

MAGNITUDE AND FREQUENCY OF FLOODS IN SMALL DRAINAGE BASINS IN NORTH DAKOTA

U. S. GEOLOGICAL SURVEY

Water-Resources Investigations 19-75

Prepared in cooperation with the
North Dakota State Highway Department



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SELECTED FACTORS FOR CONVERTING ENGLISH UNITS TO
INTERNATIONAL SYSTEM (SI) UNITS

A dual system of measurement--English units and the International System (SI) of units--is given in this report. SI is a consistent system of units adopted by the Eleventh General Conference of Weights and Measures in 1960. Selected factors for converting English units to SI units are given below.

<u>Multiply English units</u>	<u>By</u>	<u>To obtain SI units</u>
Inches	25.4	millimetres (mm)
Feet	.3048	metres (m)
Miles	1.609	kilometres (km)
Square miles (mi ²)	2.590	square kilometres (km ²)
Cubic feet per second (ft ³ /s)	.02832	cubic metres per second (m ³ /s)
Cubic feet per second per square mile [(ft ³ /s)/mi ²]	.010935	cubic metres per second per square kilometre [(m ³ /s)/km ²]
Feet per mile (ft/mi)	.1894	metres per kilometre (m/km)
Miles per square mile (mi/mi ²)	.6212	kilometres per square kilometre (km/km ²)

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ABSTRACT

This report describes methods for estimating flood-peak discharges having 2- to 50-year recurrence intervals on North Dakota streams draining less than 100 square miles (259 square kilometres). For gaged sites, frequency estimates are provided directly. For ungaged sites, flood peaks are estimated from multiple-regression equations using drainage-area size and, in two regions, soil-infiltration index as estimating variables. Nomographs provide simple solutions for the equations for flood-peak discharges having recurrence intervals of 10, 25, and 50 years. The methods described are not applicable to streams draining urban areas or basins affected by man-made regulation.

Information is also provided on the maximum flood magnitude experienced.

INTRODUCTION

Knowledge of the magnitude and frequency of floods is necessary in the planning and design of culverts, bridges and other hydraulic structures, and in the management of flood plains and flood waters. Only through the use of reliable estimates of a design flood can a structure be designed consistent with the requirements of economic feasibility, minimum risk to the populace, and least adverse environmental impact.

Purpose and Scope

The purposes of this report are: (1) to provide a method of estimating the magnitude and frequency of floods for small unregulated streams in North Dakota; (2) to evaluate the reliability of the resulting estimates; and (3) to evaluate the adequacy of the peak discharge data being collected under the present program and to determine data acquisition needs in regard to future program operation.

The methods of estimating peak discharges described in this report are based on analyses of records from 84 crest-stage stations and 13 continuous-record gaging stations (see pl. 1, in pocket). Estimating methods apply only to sites on streams where flood flows are virtually unregulated and where the drainage area size is less than 100 mi² (259 km²). Records from streams with significant regulation or diversion were not used in the analysis.

The standard error of estimate was determined for each equation developed and is used as the basis for judging the probable reliability of flood-peak estimates.

The constancy of relationships between floods of selected recurrence intervals and the basin or climatic variables was investigated to evaluate the adequacy of the data-collection program.

Gaging-Station-Numbering System

Each gaging station and partial-record station has been assigned a number in downstream order in accordance with the permanent numbering system used by the Geological Survey. Numbers are assigned in a downstream direction along the main stream, and stations on tributaries between main-stream stations are numbered in the order they enter the main stream. A similar order is followed on other ranks of tributaries. The complete 8-digit number, such as 06342300, includes the part number "06" plus a 6-digit station number.

Acknowledgments

This report was prepared by the U.S. Geological Survey in cooperation with the North Dakota State Highway Department.

Most of the flood data used in this report were collected by the U.S. Geological Survey through cooperative agreements with the North Dakota State Highway Department, W. R. Hjelle, Commissioner, and the North Dakota State Water Commission, Vernon Fahy, Chief Engineer.

Assistance in the form of funds or services was provided by other Federal agencies:

Corps of Engineers, U.S. Army,
International Joint Commission, U.S. Department of State,
Soil Conservation Service, U.S. Department of Agriculture,
Federal Highway Administration, U.S. Department of
Transportation.

Previous Studies

A report (McCabe and Crosby, 1959) was prepared utilizing all data available through 1955. That report was superseded by two reports in the water-supply paper series of the U.S. Geological Survey (Patterson, 1966 and Patterson and Gamble, 1968) that utilized data through 1961 and 1963, respectively; however, they contained very little information on floods from small drainage basins in North Dakota.

The reports by Patterson (1966) and Patterson and Gamble (1968) need to be referred to in making flood estimates for basins with drainage areas greater than 100 mi² (259 km²). The estimates thus obtained for drainage areas near 100 mi² (259 km²) will not agree with those obtained from the relationships described in this report (considered the more reliable) and the user may wish to use an averaged estimate.

A report by Crosby (1970) included a limited analysis of magnitude and frequency of North Dakota floods. That analysis was undertaken, however, for a specific purpose, which is unrelated to the practical and reliable estimation of flood magnitude and frequency for use in problems of land-use planning and structural design.

METHODS OF ESTIMATING

The most reliable estimators of future floods generally are the frequency analyses of gaging-station records. The streamflow characteristics computed by a frequency analysis are listed in table 1 and may provide satisfactory estimates for planning and design purposes at or near the gage locations shown on plate 1, particularly where long-term records are available. Therefore, the estimating technique first includes a search for available flood-frequency data for the desired site.

Peak discharges for selected recurrence intervals at most ungaged sites on small streams in North Dakota can be determined as follow:

1. Locate the site on plate 1 and determine in which region the basin is located.
2. Choose the appropriate equation from table 2 or appropriate nomograph from figures 1 through 3.
3. Determine the required basin characteristics.
 - (a) Drainage area (A) is the contributing drainage area in square miles, as determined by outlining the surface-water divide upstream from the point

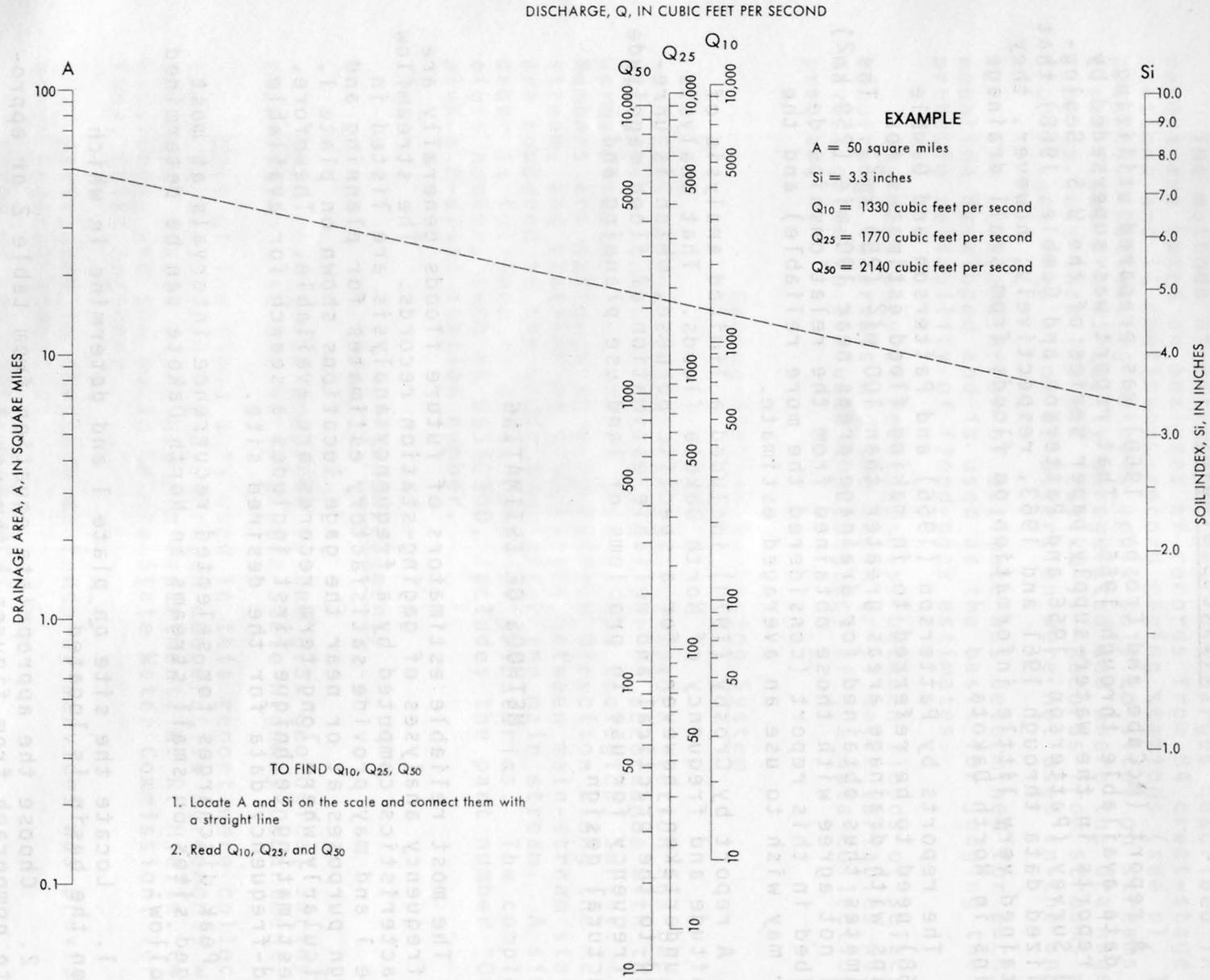


FIGURE 1.— Nomograph for determining peak discharges for floods in Region A with 10-year, 25-year, and 50-year recurrence intervals.

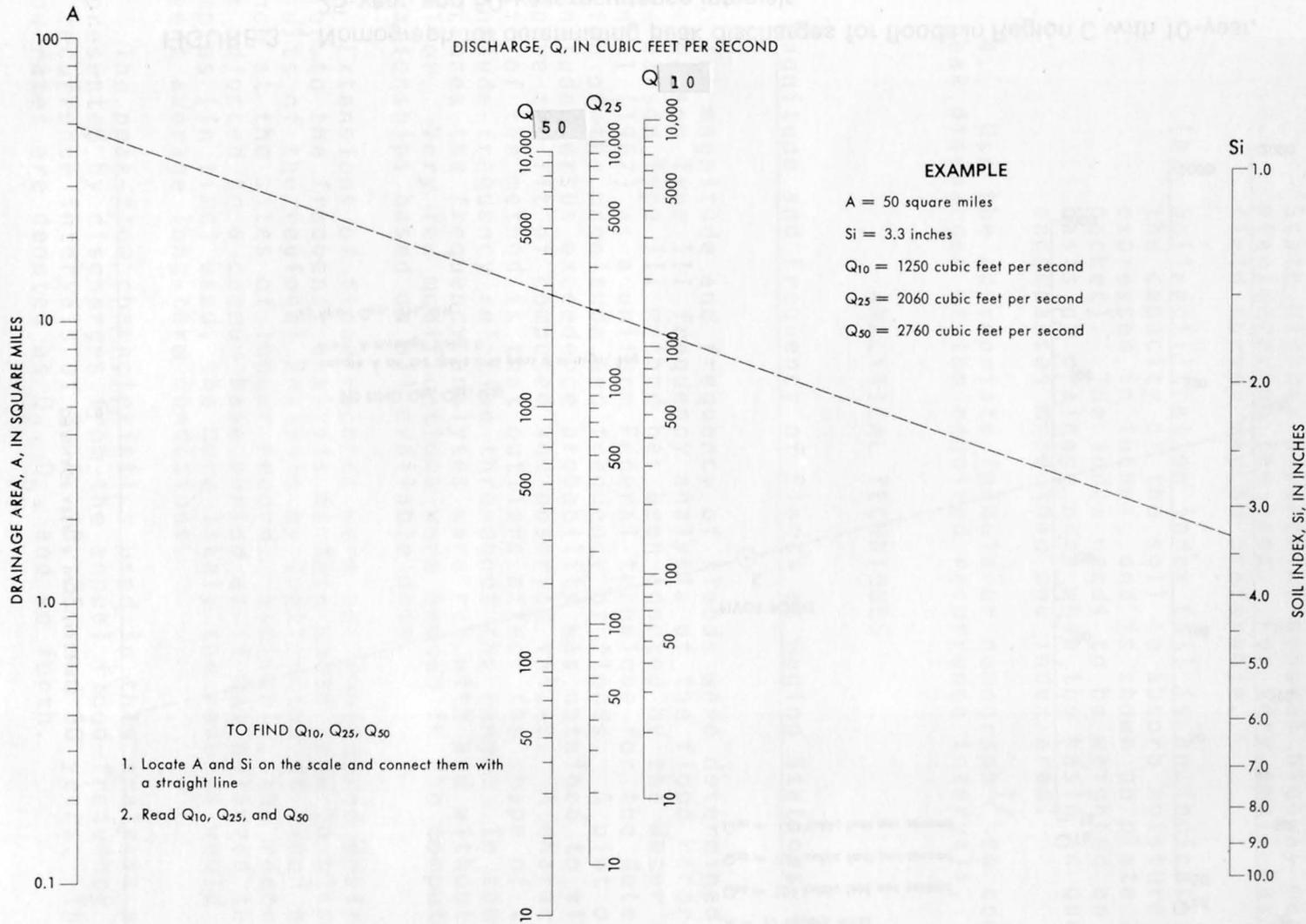


FIGURE 2.— Nomograph for determining peak discharges for floods in Region B with 10-year, 25-year, and 50-year recurrence intervals.

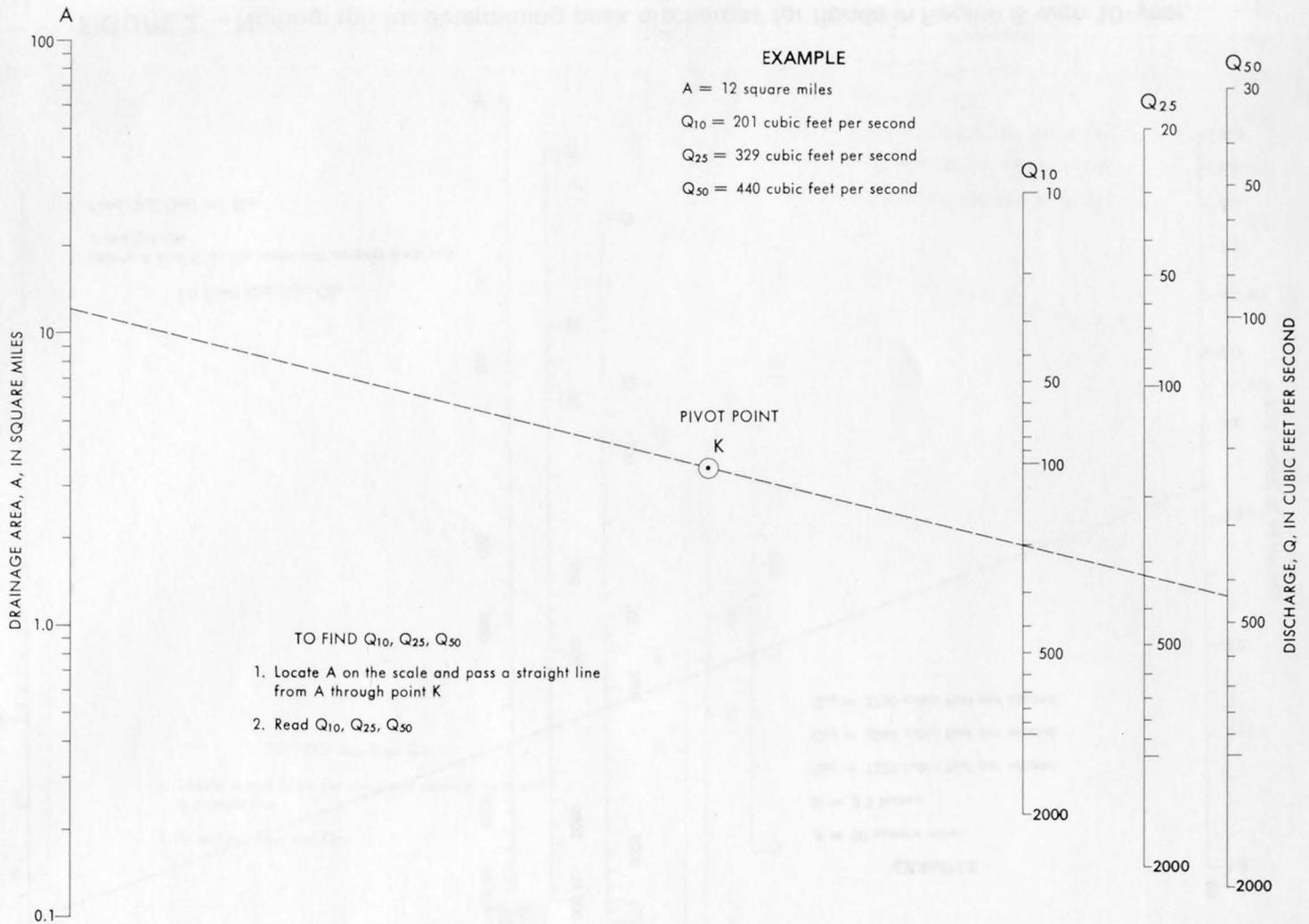


FIGURE 3.— Nomograph for determining peak discharges for floods in Region C with 10-year, 25-year, and 50-year recurrence intervals.

of interest on the best topographic map or the State Highway Department general highway maps, and planimetering the area. For very small basins a field survey may be preferable.

- (b) Soil-infiltration index (S_i) is an indication of the capacity of the soil to absorb moisture, expressed in inches, and is shown on plate 2 (in pocket). The index needs to be weighted on the basis of drainage area when the basin in question encompasses more than one index area.

4. Use the appropriate formula or nomograph^{1/} to compute the peak discharges at the required recurrence intervals.

ANALYTICAL TECHNIQUES

Magnitude and Frequency of Floods at Gaging Stations

The magnitude and frequency of floods were determined from a log-Pearson Type III frequency analysis of the flood record. The log-Pearson Type III method has been adopted by the Water Resources Council (1967) as a uniform Federal technique for the determination of the magnitude and frequency of floods. A plot of magnitude versus exceedence probability was obtained to visually compare the fit of computed and observed values. A characteristic of the method is that outliers affect the shape of the magnitude-frequency relation throughout the range. In some instances the frequency analyses were run with and without an outlier. Very few modifications were needed in the computed relationships based on all available data.

Extensions of flood records were not considered desirable prior to the frequency analysis as this would tend to bias the results of the regional analysis by duplication of flood experience at the sites of longer record. Similarly, the records were not adjusted to a common base period as it was believed the more samples (in time) used, the more likely the results would represent average long-term conditions.

The peak-flow characteristics used in this analysis are represented by discharges from the annual flood-frequency curve at recurrence intervals of 2, 5, 10, 25, and 50 years. The peak-flow rates are denoted as Q_2 , Q_5 , and so forth.

^{1/} The nomographs are not applicable for the corresponding metric units.

The reliability of frequency curves is related to the number of years of available flood record. The minimum number of years of record used to define floods of selected recurrence intervals in this study are:

Recurrence interval	10	25	50	100
Minimum years of record	10	15	20	25

Magnitudes of floods for these recurrence intervals will have about equal reliability according to Hardison (1969). These standards were relaxed slightly by using most of the records collected under the crest-stage program (18 years) to define the 50-year flood. The peak discharge for selected frequencies for all stations considered in this study are given in table 1.

Multiple-Regression Model

The regression model used in this analysis is of the form

$$Q_n = aA^bB^cC^d \dots \quad (1)$$

Where Q_n is a flood discharge having an n-year recurrence interval:

A, B, and C are basin characteristics;

a is the regression constant; and

b, c, and d are coefficients defined by regression analysis.

Past experience has indicated the general applicability of this regression model--the log transform of which is linear--in hydrologic studies. The step-backward method of multiple regression was used to relate a selected flood characteristic to one or more basin characteristics to form the regression equation. In the initial equation all basin characteristics were related to a flood characteristic. The calculations were sequentially repeated with the least significant basin characteristic omitted each time until only the most significant one remained. The standard error of estimate, the statistical significance of each basin characteristic, and other statistical data were determined for each successive calculation. This process was repeated for each of the flood characteristics.

Further analysis was made using residuals based on regression equations for which all parameters in the relation had greater than a 0.05 significance level. The residual is an index of departure from the expected and is used to check for regional groupings that would indicate a poorly defined relation or an undetected variable. The regional boundaries on plate 1 were defined on the basis of residuals.

Basin Characteristics Investigated in the Analysis

The basin characteristics tested in the regression analysis are given below. Some of the characteristics are poorly defined because much of the State is not covered by large-scale topographic maps, land use is continually changing and much of the aerial photographic coverage is 10 to 20 years old, and climatological and soils data are not available to the extent desired.

Drainage area	Basin elevation
Main-channel slope	Forest cover
Stream length	Annual precipitation
Surface storage	Rainfall intensity
Soil-infiltration index	Maximum March temperature
Basin shape	Minimum January temperature
Annual snowfall	Maximum July temperature
10-yr snowfall	Upper basin slope
100-yr snowfall	Gage vicinity slope
Annual evaporation	Cultivated area
Thunderstorm days	Main channel/wind aspect
Stream density	

FLOODS OF RECORD

The maximum discharges of record for unregulated streams draining less than 500 mi² (1,300 km²) are plotted against drainage area in figures 4, 5, and 6. The plots also include significant maximum discharges at miscellaneous sites. A curve of maximum flood experience of record is shown on each of the figures. The wide spread (three orders of magnitude) of flood peaks for a given drainage area suggests that adequate areal definition requires data at many sites.

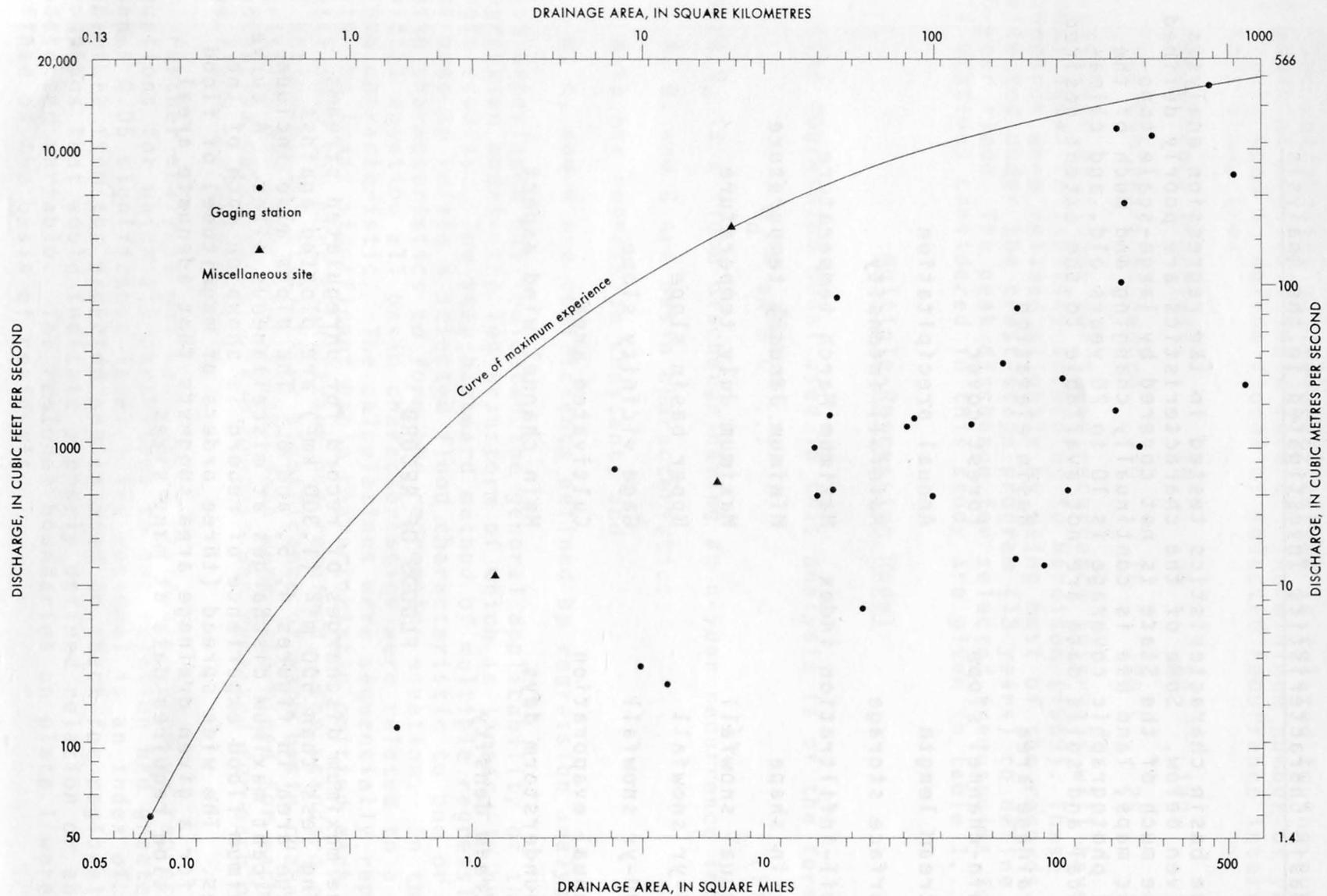


FIGURE 4. —Maximum known discharges in relation to drainage area in Region A.

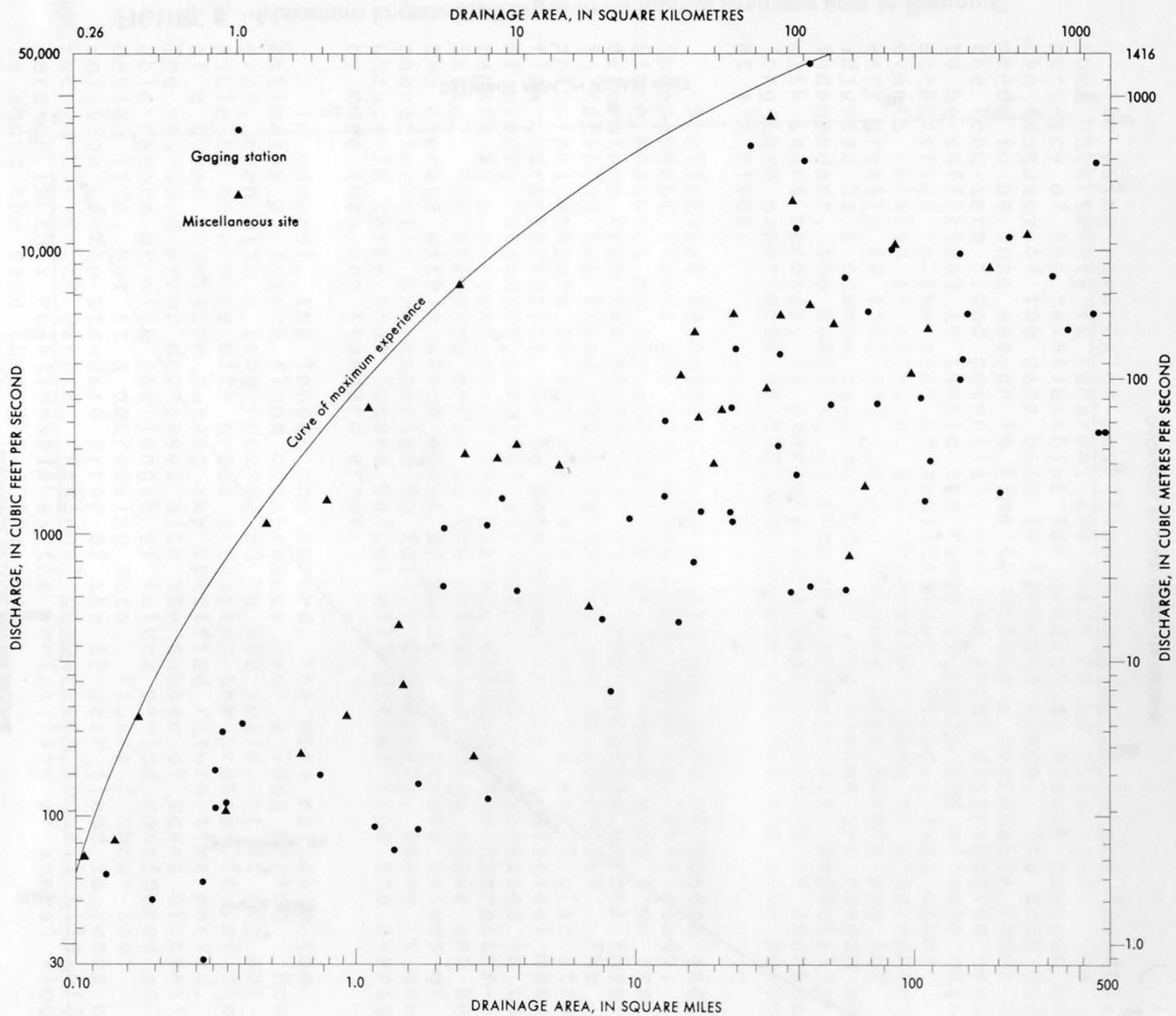


FIGURE 5.— Maximum known discharges in relation to drainage area in Region B.

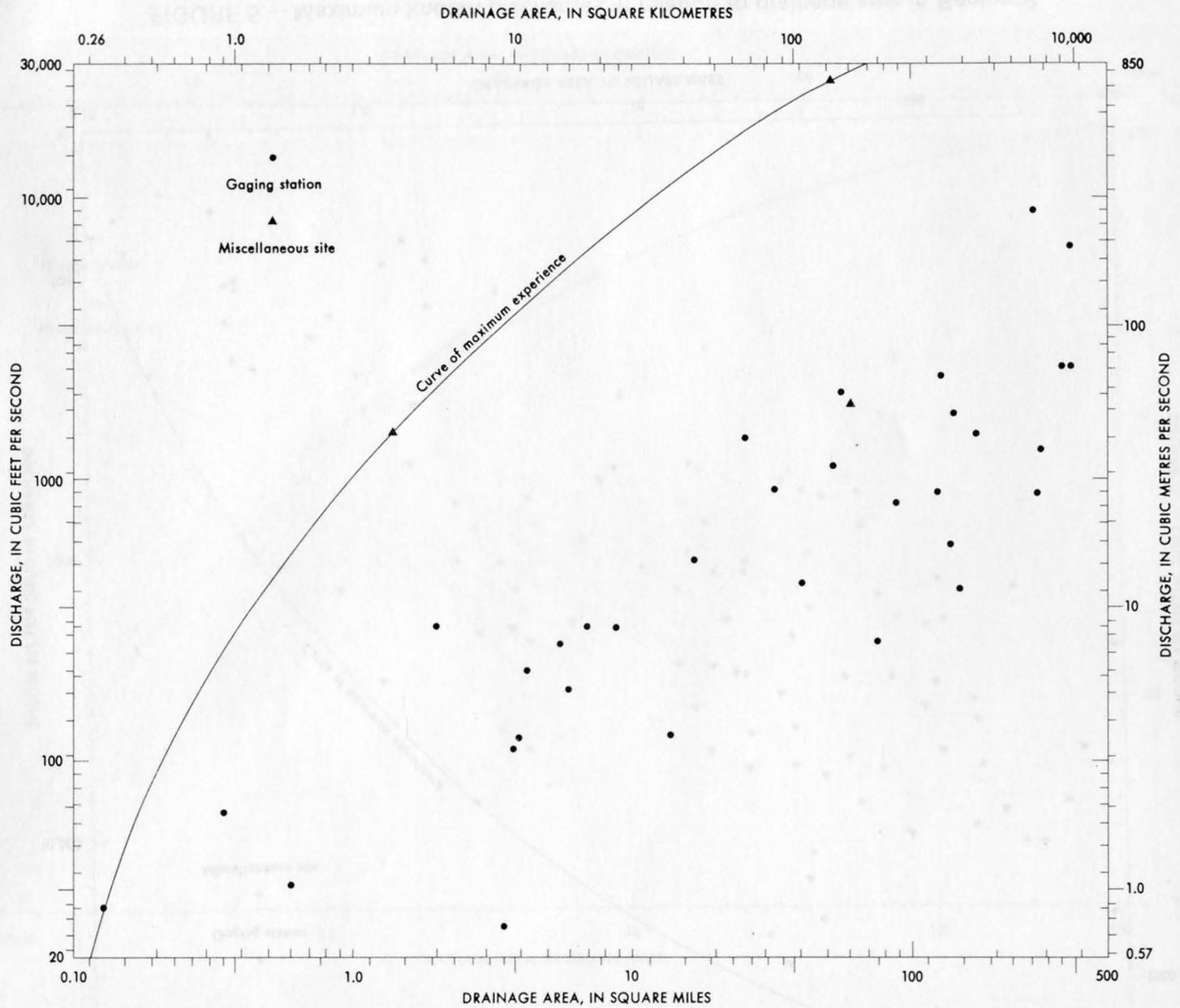


FIGURE 6.—Maximum known discharges in relation to drainage area in Region C.

ACCURACY AND LIMITATIONS

In using records of past floods to predict future floods, some uncertainty is inherent and needs to be tolerated. The accuracy of the relationships for predicting floods depends on the adequacy of the data base in time and space. The data base needs to be long enough in time to define a reasonable index of the long-term flood potential, and the areal distribution needs to be sufficient to sample the range of basin and climate characteristics in the area of application. Floods, like other events related to climate, are generally random in occurrence and vary greatly in time and space. Records used herein are not adjusted to a common base period. If the floods are random and independent, the sampling of flow characteristics, especially with short records, is probably more representative of average long-term experience if the samples are not for the same period at all sites.

Statistical techniques used in the analysis of random and independent floods are considered applicable for this study. Since measures of the variability of the floods with time are determined from the historical data, the probable errors involved in defining the flood characteristics can be appraised. The principal measure of the accuracy with which a particular stream-flow characteristics can be determined is the statistical measure of error, "standard error of estimate," and is expressed in this report as a percentage of the average value of the characteristic. The standard error is the estimated range above and below the average within which about 67 percent of future values of the characteristic are expected to fall. Conversely, there is one chance in three that future values will differ from the average by more than one standard error.

In general, the longer the record, the more reliable the estimates of future flood occurrences over a long period. However, even with a long record, 50 to 100 years, it is not possible to determine with great precision the probability of floods of a given magnitude during any specified future time period. The standard error decreases with the number of years of available record or with the length of future period considered as a design life, but at a decreasing rate. Figure 7 shows the relation of the standard error of the 50-year flood to years of record for North Dakota. Regional comparison shows little difference in this relationship as the variability of annual floods is about the same throughout the State.

The incremental economic value of additional years of record beyond a reasonable limit for planning and design of projects needs to be considered.

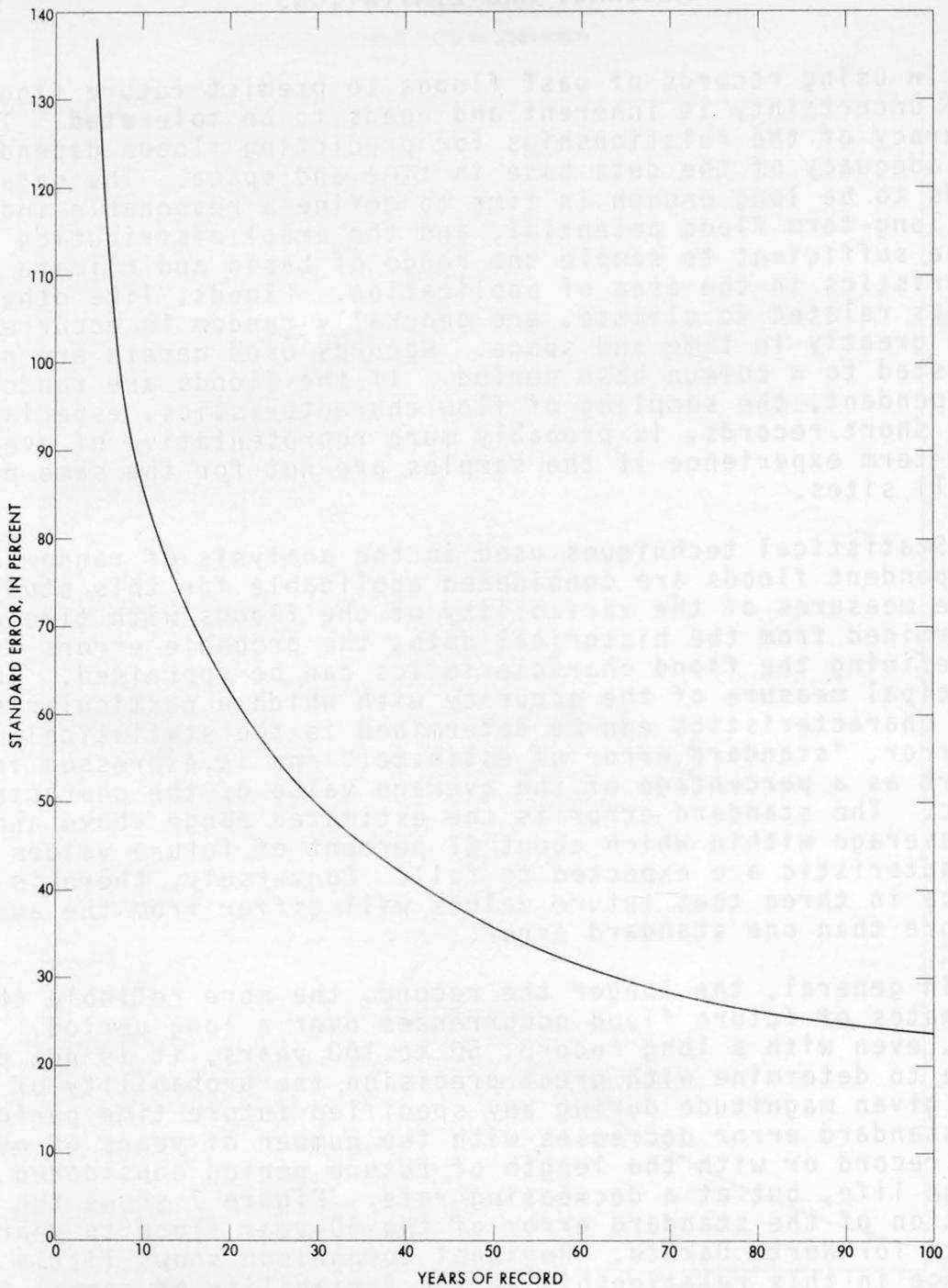


FIGURE 7.—Relation of standard error to years of record for the 50-year recurrence interval flood

This report is based on records of flood discharges from streams that are virtually unaffected by regulation or diversions. It is almost impossible to avoid areas with stock ponds so most records include their influence. The methods described herein are not applicable for streams subject to any other regulation or diversion without making allowances for the effects of these activities on the flood discharges. Effects of additional regulation need to be evaluated for the individual site.

The flood-frequency relation for a stream is altered considerably with urbanization. The effects of urbanization are not evaluated in this report and the methods described are not applicable in urban areas.

The methods in this report are applicable only for estimating flood magnitudes and frequencies for drainage areas less than 100 mi² (259 km²).

ADDITIONAL DATA ACQUISITION

The Federal Interagency Work Group, impaneled in 1971 by the Office of Water Data Coordination, U.S. Geological Survey, has recommended as an accuracy goal for flow characteristics--an accuracy equal to that with which the flow characteristic could be defined by 10 or more years of observed record at the site. The accuracy of the 50-year recurrence interval flood as stated in the statistical concept "standard error" is 86 percent (fig. 7). The average standard errors from the regression equations (table 2) range from 70 to 86 percent. Therefore, only a few of the stations need to be selected for an indefinite term of continued operation to detect long-term trends.

Effort could be directed toward preparation of a magnitude and frequency report by regression methods applicable to all drainage area sizes, as this would give some means of evaluating the accuracy of flood estimates for the larger basins.

Most small area drainage structures have some built-in upstream storage capacity. Consideration could be given to the installation of rainfall and runoff recorders at several sites for a period of time to define rainfall-runoff hydrographs to give the user of the data some indication of flood volumes from these areas.

TABLE 1.--Drainage areas, soil-infiltration indexes, flood discharges at selected frequencies, and maximum flows of record for gaging stations considered in this report

Station number	Station name	Drainage area (mi ²)	Soil-infiltration index (inches)	Discharge (ft ³ /s)					Maximum of record
				Recurrence interval (years)					
				2	5	10	25	50	
Red River of the North basin									
05051800	Grass Lake trib. nr Lidgerwood	0.61	3.2	8.9	20	30	41	51	36
05051900	Wild Rice River trib. nr Mantador	4.3	3.2	5.1	25	66	214	495	210
05055200	Big Coulee nr Maddock	90	2.7	179	412	611	900	1,140	810
05055520	Big Coulee nr Fort Totten	7.7	3.3	84	155	214	301	--	270
05056020	Mauvais Coulee trib nr Bisbee	8.9	2.6	18	78	162	335	537	300
05056040	Mauvais Coulee trib No. 2 nr Cando	17	2.8	59	191	319	508	664	520
05056060	Mauvais Coulee trib No. 3 nr Cando	129	2.8	161	665	1,110	1,650	2,020	2,300
05056080	Mauvais Coulee trib No. 4 nr Bisbee	53	2.8	81	322	611	1,120	1,650	1,100
05056900	Sheyenne River trib nr Cooperstown	15	2.4	343	661	854	1,060	1,180	1,000
05056950	Sheyenne River trib No. 2 nr Cooperstown	.08	1.6	6.2	18	31	59	87	59
05059600	Maple River nr Hope	17.4	3.2	361	585	733	916	1,050	734
05059800	Swan Creek nr Absaraka	33	3.2	90	240	400	685	970	930
05059850	Swan Creek trib nr Ayr	4.0	3.2	18	54	91	153	211	120
05059900	Swan Creek nr Casselton	57	2.9	219	695	1,090	1,580	1,920	2,000
05059950	Swan Creek trib nr Casselton	14	2.0	43	116	187	300	399	225
05060500	Rush River at Amenia	116	3.2	288	649	964	1,440	1,840	1,690
05065700	M. Br. Goose River nr Finley	33	3.0	569	1,000	1,270	1,580	--	1,250
05065800	M. Br. Goose River trib nr Finley	18	3.2	367	833	1,480	3,100	--	3,200

Station number	Station name	Drainage area (mi ²)	Soil-infiltration index (inches)	Discharge (ft ³ /s)					
				Recurrence interval (years)					Maximum of record
				2	5	10	25	50	
Red River of the North basin (cont.)									
05082600	English Coulee trib nr Grand Forks	4.7	2.1	59	123	152	174	183	164
05082680	Saltwater Coulee trib nr Grand Forks	22	3.2	143	318	393	446	466	290
05082700	Saltwater Coulee nr Emerado	110	2.9	241	509	670	835	930	730
05082900	Freshwater Coulee nr Emerado	31	2.3	355	764	1,010	1,260	1,400	1,180
05083600	M. Br. Forest River nr Whitman	73	2.5	84	274	438	652	--	425
05089100	M. Br. Park River nr Union	15	2.5	361	619	767	922	--	700
05089200	N. Br. Park River nr Gardar	52	2.7	429	1,190	1,580	1,890	2,020	1,200
05089500	Cart Creek at Mountain	17	2.7	281	604	841	1,140	1,360	1,300
05089700	Cart Creek at Crystal	74	2.6	781	2,140	2,800	3,270	3,450	2,950
05089800	Cart Creek trib nr Crystal	3.8	2.2	60	103	121	134	140	187
05098700	Hidden Island Coulee nr Hansboro	27	2.5	144	404	647	1,020	--	700
05098800	Cypress Creek nr Sarles	59	2.5	461	1,040	1,600	2,520	--	1,920
05113450	Long Creek trib No. 2 nr Crosby	5.6	2.6	39	103	177	326	491	260
05113520	Long Creek trib nr Crosby	.35	3.0	14	27	39	57	74	65
05116100	Souris River trib. nr Burlington	.13	3.3	6.6	20	31	47	60	30
05116200	Des Lacs River trib nr Donnybrook	3.8	3.0	57	132	178	227	256	210
05116550	Fuller Coulee at Foxholm	5.9	3.0	64	166	255	386	493	280
05117200	Souris River trib No. 2 nr Burlington	2.0	3.3	29	89	152	255	349	300
05122500	Willow Creek at Dunseith	91	1.9	92	281	463	742	974	476

Station number	Station name	Drainage area (mi ²)	Soil-infiltration index (inches)	Discharge (ft ³ /s)					
				Recurrence interval (years)					Maximum of record
				2	5	10	25	50	
Red River of the North basin (cont.)									
05123300	Oak Creek trib nr Bottineau	3.1	3.0	125	356	551	815	1,020	851
05123350	Oak Creek trib No. 5 nr Bottineau	.56	3.0	40	90	115	134	142	118
05123520	Egg Creek nr Glenburn	7.0	3.5	17	82	170	348	540	300
05123540	Egg Creek nr Ruthville	26	3.5	162	398	626	1,000	1,360	1,400
05123560	Egg Creek trib nr Deering	3.8	3.5	4.1	11	19	32	46	26
05123580	Egg Creek nr Deering	41	3.5	74	167	225	286	321	430
05123900	Boundary Creek nr Landa	170	2.5	184	579	1,120	2,370	3,950	3,580
Painted Woods Creek basin									
06329700	Painted Woods Creek trib nr Williston	0.37	3.7	8.5	33	65	134	214	110
06329800	Painted Woods Creek nr Williston	17	3.7	79	243	488	1,130	2,040	1,200
06329900	Painted Woods Creek trib No. 2 nr Williston	8.3	3.7	33	114	197	331	451	276
Sand Creek basin									
06330100	Sand Creek at Williston	38	3.7	141	492	892	1,620	2,320	1,600
White Earth River basin									
06331900	White Earth River trib nr Tioga	9.6	3.7	64	184	361	812	1,440	1,120
06332150	White Earth River trib nr White Earth	.32	3.0	22	58	86	122	148	107

Station number	Station name	Drainage area (mi ²)	Soil-infiltration index (inches)	Discharge (ft ³ /s)					
				Recurrence interval (years)					Maximum of record
				2	5	10	25	50	
Little Missouri River basin									
06335700	Deep Creek nr Bowman	0.29	3.9	12	26	40	63	86	58
06336100	Sheep Creek trib nr Medora	.32	2.3	24	49	72	110	145	147
06336200	Sheep Creek trib No. 2 nr Medora	.40	2.3	52	134	173	203	215	210
06336300	Little Missouri River trib nr Medora	.34	2.3	3.3	15	36	93	187	200
06336400	Jules Creek nr Medora	3.8	2.3	211	421	547	680	758	629
06336980	Little Missouri River trib nr Watford City	2.1	2.3	283	717	1,060	1,520	1,850	1,050
06337100	Spring Creek nr Watford City	23	3.2	303	747	1,110	1,620	2,010	1,100
Douglas Creek basin									
06337600	E. Br. Douglas Creek trib nr Garrison	1.4	3.7	31	57	76	99	115	76
Snake Creek basin									
06337900	Snake Creek trib nr Garrison	1.2	3.5	13	29	46	77	109	92
Knife River basin									
06340200	W. Br. Otter Creek nr Beulah	26	3.0	299	493	641	849	--	23,700
06340300	Otter Creek nr Hannover	43	3.0	605	823	896	942	--	45,300

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Station number	Station name	Drainage area (mi ²)	Soil-infiltration index (inches)	Discharge (ft ³ /s)					
				Recurrence interval (years)					Maximum of record
				2	5	10	25	50	
Square Butte Creek basin									
06342050	Sq. Butte Creek at Center	57	3.6	345	1,300	2,650	5,820	9,760	8,000
06342100	Sq. Butte Creek trib No. 2 nr Center	13	2.8	197	509	894	1,720	2,710	2,500
06342150	Sq. Butte Creek trib nr Center	.19	2.8	15	28	35	41	45	51
06342250	Sq. Butte Creek trib No. 3 nr Center	1.7	2.8	44	83	110	143	165	130
Burnt Creek basin									
06342300	Burnt Creek trib nr Baldwin	3.0	2.8	147	417	654	990	1,250	1,080
06342350	Burnt Creek trib No. 2 nr Baldwin	2.1	2.8	106	291	468	748	993	652
Heart River basin									
06343200	Heart River trib nr South Heart	.12	2.2	13	31	47	70	89	62
06344200	Heart River trib nr Dickinson	1.7	3.2	14	37	55	80	100	90
06345100	Antelope Creek nr Dickinson	69	2.3	578	2,780	5,070	8,310	10,700	6,100
06345200	Antelope Creek trib nr New England	13	3.2	177	615	1,070	1,790	2,430	1,360
06345300	Antelope Creek trib (site (No. 2) nr New England	22	3.2	326	875	1,170	1,410	1,520	1,200
06345700	Government Creek nr Richardton	33	3.2	273	1,000	1,930	3,760	5,770	4,300
06347100	Wilson Creek nr Glen Ullin	41	2.3	789	1,640	2,310	3,240	--	20,800
06347200	Hailstone Crk nr Bluegrass	39	2.6	491	997	1,330	1,710	--	12,000

Station number	Station name	Drainage area (mi ²)	Soil-infiltration index (inches)	Discharge (ft ³ /s)					Maximum of record
				Recurrence interval (years)					
				2	5	10	25	50	
Apple Creek basin									
06349100	Dead Buffalo Lake trib nr Steele	5.9	3.7	28	52	72	99	121	116
06349200	W. Br. Long Lake Creek nr Hazelton	16	3.6	71	343	634	1,060	1,410	798
Cannonball River basin									
06351650	M. Fk. Cedar Creek nr Buffalo Springs	33	3.3	301	656	1,090	2,020	--	2,050
06351680	White Butte Fork Cedar Creek nr Scranton	43	3.1	390	746	879	963	--	645
06353600	Louise Creek trib nr Brisbane	.29	4.7	9.9	19	25	32	37	31
06353700	Louise Creek trib nr Lark	.76	4.7	25	57	84	124	156	140
06353800	Louise Creek trib No. 2 nr Lark	7.7	3.2	30	99	194	406	676	500
06353900	Louise Creek above Flasher	110	3.7	258	743	1,190	1,860	2,430	1,300
Beaver Creek basin									
06354700	Spring Creek nr Linton	23	3.5	144	802	1,740	3,670	5,670	2,790
06354750	Sand Creek trib nr Hazelton	3.0	3.5	18	41	63	100	138	60
06354800	Sand Creek nr Temvik	23	2.7	151	702	1,580	3,700	6,550	4,500
Grand River basin									
06354885	N. Fk. Grand River trib nr Bowman	37	2.8	364	613	770	950	--	621
06354900	Spring Creek nr Bowman	51	2.5	123	598	1,110	1,860	2,480	2,880
06354950	Spring Creek trib nr Bowman	15	2.5	59	175	304	535	777	488

Station number	Station name	Drainage area (mi ²)	Soil-infiltration index (inches)	Discharge (ft ³ /s)					
				Recurrence interval (years)					Maximum of record
				2	5	10	25	50	

Grand River basin (cont.)

06354985	Alkali Creek nr Bowman	58	2.5	376	621	714	778	--	628
06355200	Buffalo Creek trib nr Buffalo Springs	3.4	3.0	29	147	324	698	1,150	1,320

James River basin

06467600	James River nr Manfred	56	3.3	93	315	534	867	1,150	900
06467650	James River trib nr Manfred	37	3.3	21	100	215	463	750	375
06467800	James River trib No. 3 nr Manfred	20	2.0	16	53	89	144	191	150
06469600	Mpls. Flats Creek trib nr Eldridge	9.9	3.7	14	36	56	83	107	90
06470200	Beaver Creek trib nr Eldridge	.19	3.3	9.0	23	34	47	56	45
06470300	Beaver Creek nr Sydney	62	3.3	119	486	884	1,520	2,060	1,010
06470400	Buffalo Creek trib nr Sydney	26	3.3	30	109	195	339	469	340

TABLE 2.--Summary of regression equations for estimating peak discharges in North Dakota

Region	Regression equation	Standard error of estimate (percent)	
		Average	Range
A	$Q_2 = 8A^{0.40}Si^{2.09}$	68	-47 to +89
	$Q_5 = 26A^{0.42}Si^{1.68}$	62	-44 to +79
	$Q_{10} = 43A^{0.42}Si^{1.50}$	64	-45 to +82
	$Q_{25} = 71A^{0.42}Si^{1.32}$	74	-50 to +98
	$Q_{50} = 94A^{0.42}Si^{1.24}$	86	-54 to +144
B	$Q_2 = 196A^{0.60}Si^{-1.74}$	74	-50 to +99
	$Q_5 = 465A^{0.63}Si^{-1.66}$	64	-45 to +82
	$Q_{10} = 626A^{0.64}Si^{-1.52}$	65	-46 to +84
	$Q_{25} = 766A^{0.65}Si^{-1.30}$	74	-50 to +98
	$Q_{50} = 848A^{0.65}Si^{-1.14}$	84	-53 to +115
C	$Q_2 = 13A^{0.46}$	85	-54 to +115
	$Q_5 = 34A^{0.51}$	72	-50 to +96
	$Q_{10} = 54A^{0.53}$	67	-47 to +88
	$Q_{25} = 86A^{0.54}$	67	-47 to +87
	$Q_{50} = 115A^{0.54}$	70	-48 to +92

Q_n = Peak discharge with a recurrence interval "n", in cubic feet per second;

A = Drainage area, in square miles;

Si = Soil infiltration index, in inches.

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