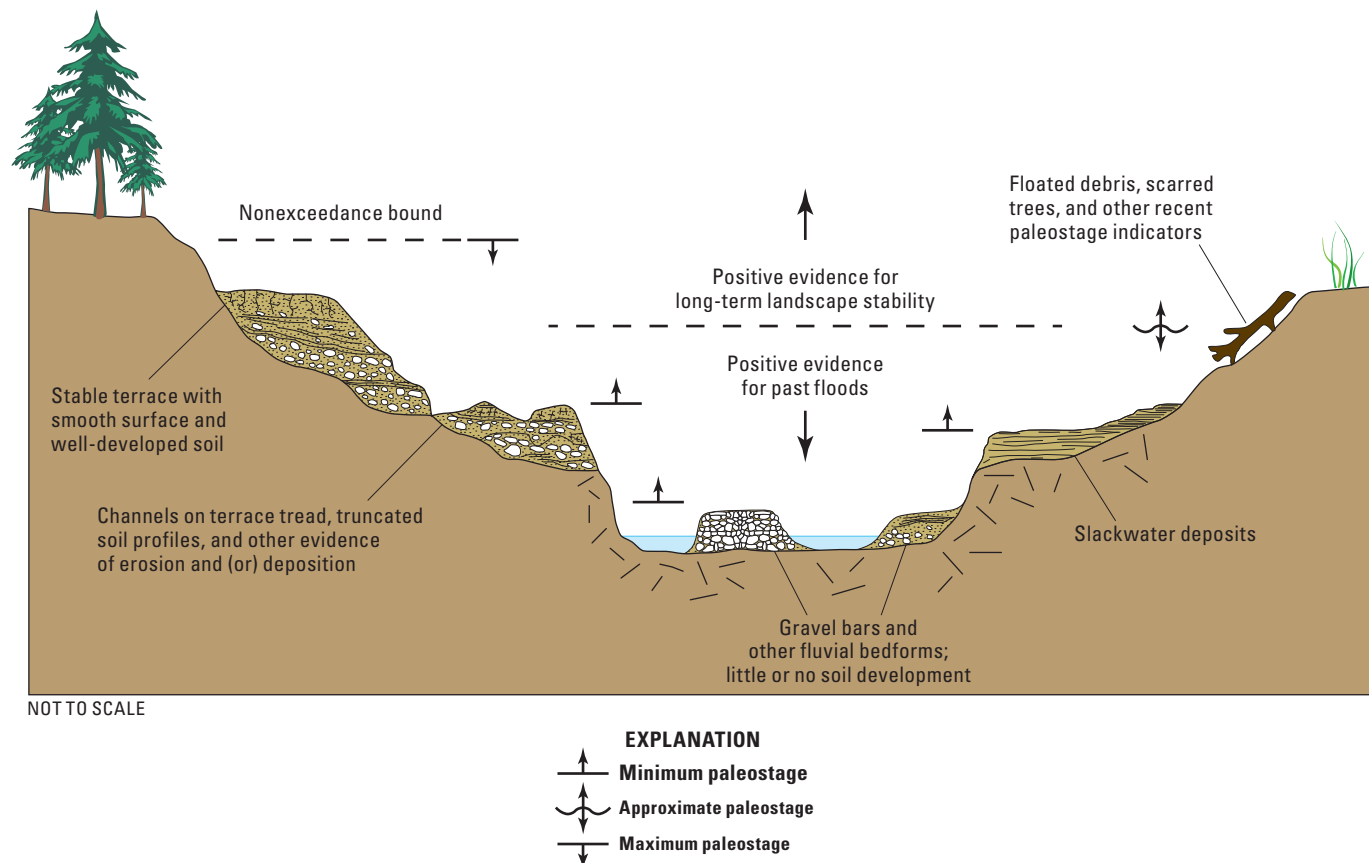


Figure 4. Idealized channel cross section illustrating the concept of a non-exceedance bound and the fluvial landforms and related deposits relative to paleoflood studies. Modified from England and others (2010).

Reclamation in the mid-1990s because the use of paleofloods alone was not adequate for quantitative risk assessments for dam safety (Levish and others, 1994).

The non-exceedance bound methodology uses the age of a terrace surface that lacks clear evidence of recent inundation, erosion, and deposition, or alternatively shows evidence for long-term stability, to establish an upper limit to flooding. This non-inundation approach can be extremely useful in flood hazard assessment (O'Connell and others, 2002) because geomorphic and stratigraphic information derived from the terrace surface can provide an upper limit or bound on the age and magnitude of extreme floods (Levish, 2002). Rather than constructing a detailed record of past floods, the non-inundation approach focuses on identifying a non-exceedance bound. Establishing a non-exceedance bound is accomplished by identifying terrace surfaces that serve as limits for the paleostage of large floods and estimating ages for those terraces (fig. 4). These bounds do not represent actual floods, but instead physical limits to peak flood stage over a measured time interval. Simply stated, a non-exceedance bound is a maximum stage that has not been exceeded in the time period since the terrace surface stabilized. The maximum stage can be used to estimate peak discharge given some knowledge of the channel characteristics. Developing evidence of specific paleofloods using this methodology is not necessary; however, determining the discharge for a flood that has not been exceeded over the time interval represented by the preserved stable landscape is important. Early studies by Mansfield (1938) on the Ohio River and Jahns (1947) on the Connecticut River recognized the concept of these non-inundation bounds. Mansfield (1938) and Jahns (1947) found that floods on those rivers overtopped stream terraces that had not been inundated for thousands of years.

Methodology Used to Identify Paleofloods and Respective Ages

An effective means of reducing uncertainties regarding probabilities of flood recurrence is to augment observational and historic peak streamflow records by applying paleoflood techniques (Costa, 1978; Stedinger and Baker, 1987; Frances and others, 1994; Webb and others, 2002) which typically use geologic, geomorphic, and paleobotanical evidence to determine the ages and magnitudes of floods that occurred before the collection of observational records.

For this study, the primary evidence for past floods consists of fine-grained deposits found on terraces and debris/flotsam lines found along the channel margin (fig. 5A). The stratigraphic record present along streams in the form of terrace and flood-plain deposits can be direct indicators of the magnitude of large floods on a river and may be 10- to 100-times longer than conventional streamgage records of large floods (Patton, 1987; Baker, 1989; Jarrett, 1991).

When a large flood occurs, sediment is transported in the flood waters and deposited on terrace surfaces, channel bars, or in slackwater settings, such as in the lee of bedrock obstructions or in the mouths of tributary junctions. A distinct

record of the flood is preserved in the stratigraphy of the terrace (fig. 5B–D). The deposits left by more recent floods will show sedimentologic characteristics of a fluvial deposit such as visible cross bedding, laminations, or other bedding features but no soil development (fig. 5C). Over time, the flood deposit will begin to form a soil and develop soil structure and horizons. Evidence for multiple floods can be detected in the stratigraphic record by the identification of stratigraphic breaks (fig. 5D), such as buried soils, organic mats, colluvium (hillslope deposits), tree rooting-horizons, and burn horizons.

Other evidence of unrecorded floods at the study sites was the presence of high-water marks such as flotsam and trimlines (erosional features caused by a flood) along the channel margins (fig. 5A and B). Although most of the floods associated with the high-water marks occurred within the last few decades, these floods were not captured in the streamgage record because of the prevalence of discontinued streamgages in the study area. Many of the drainage basins investigated had lines of fluvially transported debris such as logs, branches, leaves, pine needles, bark, and twigs that were above the stage of the peak of record for measured (known as systematic peaks in this report) discharges in the streamgage records. These lines clearly indicate (1) the occurrence of a large flood, and (2) the stage of the flood. An estimate of age can be determined from the degree of weathering of the debris. For example, logs and large branches can persist in dry environments for more than 100 years but small twigs, pine needles, and leaves last only a few years. Therefore, in drainage basins where large floods were measured from debris lines, a young age was assigned if the organic fines were still visible, but an older age if only the large debris remained. Any historic artifacts found in flotsam or flood deposits can be used to estimate the age of the flood as well (House and Baker, 2001). An example of paleoflood evidence from 06758700 Middle Bijou Creek Tributary near Deer Trail, Colo., is given in Appendix 1.

Methodology Used to Identify Non-Exceedance Bounds and Respective Ages

Evidence used to determine non-exceedance bounds for this study consisted of trimlines, soil characteristics, desert pavement (layer of flat-lying, densely-packed rocks, pebbles, or gravel overtopping a vesicular silty layer that can develop on the ground surface in arid environments), physical weathering of rocks and terraces, and vegetation (fig. 6). Trimlines, erosional features caused by a flood and typically located along the channel margin, can serve as a stage estimate for a non-exceedance bound if no other evidence of flooding is observed above the trimline (fig. 6A). This typically is used at locations with a clear trimline and a lack of terraces. Age determination typically depends on the degree of soil development above the trimline, similar to determining the age of terraces described in the following three paragraphs.

Soils formed on alluvium in stream terraces are a commonly used method to identify and age non-exceedance bounds (Levish and Ostenaa, 1996; Jacoby and others, 2007; England

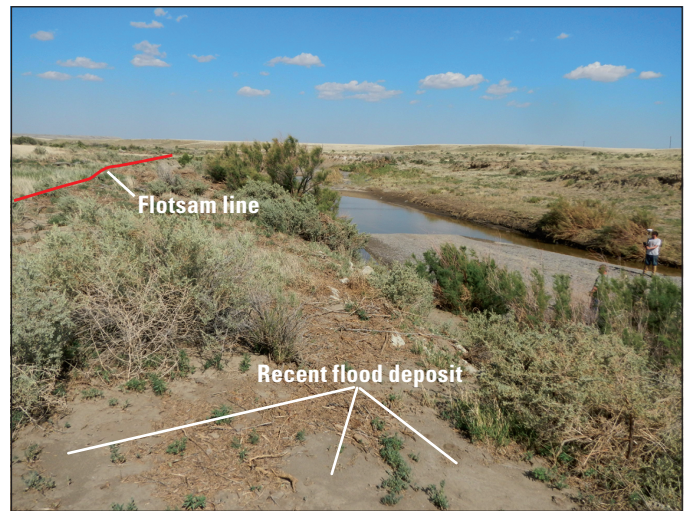
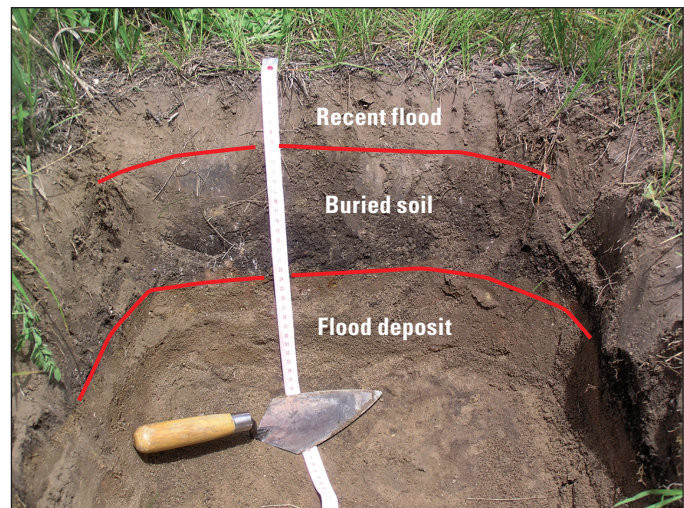
A. Little Fountain Creek near Fountain, Colorado, 07105940**B. Frijole Creek near Alfalfa, Colorado, 07125100****C. Arikaree River at Haigler, Nebraska, 06821500****D. Kettle Creek above U.S. Air Force Academy, Colorado, 07103960**

Figure 5. Evidence of flooding at *A*, 07105940 Little Fountain Creek near Fountain, Colo., *B*, 07125100 Frijole Creek near Alfalfa, Colo., *C*, 06821500 Arikaree River at Haigler, Nebr., and *D*, 07103960 Kettle Creek above U.S. Air Force Academy, Colo., *A* through *D* all show evidence of recent flooding; however, because many of the streamgages in the study area were discontinued, these floods were unrecorded, *D*, shows evidence of an older flood as indicated by the buried soil. Photographs by Tessa Harden, USGS (*A–C*) and Jeanne Godaire, Bureau of Reclamation (*D*).

and others, 2010). Stable terrace surfaces are simply flood plains that have been abandoned because of either stream incision and (or) channel migration. Once abandoned, the surface characteristics of the terrace begin to change recognizably with time. An abandoned terrace surface will tend to lose all evidence of having been inundated and become more planar and smooth with time. Once stabilized, soil will begin to form on the terrace deposits (fig. 6*B*). Thus, stable terrace surfaces represent the field expression for the level of non-inundation or non-exceedance bounds and are a direct indication for the physical upper limit of floods along a stream through time.

The rate of soil development is dependent on several factors as first described by Jenny (1941), the most important of which include climate, organisms, relief, parent material,

and time. The development of soil properties is related to these factors and can be used to estimate the age of a particular soil, and thus the age since the terrace surface stabilized. Where multiple fluvial landforms or landforms formed by other processes are present, a soil chronosequence can be developed. A soil chronosequence is defined as, “A sequence of related soils that differ in their degree of profile development because of differences in their age” (Allaby, 1998). Soil chronosequences using properties of soils and absolute or relative age dating have been developed in various regions and can be used at many of the study locations for this study to estimate the age of a soil based on these time-dependent soil properties (Birkeland, 1999). The most important soil properties that are used for correlation in this study include the presence or

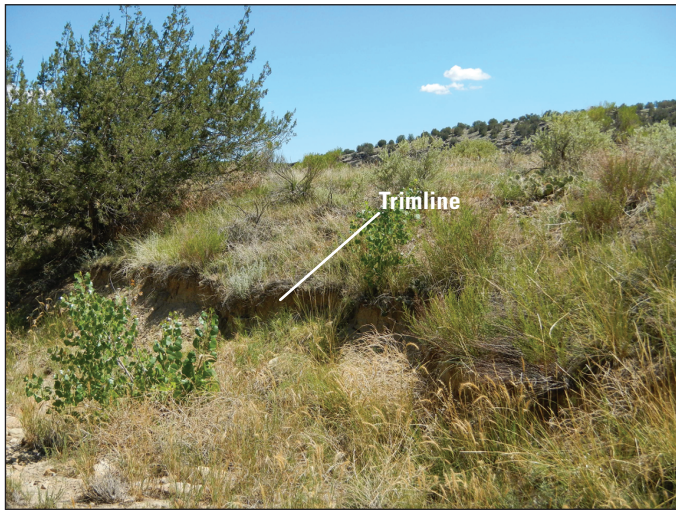
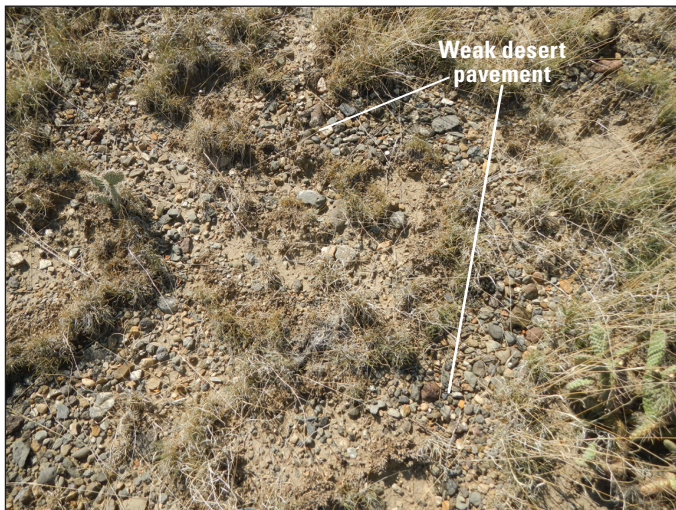
A. Lockwood Canyon Creek near Thatcher, Colorado, 07126390**B. Big Arroyo near Thatcher, Colorado, 07120620****C. Frijole Creek near Alfalfa, Colorado, 07125100****D. Frijole Creek near Alfalfa, Colorado, 07125100**

Figure 6. Evidence used to determine non-exceedance bounds. *A*, trimline at 07126390 Lockwood Canyon Creek near Thatcher, Colo., *B*, well developed, stable soil at 07120620 Big Arroyo near Thatcher, Colo., *C*, weak desert pavement at 07125100 Frijole Creek near Alfalfa, Colo., *D*, also at 07125100 Frijole Creek near Alfalfa, Colo., no desert pavement development indicating a recently disturbed surface. Photographs by Tessa Harden, USGS (A–D).

thickness of organic-rich A horizons, carbonate accumulation, and color. In addition, Bureau of Reclamation and other researchers have conducted paleoflood, geomorphic, and geologic investigations in the region of this study; radiocarbon data and soil properties from these previous studies were used to estimate the age of deposits for this study by correlating terrace surfaces and their associated soil properties (Hunt, 1954; Scott, 1960; Scott, 1963; Machette, 1985; Cornwell, 1986; May, 2003).

The ages associated with the terrace surfaces that form the non-exceedance bounds should always be considered minimum ages because of the problems related to determining the precise time when a particular surface was abandoned, stabilized, and soil began forming. Determining the age of soils can be complicated because the soils contain a wide variety of

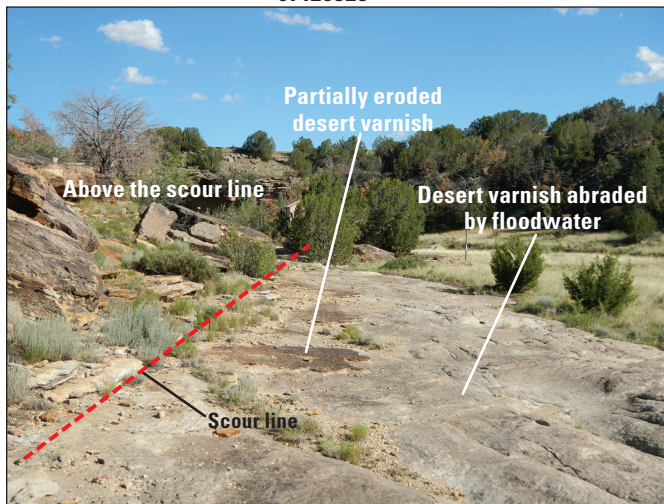
organic components because of the relatively continual input of new material introduced into the profile by biotic processes (Johnson, 1990; Hsieh, 1992, 1993; Birkeland, 1999; Puseman and Cummings, 2004). Therefore, an estimate for the maximum discharge over the minimum time interval since the terrace surface stabilized makes the non-exceedance bound a conservative estimate for the flood hazard. These estimates are made even more conservative with time because channels may down cut and erode laterally resulting in apparently larger cross sections and peak discharges than those that may have actually been present or occurred, so underestimation of the peak discharge associated with a non-exceedance bound in most cases is typically not a severe problem for flood-frequency analysis given the other uncertainties inherent in the method.

The use of desert pavement was also used to determine age and stage of non-exceedance bounds on appropriate streams. Desert pavement is a layer of flat-lying, densely-packed rocks, pebbles, or gravel overtopping a vesicular silty layer that can develop on the ground surface in arid environments (fig. 6C). Over time, a varnish or patina forms on the rocks. There is much debate on the processes that drive the formation of desert pavement (Jessup, 1960; Cook, 1970; McFadden and others, 1987; Williams and Zimelman, 1994). However, it has been well documented that the presence of the pavement indicates the area has not been disturbed for a period of time depending on the degree of development (fig. 6C and D) and can be used as a relative indicator of age (Quade, 2001; Anderson and others, 2002; Pelletier and others, 2007).

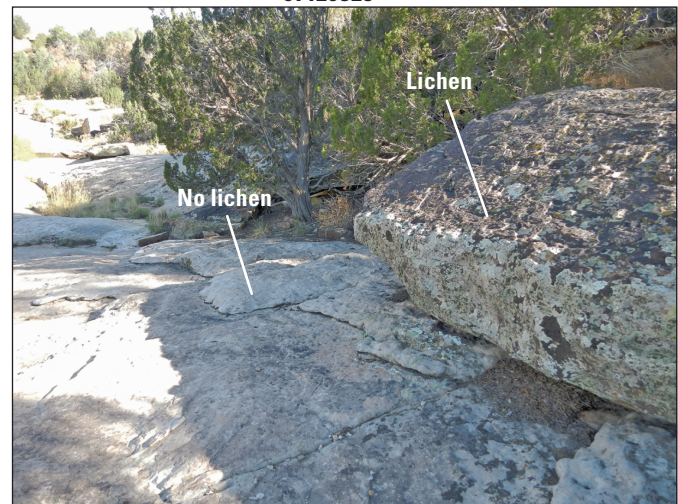
Well-developed pavements can form on time scales of hundreds to thousands of years or longer (McFadden and others, 1998; Quade, 2001; Pelletier and others, 2007).

The relative degree of weathering of rocks on the surface of terraces can also be used as an indicator of the age and stage of non-exceedance bounds. Floods will typically mobilize sediment on the surface and scour larger rocks that are not mobilized. Rock tumbling and scour of large rocks by other rocks or debris in the flood will remove weathered surfaces and thus expose fresh faces with irregular or weak weathering (fig. 7A), thus resetting the weathering “clock” on the surface of the clast. Conversely, terrace surfaces that have not been inundated for a long period of time will start to develop rills and other small scale surface drainage features. Lichen growing on the top of rocks on terrace surfaces also indicates

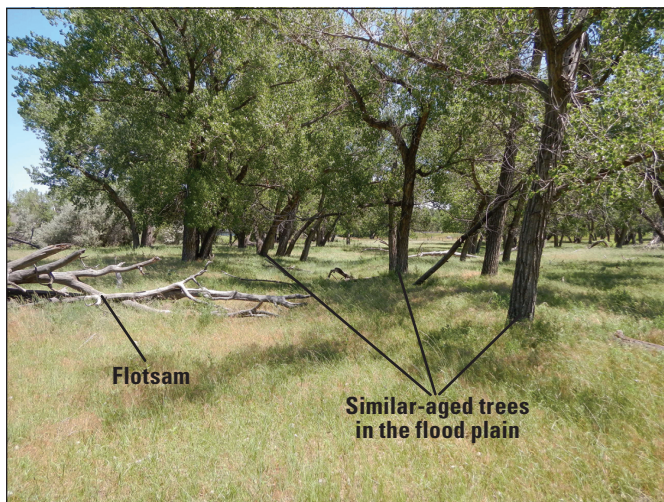
A. Taylor Arroyo below Rock Crossing, near Thatcher, Colorado, 07126325



B. Taylor Arroyo below Rock Crossing, near Thatcher, Colorado, 07126325



C. Arikaree River at Haigler, Nebraska, 06821500



D. Blue Creek near Lewellen, Nebraska, 06687000



Figure 7. Evidence used to determine non-exceedance bounds. A, freshly scoured rock (right) and weathered rock (left) at 07126325 Taylor Arroyo below Rock Crossing, near Thatcher, Colo., B, lichens that have not been scoured from recent floods (top right) and freshly scoured rock (bottom left) at 07126325 Taylor Arroyo below Rock Crossing, near Thatcher, Colo., C, similar aged stand of cottonwood trees at 06821500 Arikaree River at Haigler, Nebr., D, old cottonwood at 06687000 Blue Creek near Lewellen, Nebr. Photographs by Tessa Harden, USGS (A–D).

that the sediment has not been mobilized during the growth of the lichen (fig. 7B) because these organisms would likely be scoured from rock surfaces during a flood over the terrace surface (Gregory, 1976). Although there are challenges with estimating the age of lichen for some species (Beschel, 1973; Birkeland, 1973; Osborn and others, 2015), lichen can be used along with other indicators to estimate a minimum time over which the surface has not been inundated by a large flood.

Vegetation can aid in defining a non-exceedance bound in several ways. Assuming no anthropogenic disturbance, a stand of trees of similar age located along a river on a flood bar, terrace, or other landform, could indicate the last time a large flood of a particular magnitude occurred on the river (fig. 7C). This is particularly true for the ephemeral streams in the semi-arid regions of the study area. Cottonwoods are the dominant riparian tree in this area and have been shown to repopulate surfaces after large floods because of the abundance of bare moist ground (Scott and others, 1997; Mahoney and Rood, 1998). The age of the large flood would be close to the age of the trees assuming trees started to regrow after the flood (Friedman and others, 1996; Lukas and Woodhouse, 2006). Conversely, old trees rooted on the surface of a terrace might indicate an absence of floods large enough to significantly overtop the surface and either partially bury the tree with flood sediment or uproot the tree (fig. 7D); (Scott and others, 1997). East of Denver (fig. 1), in eastern Colorado, Friedman and Lee (2002) documented sites that have not experienced large floods in the last 80 years that have limited cottonwood production, a low tree density, and more abundant grassland than sites that have experienced a more recent large flood. The age of trees was determined in this study by using previously developed relations between tree diameter and growth rates published by the International Society of Arboriculture (Missouri Department of Conservation, 2015; Nix, 2015).

Paleoflood and non-exceedance bound ages were established based on reconnaissance-level methods as discussed previously. Multiple lines of evidence were used at each streamgage to determine the age estimate to add a higher degree of certainty to reconnaissance-level estimates. Absolute ages, such as from radiocarbon dating, were not obtained for any of the streamgages. However, because the focus of this study was on floods with annual-exceedance probabilities (AEPs) of 0.2 percent and greater, an area of the calibration curve where radiocarbon dating can have low resolution (Stuvier and Becker, 1986; Stuvier and Pearson, 1986), age estimates using a combination of geomorphic characteristics, vegetation, historical artifacts, and soil properties are credible and can provide comparable age range estimates to those obtained by radiocarbon dating. Paleoflood or non-exceedance bound evidence was documented at 41 streamgages and 3 streamgages had previously collected paleoflood data (Jarrett, 2000; England and others, 2003) for a total of 44 streamgages with paleoflood or non-exceedance bound evidence to be used to compute the AEPDs.

Computing Discharge of Paleofloods

To determine the peak discharge of a paleoflood or non-exceedance bound, two different hydraulic models were used. A hydraulic model is used to predict streamflow characteristics including water-surface elevation of a free-flowing channel given a particular discharge or discharge-time series. Models were constructed using cross sections, paleoflood stages, channel slope, and any existing high-water marks surveyed using survey-grade Global Navigation Satellite Systems following the methods described in Rydlund and Densmore (2012).

Hydrologic Engineering Center River Analysis System Computation

For streamgages where only two cross sections were surveyed, the U.S. Army Corps of Engineers Hydrologic Engineering Center River Analysis System (HEC-RAS) version 4.1 was used to simulate streamflow. HEC-RAS is applicable to a wide range of scenarios, including large networks of regulated or natural streams or rivers to constructed channels (U.S. Army Corps of Engineers, 2010a). HEC-RAS is a one-dimensional depth-averaged hydraulic model capable of computing water-surface elevations and horizontal-velocity components for subcritical and supercritical free-surface flow regimes (U.S. Army Corps of Engineers, 2010b). The model is designed for problems in which vertical accelerations are negligible, and velocity vectors generally point in the same direction over the entire depth of the water column at any discrete period in time (U.S. Army Corps of Engineers, 2010b). The Manning roughness coefficients determined in the field using the Cowan method (Cowan, 1956) and subdivided into three sections (left bank, channel, and right bank) were included in the models. The HEC-RAS models were configured with discharge as the upstream boundary condition and the channel slope was used for the normal depth downstream boundary condition. The channel slope was either surveyed in the field from channel invert elevations or high-water mark profiles or was determined from a 1:24,000 scale topographic map. Iterations of various discharges were modeled using HEC-RAS until the water-surface profile matched the stage of the paleoflood identified in the field. The corresponding discharge was then used for the paleoflood or non-exceedance bound.

Slope-Area Computation

For streamgages where three or more cross sections were surveyed, the USGS Slope-Area Computation Graphical User Interface (SACGUI) program was used (Bradley, 2012). In the slope-area method, discharge is computed on the basis of a uniform-flow equation involving channel characteristics, water-surface profiles, and a roughness or retardation coefficient (Dalrymple and Benson, 1967). The slope-area method is based on one-dimensional gradually varied, steady-flow equations and uses the conservation of energy and mass and the normal-flow equation to determine discharge (Fulford,

1994). The SACGUI program uses a high-water mark profile which was determined from the channel slope. The stage of the high-water marks was adjusted while keeping the slope constant until the water-surface profile matched the stage of the paleoflood. Resulting discharge values were used for the paleoflood or non-exceedance bound estimate. An example computation from SACGUI for 06758700 Middle Bijou Creek Tributary near Deer Trail, Colo. is located in Appendix 1.

Model Uncertainty

Estimates of discharge uncertainty for paleofloods and non-exceedance bounds can be used by the USGS software program PeakFQ (Veilleux and others, 2014) to produce more accurate flood-frequency analysis by using the upper and lower limits of the uncertainty in the paleodischarge estimate. The lower limit discharge was computed by using the lowest stage of the paleoflood or non-exceedance bound evidence, increasing the Manning roughness coefficient by 20 percent, and decreasing the channel slope by 25 percent. The upper limit discharge was computed by using the highest stage of the paleoflood or non-exceedance bound plus 1 foot (3 feet for non-exceedance bound evidence), decreasing the Manning roughness coefficient by 20 percent, and increasing the channel slope by 25 percent. These uncertainty thresholds were chosen because the thresholds fully encompass the range of reasonable values that could potentially have occurred. The type, discharge, age, uncertainties, and computation method of the paleoflood data for all 44 streamgages where paleoflood information was used to develop the PSRREs for peak-streamflow are located in Appendix 2.

Flood-Frequency Analysis

The annual series of peak-streamflow data at 188 continuous-record streamgages and crest-stage gages were used to estimate AEPDs, such as the 100-year flood discharge. AEPDs from streamgage data provide the basis for developing PSRREs. The estimated AEPDs for each streamgage were computed using the USGS software program PeakFQ version 7.1 (Veilleux and others, 2014) for annual-peak streamflows. The AEPDs were determined using systematic data through water year 2013 (October 1, 2012, through September 30, 2013). This report expresses flood-frequency estimates in terms of AEPs, which are the reciprocals of the recurrence intervals. AEP can also be represented in percent, and a particular flood-frequency estimate is then termed the “*P*-percent chance discharge,” where *P* is the probability, in percent, that the discharge will be equaled or exceeded in any year. For example, a 10-year flood discharge is the same as a discharge having a 0.10 AEP; this flood discharge also is described as a 10-percent flood or $Q_{10\%}$ (Southard and Veilleux, 2014).

For this report, the log-Pearson Type III frequency distribution was fit to the logarithms of the annual-peak flows to determine flood-frequency estimates following the guidelines established in Bulletin 17B (Interagency Advisory Committee

on Water Data, 1982). The mean, standard deviation, and skew coefficients were used to fit the distribution to describe the mid-point, slope, and curvature of the flood-frequency curve, respectively (Gotvald and others, 2012). Estimates of the *P*-percent AEPDs for each streamgage are computed by inserting the three statistics of the frequency distribution into the equation:

$$\log Q_p = \bar{X} + K_p S \quad (1)$$

where

- Q_p is the *P*-percent annual exceedance-probability discharge, in cubic feet per second;
- \bar{X} is the mean of the base 10 logarithms of the annual peak discharges;
- K_p is a factor based on the skew coefficient and the given percent AEP and is obtained from Appendix 3 in Bulletin 17B (Interagency Advisory Committee on Water Data, 1982); and
- S is the standard deviation of the logarithms of the annual peak discharges, which is a measure of the degree of variation of the annual values about the mean value.

The skew coefficient is a measure of the asymmetry of the frequency distribution and is greatly affected by the presence of high or low outliers (annual peaks that are substantially higher or lower than the trend of the data). Large positive skews typically are the result of high outliers, and large negative skews typically are the result of low outliers (Southard and Veilleux, 2014).

Skew Analysis

The skew coefficient for a streamgage is sensitive to outliers. Therefore, the station skew coefficient for streamgages with short records, common in Colorado, may not provide an accurate estimate of the data or true skew coefficient (Gotvald and others, 2009; Feaster and others, 2009; Weaver and others, 2009). Bulletin 17B (Interagency Advisory Committee on Water Data, 1982) guidelines fit a Pearson Type III distribution to the product moments (mean, standard deviation, and skew) of the logarithms of annual peak streamflow. To compensate for effects of short record at a station, station skew is combined or weighted with a generalized value that is derived from a generalized or weighted skew map, which is included in Bulletin 17B (Interagency Advisory Committee on Water Data, 1982). The weighted skew used in the analysis for this report was determined by weighting the streamgage skew and the regional skew and is inversely proportional to their respective mean square errors, as shown in equation 5 of Bulletin 17B (Interagency Advisory Committee on Water Data, 1982):

$$G_w = [MSE_{G_R}(G_s) + MSE_{G_S}(G_R)] / (MSE_{G_R} + MSE_{G_S}) \quad (2)$$

where

- G_w is the weighted skew,
- G_S is the station skew,
- G_R is the regional skew, and

MSE_{G_R} and MSE_{G_S} are the mean square errors of the regional and station skew, respectively.

The generalized or regional skew used in this report was from the national generalized skew map (plate I, Bulletin 17B [Interagency Advisory Committee on Water Data, 1982]) based on streamgage data through water year 1973, which is the most current skew map in Colorado. Additional information on skew can be found in Bulletin 17B (Interagency Advisory Committee on Water Data, 1982) and in Eash and others (2013).

Expected Moments Algorithm

In this study, the Expected Moments Algorithm (EMA) with the multiple Grubbs-Beck (MGB) test method (Grubbs and Beck, 1972) was used to compute Log-Pearson Type III exceedance-probability estimates for all 188 streamgages evaluated to develop PSRREs for eastern Colorado. The USGS software program PeakFQ version 7.1 (Veilleux and others, 2014) automates the EMA/MGB procedure described in this section of the report.

The EMA retains the essential structure and moments-based approach of the existing Bulletin 17B (Interagency Advisory Committee on Water Data, 1982) procedures to determine flood frequency but addresses several concerns about the methods used in Bulletin 17B. The EMA can accommodate interval data, simplifying the analysis of datasets containing censored observations, historic data, low outliers, and data points with high and low uncertainties common in paleofloods, while also providing enhanced confidence intervals for the AEPDs. Unlike Bulletin 17B (Interagency Advisory Committee on Water Data, 1982), which recognizes only two types of data: (1) systematic (annual streamflow peaks observed during systematic streamgage record) and (2) historic (streamflow peaks observed outside the streamgage record), the EMA employs a more general description of the historical period (the length of time that includes both systematic and historic peaks). This is accomplished through flow intervals to describe the peak streamflow in each year and perception thresholds to describe the range of measurable potential discharges in each year. Non-exceedance bounds, which were discussed in detail in the “Other Paleostage Indicators” section, are equivalent to perception thresholds and were adjusted when non-exceedance bound data were documented at a streamgage. Additional information on the EMA can be found in Eash and others (2013) and Southard and Veilleux (2014).

Multiple Grubbs-Beck Test for Detecting Potentially Influential Low Floods

Bulletin 17B (Interagency Advisory Committee on Water Data, 1982) recommends the use of the Grubbs-Beck test (Grubbs and Beck, 1972) to statistically identify low outliers in a sample of flood data. The MGB test is a generalization of the Grubbs-Beck method that creates the standard procedure for recognizing multiple Potentially Influential Low Floods (PILFs) (Cohn and others, 2013). In flood-frequency analysis, PILFs are annual streamflow peaks that meet three criteria: (1) their magnitude is much smaller than the flood quantile of interest; (2) PILFs occur below a statistically significant break in the flood-frequency plot; and (3) PILFs have great significance or leverage on the estimated frequency of large floods (Southard and Veilleux, 2014). The MGB test screens for PILFs at each streamgage and excludes them from the flood-frequency analysis. When an observation is identified as a PILF, the value of the smallest observation in the dataset determined to not be a PILF (Q_s) is used as the censoring threshold in the EMA analysis (Southard and Veilleux, 2014). All annual peaks smaller than this value will be treated as censored observations with flow intervals equal to $(0, Q_s)$ and perception thresholds equal to (Q_s, inf) (Southard and Veilleux, 2014). Identifying PILFs and recording them as censored peaks can greatly improve estimator robustness with little or no loss of efficiency (Southard and Veilleux, 2014). Thus, the use of the MGB test can improve the fit of the AEPDs, while minimizing lack-of-fit because of unimportant PILFs in an annual peak series (Cohn and others, 2013; Veilleux and others, 2014).

It is important to distinguish between low outliers and PILFs. Low outlier typically refers to one or possibly two values in a dataset that are assumed to be homogenous and that do not conform to the trend of the other observations (Southard and Veilleux, 2014). In contrast, PILFs may constitute up to one-half of the observations and are assumed to result from physical processes that are not relevant to the processes associated with large floods. Consequently, the actual magnitudes of PILFs, because PILFs reflect physical processes that are not relevant to large floods, reveal little about the upper right-hand tail of the frequency distribution representing large floods, and thus, should not have an effect when estimating the risk of large floods (Southard and Veilleux, 2014). The term “low outlier” has been replaced with the term “PILF” to more accurately describe the situation (Interagency Advisory Committee on Water Data, 2014). Additional information on the MGB test and PILFs can be found in Gotvald and others (2012), Eash and others (2013), and Southard and Veilleux (2014).

The USGS software program PeakFQ version 7.1 (Veilleux and others, 2014) was used to compute the flood-frequency estimates for streamgages presented in this report. PeakFQ automates the EMA/MGB procedure described in this section of the report. The AEPDs from all 188 streamgages used to develop the PSRREs are given in Appendix 3. All output and input files used and generated by PeakFQ for all 188 streamgages and extra runs prior to the addition of paleoflood data including the specifications file are in Appendix 4.

For the 44 streamgages where paleoflood or non-exceedance bound data existed, the period of record, perception thresholds, and flood data were determined, and PeakFQ was run with and without the paleoflood data. The PeakFQ analyses with and without the paleoflood data were done so that when the PSRREs were developed, the effects of the paleoflood data could be identified.

In PeakFQ, when paleoflood or non-exceedance bound evidence was available at the streamgage, the beginning of the period of record (including the systematic record) was entered as the age of the evidence (in Julian years [defined as 365.25 days which has been used in western societies for centuries]) and the end of the period of record (including the systematic record) was 2013 (the last full year prior to the evidence being collected). For example, for a streamgage with a paleoflood age of 500, the period of record (including the systematic record) was 1514 to 2013. For paleoflood and non-exceedance bound evidence, the perception threshold was changed to the lower limit discharge computed from the uncertainty in the paleoflood data to infinity. This was done because the low perception threshold cannot be larger than the lower limit discharge computed from the uncertainty in the paleoflood data because smaller floods were not capable of being perceived at the gage based on the lower limit discharge. For paleoflood evidence only, the age (in Julian years) and the low and high limits of discharge were entered in PeakFQ. An example PeakFQ analysis with and without the inclusion of paleoflood data is given in Appendix 1 for 06758700 Middle Bijou Creek Tributary near Deer Trail, Colo. All the PeakFQ input and output files for all 188 streamgages can be found in Appendix 4.

Basin and Climate Characteristics

Based on previous studies conducted in Colorado and neighboring States (Kircher and others, 1985; Soenksen and others, 1999; Vaill, 1999; Rasmussen and Perry, 2000; Miller, 2003; Waltemeyer, 2008; Capesius and Stephens, 2009; and Lewis, 2010) and on the availability of data, 72 characteristics (57 basin and 15 climatic characteristics) were evaluated as candidate explanatory variables in the regression analysis (table 2). The 72 characteristics consist of physical properties of the basin, precipitation, land cover, and soil characteristics. Environmental Systems Research Institute, Inc. (Esri), ArcMap 10.1 (Esri, 2014) was used to compute the basin and climate characteristics, which were used as potential explanatory variables in the PSRREs. Values for all 72 characteristics were calculated for the 188 streamgages using various Esri ArcMap 10.1 (Esri, 2014) techniques, tools, and algorithms (table 2).

Esri ArcMap 10.1 (Esri, 2014) analyses were performed to determine basin and climate characteristics that could be used in the PSRREs at each streamgage. The basin perimeter delineations were performed using the National Hydrography Dataset Plus (McKay and others, 2014) and elevation data

were analyzed to produce the parameter data listed in table 2. The elevation, mean precipitation, precipitation frequency, soil, surface lithology, and land cover data were processed with the basin outlines using the National Water Quality Assessment Area-Characterization Toolset (Price, 2010) to produce the elevation-related parameters for each basin in table 2.

Regional-Regression Analyses

Multilinear regression was used to define statistical relations between peak streamflow and 2 or more of the 72 basin and climatic characteristics from table 2. Detailed description of the principles of regional regression is available in Helsel and Hirsch (2002) or Montgomery and others (2001). The statistical tests to evaluate model performance are described in Tasker and Stedinger (1989), Eng and others (2009), and Eash and others (2013).

Definition of Flood Regions

McCain and Jarrett (1976) originally defined five unique hydrologic regions in Colorado on the basis of physiographic and climatic characteristics. These same five hydrologic regions have been used for all Colorado peak- and low-flow studies since 1976 with minor modification (Kircher and others, 1985; Vaill, 1999; Capesius and Stephens, 2009). The scope of this report was to update the PSRREs in eastern Colorado which is defined as the Plains hydrologic region in Vaill (1999). The study area was extended 100 miles into adjacent States to include more streamgages and improve statistical robustness for development of PSRREs, because the hydrology is not affected by State boundaries. After analyzing the study area for potential regional subdivisions, dividing eastern Colorado into two new individual regions at -104° longitude resulted in PSRREs with the smallest *SME* and *SEP* and largest pseudo R^2 . The new hydrologic region located between -104° longitude and the Kansas-Nebraska State line will still be designated the Plains hydrologic region in this report. The hydrologic region comprising the rest of eastern Colorado will be designated the Foothills hydrologic region (fig. 8). The western edge of the Foothills hydrologic region was established by Vaill (1999) and this report follows that boundary. The western edge of the Foothills hydrologic region follows the 7,500-foot (ft) contour south from the Wyoming State line to the Chaffee-Fremont County line where the hydrologic region boundary follows the Chaffee-Fremont County line south across the Arkansas River and transitions up to the 9,000-ft contour, which is followed south to the New Mexico State line. The subdivision of eastern Colorado into two hydrologic regions was determined by maximizing the R^2 and $\text{adj}R^2$ and minimizing the standard error, Mallow's C_p , and the predicted residual sum of squares (PRESS) statistic

Table 2. Basin and climate characteristics evaluated for use in the peak-streamflow regional-regression equations in eastern Colorado, 2015.

[NWIS, National Water Information System; USGS, U.S. Geological Survey; NHD, National Hydrography Dataset; NED, 1 meter = 3.281 feet; National Elevation Dataset; PRISM, Parameter-Elevation Regressions on Independent Slopes Model; NOAA, National Oceanic and Atmospheric Administration; TP-40, Technical Paper 40; STATSGO, State Soil Geographic; NLCD, National Land Cover Dataset]

Characteristic	Unit	Dataset	Dataset source
Drainage area ¹ , A	square miles	NWIS	USGS (2013b)
Latitude	decimal degrees	NWIS	USGS (2013b)
Longitude	decimal degrees	NWIS	USGS (2013b)
Elevation of basin outlet ¹ , E_{out}	feet	NWIS	USGS (2013b)
Basin perimeter	feet	NHD Plus	McKay and others (2014)
Minimum basin elevation	feet	10-meter NED	Gesch (2007)
Maximum basin elevation	feet	10-meter NED	Gesch (2007)
Elevation relief	feet	10-meter NED	Gesch (2007)
Mean basin elevation	feet	10-meter NED	Gesch (2007)
Percentage of basin above 7,500 feet	percent	10-meter NED	Gesch (2007)
Percentage of basin with slopes greater than 30 percent	percent	10-meter NED	Gesch (2007)
Minimum basin slope	percent	10-meter NED	Gesch (2007)
Maximum basin slope	percent	10-meter NED	Gesch (2007)
Mean basin slope ¹ , S	percent	10-meter NED	Gesch (2007)
Mean annual precipitation	inches	800-meter PRISM	Daly and others (1994)
Mean precipitation in January	inches	800-meter PRISM	Daly and others (1994)
Mean precipitation in February	inches	800-meter PRISM	Daly and others (1994)
Mean precipitation in March	inches	800-meter PRISM	Daly and others (1994)
Mean precipitation in April	inches	800-meter PRISM	Daly and others (1994)
Mean precipitation in May	inches	800-meter PRISM	Daly and others (1994)
Mean precipitation in June	inches	800-meter PRISM	Daly and others (1994)
Mean precipitation in July	inches	800-meter PRISM	Daly and others (1994)
Mean precipitation in August	inches	800-meter PRISM	Daly and others (1994)
Mean precipitation in September	inches	800-meter PRISM	Daly and others (1994)
Mean precipitation in October	inches	800-meter PRISM	Daly and others (1994)
Mean precipitation in November	inches	800-meter PRISM	Daly and others (1994)
Mean precipitation in December	inches	800-meter PRISM	Daly and others (1994)
6-hour, 100-year precipitation ¹ , ${}_6P_{100}$	inches	NOAA Atlas 2; NOAA Atlas 14; TP-40	Bonnin and others (2011) ² , Miller and others (1973) ³ , Perica and others (2013) ⁴ , Hershfield (1961) ⁵
24-hour, 100-year precipitation	inches	NOAA Atlas 2; NOAA Atlas 14; TP-40	Bonnin and others (2011) ² , Miller and others (1973) ³ , Perica and others (2013) ⁴ , Hershfield (1961) ⁵
Mean percent of soil consisting of clay ¹ , C	percent	STATSGO dataset	Wolock (1997)
Mean percent of soil consisting of sand	percent	STATSGO dataset	Wolock (1997)
Mean permeability of soil	inches per hour	STATSGO dataset	Schwarz and Alexander (1995)
Mean available water capacity	inches per hour	STATSGO dataset	Schwarz and Alexander (1995)
Percent of hydrologic group A soils	percent	STATSGO dataset	Wolock (1997)
Percent of hydrologic group B soils	percent	STATSGO dataset	Wolock (1997)
Percent of hydrologic group C soils	percent	STATSGO dataset	Wolock (1997)
Percent of hydrologic group D soils	percent	STATSGO dataset	Wolock (1997)
Percent carbonate residual material	percent	USGS Surficial Lithology	Cress and others (2010)
Percent non-carbonate residual material	percent	USGS Surficial Lithology	Cress and others (2010)
Percent alkaline intrusive volcanic rock	percent	USGS Surficial Lithology	Cress and others (2010)
Percent silicic residual material	percent	USGS Surficial Lithology	Cress and others (2010)
Percent extrusive volcanic rock	percent	USGS Surficial Lithology	Cress and others (2010)
Percent colluvial sediment	percent	USGS Surficial Lithology	Cress and others (2010)
Percent glacial till, clayey	percent	USGS Surficial Lithology	Cress and others (2010)
Percent glacial till, loamy	percent	USGS Surficial Lithology	Cress and others (2010)
Percent glacial till, coarse textured	percent	USGS Surficial Lithology	Cress and others (2010)
Percent glacial outwash and glacial lake sediment	percent	USGS Surficial Lithology	Cress and others (2010)
Percent glacial lake sediment, fine textured	percent	USGS Surficial Lithology	Cress and others (2010)
Percent glacial outwash and glacial lake sediment	percent	USGS Surficial Lithology	Cress and others (2010)
Percent hydric, peat, and muck	percent	USGS Surficial Lithology	Cress and others (2010)
Percent eolian sediment, coarse textured	percent	USGS Surficial Lithology	Cress and others (2010)
Percent eolian sediment, fine textured	percent	USGS Surficial Lithology	Cress and others (2010)

Table 2. Basin and climate characteristics evaluated for use in the peak-streamflow regional-regression equations in eastern Colorado, 2015.—Continued

[NWIS, National Water Information System; USGS, U.S. Geological Survey; NHD, National Hydrography Dataset; NED, 1 meter = 3,281 feet; National Elevation Dataset; PRISM, Parameter-Elevation Regressions on Independent Slopes Model; NOAA, National Oceanic and Atmospheric Administration; TP-40, Technical Paper 40; STATSGO, State Soil Geographic; NLCD, National Land Cover Dataset]

Characteristic	Unit	Dataset	Dataset source
Percent saline lake sediment	percent	USGS Surficial Lithology	Cress and others (2010)
Percent alluvium and fine textured coastal zone	percent	USGS Surficial Lithology	Cress and others (2010)
Percent coastal zone sediment, coarse textured	percent	USGS Surficial Lithology	Cress and others (2010)
Percent water	percent	USGS Surficial Lithology	Cress and others (2010)
Percent open water	percent	NLCD 2006	Fry and others (2011)
Percent perennial ice/snow	percent	NLCD 2006	Fry and others (2011)
Percent developed, open space	percent	NLCD 2006	Fry and others (2011)
Percent developed, low intensity	percent	NLCD 2006	Fry and others (2011)
Percent developed, medium intensity	percent	NLCD 2006	Fry and others (2011)
Percent developed, high intensity	percent	NLCD 2006	Fry and others (2011)
Percent barren land	percent	NLCD 2006	Fry and others (2011)
Percent deciduous forest	percent	NLCD 2006	Fry and others (2011)
Percent evergreen forest	percent	NLCD 2006	Fry and others (2011)
Percent mixed forest	percent	NLCD 2006	Fry and others (2011)
Percent shrub/scrub	percent	NLCD 2006	Fry and others (2011)
Percent grassland/herbaceous	percent	NLCD 2006	Fry and others (2011)
Percent pasture/hay ^a , GP	percent	NLCD 2006	Fry and others (2011)
Percent cultivated crops	percent	NLCD 2006	Fry and others (2011)
Percent woody wetlands	percent	NLCD 2006	Fry and others (2011)
Percent woody wetlands	percent	NLCD 2006	Fry and others (2011)
Percent emergent herbaceous wetlands	percent	NLCD 2006	Fry and others (2011)

¹Variables used in the regional regression equations.

²Used for New Mexico.

³Used for Wyoming.

⁴Used for Colorado, Kansas, Nebraska, and Oklahoma.

⁵Used for Texas.

by Ordinary-Least Squares (OLS) regression procedures. Using GLS regression procedures, it was confirmed that the new regions reduced the uncertainty in the PSRREs compared to the not-subdivided region used in previous studies (Kircher and others, 1985; Vaill, 1999; Capesius and Stephens, 2009). Prior to determining that the -104° longitude minimized the PSRRE uncertainty, maps of streamgage residuals and various other subdivisions were tested based on longitude, basin outlet elevation, mean basin elevation, maximum basin elevation, drainage area, hydrologic forcing (rainfall and snowmelt compared to predominantly rainfall), and basin (Arkansas compared to lower Missouri).

Development of Peak-Streamflow Regional-Regression Equations

The PSRREs were developed for use in estimating peak streamflows for selected AEPs of 50 to 0.2 percent at gaged and ungaged locations for basins in the Plains and Foothills hydrologic regions of eastern Colorado. OLS regression techniques were performed to select the basin and climate characteristics for use as independent variables. A linear relation between the dependent and independent variables are required for OLS regression. To satisfy this criterion, variables often are transformed, and in hydrologic analyses, typically the

log-transformation is used (Southard and Veilleux, 2014). The dependent response variable is the AEPD, and the independent explanatory variables are the basin and climate characteristics that describe the AEPDs. For the current (2015) study, all variables were transformed to base 10 logarithms to increase linearity prior to the development of the PSRREs.

A constant variance in the dependent variable for the range of the independent variables, referred to as homoscedasticity, about the regression line and normality of residuals also is a criterion for OLS regression. Transformation of the AEPD, in percent, and certain other variables to base 10 logarithms can enhance the homoscedasticity of the data about the regression line (Southard and Veilleux, 2014). Linearity, homoscedasticity, and normality of residuals were examined in residual plots.

The hydrologic model used in the regression analysis in this report is of the form:

$$Q_p = aA^bB^cC^d \quad (3)$$

where

Q_p	is the dependent variable, P -percent AEPD, in cubic feet per second;
A, B, C	are explanatory (independent) variables; and
a, b, c, d	are regression coefficients.

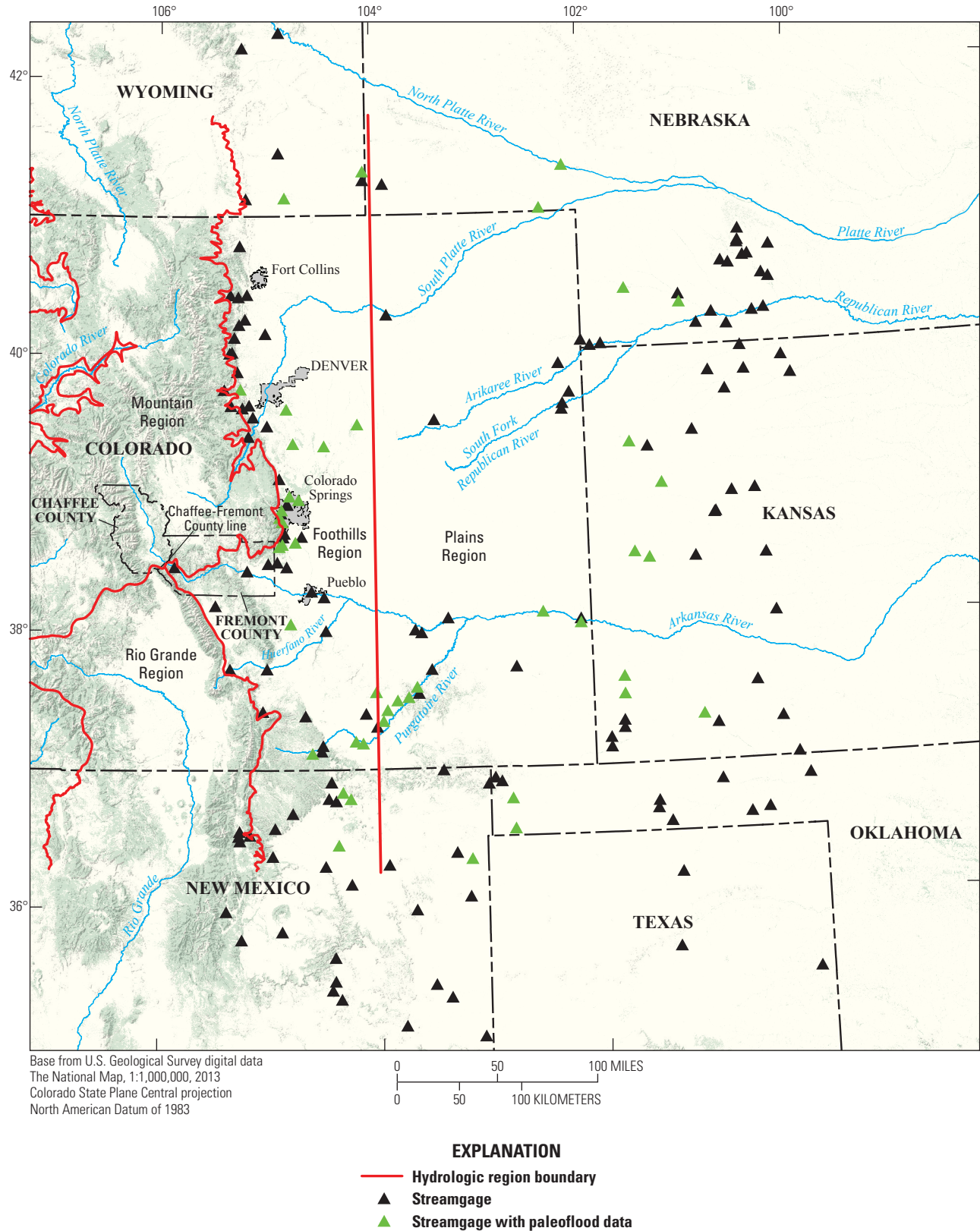


Figure 8. Boundaries of the new hydrologic regions created in eastern Colorado: the Plains hydrologic region and the Foothills hydrologic region with the location of the 99 and 89 streamgages were used to develop the peak-streamflow regional-regression equations in the Plains and Foothills hydrologic regions, respectively.

If the dependent variable Q_p and the independent variables A and B are logarithmically transformed then the hydrologic model has the following linear form:

$$\log Q_p = \log a + b(\log A) + c(\log B) + d(\log C) \quad (4)$$

where the variables are as previously defined. This equation is commonly written as:

$$Q_p = 10^a A^b B^c C^d \quad (5)$$

where the variables are as previously defined.

The basin and climate characteristics (table 2) with the best correlation to peak streamflow were identified, checked for any substantial cross-correlation with other variables in the group, and were selected as the potential explanatory variables for the PSRREs. The final AEPDs for the 188 streamgages used in the regional-regression analysis are in Appendix 3.

Ordinary-Least Squares Regression

OLS regression was used in this report as an exploratory tool to limit potential explanatory variables, to a reasonable number for subsequent analysis by GLS regression, and to help determine if new hydrologic boundaries could improve the accuracy of the PSRREs. The potential explanatory variables were assessed for linear correlation with the $Q_{1\%}$ AEPD streamflow using plots, parametric and nonparametric correlation coefficients, and the statistical significance of each explanatory variable when regressed with peak streamflow (p -values), to determine the strongest predictors of peak streamflow. Transformations of equation variables were considered to determine potential for improvement of correlations with streamflow and conformance to the assumptions of linear regression application. Logarithmic transformation (base 10) of both the streamflow and the explanatory variables prior to OLS regression was found to improve normality and homoscedasticity (assumptions for parametric regression), and in most cases, substantially improve both $\text{adj}R^2$ and statistical significance of the slope of each explanatory variable in a regression with streamflow.

The best OLS regression models (with one-, two-, three-, and four-explanatory variables using the best of all subsets routine) for prediction of $Q_{1\%}$ AEPD streamflow were identified and then used to limit the number of potential explanatory variables in the subsequent GLS analyses. The best PSRREs were determined by assessing the following metrics for each model: Standard Error of Estimate, $\text{Adj}R^2$, Mallow's C_p , and PRESS statistics. Statistical diagnostics and plots also were used to assess the regression models for meeting the assumptions of parametric regression methods. The $\text{adj}R^2$ statistic is maximized and the standard error of estimate, Mallow's C_p , and PRESS statistics are minimized with accurate sets of independent variables in a regression model that explain more of

the variance in the dependent variable. Additional information on OLS regression can be found in Eash and others (2013) and Southard and Veilleux (2014).

Multicollinearity (high correlation among the explanatory variables) can make results based on a multiple linear regression model misleading or erroneous, and would generally disqualify the use of both variables within a single final PSRRE. Early in the variable selection process, parametric and nonparametric correlation coefficients and plots of a particular explanatory variable with each of the other explanatory variables were used to make preliminary assessments of potential multicollinearity. When assessing candidate variables and the apparent best OLS PSRRE for further refinement by GLS regression, multicollinearity was assessed primarily by using the variance inflation factor statistic (Gotvald and others, 2012) to screen for correlated or unnecessary variables in candidate PSRREs.

Generalized-Least Squares Regression

GLS multilinear regression, as described by Stedinger and Tasker (1985), Tasker and Stedinger (1989), and Griffis and Stedinger (2007), is a method of weighting streamflow station AEPD data in the regression analysis according to differences in streamflow reliability (record lengths) and variability (record variance), and according to spatial cross correlations of concurrent streamflow among streamgages. Comparison of OLS, Weighted Least Squares (WLS), and GLS in a study by Stedinger and Tasker (1985) indicated that the weighted methods (WLS and GLS) produced results that were more accurate than the OLS regression method. When streamflow records of varying lengths or with correlated concurrent flows occurred in the dataset, the weighting technique used in GLS produces equations that are both improved in estimates of streamflow statistics and estimates of the predictive accuracy of the statistics (Stedinger and Tasker, 1985).

Based on the exploratory results from OLS regression and explanatory variable multicollinearity analysis, the computer program WREG (Eng and others, 2009) was used to compute GLS PSRREs from the best candidate variables. Five basin and climate characteristics were used as potential explanatory variables to develop the PSRREs with $Q_{1\%}$ AEPD streamflows. The general form of the equation was defined by equation 4. Final GLS regression models were selected on the basis of minimizing values of the SME and the SEP , and maximizing values of $\text{pseudo}R^2$.

GLS models need to develop a correlation model and weighting matrix. The correlation smoothing function relates the correlation between annual-peak discharges at two streamgages to the geographic distance between the streamgages for every paired combination of the streamgages with a given number of years of concurrent streamflow. The correlation smoothing function was defined in equation 18 in Eng and others (2009). Using 30 years of concurrent streamflow, alpha of 0.001, and theta of 0.99, the correlation

smoothing function was used by WREG to compute a weighting matrix for the data from 89 streamgages included in the development of the GLS PSRREs for the Foothills hydrologic region. Using 40 years of concurrent streamflow, alpha of 0.001, and theta of 0.99, the correlation smoothing function was used by WREG to compute a weighting matrix for the data from 99 streamgages included in the development of the GLS PSRREs for the Plains hydrologic region.

Final Peak-Streamflow Regional-Regression Equations

A total of 89 streamgages, of which 60 are in Colorado, were used to develop the PSRREs in the Foothills hydrologic region, and 99 streamgages, of which 21 are in Colorado, were used to develop the PSRREs in the Plains hydrologic region (no streamgages were used in both regions). The Foothills hydrologic region encompasses approximately 18,900 mi², and the Plains hydrologic region encompasses approximately 29,500 mi². The selection of the final basin and climatic characteristics and the evaluation of the accuracy of the PSRREs were based on the $Q_{1\%}$ AEPD for each hydrologic region. The PSRREs from GLS regression are listed for the Foothills hydrologic region (fig. 9) and Plains hydrologic region (fig. 10).

The SEP , pseudo R^2 , and SME for each PSRRE are reported in figures 9 and 10. The performance metrics pseudo R^2 and SME describe how well the PSRREs perform on the streamgages used in the regression analyses and the SEP measures how well the GLS regression models can

predict AEPDs at ungaged sites (Eash and others, 2013).

The same four explanatory variables were used to develop all eight AEPD PSRREs in the Foothills hydrologic region, and the same three explanatory variables were used to develop all eight AEPD PSRREs in the Plains hydrologic region to minimize the possibility of predictive inconsistencies between estimates of different probabilities, so that discharge estimates will increase as the discharge probability decreases. For example, maintaining the same explanatory variables helps to maximize the chance the estimate for the $Q_{1\%}$ flood discharge is greater than the estimate for the $Q_{2\%}$ flood discharge and the estimate for the $Q_{2\%}$ flood discharge is greater than the estimate for the $Q_{4\%}$ flood discharge, and so forth for all eight AEPDs in both hydrologic regions (Eash and others, 2013).

Comparisons of the number of streamgages used in the analysis and the SEP reported in the previous study (Capesius and Stephens, 2009) for the Foothills hydrologic region, and for the Plains hydrologic region are listed in table 3. The mean SEP for all 8 AEPDs in both the Foothills and Plains hydrologic regions was reduced approximately 25 percent compared to the previous study, from 147 percent to 92 percent in the Foothills hydrologic region and to 129 percent in the Plains hydrologic region (table 3). Additional information on SEP is presented in Olson (2014).

The four explanatory variables used in the PSRREs in the Foothills hydrologic region are drainage area in mi² (A); 6-hour, 100-year precipitation in inches (${}_6P_{100}$); amount of soil consisting of clay in the basin in percent (C); and elevation of the basin outlet in feet (E_{out}). The three explanatory variables used in the PSRREs in the Plains hydrologic region are A , C ,

Peak-Streamflow Equations for the Foothills Hydrologic Region									
Generalized-least squares regression, 89 streamgages									
A , drainage area in square miles; ${}_6P_{100}$, 6-hour, 100-year precipitation; C , amount of clay in basin in percent; E_{out} , basin outlet elevation in feet; $Q_{0.5}$, $Q_{0.2}$, $Q_{0.1}$, $Q_{0.04}$, $Q_{0.02}$, $Q_{0.01}$, $Q_{0.005}$, $Q_{0.002}$, discharge with an annual exceedance probability of 0.5, 0.2, 0.1, 0.04, 0.02, 0.01, 0.005, 0.002, respectively									
Approximate range of predictor variables									
A : 0.60–2,850 square miles, ${}_6P_{100}$: 2.38–4.89 inches, C : 9.87–37.5 percent, and E_{out} : 4,290–8,270 feet									
$Q_{0.5}$	$= 10^{9.952}$	$A^{0.626}$	${}_6P_{100}^{1.401}$	$C^{0.836}$	$E_{out}^{-2.774}$	$SEP = 117,$	$pseudoR^2 = 68,$	$SME = 111,$	
$Q_{0.2}$	$= 10^{11.424}$	$A^{0.594}$	${}_6P_{100}^{2.052}$	$C^{0.935}$	$E_{out}^{-3.164}$	$SEP = 87,$	$pseudoR^2 = 77,$	$SME = 82,$	
$Q_{0.1}$	$= 10^{12.107}$	$A^{0.584}$	${}_6P_{100}^{2.372}$	$C^{0.974}$	$E_{out}^{-3.341}$	$SEP = 80,$	$pseudoR^2 = 79,$	$SME = 76,$	
$Q_{0.04}$	$= 10^{12.675}$	$A^{0.578}$	${}_6P_{100}^{2.725}$	$C^{0.998}$	$E_{out}^{-3.486}$	$SEP = 80,$	$pseudoR^2 = 80,$	$SME = 75,$	
$Q_{0.02}$	$= 10^{12.977}$	$A^{0.575}$	${}_6P_{100}^{2.963}$	$C^{1.010}$	$E_{out}^{-3.564}$	$SEP = 83,$	$pseudoR^2 = 79,$	$SME = 79,$	
$Q_{0.01}$	$= 10^{13.244}$	$A^{0.572}$	${}_6P_{100}^{3.190}$	$C^{1.013}$	$E_{out}^{-3.631}$	$SEP = 88,$	$pseudoR^2 = 78,$	$SME = 83,$	
$Q_{0.005}$	$= 10^{13.495}$	$A^{0.570}$	${}_6P_{100}^{3.386}$	$C^{1.024}$	$E_{out}^{-3.697}$	$SEP = 94,$	$pseudoR^2 = 76,$	$SME = 88,$	and
$Q_{0.002}$	$= 10^{13.820}$	$A^{0.566}$	${}_6P_{100}^{3.621}$	$C^{1.038}$	$E_{out}^{-3.783}$	$SEP = 104,$	$pseudoR^2 = 74,$	$SME = 97.$	

Figure 9. Peak-streamflow regional-regression equations for the Foothills hydrologic region. [SEP , standard error of prediction; SME , standard model error]

Peak-Streamflow Equations for the Plains Hydrologic Region				
Generalized-least squares regression, 99 streamgages				
<i>A</i> , drainage area in square miles; <i>S</i> , mean basin slope in percent; <i>C</i> , amount of clay in basin in percent; $Q_{0.5}$, $Q_{0.2}$, $Q_{0.1}$, $Q_{0.04}$, $Q_{0.02}$, $Q_{0.01}$, $Q_{0.005}$, $Q_{0.002}$, discharge with an annual exceedance probability of 0.5, 0.2, 0.1, 0.04, 0.02, 0.01, 0.005, 0.002, respectively				
Approximate range of predictor variables				
<i>A</i> : 0.26–3,560 square miles, <i>S</i> : 0.41–21.9 percent, <i>C</i> : 5.20–38.5 percent				
$Q_{0.5} = 10^{-1.033} A^{0.378} S^{0.683} C^{1.742}$	$SEP = 131$	pseudo $R^2 = 49$	$SME = 126$,	
$Q_{0.2} = 10^{-0.934} A^{0.385} S^{0.652} C^{2.046}$	$SEP = 102$	pseudo $R^2 = 57$	$SME = 98$,	
$Q_{0.1} = 10^{-0.877} A^{0.390} S^{0.640} C^{2.190}$	$SEP = 103$	pseudo $R^2 = 58$	$SME = 98$,	
$Q_{0.04} = 10^{-0.797} A^{0.396} S^{0.621} C^{2.329}$	$SEP = 113$	pseudo $R^2 = 55$	$SME = 107$,	
$Q_{0.02} = 10^{-0.728} A^{0.399} S^{0.606} C^{2.408}$	$SEP = 123$	pseudo $R^2 = 53$	$SME = 117$,	
$Q_{0.01} = 10^{-0.654} A^{0.403} S^{0.593} C^{2.470}$	$SEP = 136$	pseudo $R^2 = 50$	$SME = 129$,	
$Q_{0.005} = 10^{-0.577} A^{0.407} S^{0.581} C^{2.518}$	$SEP = 150$	pseudo $R^2 = 48$	$SME = 142$, and	
$Q_{0.002} = 10^{-0.473} A^{0.412} S^{0.567} C^{2.567}$	$SEP = 170$	pseudo $R^2 = 44$	$SME = 160$.	

Figure 10. Peak-streamflow regional-regression equations for the Plains hydrologic region. [*SEP*, standard error of prediction; *SME*, standard model error]

and mean basin slope in percent (*S*). As mentioned earlier, data transformations were tested for each explanatory variable used in the PSRREs and all variables were log-transformed because doing so provided the most linear relation for $Q_{1\%}$. All explanatory variables included in the final PSRREs for both hydrologic regions were statistically significant for all AEPDs as determined by WREG and were not correlated with other explanatory variables used in the same PSRRE.

When tested independently, the most significant explanatory variable in the Foothills hydrologic region was *A* and the second most significant explanatory variable in the Foothills hydrologic region was *E_{out}*. In the Plains hydrologic region, *A* was also the most significant explanatory variable followed by mean basin elevation which was the second most significant explanatory variable.

In figure 11, the at-site $Q_{1\%}$ AEPDs from PeakFQ were plotted with the estimated values from the PSRREs presented in figures 9 and 10, and data for both hydrologic regions document a fairly uniform distribution around the line of equality. The greatest reduction in *SEP* for the PSRREs was observed when additional basin characteristics were included in the PSRREs, particularly land-use characteristics that had never been analyzed in previous studies such as amount of soil consisting of clay in the basin in percent. Dividing eastern Colorado at -104° longitude into two separate hydrologic regions dramatically reduced the *SEP* for the Foothills hydrologic region, whereas in the Plains hydrologic region a less pronounced reduction in the *SEP* was computed (fig. 8; table 3). Expanding the study from 50 miles beyond the State line, as was done in previous studies (McCain and Jarrett, 1976; Kircher and others, 1985; Vaill, 1999; Capesius and Stephens, 2009), to 100 miles was critical in significantly reducing the *SEP* for the new Foothills hydrologic

region from that computed for the 2009 Plains hydrologic region, and also slightly decreasing the *SEP* in the new Plains hydrologic region.

Figure 12 plots the estimated $Q_{1\%}$ AEPD from the PSRREs for the 188 streamgages used in the analysis from both the Foothills and Plains hydrologic region with the estimated $Q_{1\%}$ AEPD from the PSRREs published in Capesius and Stephens (2009). In general, the PSRREs presented in the current (2015) report reduce the estimated $Q_{1\%}$ AEPD at the 188 streamgages where both sets of PSRREs were tested, particularly when the $Q_{1\%}$ AEPDs approach or exceed 20,000 cubic feet per second (ft^3/s).

Table 3. Standard error of prediction of the peak-streamflow regional-regression equations for eastern Colorado presented in the previous study (Capesius and Stephens, 2009), Foothills hydrologic region, and Plains hydrologic region.

[*SEP*, standard error of prediction; $Q_{0.5}$, $Q_{0.2}$, $Q_{0.1}$, $Q_{0.04}$, $Q_{0.02}$, $Q_{0.01}$, $Q_{0.005}$, $Q_{0.002}$, discharge with an annual exceedance-probability of 0.5, 0.2, 0.1, 0.04, 0.02, 0.01, 0.005, 0.002, respectively]

Annual exceedance- probability discharge	Previous <i>SEP</i> (69 streamgages)	Foothills <i>SEP</i> (89 streamgages)	Plains <i>SEP</i> (99 streamgages)
$Q_{0.5}$	183	117	131
$Q_{0.2}$	142	87	102
$Q_{0.1}$	136	80	103
$Q_{0.04}$	137	80	113
$Q_{0.02}$	139	83	123
$Q_{0.01}$	141	88	136
$Q_{0.005}$	160	94	150
$Q_{0.002}$	141	104	170
Mean	147	92	129

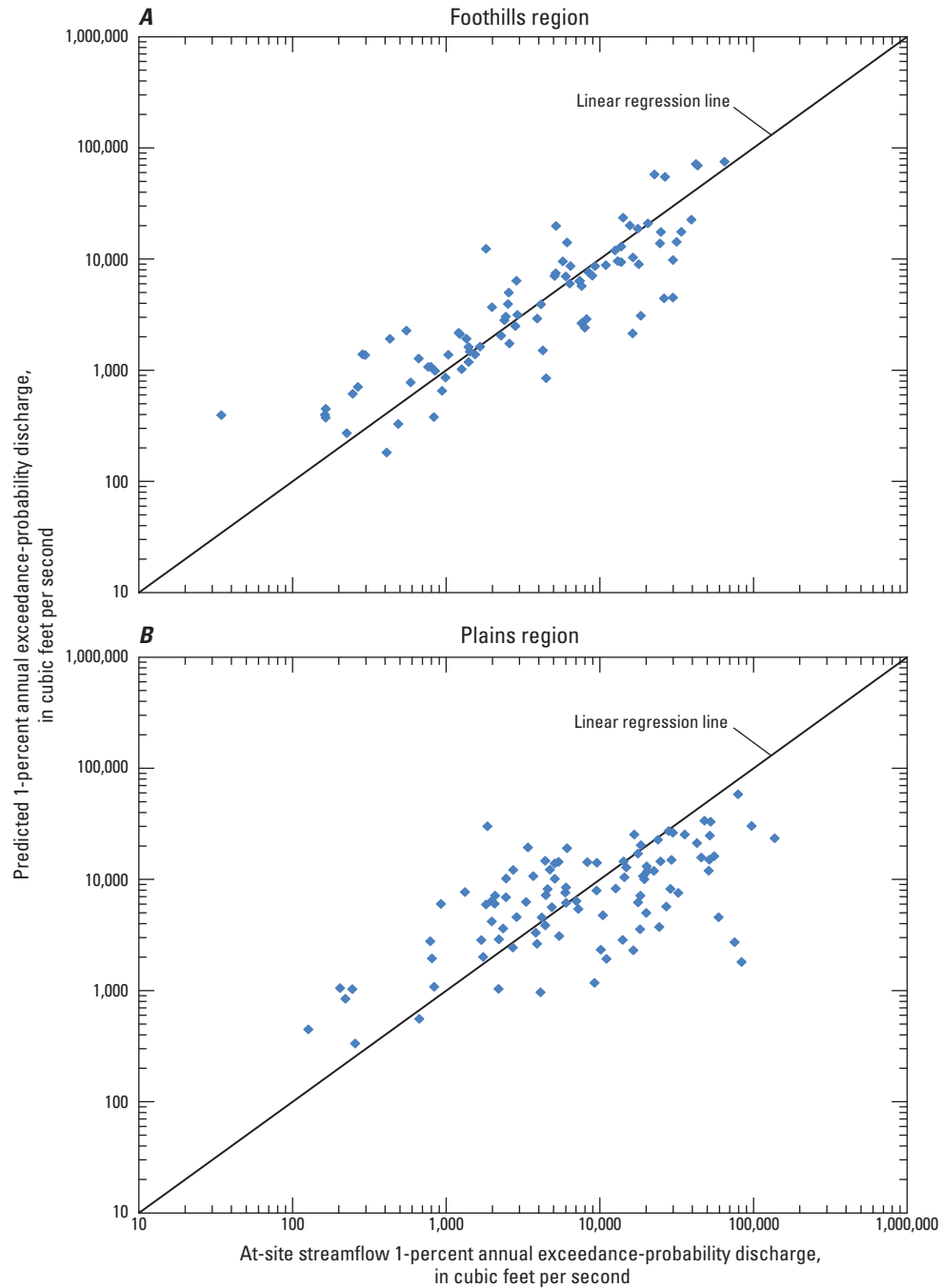


Figure 11. Relation between 1-percent annual exceedance-probability discharges computed from at-site streamflow to those predicted from the peak-streamflow regional-regression equations for the Foothills and Plains hydrologic regions of Colorado.

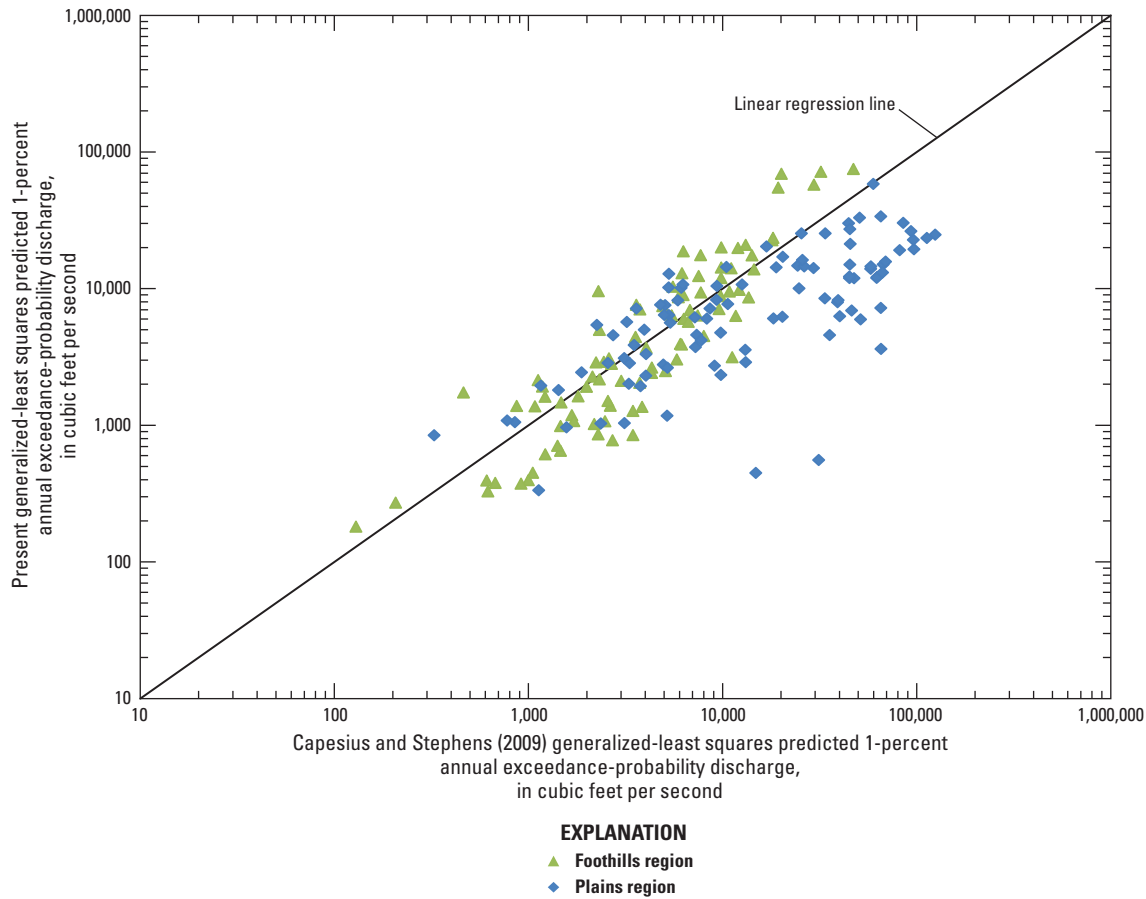


Figure 12. Relation between 1-percent annual exceedance-probability discharges computed from the peak-streamflow regional-regression equations in the Foothills and Plains hydrologic regions of Colorado and the 1-percent annual exceedance-probability discharges computed from the peak-streamflow regional-regression equations in Capesius and Stephens (2009).

Application and Limitations of Peak-Streamflow Regional-Regression Equations

This section provides four methods to estimate AEPDs at streamgages in eastern Colorado. The best method may depend on several factors: (1) if the site of interest is located at a streamgage and the record length and quality at a streamgage, where weighting of different estimates can result in more reliable AEPDs, (2) the drainage-area ratio between the site of interest and the streamgage, if the site of interest is on the same stream as a streamgage, (3) if the streamgage data are representative of the flow characteristics at the site of interest, and (4) whether or not the site of interest spans more than one hydrologic region.

Use of Peak-Streamflow Regional-Regression Equations at Streamgages

Improved estimates of AEPDs at streamgages can be obtained by weighting the AEPD EMA/MGB estimate with the PSRRE estimate as recommended by Bulletin 17B

(Interagency Advisory Committee on Water Data, 1982).

The variance of prediction is a measure of the uncertainty of both estimates and can be used to lower the uncertainty of the weighted estimate by weighting the variance of prediction of both estimates, which are inversely proportional to their associated estimates of peak flow (Gotvald and others, 2012). Assuming the estimates from PeakFQ and the PSRREs are independent, the variance of the weighted estimate will be less than the variance of either of the independent estimates (Gotvald and others, 2012). Optimal weighted estimates of AEPDs were computed for this study in Appendix 5 using the AEPDs and the corresponding variances determined by PeakFQ and WREG.

After both the variances have been computed, the two independent annual exceedance-probability estimates can be weighted using the following equation (Verdi and Dixon, 2011; Cohn and others, 2012; Gotvald and others, 2012).

$$\log Q_{P(g)w} = \frac{VP_{P(g)r} \log Q_{P(g)s} + VP_{P(g)s} \log Q_{P(g)r}}{VP_{P(g)s} + VP_{P(g)r}} \quad (6)$$

where

- $Q_{P(g)w}$ is the weighted independent estimate of annual peak flow for the selected P -percent AEP for a streamgage, g , in cubic feet per second (Appendix 5);
- $VP_{P(g)r}$ is the variance of prediction at the streamgage derived from the applicable PSRREs for the selected P -percent AEP, in log units (Appendix 5);
- $Q_{P(g)s}$ is the at-site estimate from the EMA or MGB log-Pearson Type III analysis for the selected P -percent AEP for a streamgage, g , in cubic feet per second from Appendix 3 (Appendix 5);
- $VP_{P(g)s}$ is the variance of prediction at the streamgage from the EMA or MGB log-Pearson Type III analysis for the selected P -percent AEP, in log units (Appendix 5); and
- $Q_{P(g)r}$ is the peak flow estimate for the selected P -percent AEP at the streamgage derived from the applicable PSRREs, in cubic feet per second (Appendix 5).

Weighting the variances inversely proportional minimizes the effect of an estimate with high uncertainty; likewise, if the uncertainty is low then the weight of the estimate is large (Southard and Veilleux, 2014). The computed variance of prediction associated with the weighted estimate, $VP_{P(g)w}$, is shown in the following equation (Verdi and Dixon, 2011; Gotvald and others, 2012):

$$VP_{P(g)w} = \frac{VP_{P(g)s} VP_{P(g)r}}{VP_{P(g)s} + VP_{P(g)r}} \quad (7)$$

where the variables are previously defined. The weighted AEPDs estimates that were computed from equation 6, the variance of prediction values derived from the applicable PSRREs, the PeakFQ AEPDs, the variance of prediction values for the 188 streamgages included in this study, and the peak flow estimate from the PSRREs are listed in Appendix 5.

Use of Peak-Streamflow Regional-Regression Equations on Gaged Streams

Sites of interest on streams with streamgages may have estimates determined by area weighting the AEPDs on the basis of the drainage-area ratio between the site of interest and the streamgage on the same stream (Olson, 2014). The weighting procedure is not applicable when the drainage-area ratio is less than 0.5 or greater than 1.5, or when the flood characteristics substantially change between sites (Eash and others, 2013). To compute the area-weighted estimate at the ungaged site, the $Q_{P(u)w}$ estimate (Appendix 5) for the streamgage from

equation 6 must be computed, then the area-weighted AEPD for the ungaged site, $Q_{P(u)aw}$, is then computed using the following equation (Olson, 2014):

$$Q_{P(u)aw} = \left(\frac{A_{(u)}}{A_{(g)}} \right)^b Q_{P(g)w} \quad (8)$$

where

- $Q_{P(u)aw}$ is the area-weighted estimate of flood discharge for the selected P -percent AEP for the ungaged site, u , in cubic feet per second;
- $A_{(u)}$ is the drainage area of the ungaged site, in square miles;
- $A_{(g)}$ is the drainage area of the gaged site, in square miles;
- $Q_{P(g)w}$ is described for equation 6; and
- b is the exponent of drainage area from the appropriate P -percent AEP regional exponent for the region (table 4).

Following Southard and Veilleux (2014), a GLS analysis using only drainage area (A) as an independent variable was performed to define the regional exponent for area-weighted estimates, and the resulting regional exponents ranged from 0.592 to 0.674 for the Foothills hydrologic region and from 0.331 to 0.354 for Plains hydrologic region. The resulting regional constants ranged from 1.237 to 2.828 for the Foothills hydrologic region and from 1.793 to 3.428 for Plains hydrologic region (table 4). The regional constants represent the unit drainage area for a given flood-frequency event for the reference basin (Vogel and Sankarasubramanian, 2000; Farmer and others, 2015).

Table 4. Regional exponents and constants determined from regional regression of log-transformed drainage area for area-weighting method to estimate annual exceedance-probability discharges for ungaged sites on gaged streams.

Annual exceedance probability	Foothills hydrologic region		Plains hydrologic region	
	Exponent b	Constant	Exponent b	Constant
0.5	0.674	1.237	0.333	1.793
0.2	0.642	1.720	0.331	2.305
0.1	0.628	1.969	0.333	2.556
0.04	0.616	2.231	0.337	2.817
0.02	0.609	2.395	0.340	2.985
0.01	0.602	2.543	0.344	3.134
0.005	0.597	2.673	0.348	3.268
0.002	0.592	2.828	0.354	3.428

[b , exponent of drainage area from the appropriate P -percent annual exceedance-probability regional exponent for the region]

Use of Peak-Streamflow Regional-Regression Equations on Ungaged Streams

The PSRREs can be used if the ungaged site meets the criteria for use of this method. The PSRREs presented in figures 9 and 10 are applicable for streams that are minimally affected by anthropogenic activities and urbanization within the basin. The applicable range of basin characteristics that are used as explanatory variables for the PSRREs is listed in figures 9 and 10. These PSRREs are to be used with caution at ungaged locations for which the basin characteristics are outside the range of those used to develop the PSRREs. These PSRREs also are to be used with caution at ungaged locations outside Colorado or outside the Foothills and Plains hydrologic region of Colorado. The Foothills hydrologic region has four basin-characteristic ranges for applying the PSRREs. For the Foothills hydrologic region, the applicable range for drainage area is from 0.60 to 2,850 mi², the applicable range for 6-hour, 100-year precipitation is from 2.38 to 4.89 in., the applicable range for amount of soil consisting of clay in basin is from 9.87 to 37.5 percent, and the applicable range for basin outlet elevation is from 4,290 to 8,270 ft. The Plains hydrologic region has three basin-characteristics ranges for applying the PSRREs. For the Plains hydrologic region, the applicable range for drainage area is from 0.26 to 3,560 mi², the applicable range for mean basin slope is from 0.41 to 21.9 percent, and the applicable range for amount of soil consisting of clay in basin is from 5.20 to 38.5 percent.

Use of Peak-Streamflow Regional-Regression Equations on Ungaged Streams in Two Hydrologic Regions

For an ungaged site on a stream that crosses hydrologic regions, the PSRREs for each region can be applied separately using basin characteristics for the entire drainage basin above the ungaged site (Gotvald and others, 2012). The individual peak flows from each region can then be weighted by the proportion of drainage area within each region and added to produce final estimates for the ungaged site. For example, if 25 percent of the drainage area at an ungaged site is in the upstream region and 75 percent is in the downstream region, the discharge estimate based on PSRREs for the upstream region are multiplied by 0.25 and added to 0.75 times the regression estimate based on PSRREs for the downstream region. The variance of prediction for such a weighted estimate can also be approximated by using the same weighting procedure based on proportional drainage areas (Gotvald and others, 2012).

Effect of Paleoflood Data on the Peak-Streamflow Regional-Regression Equations

The amount of information collected from paleoflood data varied across the 44 streamgages where paleoflood data were collected. For some sites, the paleoflood data were informative and added hundreds or even thousands of years of record, where at other sites the information collected was minimal with only a decade or two added to the period of record. Appendix 2 lists the estimated discharge, uncertainty of the discharge, age, and uncertainty of the age for all the paleoflood data used in the analysis at the 44 streamgages where paleoflood data were available. Paleoflood data can be used in the context of the EMA to extend the record length and the amount of flood information known at a streamgage. While not comparable to adding equivalent years of systematic record, the addition of up to hundreds of years of paleoflood data to a streamgage can substantially improve the low probability AEPDs. Increasing the record length at streamgages where paleoflood data are collected provides more weight to these streamgages when the GLS regression is performed. The new record length created by the paleoflood data, the effective record length, is determined from the age of the paleoflood data and the AEPD and also includes the existing systematic data; the effective record length was computed using computer programming code written in R (Venables and Smith, 2014) following techniques outlined in Stedinger and Cohn (1986) and Cohn and others (2001). Table 5 presents the record length and resulting effective record length added for each AEPD at the 44 streamgages where paleoflood data were available. Table 6 presents the record length by hydrologic region and the record length added by including paleoflood data at 44 of the 188 streamgages. While the mean age of paleoflood data at the 44 streamgages was 630 years (Appendix 2), that only resulted in a mean of 28 and 21 years (table 6) of increase in effective record length for the 1-percent AEPD when included with all 89 and 99 streamgages in the Foothills and Plains hydrologic regions, respectively. In addition, 24 of the 44 streamgages with paleoflood data have effective record lengths of 100 years or less even after the addition of the paleoflood data for the 1-percent AEPD (table 5).

At the 44 streamgages where paleoflood data were available, the mean record length prior to the inclusion of paleoflood data was 23 and 30 years in the Foothills and Plains hydrologic regions, respectively (tables 5 and 6). After the inclusion of the paleoflood data, the mean record length increased to 24–363 years (depending on AEPD) and 30–268 years (depending on AEPD) in the Foothills and Plains hydrologic regions, respectively (table 5). With the increase in record length, the mean *SEP* of the 8 AEPDs was reduced from 145 to 129 percent (10.7 percent mean reduction) and even more substantially for AEPDs of 1-percent and less (100-year flood discharge and greater) when the paleoflood data were added to the analyses

Table 5. Number of years of record at the 44 streamgages where paleoflood data were collected.[USGS, U.S. Geological Survey; $Q_{0.5}$, $Q_{0.2}$, $Q_{0.1}$, $Q_{0.04}$, $Q_{0.02}$, $Q_{0.01}$, $Q_{0.005}$, $Q_{0.002}$, discharge with an annual exceedance-probability of 0.5, 0.2, 0.1, 0.04, 0.02, 0.01, 0.005, 0.002, respectively; NA, not applicable]

USGS streamgage number	Streamgage name	Hydro- logic region	Number of systematic peaks	Number of historic peaks	Number of paleoflood peaks	$Q_{0.5}$ effective record length	$Q_{0.2}$ effective record length	$Q_{0.1}$ effective record length	$Q_{0.04}$ effective record length	$Q_{0.02}$ effective record length	$Q_{0.01}$ effective record length	$Q_{0.005}$ effective record length	$Q_{0.002}$ effective record length
06687000	Blue Creek near Lewellen, Nebr.	Plains	81	0	1	80	81	81	81	81	81	80	80
06712000	Cherry Creek near Franktown, Colo.	Foothills	74	0	1	77	79	105	194	328	548	888	1,574
06712500	Cherry Creek near Melvin, Colo.	Foothills	30	0	1	31	32	43	89	169	334	692	2043
06719500	Clear Creek near Golden, Colo.	Foothills	62	0	1	64	64	65	67	68	69	69	69
06755960	Crow Creek at 19th Street, at Cheyenne, Wyo.	Foothills	23	0	1	22	30	45	73	89	96	96	93
06758200	Kiowa Creek at Kiowa, Colo.	Foothills	10	0	1	11	15	23	45	64	79	83	82
06758700	Middle Bijou Creek Tributary near Deer Trail, Colo.	Foothills	10	0	1	12	18	31	82	156	234	266	247
06762600	Lodgepole Creek Tributary Number 2 near Albin, Wyo.	Foothills	25	0	1	25	25	27	30	32	33	34	35
06763500	Lodgepole Creek at Ralton, Nebr.	Plains	36	1	1	37	41	51	65	72	76	77	78
06831500	Frenchman Creek near Imperial, Nebr.	Plains	31	0	1	31	36	45	59	66	69	70	70
06835100	Bobtail Creek near Palisade, Nebr.	Plains	13	0	1	13	17	26	54	92	141	189	231
06844800	South Fork Sappa Creek Tributary near Goodland, Kans.	Plains	33	0	1	34	39	46	55	59	61	61	61
06858500	North Fork Smoky Hill River near McAllaster, Kans.	Plains	33	0	1	33	36	48	85	130	180	225	264
07099215	Turkey Creek near Fountain, Colo.	Foothills	30	0	1	30	34	44	72	97	119	134	144
07099220	Little Turkey Creek near Fountain, Colo.	Foothills	10	0	1	10	11	14	22	31	41	50	60
07103703	Camp Creek at Garden of the Gods, Colo.	Foothills	22	0	1	23	29	42	72	97	115	125	128
07103960	Kettle Creek above U.S. Air Force Academy, Colo.	Foothills	13	0	2	13	14	18	27	35	44	50	57
07103977	Cottonwood Creek at Cowpoke Road at Colorado Springs, Colo.	Foothills	11	0	1	11	11	11	11	11	10	10	10
07105000	Bear Creek near Colorado Springs, Colo.	Foothills	22	0	2	22	22	24	28	31	33	35	37
07105490	Cheyenne Creek at Evans Avenue at Colorado Springs, Colo.	Foothills	22	0	1	22	24	28	35	40	44	47	48
07105940	Little Fountain Creek near Fountain, Colo.	Foothills	12	0	1	13	17	24	39	52	64	71	77
07107500	St. Charles River at Burnt Mill, Colo.	Foothills	27	0	2	28	35	49	78	100	116	124	126
07120620	Big Arroyo near Thatcher, Colo.	Foothills	30	0	1	31	33	47	103	210	447	972	2,596
07124300	Long Canyon Creek near Madrid, Colo.	Foothills	18	0	1	18	19	22	30	35	38	40	41
07125100	Frijole Creek near Alfalfa, Colo.	Foothills	13	2	2	21	32	47	66	71	71	69	65
07125500	San Francisco Creek near Alfalfa, Colo.	Foothills	15	0	2	16	19	28	54	83	111	131	142
07126200	Van Bremer Arroyo near Model, Colo.	Plains	48	0	1	48	50	58	74	85	92	97	100
07126325	Taylor Arroyo below Rock Crossing, near Thatcher, Colo.	Plains	31	0	1	31	35	45	69	89	106	118	125
07126390	Lockwood Canyon Creek near Thatcher, Colo.	Plains	30	0	1	32	40	52	71	80	83	83	82
07126415	Red Rock Canyon Creek at Mouth near Thatcher, Colo.	Plains	31	0	1	31	31	33	35	37	38	38	39
07126480	Bent Canyon Creek at Mouth near Timpas, Colo.	Plains	29	0	1	29	32	42	64	83	99	110	118
07134100	Big Sandy Creek near Lamar, Colo.	Plains	33	1	1	34	39	46	55	60	62	64	64
07135000	Two Butte Creek near Holly, Colo.	Plains	10	0	1	12	17	29	70	126	181	204	195
07138600	White Woman Creek Tributary near Selkirk, Kans.	Plains	39	0	1	39	43	54	82	107	130	148	164
07138650	White Woman Creek near Leoti, Kans.	Plains	20	0	1	20	25	37	83	164	321	587	1,016
07155100	Cold Springs Creek near Wheelless, Okla.	Plains	17	1	1	19	24	35	60	81	97	105	106
07156100	Sand Arroyo Creek near Johnson, Kans.	Plains	15	0	1	16	20	31	72	145	274	447	601
07156220	Bear Creek near Johnson, Kans.	Plains	32	0	1	32	37	47	64	75	82	85	86
07156600	Cimarron River Tributary near Moscow, Kans.	Plains	33	0	2	34	36	44	63	77	87	94	98
07201200	Chicorica Creek Tributary near Raton, N. Mex.	Foothills	31	0	1	31	34	44	71	98	126	149	171
07201450	Green Mountain Arroyo near Raton, N. Mex.	Foothills	12	0	1	13	19	32	72	122	172	199	198
07203600	Rio Del Plano Tributary near Taylor Springs, N. Mex.	Foothills	12	1	2	13	15	20	40	69	118	191	317
07227300	Sand Draw near Clayton, N. Mex.	Plains	18	0	1	19	22	32	70	138	281	565	1,253
07232650	Aqua Frio Creek near Felt, Okla.	Plains	12	0	2	13	16	23	50	95	185	356	799
Foothills mean	NA	NA	23	0	1	24	27	36	61	91	133	197	363
Plains mean	NA	NA	30	0	1	30	34	43	66	92	130	181	268
Mean	NA	NA	26	0	1	27	31	40	63	92	132	189	318

Table 6. Record length of the 188 streamgages by region used in the analysis by hydrologic region and the effect of paleoflood data on the record length for various annual exceedance-probability discharges.[$Q_{0.5}$, $Q_{0.2}$, $Q_{0.1}$, $Q_{0.04}$, $Q_{0.02}$, $Q_{0.01}$, $Q_{0.005}$, $Q_{0.002}$, discharge with an annual exceedance-probability of 0.5, 0.2, 0.1, 0.04, 0.02, 0.01, 0.005, 0.002, respectively]

Hydrologic region	Foothills	Plains	Both regions
Number of streamgages in region	89	99	188
Mean record length of all streamgages, in years	35	34	35
Number of streamgages with paleoflood data	23	21	44
Mean record length at paleoflood streamgages before paleoflood data, in years	23	30	26
$Q_{0.5}$ mean effective record length added by paleoflood data, in years	0	0	0
$Q_{0.2}$ mean effective record length added by paleoflood data, in years	1	1	1
$Q_{0.1}$ mean effective record length added by paleoflood data, in years	3	3	3
$Q_{0.04}$ mean effective record length added by paleoflood data, in years	10	8	9
$Q_{0.02}$ mean effective record length added by paleoflood data, in years	17	13	15
$Q_{0.01}$ mean effective record length added by paleoflood data, in years	28	21	25
$Q_{0.005}$ mean effective record length added by paleoflood data, in years	45	32	38
$Q_{0.002}$ mean effective record length added by paleoflood data, in years	88	51	68
$Q_{0.5}$ mean record length with paleoflood data, in years	35	34	35
$Q_{0.2}$ mean record length with paleoflood data, in years	36	35	36
$Q_{0.1}$ mean record length with paleoflood data, in years	39	37	38
$Q_{0.04}$ mean record length with paleoflood data, in years	45	42	43
$Q_{0.02}$ mean record length with paleoflood data, in years	53	48	50
$Q_{0.01}$ mean record length with paleoflood data, in years	64	56	59
$Q_{0.005}$ mean record length with paleoflood data, in years	80	66	73
$Q_{0.002}$ mean record length with paleoflood data, in years	123	85	103

Table 7. Standard error of prediction of the peak-streamflow regional-regression equations for eastern Colorado across both hydrologic regions for the 44 streamgages where paleoflood data were collected with and without the inclusion of the paleoflood data.[SEP , standard error of prediction; $Q_{0.5}$, $Q_{0.2}$, $Q_{0.1}$, $Q_{0.04}$, $Q_{0.02}$, $Q_{0.01}$, $Q_{0.005}$, $Q_{0.002}$, discharge with an annual exceedance probability of 0.5, 0.2, 0.1, 0.04, 0.02, 0.01, 0.005, 0.002, respectively; %, percent]

Annual exceedance-probability discharge	SEP without paleoflood data across both hydrologic regions (44 streamgages)	SEP with paleoflood data across both hydrologic regions (44 streamgages)	Mean change from previous SEP (percent)
$Q_{0.5}$	166	157	-5.5
$Q_{0.2}$	132	123	-6.3
$Q_{0.1}$	126	117	-7.6
$Q_{0.04}$	129	117	-9.1
$Q_{0.02}$	135	121	-10.5
$Q_{0.01}$	143	126	-12.3
$Q_{0.005}$	154	132	-14.4
$Q_{0.002}$	172	141	-18.0
Mean	145	129	-10.7

(table 7). When all 188 streamgages were considered, including the paleoflood data at the 44 streamgages, the record length was increased from 35 and 34 years to 35–123 years (depending on AEPD) and 34–85 years (depending on AEPD) in the Foothills and Plains hydrologic regions, respectively (table 6). However, even with a substantial increase in mean record length from paleoflood data, the paleoflood data had minimal effect on the SEP of the AEPDs (table 8). With the addition of the paleoflood data at 44 of the sites, the mean SEP for the Foothills hydrologic region went from 95 to 92 percent, the mean SEP for the Plains hydrologic region went from 128 to 129 percent, and the mean SEP for both hydrologic regions went from 137 to 139 percent (table 8). These results lead the authors to conclude that for paleoflood data to be effective in reducing the SEP in the

Foothills and Plains hydrologic regions of eastern Colorado, a larger ratio than 44 of 188 (23 percent) streamgages would need paleoflood data and that paleoflood data would need to increase the record length by more than 25 years for the 1-percent AEPD. However, in other regions where the hydrology is less variable, it may be possible to collect paleoflood data at a similar fraction of the streamgages in a hydrologic region with a similar increase in record length and obtain significant reduction in the uncertainty in the PSRREs. To make further reductions in the uncertainties of the PSRREs in the Foothills and Plains hydrologic regions, additional streamgages or crest-stage gages are needed to collect peak-streamflow data on natural streams in eastern Colorado.

Table 8. Standard error of prediction of the peak-streamflow regional-regression equations for eastern Colorado with and without the inclusion of the paleoflood data.

[SEP, standard error of prediction; $Q_{0.5}$, $Q_{0.2}$, $Q_{0.1}$, $Q_{0.04}$, $Q_{0.02}$, $Q_{0.01}$, $Q_{0.005}$, $Q_{0.002}$, discharge with an annual exceedance probability of 0.5, 0.2, 0.1, 0.04, 0.02, 0.01, 0.005, 0.002, respectively]

Annual exceedance-probability discharge (percent)	SEP without paleo-flood data for both hydrologic regions (188 streamgages)	SEP with paleo-flood data for both hydrologic regions (188 streamgages)	Foothills SEP without paleoflood data (89 streamgages)	Foothills SEP with paleoflood data (89 streamgages)	Plains SEP without paleoflood data (99 streamgages)	Plains SEP with paleoflood data (99 streamgages)
$Q_{0.5}$	154	156	117	117	128	131
$Q_{0.2}$	121	124	87	87	99	102
$Q_{0.1}$	117	121	81	80	101	103
$Q_{0.04}$	122	126	83	80	112	113
$Q_{0.02}$	129	132	87	83	123	123
$Q_{0.01}$	138	140	94	88	137	136
$Q_{0.005}$	149	150	101	94	153	150
$Q_{0.002}$	166	164	114	104	175	170
Mean	137	139	95	92	128	129

StreamStats

The USGS has developed a Web-based computer program, StreamStats, (<http://water.usgs.gov/osw/streamstats/index.html>) (Ries and others, 2004; USGS, 2015) which facilitates the computation of various streamflow statistics, including peak streamflow, using regional-regression equations published in this and previous reports (Capesius and Stephens, 2009). StreamStats allows the user to obtain streamflow statistics for both gaged and ungaged sites by selecting a specific stream location on a map interface. If a user selects the location of a USGS streamgage, the user will receive previously published information for the streamgage from a database. If the location of interest lacks a streamgage, StreamStats delineates the basin upstream from the selected location, computes basin and climatic characteristics, and provides estimates of the streamflow statistics using the regional-regression equations. The results are presented in a table and a map showing the basin-boundary outline. The estimates are applicable for stream sites not significantly affected by regulation, diversions, channelization, backwater, or urbanization. In the past, it could take an experienced person more than a day to estimate this information at an ungaged site. StreamStats reduces the effort to only a few minutes.

StreamStats makes the process of computing streamflow statistics for ungaged sites much faster, more accurate, and more consistent than previously used manual methods (Eash and others, 2013). It also makes streamflow statistics for streamgages available without the need to locate, obtain, and read the publications in which streamflow statistics were originally provided. Examples of streamflow statistics that can be provided by StreamStats include the $Q_{1\%}$ flood discharge, the mean-daily and mean-annual streamflow, and the mean 7-day, 10-year low streamflow. Examples of basin characteristics include the drainage area, basin elevation, mean-annual precipitation, percent of area underlain by hydrologic soil types,

and so forth. Basin characteristics provided by StreamStats are the physical, geologic, and climatic properties that have been statistically related to movement of water through a drainage basin to a stream site.

Explanatory variables used to develop the PSRREs presented in this report are compatible for use with StreamStats. Finally, PSRREs developed in this report for Colorado have been included in StreamStats to ease the computation of estimates of peak streamflow for users.

Summary

The U.S. Geological Survey (USGS), in cooperation with the Colorado Department of Transportation, developed regional-regression equations for estimating the 50-, 20-, 10-, 4-, 2-, 1-, 0.5-, 0.2-percent annual exceedance-probability discharge (AEPD) for natural streamflow in eastern Colorado. At the completion of the selection process, 37 streamgages were excluded from the analysis because of redundancy, and a total of 188 streamgages, consisting of 6,536 years of record and a mean of approximately 35 years of record per streamgage, were used to develop the peak-streamflow regional-regression equations. Paleoflood and non-exceedance bound ages were established based on reconnaissance-level methods. Multiple lines of evidence were used at each streamgage to arrive at a conclusion (age estimate) to add a higher degree of certainty to reconnaissance-level estimates. Paleoflood or non-exceedance bound evidence was documented at 41 streamgages, and 3 streamgages had previously collected paleoflood data. To determine the peak discharge of a paleoflood or non-exceedance bound, two different hydraulic models were used. The estimated AEPDs for each streamgage were computed using the USGS computer program PeakFQ. The AEPDs were determined using systematic data through water year 2013 (October 1, 2012, through

September 30, 2013). Based on previous studies conducted in Colorado and neighboring States and on the availability of data, a total of 72 characteristics (57 basin and 15 climatic characteristics) were evaluated as candidate explanatory variables in the regression analysis.

After analyzing the study area for potential regional subdivisions, the new hydrologic region located between -104° longitude and the Kansas-Nebraska State line will still be designated the Plains hydrologic region in this report. The hydrologic region comprising the rest of eastern Colorado will be designated the Foothills hydrologic region. The western edge of the Foothills hydrologic region was established by Vaill in 1999, and this report follows that boundary. The western edge of the Foothills hydrologic region follows the 7,500-foot contour south from the Wyoming State line to the Chaffee-Fremont County line where the hydrologic region boundary follows the Chaffee-Fremont County line south across the Arkansas River and transitions up to the 9,000-foot contour which is followed south to the New Mexico State line.

Generalized-Least Squares regression was used exclusively to compute the final peak-streamflow regional-regression equations for peak streamflow. Dividing eastern Colorado into two new individual regions at -104° longitude resulted in peak-streamflow regional-regression equations with the smallest standard error of prediction (*SEP*).

The mean *SEP* for all 8 AEPDs in both the Foothills and Plains hydrologic region was reduced approximately 25 percent compared to the previous study, from 147 percent to 92 percent in the Foothills hydrologic region and to 129 percent in the Plains hydrologic region. When all 188 streamgages were considered, including the paleoflood data at the 44 streamgages, the paleoflood data had minimal effect on the *SEP* of the AEPDs. With the addition of the paleoflood data at 44 of the sites, the mean *SEP* for the Foothills hydrologic region went from 95 to 92 percent, the mean *SEP* for the Plains hydrologic region went from 128 to 129 percent, and the mean *SEP* for both hydrologic regions went from 137 to 139 percent. For paleoflood data to be effective in eastern Colorado, a larger ratio than 44 of 188 (23 percent) streamgages would need paleoflood data and that paleoflood data would need to increase the record length by more than 25 years for the 1-percent AEPD. The greatest reduction in *SEP* for the peak-streamflow regional-regression equations was observed when additional basin characteristics, particularly land-use characteristics that had never been analyzed in previous studies, were included in the peak-streamflow regional-regression equations and by dividing eastern Colorado into two separate regions. To make further reductions in the uncertainties of the PSRREs in the Foothills and Plains hydrologic regions, additional streamgages or crest-stage gages are needed to collect peak-streamflow data on natural streams in eastern Colorado.

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Appendixes

Appendix 1. Example Paleoflood Investigation and Computation of Streamgage Peak-Streamflow Frequency at 06758700 Middle Bijou Creek Tributary near Deer Trail, Colo.

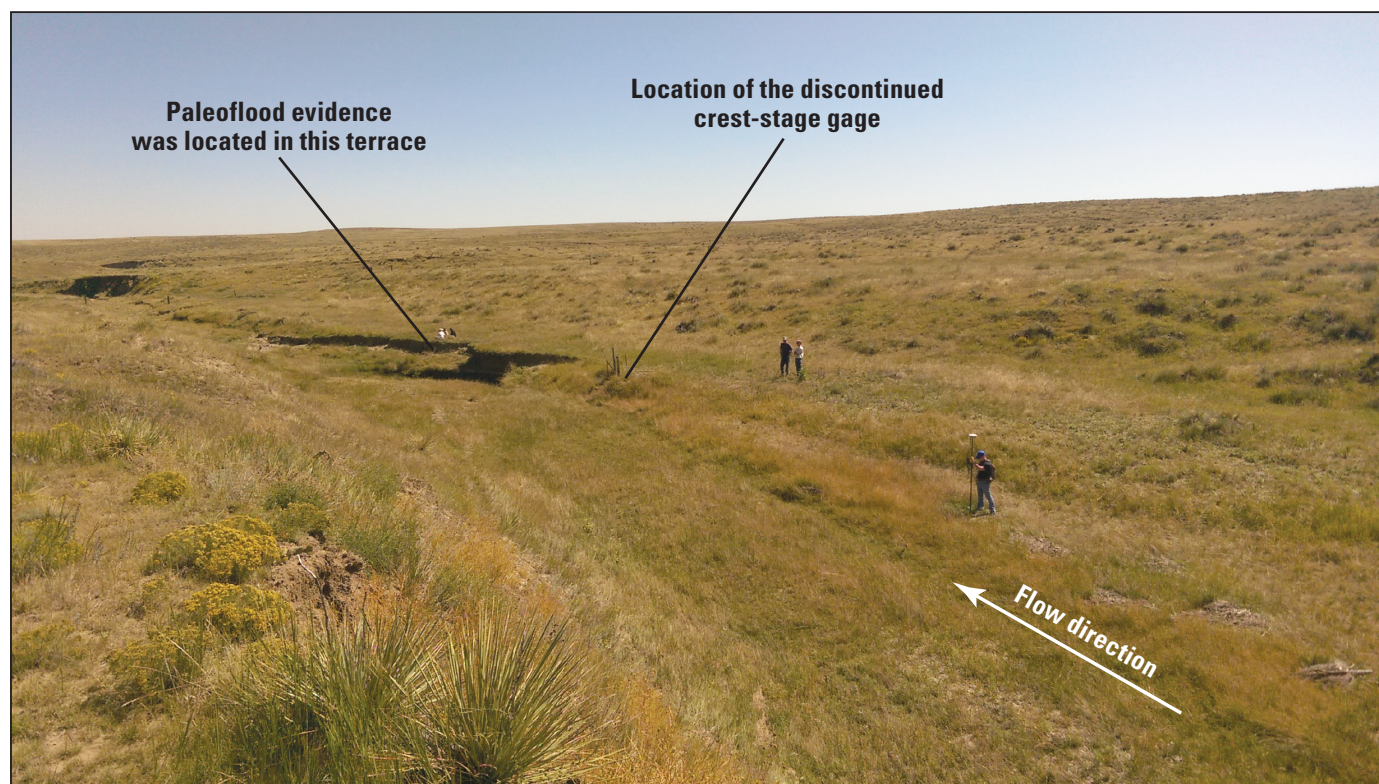


Figure 1–1. Looking downstream toward the discontinued crest-stage gage at 06758700 Middle Bijou Creek Tributary near Deer Trail, Colo.; paleoflood evidence was identified in a terrace along the right bank approximately 100 feet downstream from the streamgage. Photograph by Michael Kohn, USGS.

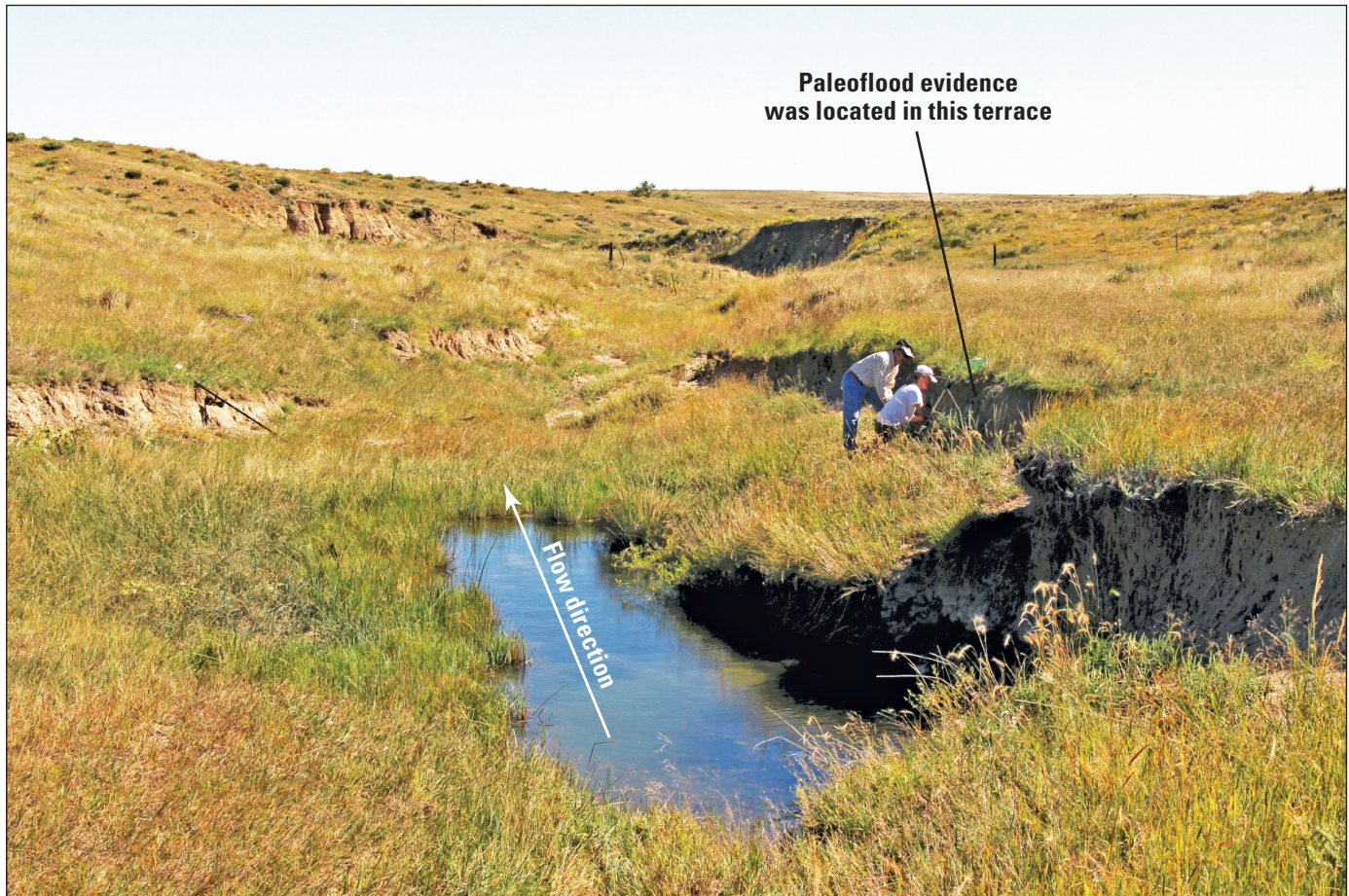


Figure 1–2. Looking downstream at 06758700 Middle Bijou Creek Tributary near Deer Trail, Colo.; paleoflood evidence was identified in a terrace along the right bank approximately 100 feet downstream from the streamgage; photograph taken from the discontinued crest-stage gage. Photograph by Michael Stevens, USGS.



Figure 1–3. Four horizons are present in this terrace exposure; *A*, an organic rich A horizon, *B*, a B horizon with sand and silt interbeds, *C*, a more developed B₂ horizon, and *D*, and older Bk horizon at the bottom. Photograph by Jeanne Godaire, Bureau of Reclamation.



Figure 1–4. These soil samples correspond to the soil horizons in figure 1–3. The soil structure goes from single grained and weakly granular in the A horizon to weak to moderate subangular blocky in the Bk horizon. Photograph by Jeanne Godaire, Bureau of Reclamation.

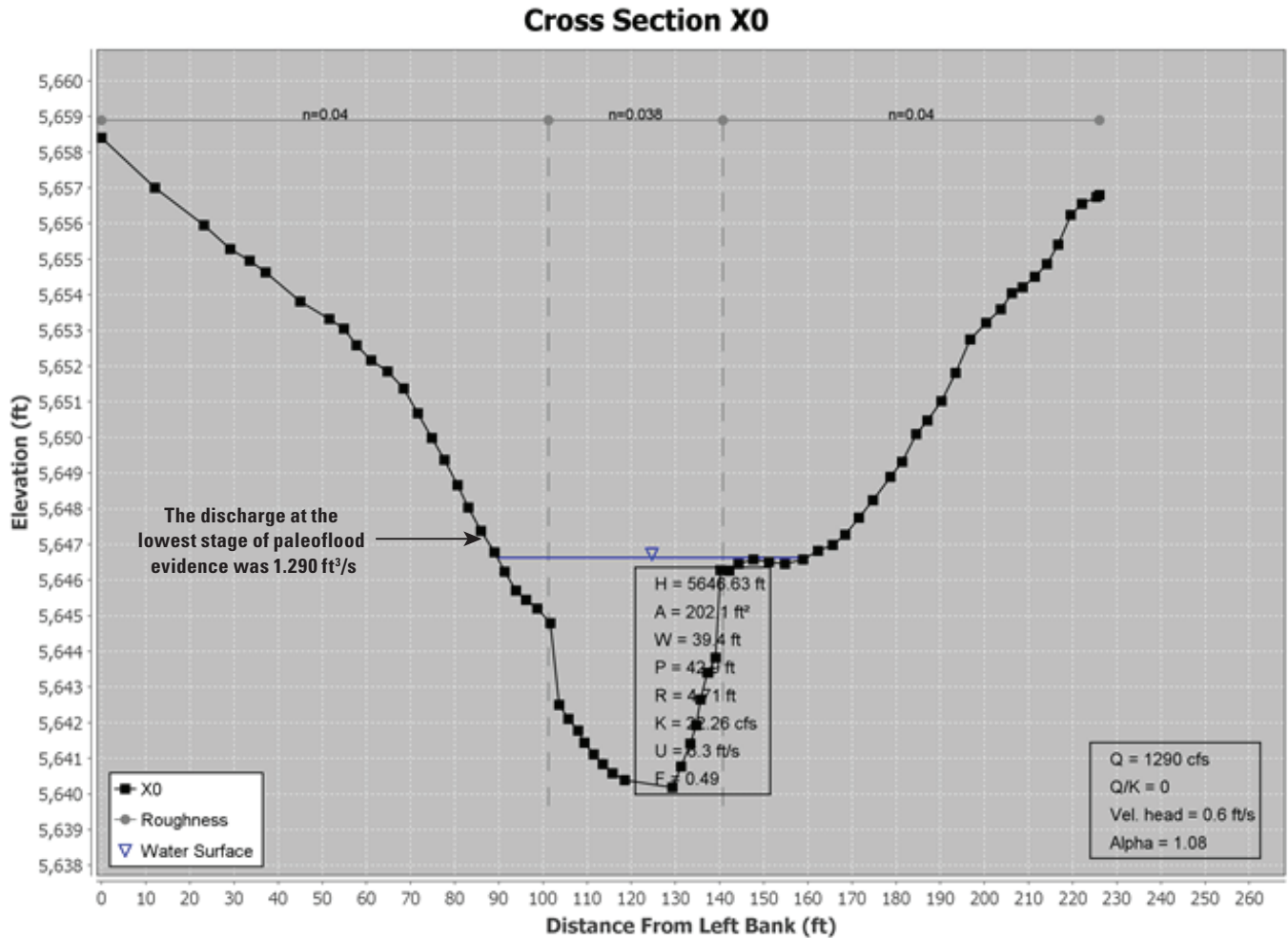


Figure 1–5. Lower limit discharge model output from Slope-Area Computation Graphical User Interface at the cross section where the terrace with paleoflood evidence was located; the lower limit discharge of 1,290 cubic feet per second (ft³/s or cfs) was computed by using the lowest stage of the paleoflood evidence, increasing the Manning roughness coefficient (n) by 20 percent, and decreasing the channel slope by 25 percent. H, water-surface elevation; A, cross-sectional area; W, width; P, wetted perimeter; R, hydraulic radius; K, conveyance; U, velocity; F, Froude number; Q, discharge; Vel. head, velocity head; n, Manning roughness coefficient; ft, foot; alpha, kinetic energy flux correction.

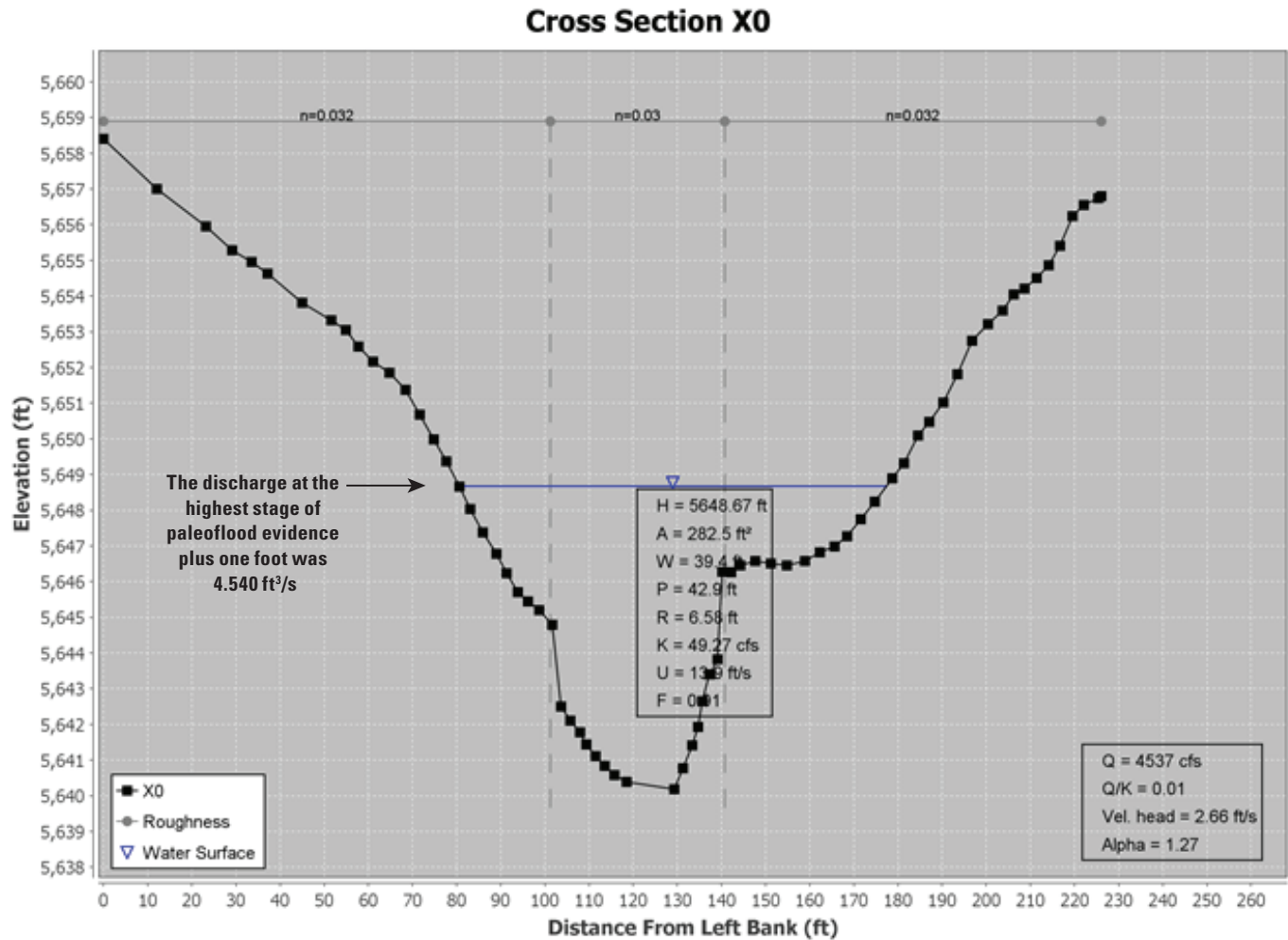


Figure 1–6. The upper limit discharge model output from Slope-Area Computation Graphical User Interface at the cross section where the terrace with paleoflood evidence was located; the upper limit discharge of 4,540 cubic feet per second (ft³/s or cfs) was computed by using the highest stage of the paleoflood evidence plus 1 foot (3 feet for non-exceedance bounds), decreasing the Manning roughness coefficient (n) by 20 percent, and increasing the channel slope by 25 percent. H, water-surface elevation; A, cross-sectional area; W, width; P, wetted perimeter; R, hydraulic radius; K, conveyance; U, velocity; F, Froude number; Q, discharge; Vel. head, velocity head; n, Manning roughness coefficient; ft, foot; alpha, kinetic energy flux correction.

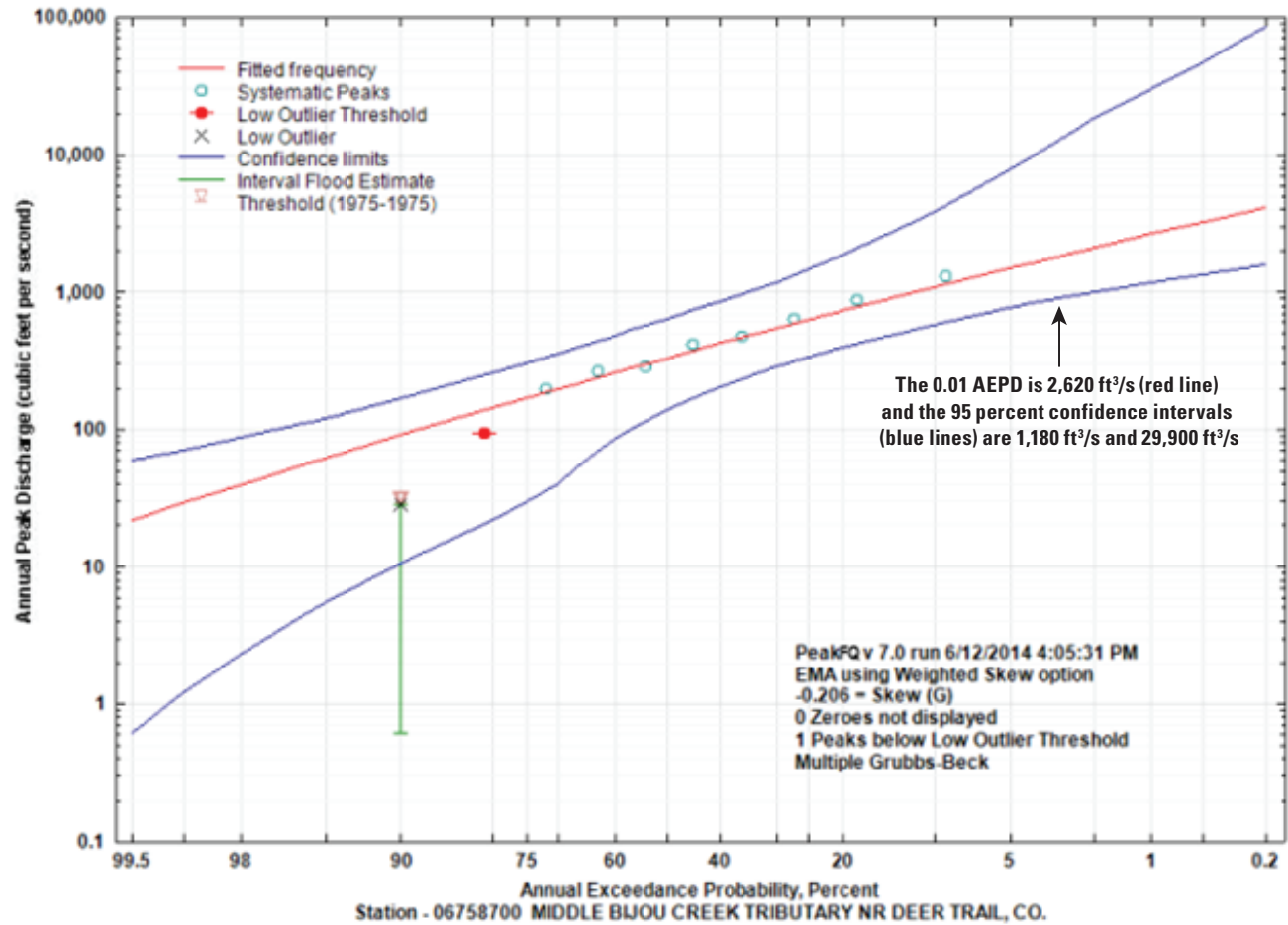


Figure 1–7. The flood-frequency curve for 06758700 Middle Bijou Creek Tributary near Deer Trail, Colo. prior to the inclusion of any paleoflood data; the 0.01 annual exceedance-probability discharge is 2,620 cubic feet per second (ft³/s) (red line) and the 95 percent confidence intervals (blue lines) are 1,180 ft³/s and 29,900 ft³/s. AEPD, annual exceedance-probability discharge; EMA, Expected Moments Algorithm; Skew (G), weighted skew.

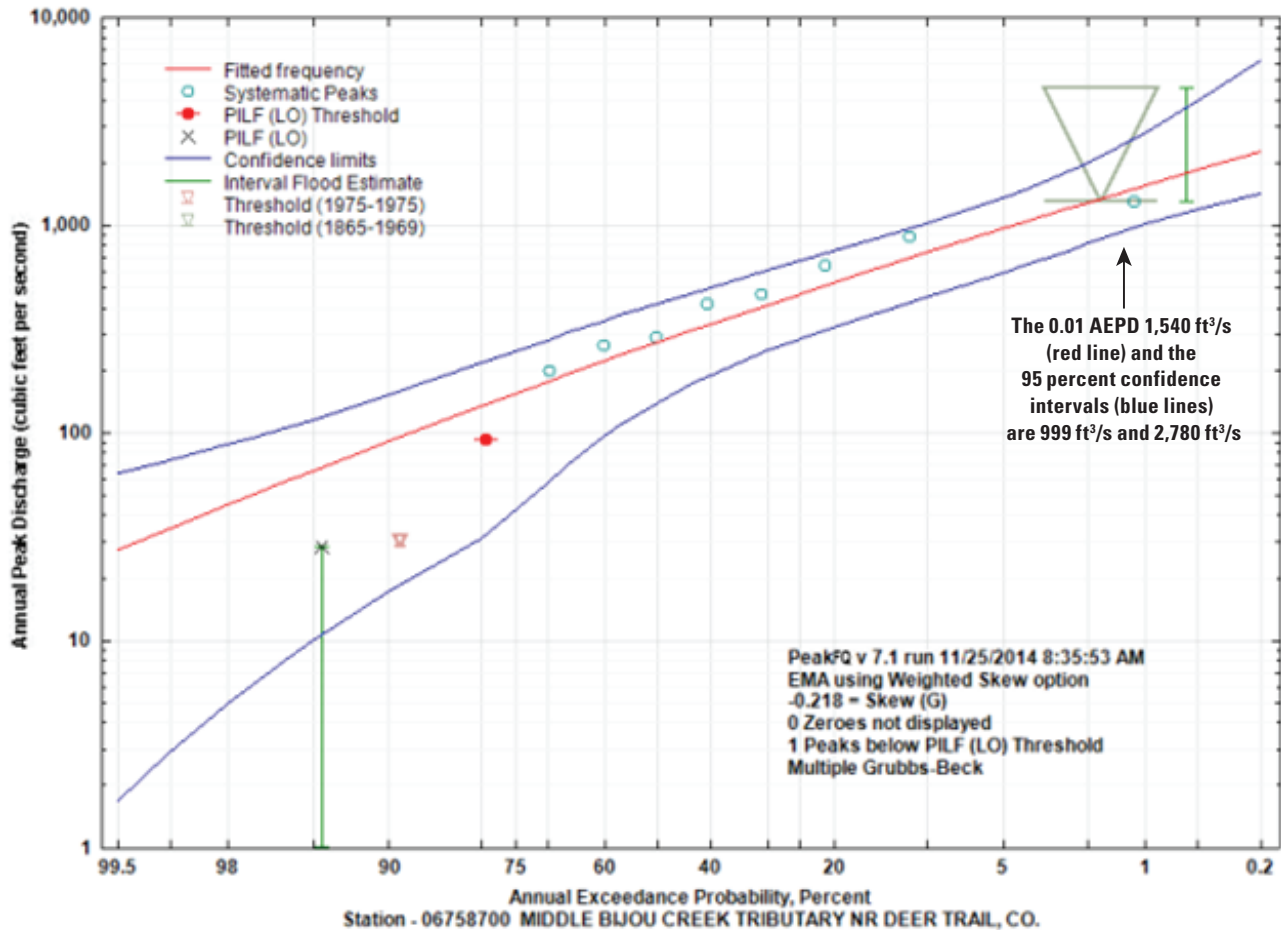


Figure 1-8. The flood-frequency curve for 06758700 Middle Bijou Creek Tributary near Deer Trail, Colo. with a paleoflood that has a lower limit discharge of 1,290 cubic feet per second (ft³/s), upper limit discharge of 4,540 ft³/s, and age of 150 years; the 0.01 annual exceedance-probability discharge has been reduced by 41 percent to 1,540 ft³/s (red line) and the 95 percent confidence intervals (blue lines) are 999 ft³/s and 2,780 ft³/s. AEPD, annual exceedance-probability discharge; PILF (LO), low outlier; EMA, Expected Moments Algorithm; Skew (G), weighted skew.

Appendix 2. Paleoflood Data

Table 2-1. Summary of the paleoflood data collected and applied at 44 streamgages used in the regression analysis of natural streams in eastern Colorado, 2015.

[USGS, U.S. Geological Survey; Q , discharge; Q_{low} , low uncertainty discharge; Q_{high} , high uncertainty discharge; HEC-RAS, Hydrologic Engineering Center River Analysis System; SAC, Slope-Area Computation, Nebr., Nebraska; Colo., Colorado; Wyo., Wyoming; Kans., Kansas; Do., repeat from row above; Okla., Oklahoma; NA, not applicable; N. Mex., New Mexico]

USGS stream-gage number	Streamgage name	Date collected	Data type	Annual exceedance-probability discharge (percent)			Age, in years from date collected	Age uncertainty, in years	Method
				Q (in cubic feet per second)	Q_{low} (in cubic feet per second)	Q_{high} (in cubic feet per second)			
06687000	Blue Creek near Lewellen, Nebr.	2014	Non-exceedance bound	3,970	2,860	18,700	350	±150	HEC-RAS
06712000	Cherry Creek near Franktown, Colo.	1999	Paleoflood	37,000	27,800	46,300	7,500	±2,500	Critical depth Slope conveyance Flow competence HEC-RAS
06712500	Cherry Creek near Melvin, Colo.	1999	Paleoflood	74,000	55,500	92,500	3,200	±1,600	
06719500	Clear Creek near Golden, Colo.	2002	Paleoflood	50,100	37,600	62,600	6,000	Unknown	
06755960	Crow Creek at 19th Street, at Cheyenne, Wyo.	2014	Non-exceedance bound	11,000	7,140	22,700	125	±25	HEC-RAS
06758200	Kiowa Creek at Kiowa, Colo.	2014	Non-exceedance bound	19,700	19,700	19,700	49	0	Gage record
06758700	Middle Bijou Creek Tributary near Deer Trail, Colo.	2014	Paleoflood	1,820	1,290	4,540	150	±50	SAC
06762600	Lodgepole Creek Tributary Number 2 near Albin, Wyo.	2014	Non-exceedance bound	4,500	4,500	25,300	200	±100	HEC-RAS
06763500	Lodgepole Creek at Ralton, Nebr.	2014	Non-exceedance bound	18,500	13,400	48,600	300	±200	HEC-RAS
06831500	Frenchman Creek near Imperial, Nebr.	2014	Non-exceedance bound	4,000	3,150	21,900	200	±100	HEC-RAS
06835100	Bobtail Creek near Palisade, Nebr.	2014	Non-exceedance bound	15,200	15,200	105,000	350	±150	HEC-RAS
06844800	South Fork Sappa Creek Tributary near Goodland, Kans.	2014	Non-exceedance bound	3,450	3,450	3,450	57	0	Gage record
06858500	North Fork Smoky Hill River near McAllaster, Kans.	2014	Non-exceedance bound	50,000	32,700	129,000	300	±200	HEC-RAS
07099215	Turkey Creek near Fountain, Colo.	2014	Non-exceedance bound	1,650	1,560	2,480	150	±50	SAC
07099220	Little Turkey Creek near Fountain, Colo.	2014	Non-exceedance bound	1,230	1,010	1,400	150	±50	SAC
07103703	Camp Creek at Garden of the Gods, Colo.	2014	Non-exceedance bound	875	714	1,360	150	±50	SAC
07103960	Kettle Creek above U.S. Air Force Academy, Colo.	2014	Paleoflood	12,800	12,800	13,000	49	0	HEC-RAS
	Do.	2014	Non-exceedance bound	12,800	12,800	24,400	150	±50	HEC-RAS
07103977	Cottonwood Creek at Cowpoke Road at Colorado Springs, Colo.	2014	Non-exceedance bound	6,100	5,500	9,630	300	±200	HEC-RAS
07105000	Bear Creek near Colorado Springs, Colo.	2014	Non-exceedance bound	1,600	918	8,810	300	±200	SAC
	Do.	2014	Non-exceedance bound	4,060	3,000	13,700	750	±250	SAC
07105490	Cheyenne Creek at Evans Avenue at Colorado Springs, Colo.	2014	Non-exceedance bound	3,000	2,700	6,800	125	±75	HEC-RAS
07105940	Little Fountain Creek near Fountain, Colo.	2014	Non-exceedance bound	18,000	7,140	41,600	150	±50	SAC
07107500	St. Charles River at Burnt Mill, Colo.	2014	Non-exceedance bound	21,800	21,800	21,800	91	0	Gage record
	Do.	2014	Non-exceedance bound	24,500	21,800	45,400	150	±50	HEC-RAS

Table 2-1. Summary of the paleoflood data collected and applied at 44 streamgages used in the regression analysis of natural streams in eastern Colorado, 2015.—Continued

[USGS, U.S. Geological Survey; Q , discharge; Q_{low} , low uncertainty discharge; Q_{high} , high uncertainty discharge; ft³/s, cubic foot per second; HEC-RAS, Hydrologic Engineering Center River Analysis System; SAC, Slope-Area Computation, Nebr., Nebraska; Colo., Colorado; Wyo., Wyoming; Kans., Kansas; Do., repeat from row above; Okla., Oklahoma; NA, not applicable; N. Mex., New Mexico]

USGS stream-gage number	Streamgage name	Date collected	Data type	Annual exceedance-probability discharge (percent)			Age, in years from date collected	Age uncertainty, in years	Method
				Q (in cubic feet per second)	Q_{low} (in cubic feet per second)	Q_{high} (in cubic feet per second)			
07120620	Big Arroyo near Thatcher, Colo.	2014	Non-exceedance bound	5,120	4,260	15,600	750	±250	HEC-RAS
07124300	Long Canyon Creek near Madrid, Colo.	2014	Non-exceedance bound	13,000	9,500	36,200	200	±100	HEC-RAS
07125100	Frijole Creek near Alfalfa, Colo.	2014	Non-exceedance bound	28,400	28,400	62,000	200	±100	HEC-RAS
07125500	Do.	2014	Paleoflood	6,540	5,420	7,660	32	0	HEC-RAS
	San Francisco Creek near Alfalfa, Colo.	2014	Non-exceedance bound	26,300	26,300	26,300	60	0	Gage record
	Do.	2014	Non-exceedance bound	26,300	26,300	55,500	2,000	±1,000	HEC-RAS
07126200	Van Bremer Arroyo near Model, Colo.	2014	Non-exceedance bound	29,500	21,300	46,600	150	±50	HEC-RAS
07126325	Taylor Arroyo below Rock Crossing, near Thatcher, Colo.	2014	Non-exceedance bound	9,090	7,650	20,100	150	±50	HEC-RAS
07126390	Lockwood Canyon Creek near Thatcher, Colo.	2014	Non-exceedance bound	1,110	1,110	1,110	75	±25	Gage record
07126415	Red Rock Canyon Creek at Mouth near Thatcher, Colo.	2014	Non-exceedance bound	21,200	15,700	45,300	150	±50	HEC-RAS
07126480	Bent Canyon Creek at Mouth near Timpas, Colo.	2014	Non-exceedance bound	7,530	5,470	25,000	150	±50	SAC
07134100	Big Sandy Creek near Lamar, Colo.	2014	Non-exceedance bound	3,600	3,600	3,600	80	±10	Gage record
07135000	Two Butte Creek near Holly, Colo.	2014	Non-exceedance bound	2,820	2,090	10,100	70	±10	SAC
07138600	White Woman Creek Tributary near Selkirk, Kans.	2014	Non-exceedance bound	1,870	1,440	4,790	500	±100	SAC
07138650	White Woman Creek near Leoti, Kans.	2014	Non-exceedance bound	8,300	5,800	20,700	300	±100	SAC
07155100	Cold Springs Creek near Wheelless, Okla.	2014	Non-exceedance bound	2,520	2,520	2,520	75	0	Gage record
07156100	Sand Arroyo Creek near Johnson, Kans.	2014	Non-exceedance bound	2,300	2,300	2,300	300	±100	Gage record
07156220	Bear Creek near Johnson, Kans.	2014	Non-exceedance bound	15,000	15,000	15,000	100	±10	Gage record
07156600	Cimarron River Tributary near Moscow, Kans.	2014	Non-exceedance bound	3,900	3,900	3,900	24	0	Gage record
	Do.	2014	Non-exceedance bound	9,620	7,100	NA	2,000	±1,000	SAC
07201200	Chicorica Creek Tributary near Raton, N. Mex.	2014	Non-exceedance bound	3,500	2,580	9,800	350	±150	SAC
07201450	Green Mountain Arroyo near Raton, N. Mex.	2014	Paleoflood	15,000	10,200	28,100	100	±40	SAC
07203600	Rio Del Plano Tributary near Taylor Springs, N. Mex.	2014	Non-exceedance bound	2,820	1,820	5,600	500	±100	r
	Do.	2014	Indirect-discharge measurement	938	938	938	0	0	SAC
07227300	Sand Draw near Clayton, N. Mex.	2014	Non-exceedance bound	12,200	8,730	42,100	500	±100	SAC
07232650	Aqua Frio Creek near Felt, Okla.	2014	Non-exceedance bound	1,900	1,900	1,900	38	0	Gage record
	Do.	2014	Non-exceedance bound	6,540	5,600	11,200	2,000	±500	Step-rackwater
							Mean	630 years	

Appendix 3. Annual Exceedance-Probability Discharges

Table 3-1. Summary of the annual exceedance-probability discharges at the 188 streamgages used in the regression analysis of natural streams in eastern Colorado, 2015.

[USGS, U.S. Geological Survey; $Q_{0.5}$, $Q_{0.2}$, $Q_{0.1}$, $Q_{0.04}$, $Q_{0.02}$, $Q_{0.01}$, $Q_{0.005}$, $Q_{0.002}$, discharge with an annual exceedance-probability of 0.5, 0.2, 0.1, 0.04, 0.02, 0.01, 0.005, 0.002, respectively; Wyo., Wyoming; Nebr., Nebraska; Colo., Colorado; Kans., Kansas; Okla., Oklahoma; N. Mex., New Mexico; Tex., Texas; NA, not applicable]

Streamgage information		Annual exceedance-probability discharge, in cubic feet per second							
USGS streamgage number	Streamgage name	Q_2	Q_5	Q_{10}	Q_{25}	Q_{50}	Q_{100}	Q_{200}	Q_{500}
06655000	Cottonwood Creek at Wendover, Wyo.	284	926	1,750	3,520	5,560	8,450	12,400	20,000
06668040	Rabbit Creek near Wheatland, Wyo.	26.7	72.9	122	211	300	410	545	768
06675300	Horse Creek Tributary near Little Bear, Wyo.	13.8	55.8	110	220	337	488	678	996
06687000	Blue Creek near Lewellen, Nebr.	194	285	359	467	562	668	789	973
06707000	North Fork South Platte River at South Platte, Colo.	836	1,290	1,600	1,980	2,260	2,530	2,800	3,150
06708500	Deer Creek near Littleton, Colo.	63.5	142	213	328	432	551	688	897
06709500	Plum Creek near Louviers, Colo.	495	1,810	3,870	9,310	17,000	30,000	51,500	102,000
06710385	Bear Creek above Evergreen, Colo.	204	341	457	639	801	990	1,210	1,550
06710990	Parmalee Gulch at Mouth, at Indian Hills, Colo.	18.8	52.4	87.9	150	211	286	374	518
06711000	Turkey Creek near Morrison, Colo.	124	357	625	1,140	1,680	2,400	3,310	4,910
06712000	Cherry Creek near Franktown, Colo.	627	1,710	2,840	4,800	6,680	8,950	11,700	15,900
06712500	Cherry Creek near Melvin, Colo.	2,180	5,610	8,920	14,300	19,100	24,700	30,900	40,300
06718550	North Clear Creek above Mouth near Black Hawk, Colo.	162	309	431	614	769	940	1,130	1,410
06719500	Clear Creek near Golden, Colo.	1,690	2,960	3,930	5,300	6,420	7,610	8,890	10,700
06724000	Saint Vrain Creek at Lyons, Colo.	1,010	1,850	2,670	4,120	5,570	7,430	9,790	13,900
06724500	Left Hand Creek near Boulder, Colo.	355	555	702	903	1,060	1,230	1,410	1,660
06726900	Bummers Gulch near El Vado, Colo.	3.80	8.10	12.2	19.3	26.0	34.4	44.5	61.3
06727500	Fourmile Creek at Orodell, Colo.	82.7	226	386	685	997	1,400	1,910	2,800
06730300	Coal Creek near Plainview, Colo.	51.1	146	250	435	619	846	1,120	1,570
06730500	Boulder Creek at Mouth near Longmont, Colo.	725	1,450	2,100	3,140	4,090	5,190	6,470	8,480
06736000	North Fork Big Thompson River at Drake, Colo.	213	469	739	1,240	1,760	2,450	3,340	4,940
06736650	Cedar Creek at Cedar Cove, Colo.	36.6	174	397	962	1,710	2,870	4,630	8,270
06739500	Buckhorn Creek near Masonville, Colo.	326	1,650	3,750	8,880	15,400	25,000	38,800	65,900
06742000	Little Thompson River near Berthoud, Colo.	508	1,680	3,130	6,080	9,360	13,800	19,600	30,200
06751490	North Fork Cache La Poudre River at Livermore, Colo.	400	1,070	1,770	3,010	4,240	5,750	7,590	10,600
06755000	South Crow Creek near Hecla, Wyo.	16.7	36.5	56.1	90.0	123	165	215	300
06755960	Crow Creek at 19th Street, at Cheyenne, Wyo.	205	596	1,050	1,940	2,880	4,140	5,780	8,670
06758200	Kiowa Creek at Kiowa, Colo.	522	2,400	5,180	11,500	19,100	30,000	44,900	72,700
06758700	Middle Bijou Creek Tributary near Deer Trail, Colo.	270	526	733	1,030	1,280	1,540	1,830	2,230
06759100	Bijou Creek near Fort Morgan, Colo.	175	384	597	978	1,360	1,860	2,480	3,560
06761900	Lodgepole Creek Tributary near Pine Bluffs, Wyo.	29.4	60.7	89.2	135	177	226	283	372
06762500	Lodgepole Creek at Bushnell, Nebr.	120	760	1,960	5,320	10,100	17,800	29,800	55,500
06762600	Lodgepole Creek Tributary Number 2 near Albin, Wyo.	98.0	232	362	581	787	1,030	1,330	1,790
06763500	Lodgepole Creek at Ralton, Nebr.	42.5	161	350	847	1,550	2,730	4,680	9,200
06767200	North Fork Plum Creek Tributary near Farnam, Nebr.	18.6	71.8	146	314	517	809	1,220	2,010
06821300	North Fork Arikaree Tributary near Shaw, Colo.	128	401	717	1,310	1,930	2,720	3,710	5,370
06821400	North Fork Black Wolf Creek near Vernon, Colo.	128	824	2,080	5,400	9,800	16,600	26,500	46,200
06821500	Arikaree River at Haigler, Nebr.	666	3,600	8,250	19,200	32,400	51,200	76,900	124,000
06823000	North Fork Republican River at Colorado- Nebraska State Line	175	403	653	1,130	1,650	2,350	3,290	5,010
06823500	Buffalo Creek near Haigler, Nebr.	22.4	39.4	54.4	78.5	101	127	158	208
06825000	South Fork Republican River near Idalia, Colo.	3,460	8,620	14,300	25,100	36,600	51,700	71,600	107,000
06825500	Landsman Creek near Hale, Colo.	1,280	3,640	6,110	10,400	14,500	19,500	25,300	34,600
06826900	Sand Creek near Hale, Colo.	55.4	393	1,070	3,090	6,060	11,100	19,100	36,800
06829700	Thompson Canyon near Trenton, Nebr.	267	700	1,180	2,070	2,990	4,190	5,720	8,390
06831500	Frenchman Creek near Imperial, Nebr.	298	602	893	1,390	1,870	2,450	3,170	4,370
06835000	Stinking Water Creek near Palisade, Nebr.	385	869	1,370	2,300	3,240	4,460	6,020	8,760
06835100	Bobtail Creek near Palisade, Nebr.	316	924	1,660	3,140	4,790	7,050	10,100	15,700

Table 3-1. Summary of the annual exceedance-probability discharges at the 188 streamgages used in the regression analysis of natural streams in eastern Colorado, 2015.—Continued

[USGS, U.S. Geological Survey; $Q_{0.5}$, $Q_{0.2}$, $Q_{0.1}$, $Q_{0.04}$, $Q_{0.02}$, $Q_{0.01}$, $Q_{0.005}$, $Q_{0.002}$, discharge with an annual exceedance-probability of 0.5, 0.2, 0.1, 0.04, 0.02, 0.01, 0.005, 0.002, respectively; Wyo., Wyoming; Nebr., Nebraska; Colo., Colorado; Kans., Kansas; Okla., Oklahoma; N. Mex., New Mexico; Tex., Texas; NA, not applicable]

Streamgage information		Annual exceedance-probability discharge, in cubic feet per second							
USGS streamgage number	Streamgage name	Q_2	Q_5	Q_{10}	Q_{25}	Q_{50}	Q_{100}	Q_{200}	Q_{500}
06836000	Blackwood Creek near Culbertson, Nebr.	388	940	1,490	2,430	3,330	4,420	5,720	7,820
06836500	Driftwood Creek near McCook, Nebr.	781	1,890	3,030	5,060	7,070	9,580	12,700	17,900
06838200	Coon Creek at Indianola, Nebr.	129	299	464	743	1,010	1,330	1,710	2,320
06838550	Dry Creek at Bartley, Nebr.	249	388	496	652	782	925	1,080	1,320
06839200	Elkhorn Canyon near Maywood, Nebr.	182	642	1,280	2,750	4,570	7,270	11,200	19,100
06839600	Frazier Creek near Maywood, Nebr.	660	2,250	4,500	9,820	16,600	27,100	43,000	76,400
06839850	Fox Creek north of Curtis, Nebr.	98.5	628	1,680	4,880	9,790	18,400	32,900	66,800
06839900	Fox Creek above Cut Canyon near Curtis, Nebr.	247	714	1,260	2,350	3,520	5,100	7,170	10,900
06839950	Cut Canyon near Curtis, Nebr.	321	648	951	1,450	1,910	2,460	3,110	4,160
06840000	Fox Creek at Curtis, Nebr.	434	1,070	1,730	2,880	4,010	5,400	7,100	9,900
06840500	Dry Creek near Curtis, Nebr.	798	2,620	5,220	11,500	19,600	32,400	52,300	95,100
06841000	Medicine Creek above Harry Strunk Lake, Nebr.	2,710	6,190	9,690	15,800	21,800	29,300	38,600	54,000
06841500	Mitchell Creek above Harry Strunk Lake, Nebr.	509	1,670	3,070	5,840	8,800	12,700	17,700	26,500
06844700	South Fork Sappa Creek near Brewster, Kans.	123	344	594	1,070	1,560	2,200	3,020	4,450
06844800	South Fork Sappa Creek Tributary near Goodland, Kans.	39.3	333	945	2,720	5,240	9,250	15,300	27,700
06844900	South Fork Sappa Creek near Achilles, Kans.	282	1,060	2,060	4,140	6,450	9,540	13,600	20,700
06845000	Sappa Creek near Oberlin, Kans.	467	1,990	4,090	8,540	13,500	20,200	28,900	44,200
06845100	Long Branch draw near Norcatur, Kans.	234	596	961	1,580	2,170	2,880	3,720	5,040
06846200	Beaver Creek Tributary near Ludell, Kans.	325	610	835	1,160	1,420	1,690	1,990	2,410
06846500	Beaver Creek at Cedar Bluffs, Kans.	286	837	1,490	2,790	4,210	6,130	8,670	13,300
06847600	Prairie Dog Creek Tributary at Colby, Kans.	163	410	669	1,140	1,610	2,190	2,930	4,160
06847900	Prairie Dog Creek above Keith Sebelius Lake, Kans.	414	1,570	3,270	7,310	12,500	20,400	32,300	56,800
06858500	North Fork Smoky Hill River near McAllaster, Kans.	283	1,540	3,540	8,310	14,100	22,500	34,100	55,700
06859500	Ladder Creek beowl Chalk Creek nrea Scott City, Kans.	733	3,280	7,170	16,500	28,300	45,900	71,400	122,000
06860000	Smoky Hill River at Elkader, Kans.	1,170	5,270	10,900	22,600	35,400	52,100	73,200	109,000
06860300	South Branch Hackberry Creek near Orion, Kans.	358	1,170	2,190	4,340	6,780	10,200	14,800	23,400
06860500	Hackberry Creek near Gove, Kans.	421	2,150	4,820	11,000	18,400	28,800	43,000	69,000
07093775	Badger Creek, Lower Station, near Howard, Colo.	255	935	1,800	3,560	5,470	7,990	11,300	16,900
07095000	Grape Creek near Westcliffe, Colo.	378	807	1,190	1,780	2,310	2,910	3,590	4,610
07096500	Fourmile Creek near Canon City, Colo.	525	1,260	2,000	3,300	4,570	6,130	8,030	11,200
07099060	Beaver Creek above Highway 115 near Penrose, Colo.	170	644	1,280	2,640	4,210	6,380	9,310	14,700
07099080	Red Creek below Sullivan Park at Fort Carson, Colo.	256	1,510	3,630	9,000	15,900	26,200	41,000	69,600
07099215	Turkey Creek near Fountain, Colo.	53.5	178	323	593	866	1,210	1,630	2,310
07099220	Little Turkey Creek near Fountain, Colo.	26.2	73.4	125	218	313	431	576	818
07099230	Turkey Creek above Teller Reservoir near Stone City, Colo.	119	831	2,160	5,750	10,600	18,000	28,900	50,500
07103700	Fountain Creek near Colorado Springs, Colo.	400	824	1,190	1,760	2,260	2,830	3,460	4,410
07103703	Camp Creek at Garden of the Gods, Colo.	12.7	57.5	126	289	493	796	1,230	2,090
07103747	Monument Creek at Palmer Lake, Colo.	62.6	152	233	359	467	587	719	909
07103960	Kettle Creek above U.S. Air Force Academy, Colo.	188	587	1,060	1,980	2,970	4,260	5,940	8,860
07103977	Cottonwood Creek at Cowpoke Road at Colorado Springs, Colo.	69.2	114	147	193	229	267	306	362
07104000	Monument Creek at Pikeview, Colo.	1,420	2,730	3,740	5,140	6,240	7,380	8,560	10,200
07104050	North Rockrimmon Creek above Delmonico Drive at Colorado Springs, Colo.	429	552	626	713	775	833	890	962
07105000	Bear Creek near Colorado Springs, Colo.	13.9	40.1	69.1	123	178	247	333	478
07105490	Cheyenne Creek at Evans Avenue at Colorado Springs, Colo.	104	254	408	676	939	1,260	1,660	2,310
07105900	Jimmy Camp Creek at Fountain, Colo.	516	2,050	4,470	10,800	19,500	33,900	57,100	110,000
07105920	Little Fountain Creek above Keaton Reservoir, near Fort Carson, Colo.	69.1	210	381	732	1,130	1,660	2,390	3,730
07105940	Little Fountain Creek near Fountain, Colo.	266	808	1,420	2,540	3,670	5,080	6,820	9,690

Table 3-1. Summary of the annual exceedance-probability discharges at the 188 streamgages used in the regression analysis of natural streams in eastern Colorado, 2015.—Continued

[USGS, U.S. Geological Survey; $Q_{0.5}$, $Q_{0.2}$, $Q_{0.1}$, $Q_{0.04}$, $Q_{0.02}$, $Q_{0.01}$, $Q_{0.005}$, $Q_{0.002}$, discharge with an annual exceedance-probability of 0.5, 0.2, 0.1, 0.04, 0.02, 0.01, 0.005, 0.002, respectively; Wyo., Wyoming; Nebr., Nebraska; Colo., Colorado; Kans., Kansas; Okla., Oklahoma; N. Mex., New Mexico; Tex., Texas; NA, not applicable]

Streamgage information		Annual exceedance-probability discharge, in cubic feet per second							
USGS streamgage number	Streamgage name	Q_2	Q_5	Q_{10}	Q_{25}	Q_{50}	Q_{100}	Q_{200}	Q_{500}
07105945	Rock Creek above Fort Carson Reservation, Colo.	39.9	133	260	547	900	1,430	2,200	3,750
07106500	Fountain Creek at Pueblo, Colo.	4,630	10,000	15,200	23,900	32,200	42,300	54,400	73,900
07107500	St. Charles River at Burnt Mill, Colo.	921	2,890	5,210	9,700	14,400	20,600	28,500	42,000
07108900	St. Charles River at Vineland, Colo.	2,370	5,460	8,600	14,200	19,700	26,600	35,100	49,600
07111000	Huerfano River at Manzanares Crossing, near Redwing, Colo.	205	542	961	1,860	2,920	4,470	6,720	11,200
07112500	Huerfano River at Badito, Colo.	963	2,180	3,350	5,300	7,130	9,300	11,900	16,000
07114000	Cucharas River at Boyd Ranch near La Veta, Colo.	111	220	308	438	546	663	789	970
07116000	Huerfano River below Huerfano Valley Dam near Undercliffe, Colo.	3,670	7,320	10,400	14,800	18,600	22,700	27,200	33,700
07118000	Apishapa River near Aguilar, Colo.	2,430	3,750	4,670	5,840	6,730	7,620	8,520	9,730
07120620	Big Arroyo near Thatcher, Colo.	56.0	250	518	1,090	1,720	2,560	3,650	5,520
07121500	Timpas Creek at Mouth near Swink, Colo.	1,240	3,250	5,370	9,180	13,000	17,700	23,600	33,300
07122400	Crooked Arroyo near Swink, Colo.	167	395	635	1,070	1,520	2,090	2,810	4,060
07123675	Horse Creek near Las Animas, Colo.	88.8	338	712	1,630	2,840	4,750	7,670	13,900
07124300	Long Canyon Creek near Madrid, Colo.	1,190	2,240	3,100	4,330	5,350	6,450	7,640	9,350
07124500	Purgatoire River at Trinidad, Colo.	5,790	12,100	17,400	25,400	32,100	39,500	47,600	59,200
07125100	Frijole Creek near Alfalfa, Colo.	1,640	3,770	5,840	9,300	12,600	16,500	21,100	28,400
07125500	San Francisco Creek near Alfalfa, Colo.	2,530	5,010	7,090	10,200	12,800	15,700	18,900	23,500
07126100	Luning Arroyo near Model, Colo.	580	1,730	3,140	6,030	9,290	13,800	19,900	31,400
07126140	Van Bremer Arroyo near Tyrone, Colo.	43.3	186	379	783	1,230	1,820	2,570	3,880
07126200	Van Bremer Arroyo near Model, Colo.	476	1,790	3,440	6,680	10,100	14,500	19,900	29,100
07126325	Taylor Arroyo below Rock Crossing, near Thatcher, Colo.	188	705	1,360	2,690	4,110	5,980	8,360	12,400
07126390	Lockwood Canyon Creek near Thatcher, Colo.	108	333	580	1,020	1,460	1,990	2,620	3,620
07126415	Red Rock Canyon Creek at Mouth near Thatcher, Colo.	407	1,010	1,620	2,660	3,660	4,870	6,330	8,670
07126470	Chacuaco Creek at Mouth near Timpas, Colo.	1,790	4,650	7,650	13,000	18,300	24,900	32,900	46,300
07126480	Bent Canyon Creek at Mouth near Timpas, Colo.	141	529	1,030	2,040	3,130	4,570	6,430	9,620
07126500	Purgatoire River at Ninemile Dam, near Higbee, Colo.	5,800	14,700	24,100	41,100	58,100	79,500	106,000	151,000
07133200	Clay Creek Tributary near Deora, Colo.	33.5	711	3,170	14,400	36,900	83,600	173,000	405,000
07134100	Big Sandy Creek near Lamar, Colo.	235	623	1,030	1,770	2,500	3,410	4,530	6,380
07134990	Wild Horse Creek above Holly, Colo.	193	446	697	1,130	1,550	2,070	2,700	3,740
07135000	Two Butte Creek near Holly, Colo.	101	464	982	2,100	3,380	5,100	7,350	11,300
07138600	White Woman Creek Tributary near Selkirk, Kans.	62.3	159	257	427	590	788	1,030	1,410
07138650	White Woman Creek near Leoti, Kans.	203	744	1,420	2,760	4,180	6,030	8,360	12,300
07138800	Lion Creek Tributary near Modoc, Kans.	134	168	189	213	229	245	260	279
07140600	Pawnee River Tributary near Kalvesta, Kans.	234	710	1,280	2,440	3,710	5,440	7,740	11,900
07141400	South Fork Walnut Creek Tributary near Dighton, Kans.	66.6	99.4	123	154	179	204	231	268
07153500	Dry Cimarron River near Guy, N. Mex.	2,800	6,680	10,800	18,200	25,900	35,700	48,200	69,900
07154400	Carrizozo Creek near Kenton, Okla.	1,590	4,090	6,570	10,700	14,600	19,100	24,400	32,700
07154500	Cimarron River near Kenton, Okla.	3,650	10,000	16,600	28,200	39,300	52,700	68,700	94,200
07154650	Tesesquite Creek near Kenton, Okla.	1,220	3,920	6,950	12,400	17,800	24,400	32,200	44,700
07155100	Cold Springs Creek near Wheelless, Okla.	55.2	282	632	1,450	2,440	3,840	5,760	9,320
07155590	Cimarron River near Elkhart, Kans.	131	2,140	8,120	30,600	68,700	138,000	253,000	511,000
07155900	North Fork Cimarron River Tributary near Elkhart, Kans.	34.5	736	3,200	14,000	34,600	75,400	150,000	333,000
07156000	North Fork Cimarron River Tributary near Richfield, Kans.	709	2,460	4,570	8,640	12,900	18,300	25,100	36,400
07156010	North Fork Cimarron River at Richfield, Kans.	763	3,530	8,030	19,500	35,000	59,500	97,000	177,000
07156100	Sand Arroyo Creek near Johnson, Kans.	70.8	247	458	864	1,290	1,820	2,480	3,580
07156220	Bear Creek near Johnson, Kans.	927	2,580	4,350	7,470	10,500	14,300	18,800	26,200
07156600	Cimarron River Tributary near Moscow, Kans.	636	1,270	1,790	2,560	3,210	3,910	4,670	5,780

Table 3-1. Summary of the annual exceedance-probability discharges at the 188 streamgages used in the regression analysis of natural streams in eastern Colorado, 2015.—Continued

[USGS, U.S. Geological Survey; $Q_{0.5}$, $Q_{0.2}$, $Q_{0.1}$, $Q_{0.04}$, $Q_{0.02}$, $Q_{0.01}$, $Q_{0.005}$, $Q_{0.002}$, discharge with an annual exceedance-probability of 0.5, 0.2, 0.1, 0.04, 0.02, 0.01, 0.005, 0.002, respectively; Wyo., Wyoming; Nebr., Nebraska; Colo., Colorado; Kans., Kansas; Okla., Oklahoma; N. Mex., New Mexico; Tex., Texas; NA, not applicable]

Streamgage information		Annual exceedance-probability discharge, in cubic feet per second							
USGS streamgage number	Streamgage name	Q_2	Q_5	Q_{10}	Q_{25}	Q_{50}	Q_{100}	Q_{200}	Q_{500}
07156700	Cimarron River Tributary near Satanta, Kans.	127	493	957	1,880	2,860	4,110	5,690	8,330
07157100	Crooked Creek near Copeland, Kans.	454	1,550	2,820	5,180	7,550	10,500	14,000	19,700
07157400	Crooked Creek Tributary at Meade, Kans.	238	1,360	3,170	7,460	12,700	20,100	30,200	48,700
07157500	Crooked Creek near Englewood, Kans.	2,030	5,320	8,520	13,800	18,500	24,000	30,100	39,500
07157550	West Fork Creek near Knowles, Okla.	90.1	299	528	927	1,300	1,740	2,250	3,010
07199000	Canadian River near Hebron, N. Mex.	3,120	7,170	11,100	17,700	24,000	31,600	40,700	55,200
07201000	Raton Creek at Raton, N. Mex.	339	841	1,340	2,180	2,970	3,920	5,030	6,800
07201200	Chicorica Creek Tributary near Raton, N. Mex.	50.2	175	329	637	968	1,400	1,960	2,920
07201450	Green Mountain Arroyo near Raton, N. Mex.	43.4	432	1,370	4,500	9,540	18,500	33,500	67,900
07202000	Chicorica Creek near Hebron, N. Mex.	1,660	3,700	5,560	8,500	11,100	14,200	17,600	22,800
07203000	Vermejo River near Dawson, N. Mex.	1,370	3,220	4,910	7,570	9,920	12,600	15,600	20,000
07203600	Rio Del Plano Tributary near Taylor Springs, N. Mex.	121	301	476	768	1,040	1,350	1,720	2,290
07204000	Moreno Creek at Eagle Nest, N. Mex.	60.8	116	157	212	254	297	340	398
07204500	Cieneguilla Creek near Eagle Nest, N. Mex.	101	224	330	488	621	766	921	1,140
07205000	Sixmile Creek near Eagle Nest, N. Mex.	24.7	48.1	68.6	101	130	163	201	260
07206400	Clear Creek near Ute Park, N. Mex.	22.7	46.3	67.4	101	130	165	204	265
07207500	Ponil Creek near Cimarron, N. Mex.	343	1,010	1,740	3,060	4,380	6,010	7,990	11,200
07208500	Rayado Creek near Cimarron, N. Mex.	134	331	549	964	1,400	1,990	2,750	4,120
07211500	Canadian River near Taylor Springs, N. Mex.	3,890	9,550	16,100	29,300	44,100	64,800	93,300	148,000
07213700	Canadian River Tributary near Mills, N. Mex.	121	873	2,230	5,690	10,100	16,400	25,100	41,200
07214800	Rio La Casa near Cleveland, N. Mex.	194	450	715	1,190	1,670	2,280	3,050	4,360
07220000	Sapello River at Sapello, N. Mex.	2,430	4,220	5,610	7,590	9,210	11,000	12,800	15,500
07220900	Dog Creek near Shoemaker, N. Mex.	710	1,830	2,920	4,710	6,330	8,200	10,300	13,600
07221600	Lagartija Creek Tributary near Sanchez, N. Mex.	194	503	822	1,380	1,920	2,580	3,380	4,670
07222300	Trementina Creek at Trementina, N. Mex.	1,980	4,590	6,980	10,700	14,000	17,700	21,900	28,000
07222500	Conchas River at Variadero, N. Mex.	3,000	7,180	11,800	20,700	30,400	43,400	60,600	92,300
07222800	Garita Creek Tributary near Variadero, N. Mex.	635	2,160	3,860	6,860	9,720	13,100	17,000	23,000
07225300	Bluewater Creek near Tucumcari, N. Mex.	497	1,160	1,760	2,700	3,510	4,420	5,420	6,900
07225500	Ute Creek near Gladstone, N. Mex.	6,200	9,230	11,400	14,200	16,300	18,500	20,800	23,900
07226200	Bueyeros Creek at Bueyeros, N. Mex.	466	1,650	3,180	6,370	9,960	14,900	21,400	33,300
07226500	Ute Creek near Logan, N. Mex.	4,540	11,000	17,200	27,500	37,000	48,100	60,900	80,800
07227100	Revuelto Creek near Logan, N. Mex.	5,230	9,430	12,900	18,300	22,900	28,100	34,000	42,800
07227150	Arroyo Del Puerto near Endee, N. Mex.	196	607	1,060	1,890	2,700	3,690	4,880	6,790
07227200	Tramperos Creek near Stead, N. Mex.	841	2,690	4,770	8,540	12,300	16,800	22,200	30,900
07227295	Sandy Arroyo Tributary near Clayton, N. Mex.	39.1	130	234	423	609	837	1,110	1,540
07227300	Sand Draw near Clayton, N. Mex.	115	521	1,110	2,410	3,930	6,040	8,880	14,000
07227920	Dixon Creek near Borger, Tex.	1,130	2,440	3,570	5,270	6,720	8,310	10,000	12,500
07232500	Beaver River near Guymon, Okla.	3,810	13,400	24,800	46,700	69,100	97,300	132,000	189,000
07232550	South Fork Tributary near Guymon, Okla.	6.80	26.3	51.2	101	153	221	307	450
07232650	Aqua Frio Creek near Felt, Okla.	60.1	224	435	869	1,350	1,990	2,820	4,280
07232900	Coldwater Creek near Guymon, Okla.	62.6	713	2,320	7,630	15,900	30,000	52,500	101,000
07233500	Palo Duro Creek near Spearman, Tex.	1,890	5,940	10,700	20,100	30,000	42,900	59,500	88,300
07233850	Sharp Creek Tributary near Turpin, Okla.	32.3	70.0	104	156	203	256	316	406
07234050	North Fork Clear Creek Tributary near Balko, Okla.	31.2	358	1,150	3,720	7,600	14,100	24,200	45,400
07234100	Clear Creek near Elmwood, Okla.	1,050	5,030	10,700	23,100	37,000	55,600	79,700	121,000
07301410	Sweetwater Creek near Kelton, Tex.	388	898	1,350	2,050	2,650	3,310	4,040	5,090
NA	Raton Creek above Starkville, Colo.	137	506	1,000	2,090	3,360	5,160	7,650	12,300

Appendixes 4–6

[available at <http://dx.doi.org/10.3133/sir20165099>]

Appendix 4. Flood-frequency analysis at the 188 streamgages included in the report

Appendix 5. Variance of prediction values and annual exceedance-probability discharges for streamgages in the foothills and plains hydrologic regions of Colorado and selected streamgages in adjacent states

Appendix 6. Final peak-streamflow regional-regression equation analyses in the foothills and plains hydrologic regions of Colorado

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