(200) WRi no. 75-55

# 'HYDROLOGIC ENVIRONMENTAL EFFECT OF SPRAYED SEWAGE EFFLUENT, TALLAHASSEE, FLORIDA





Prepared in cooperation with the CITY OF TALLAHASSEE, FLORIDA and the U.S. ARMY CORPS OF ENGINEERS



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HYDROLOGIC ENVIRONMENTAL EFFECTS OF SPRAYED

SEWAGE EFFLUENT, TALLAHASSEE,

FLORIDA

By Larry J. Slack

U.S. GEOLOGICAL SURVEY

Water-Resources Investigations 55-75

Prepared in cooperation with the City of Tallahassee, Florida and the U.S. Army, Corps of Engineers

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## HYDROLOGIC ENVIRONMENTAL EFFECTS OF SPRAYED SEWAGE EFFLUENT,

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#### ABSTRACT

Since 1966, Tallahassee has been experimentally disposing of the effluent from secondarily-treated sewage by spraying at the Thomas P. Smith Wastewater Renovation Plant. This report describes the disposal system, assesses the hydrologic and chemical effects of sewage effluent disposal on ground water in the area around the plant, and provides data useful for the development of basic criteria for land disposal of sewage effluent.

Potentiometric maps indicate ground water moves southwesterly from the spray area. Because of a high rate of natural underflow and large aquifer permeability the application of effluent caused no appreciable distortion of the regional ground-water flow pattern.

The quality of the sprayed effluent was generally improved as it filtered through the soil to the water table. BOD was reduced to less than 5 milligrams per litre and fecal coliform bacteria were reduced to zero. Almost all the phosphorus was removed.

Based on average data (collected from July 1972 to June 1974) there was a reduction in concentration of approximately 12 milligrams per litre of the total nitrogen from the effluent which was applied at a rate of 2 to 8 inches per week in an area with a cover crop. On the same basis there was an approximate concentration reduction of 5 milligrams per litre of the total nitrogen from the effluent applied at a rate of 14 inches per week to an undisturbed forest.

The high-rate application of effluent (14 inches per week) resulted in increased chloride and nitrogen concentrations to a depth of at least 270 feet in the underlying fresh-water aquifer.

Effluent-percolate has moved from the plant at about 2,400 feet per year. The effluent-percolate reached a well about 1,800 feet downgradient of the heavily-sprayed area in 3/4 year; the mixing ratio of effluent to ground water at that well was 1 to 4 in less than 2 years. As of June 1974 the leading edge of the waste front is somewhere between 1,800 and 4,000 feet downgradient from the plant and the ground water in about 140 acres of the underlying aquifer has increased in nitrate-N and chloride concentrations.

#### ACKNOWLEDGMENTS

The author gratefully acknowledges the cooperation of Mr. Thomas P. Smith, City Sanitary Engineer, and the personnel at the Thomas P. Smith Wastewater Renovation Plant.

Thanks are extended to Mr. William G. Leseman of the city of Tallahassee who supplied data and technical assistance and to the U.S. Forest Service for permission to drill observation wells in the Apalachicola National Forest.

#### PURPOSE AND SCOPE

In cooperation with the city of Tallahassee, Florida, and the U.S. Army Corps of Engineers, Mobile District, the U.S. Geological Survey is investigating the hydrologic and chemical effects on the underlying fresh ground-water system of sewage-effluent disposal by spray irrigation on land at the Thomas P. Smith Plant. The plant is also referred to as the Southwest Plant. The investigation is intended to provide data that will be useful for development of basic criteria for land disposal of sewage effluent. Figure 1 indicates the general location of the project area.

This report describes significant results for the period July 1972 through June 1974. It includes: (1) evaluation of the direction and rate of movement of water and the extent and location of the effluent plume in the aquifer underlying the disposal site and, (2) assessment of changes that occur in quality of the effluent and the receiving ground water.

The ground-water system in and adjacent to the plant is being monitored on a continuing basis to detect any long-term trend in water quality chargeable to spray irrigation of sewage effluent at the plant. In the continuation study the application of effluent has been discontinued in part of the study area in order to allow evaluation of recovery rates.

#### HISTORICAL BACKGROUND AND OPERATING HISTORY

As of June 1974 Tallahassee has four secondary sewage treatment plants: (1) Lake Bradford Road Plant, 4.5 Mgal/d (0.197 m $^3$ /s) capacity; (2) Dale Mabry Field Plant, 0.9 Mgal/d(0.039 m $^3$ /s) capacity; (3) Municipal Airport Plant, 0.06 Mgal/d (0.0026 m $^3$ /s) capacity; and (4) Thomas P. Smith Wastewater Renovation Plant, 2.5 Mgal/d (0.110 m $^3$ /s) capacity.

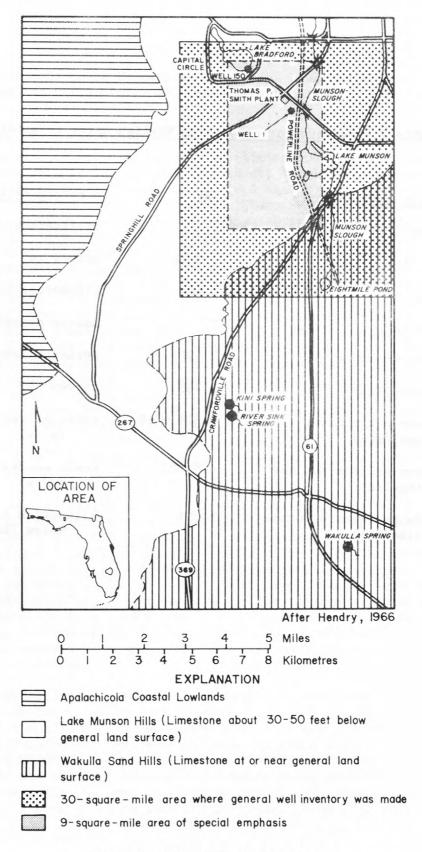


FIGURE 1. -- Area of investigation.

Table 1.--Factors for converting English units to metric units.

Multiply English units	<u>By</u>	To obtain metric units
inches (in)	25.4	millimetres (mm)
feet (ft)	.3048	metres (m)
miles (mi)	1.609	kilometres (km)
acres (ac)	4047	square metres $(m^2)$
square miles (mi <sup>2</sup> )	2.590	square kilometres $(km^2)$
gallons (gal)	3.785	litres (1)
gallons per minute (gal/min)	$6.309 \times 10^{-5}$	cubic metres per second $(m^3/s)$
million gallons per day (Mgal/d)	.04381	cubic metres per second $(m^3/s)$
pounds per square inches (1bs/in <sup>2</sup> )	6.896	kilonewton per square metre (kN/m <sup>2</sup> )

The Thomas P. Smith Wastewater Renovation Plant is at Springhill Road and Capital Circle (fig. 2), immediately southwest of the city limit.

Mr. Thomas P. Smith, the City Sanitary Engineer, advanced the idea of using spray irrigation as a means of eliminating the discharge of effluent, not only to Lake Munson, but to all local surface waters. 1966, Tallahassee began disposing about 0.5 Mgal/d (0.02 m<sup>3</sup>/s) of secondary-treated sewage effluent at its Thomas P. Smith Wastewater Renovation Plant, on an experimental basis. Other than the effluent disposed of by spray-irrigation methods, all domestic wastewater treatment-plant effluents from the city of Tallahassee, along with almost 85 percent of the runoff from its paved areas, flows to Lake Munson (Bishop, 1971) (fig. 1). The overflow from this lake empties into an area where the limestone of the Floridan aquifer is at or near land surface permitting direct recharge of the lake overflow to this aquifer. Severe algal blooms have caused the lake to become pea green in color and the city has received numerous complaints from local citizens of fish kills alleged to have been caused primarily by the effluent being discharged to the lake.

Nutrient uptake by crops and associated variations in plant growth rate with different application rates of sewage effluent have been investigated by the city of Tallahassee since 1970 in cooperation with researchers from the University of Florida (Overman, 1971; and Overman and Smith, 1973).

To allow flexibility in experimenting with dosing rates, four 4-acre  $(16,000-m^2)$  fields were established (fig. 2). Sprinkler heads were installed 100 ft (30.5~m) apart. Each head was capable of delivering 45 gal/min  $(0.00284~\text{m}^3/\text{s})$  at 40 lb/in²  $(276~\text{kN/m}^2)$  to produce a wetted area 155 ft (47.2~m) in diameter. The pump capacity was 720 gal/min  $(0.045~\text{m}^3/\text{s})$  at 160 ft (49~m) total head. Each 4-acre field contained 16 sprinklers. Effluent was applied in each field at a different rate--2, 4, 6, and 8 inches (50, 100, 150, and 200~mm) per week, Monday through Friday. The 4.5-acre  $(18,000~\text{m}^2)$  Single-Gun Area (fig. 2) was sprayed 24 hours a day Saturday and Sunday, or a total of 16 in (406~mm) a week.

In March 1972 the installation of four 1,000-gal/min  $(0.063 \text{ m}^3/\text{s})$  guns was completed in an 18-acre  $(72,000 \text{ m}^2)$  forested area (the 4-Big-Gun Area of fig. 2) east of the plowed field. These guns were set on 400-foot (120-m) centers and were designed to reach 250 ft (76 m) at 90  $1\text{bs/in}^2$   $(620 \text{ kN/m}^2)$ . The guns were operated in pairs: on alternate days guns A' and B' would operate for 8 hours while guns C' and D' would be idle. The 4-Big-Gun Area received an average of 1 Mgal/d  $(0.0438 \text{ m}^3/\text{s})$  7 days a week or 14 in (360 mm) per week. From 1972 to 1974,

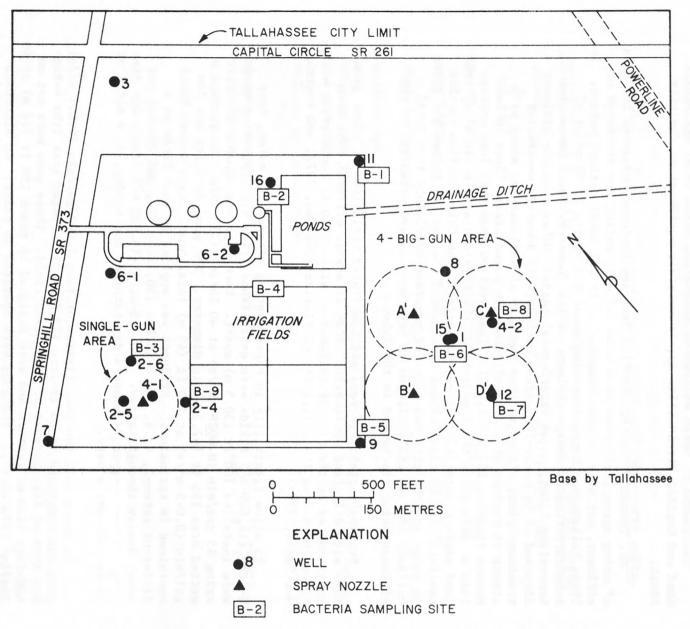


FIGURE 2.--Location of the Thomas P. Smith Wastewater Renovation Plant.

about 140 ft (43 m) of effluent was sprayed on the forested area (the 4-Big-Gun area). This heavy application rate was for the purpose of gathering needed design information and not as a normal spray-irrigation project (Thomas P. Smith, oral commun., July 1972).

Plant effluent in excess of the volume sprayed is discharged to Munson Slough, which drains into Lake Munson.

The spray-irrigation operation was essentially the same as outlined above until February 1973, when spray irrigation was discontinued in the Single-Gun Area. From February 1973 to June 1974, 1 Mgal/d (0.04 m $^3$ /s) or 14 in (360 mm) per week on the average was sprayed on the 4-Big-Gun Area and about 0.3 Mgal/d (0.01 m $^3$ /s) or 5 in (130 mm) per week on the Irrigation-Fields Area.

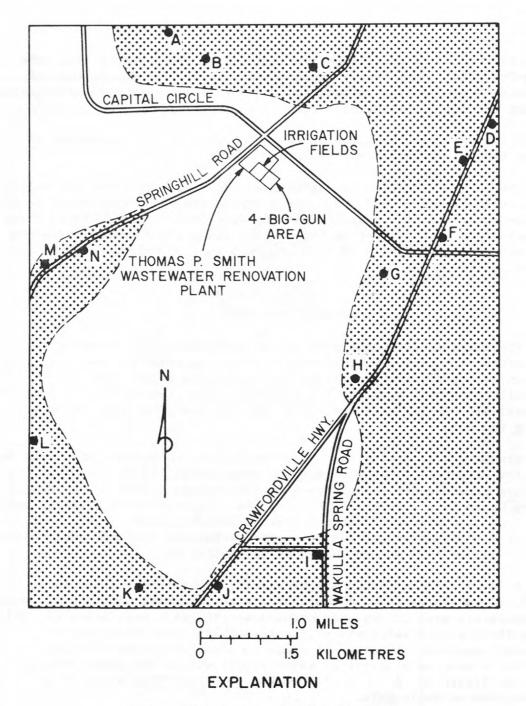
#### METHODS OF STUDY

The investigation consisted of the following: (1) reviewing the literature; (2) making an inventory of existing wells in the area; (3) installing observation wells; (4) collecting water samples; (5) collecting soil samples; (6) measuring water levels; (7) obtaining geophysical logs for selected wells; and (8) analyzing and interpretating the data.

Standard U.S. Geological Survey analytical techniques were used in this investigation to provide maximum transferability of results. The standard ground-water techniques used are published in U.S. Geological Survey Water-Supply Papers (WSP) 489, 494, 1536 E, 1536 I, 1544 H, and 1545 C and Techniques of Water-Resources Investigations (TWRI) Book 2, chapter El. The standard quality-water techniques used are published in WSP's 1454 and 1473 and in TWRI Book 5, chapter Al.

A  $30\text{-mi}^2$   $(78\text{-km}^2)$  area was canvassed; emphasis was given the  $9\text{-mi}^2$   $(23\text{-km}^2)$  area (fig. 1) surrounding the disposal site. Well owners and operators were interviewed, ground-water levels were measured, and water (both ground water and effluent) samples were collected for chemical analyses. Location of wells in areas where the Floridan aquifer is used as a source of water supply within the study area is shown on figure 3. A table of chemical data for these wells is given in the section on Basic Data.

Sixteen test wells were drilled during the first year of the study and seven during the second year. The location of these wells is shown in figure 4. The test wells were numbered in sequence. Table 2 gives a brief summary of well data. Well cuttings were collected at each site except wells 6 and 20. Cuttings from well 23 at depths greater than 60 ft (18 m) were considered unreliable due to drilling problems. All the chemical data for these wells are available from the U.S. Geological Survey, Florida District Office, Tallahassee, Florida.



G SELECTED SUPPLY WELLS

AREA WITH WELLS USED FOR WATER SUPPLY

FIGURE 3.--Location of wells in area where the Floridan aquifer is used as a source of water supply.

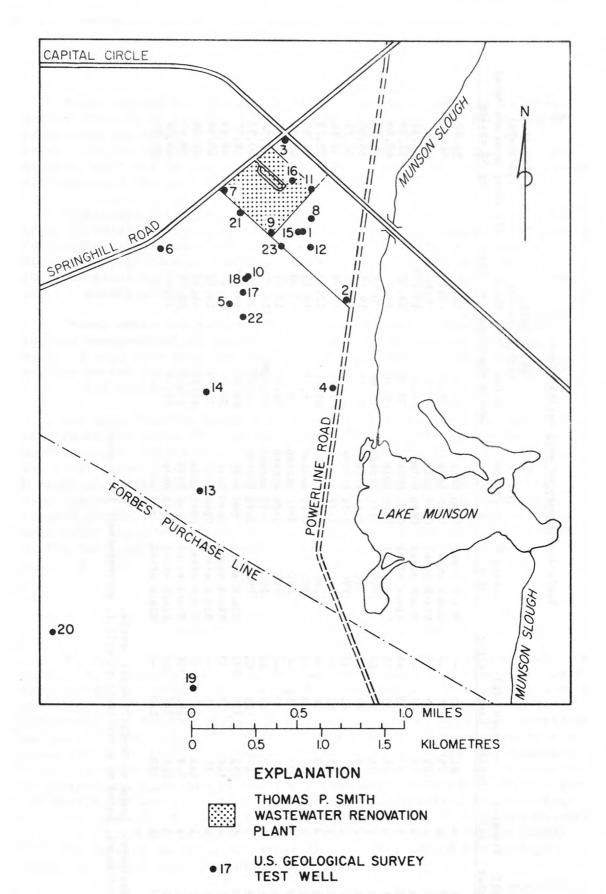


FIGURE 4. -- Location of U.S. Geological Survey test wells.

Table 2.--Summary of well descriptions.

							Water Level (ft)(05-24-74)		Elevation (05-24-74)
Well	Diameter		pth (ft		Casing	Finish	Below	Above	(ft above mean sea level)
number	mber (in) Drilled Cased Logged		type		land surface	mean sea level	Top of casing		
1	4	61	45		Black steel	Open hole	35.24	20.77	59.42
1 2 3	4	75	51		Black steel	Open hole	29.48	22.18	54.16
3	4	104	93	98	Black steel	Open hole	Dry	Dry	
4	4	53	38		Black steel	Open hole	24.5*	19.71*	47.31
5	4	51	33	45	Black steel	Open hole	30.25	19.78	52.09
6	4	45	35	45	Black steel	Open hole	20.20	19.98	42.66
7	2	35	27	25	PVC	2-ft screen	16.6*	20.4*	39.52
8	2	49	47	47	PVC	2-ft screen	37.77	19.13	59.90
9	2	42	42	42	PVC	2-ft screen	27.55	20.66	50.71
10	2	43	41		PVC	2-ft screen	37.7*	19.3*	59.64
11	2	40	38		PVC	2-ft screen	29.22	22.10	53.32
12	2	50	48	48	PVC	2-ft screen	38.75	19.21	61.96
13	2	37	35		PVC	2-ft screen	24.72	17.09	45.31
14	2	55	53		PVC	2-ft screen	26.18	17.94	47.12
15	2	47	45	45	PVC	2-ft screen	35.24**	20.77**	57.50
16	2	67	65	65	PVC	2-ft screen	36.25	21.89	60.37
17	4	152	152	152	Black steel	Open hole	29.08*	19.75*	48.83
18	4	160	54	53	Black steel	Open hole	38.00	19.90	58.35
19	4	75	53		Black steel	Open hole	24.63	14.31	40.94
20	4	145	145		Black steel	Open hole	14.52	14.32	30.84
21	4	248	247	248	Black steel	Open hole	37.50	20.42	58.92
22	4	268	267	268	Black steel	Open hole	28.79	19.45	48.74
23	4	270	240		Black steel	Open hole			53.67

<sup>\*</sup>Estimated, based on 06-12-74 water levels. \*\*Estimated, based on water level at well 1, approximately 15 ft away.

Wells drilled earlier by the Florida Bureau of Geology are identified (fig. 2) by two numbers separated by a dash. The first number represents the diameter of the well in inches; the second number represents the chronological order in which the well was drilled. For example, well 4-2 is the second 4-in (100-mm) well drilled by the Bureau of Geology in the test area.

Continuous water-level recorders were installed: (1) on well 150 (fig. 1) outside the spray effluent area--to monitor ground-water levels for benchmark purposes; and (2) on well 1 (fig. 1)--in the area of heaviest application of sewage effluent. Water levels were measured in the Geological Survey wells and Bureau of Geology wells 2-4, 2-5, 2-6, and 4-1 weekly until April 1974 and monthly until July 1974.

Ground water was sampled monthly; the effluent bimonthly; and springs downgradient of the plant, quarterly for chemical analyses. Rainfall data were obtained from the National Oceanic and Atmospheric Administration for the Tallahassee Municipal Airport approximately 2 mi (3 km) west-northwest from the study area.

The investigation began 6 years after effluent was first applied to the irrigation plots (Overman and Smith, 1973). This eliminated an opportunity to obtain data "before the fact" and to follow changes in the local ground waters from point zero in time and space. However, historical perspective of activities at the plant was obtained through discussions with Messrs. Smith and Overman and the review of their related papers. Also, the total effect of application of wastes before this study began was evaluated by areal determination of the character of the water in the aquifer beyond the area of influence of the effluent.

#### HYDROGEOLOGY

## Importance of Hydrogeologic Factors

Hydrogeologic factors are especially important to the investigation because they largely determine the quality of the effluent that reaches the underlying aquifer. For any such investigation, hydrogeology limits the the amount of treatment wastewater receives from the disposal medium (Goldstein and Moberg, 1973). Most soils and cover crops can beneficiate sewage effluent when application rates are less than evapotranspiration rates (Parizek, 1973). The more irrigation rates exceed evapotranspiration rates the more important hydrogeologic factors become in insuring sufficient and long-term renovation of wastewater (Parizek, 1973). The 730 in (18,500 mm) per year of sprayed effluent received by the 4-Big-Gun Area is about 16 times the average annual potential evapotranspiration of 46 in (1,170 mm) reported for the area by Visher and Hughes (1969). Effluent sprayed in the other areas also greatly exceed the average annual potential evapotranspiration.

# Regional Hydrogeology

Geologically, the Thomas P. Smith Wastewater Renovation Plant is situated on the Lake Munson Hills sediments of the Woodville Karst Plain (Hendry and Sproul, 1966). The Plain extends from the south edge of Tallahassee to the Gulf. Its western boundary is approximately along the range line between townships 1 and 2 west, and its eastern edge extends into Jefferson County. The Plain is characterized by loose quartz sands thinly veneering a limestone substratum—a sinkhole—sand dune topography. Elevations are 20 to 60 ft (6 to 18 m), with dune tops to 80 ft (24 m) above mean sea level. The dunes are of barchan type, oriented to northeasterly winds. The porous and permeable sand allows quick entry of rain to the soluble and sink-dotted limestone and permits little stream development.

Although some water is contained in the overlying sand, silt, and clay, the principal source of ground water in Leon County consists of a sequence of limestones and dolomites that generally act as a hydrologic unit (Hendry and Sproul, 1966) and is referred to as the Floridan aquifer (Parker and others, 1955). The Floridan aquifer underlies all of Florida and parts of southern Alabama and Georgia. Recharge to the aquifer in Leon County is derived from rain which falls on Gadsden, Leon, and Jefferson Counties, and on adjacent areas in Georgia. Discharge from the aquifer is generally southerly, to the gulf. Over 95 percent of all water used in Leon County is obtained from the Floridan aquifer (Hendry and Sproul, 1966). All of the city and industrial-supply wells are upgradient of the Smith Plant.

The limiting factors to downward percolation of the effluent or rain at the plant are the clay layers above the limestone. The movement of soil water, especially excess water that moves downward as a result of gravity drainage is limited by the least permeable strata (those having the lowest hydraulic conductivity) through which it must pass.

Lithologic logs and natural-gamma logs of 21 Survey wells and 4 Bureau of Geology wells (Basic Data B) were used in determining the positions and thicknesses of clay layers penetrated by cased wells. The natural-gamma radioactivity of quartz sand and pure limestone is negligible. Because clays tend to concentrate the heavy radioactive elements through the processes of ion exchange and adsorption, the natural-gamma activity of clay-bearing sediments generally is much higher than that of quartz sands and carbonates. Thus, an increase in natural-gamma count (which is directly proportional to radioactivity) may indicate an increase in clay content of the material (Keys and MacCary, 1971).

The lithologic logs and natural-gamma logs indicate that sandy clay, clayey sand, or clay were present in one or more layers at shallow depths beginning within the first 32 ft  $(9.6\ m)$  at all the wells logged except wells 2 and 3. Well 2 passed through a thin limestone layer but was still in sand at 75 ft  $(23\ m)$ . Well 3 penetrated its first clay layer at 82 ft  $(25\ m)$ .

The location of two hydrogeologic sections is shown on figure 5. In both sections wells were projected into the line at section as shown. Section A-A' (fig. 6) was drawn based on lithologic and natural-gamma logs. Section B-B' (fig. 7) was drawn based on lithologic, driller, and natural-gamma logs and chemical analyses of ground water. The midpoint or center of the minimum-maximum intensity value on the gamma-ray curve was assumed to represent the correct point for determining the top and bottom of the clay zones (Mercier, 1950). For the convenience of the reader, on figure 6 gamma-ray curves were plotted to the immediate left of the well-location lines at the same scale. Tic marks were then drawn on the well-location lines to represent the boundaries of the clay layers. The potentiometric surface shows the altitude at which water level would have stood in tightly cased wells open to the uppermost Floridan aquifer in February 1974.

The sand aquifer and the limestone aquifer appear to act as one hydraulic unit. Apparently the clay layers are discontinuous; otherwise, little of the effluent would be reaching the limestone aquifer at well 1 and there would be a large amount of water perched above the On the contrary, no water was observed by the drillers above the clay at well 1 when it was drilled after the area received 2 in (50 mm) of sewage effluent per day for 6 months. Furthermore, chloride concentrations for water samples collected at well 1 from the limestone aquifer have been approximately equal to that of the sprayed effluent, indicating that large volumes of the effluent are reaching the underlying aquifer regardless of the clay layers. The movement of large volumes of water through the limestone is related to solution openings rather than "primary" porosity. (This point is discussed in detail in the "Changes in Quality of Effluent" section.) The effective porosities (table 3) for the clay and limestone at well 10 are approximately twice as great as those at well 8, indicating they may not represent portions of the same layers.

# Basic Hydrogeologic Concepts

Hydraulic conductivity, as reported in table 3, is the rate of flow, in cubic feet per day, through a cross-sectional area of 1 square foot, under a hydraulic gradient of 1 foot per foot, at the prevailing kinematic viscosity in units of feet per day (Lohman, 1972). For convenience it is also shown in metres per day. The older "field

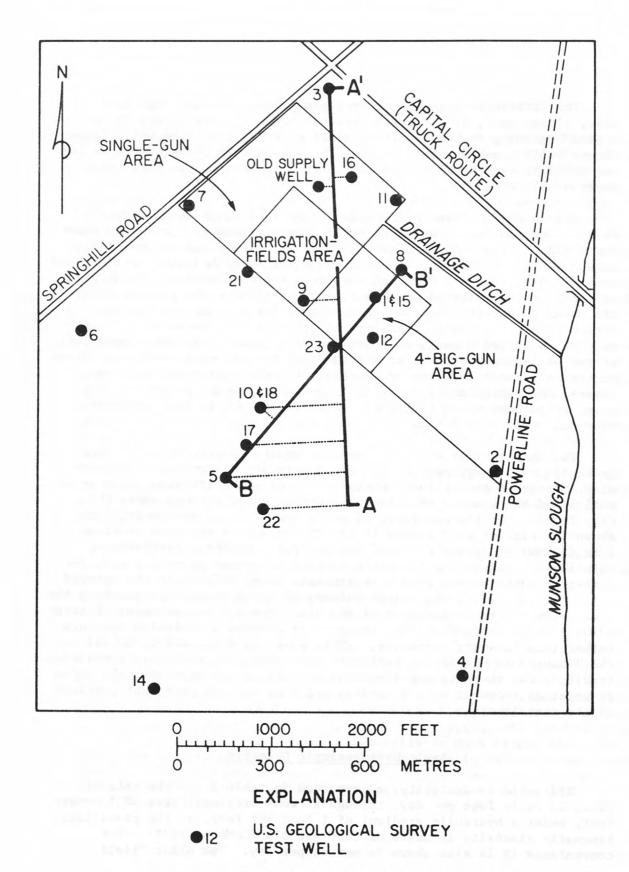


FIGURE 5.--Location of sections A-A' and B-B'.

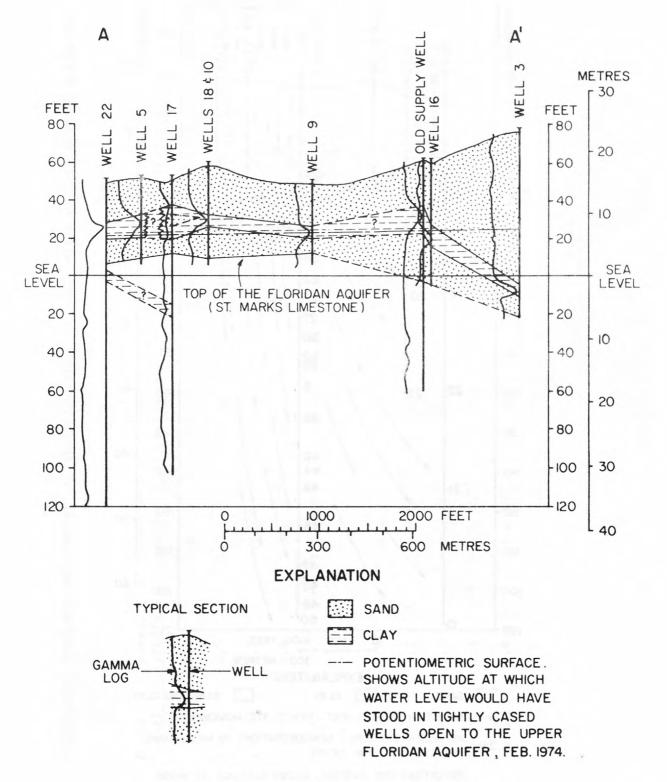
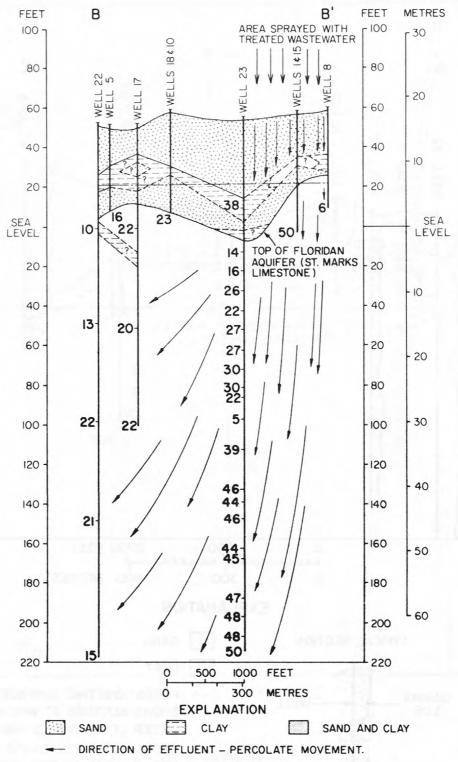


FIGURE 6--Hydrogeologic section A-A'.



39 REPRESENTS CHLORIDE CONCENTRATIONS IN MILLIGRAMS PER LITRE AT THAT DEPTH.

POTENTIOMETRIC SURFACE. SHOWS ALTITUDE AT WHICH
WATER LEVEL WOULD HAVE STOOD IN TIGHTLY CASED
WELLS OPEN TO THE UPPER FLORIDAN AQUIFER, FEB. 1974.

FIGURE 7. -- Hydrogeologic section B-B'.

Table 3.--Laboratory analyses and statistical characteristics of grain sizes of core samples collected at wells 8 and 10.

Depth (feet)	hydraulic co	nductivity	Median diameter (millimetres)	Effective porosity (percent)	Sorting coefficient	Lithology
7- 8	5.9	1.8	0.25	36.0	1.6	Sand
13-14	4.3	1.3	.23	34.1	1.7	Sand
17-18	6.2	1.9	.13	35.8	1.6	Sand
27-28	$4.6 \times 10^{-6}$	$1.4 \times 10^{-6}$	> .001	5.3		Clay
37–38	$3.05 \times 10^{-3}$	$9.3 \times 10^{-4}$	.012	12.5	830	Limestone
3- 4	14.4	4.4	.26	35.5		Sand
13-14	$7.5 \times 10^{-1}$	$2.3 \times 10^{-1}$	.20	28.3	1.6	Sand
23-24	1.3	$3.9 \times 10^{-1}$	.21	21.8	1.6	Sand
33-34	$1.5 \times 10^{-6}$	$4.7 \times 10^{-7}$	> .001	12.4		Clay
41-42	$4.6 \times 10^{-4}$	$1.4 \times 10^{-4}$	.026	26.6	8.0	Limestone
	(feet)  7- 8  13-14  17-18  27-28  37-38  3- 4  13-14  23-24  33-34	Depth (feet) (ft/day) (m  7-8 5.9  13-14 4.3  17-18 6.2  27-28 4.6 x 10 <sup>-6</sup> 37-38 3.05 x 10 <sup>-3</sup> 3-4 14.4  13-14 7.5 x 10 <sup>-1</sup> 23-24 1.3  33-34 1.5 x 10 <sup>-6</sup>	(feet) (ft/day) (metres/day)  7- 8 5.9 1.8  13-14 4.3 1.3  17-18 6.2 1.9  27-28 4.6 x 10 <sup>-6</sup> 1.4 x 10 <sup>-6</sup> 37-38 3.05 x 10 <sup>-3</sup> 9.3 x 10 <sup>-4</sup> 3- 4 14.4 4.4  13-14 7.5 x 10 <sup>-1</sup> 2.3 x 10 <sup>-1</sup> 23-24 1.3 3.9 x 10 <sup>-1</sup> 33-34 1.5 x 10 <sup>-6</sup> 4.7 x 10 <sup>-7</sup>	Depth (feet) hydraulic conductivity (feet) (ft/day) (metres/day) diameter (millimetres)  7-8 5.9 1.8 0.25  13-14 4.3 1.3 .23  17-18 6.2 1.9 .13  27-28 4.6 $\times$ 10 <sup>-6</sup> 1.4 $\times$ 10 <sup>-6</sup> > .001  37-38 3.05 $\times$ 10 <sup>-3</sup> 9.3 $\times$ 10 <sup>-4</sup> .012  3-4 14.4 4.4 .26  13-14 7.5 $\times$ 10 <sup>-1</sup> 2.3 $\times$ 10 <sup>-1</sup> .20  23-24 1.3 3.9 $\times$ 10 <sup>-1</sup> .21  33-34 1.5 $\times$ 10 <sup>-6</sup> 4.7 $\times$ 10 <sup>-7</sup> > .001	Depth (feet)       hydraulic conductivity (ft/day)       diameter (millimetres)       porosity (percent)         7- 8       5.9       1.8       0.25       36.0         13-14       4.3       1.3       .23       34.1         17-18       6.2       1.9       .13       35.8         27-28       4.6 x $10^{-6}$ 1.4 x $10^{-6}$ > .001       5.3         37-38       3.05 x $10^{-3}$ 9.3 x $10^{-4}$ .012       12.5         3- 4       14.4       4.4       .26       35.5         13-14       7.5 x $10^{-1}$ 2.3 x $10^{-1}$ .20       28.3         23-24       1.3       3.9 x $10^{-1}$ .21       21.8         33-34       1.5 x $10^{-6}$ 4.7 x $10^{-7}$ > .001       12.4	Depth (feet)         hydraulic conductivity (ft/day)         diameter (millimetres)         porosity (percent)         Sorting coefficient           7-8         5.9         1.8         0.25         36.0         1.6           13-14         4.3         1.3         .23         34.1         1.7           17-18         6.2         1.9         .13         35.8         1.6           27-28         4.6 x 10 <sup>-6</sup> 1.4 x 10 <sup>-6</sup> >.001         5.3            37-38         3.05 x 10 <sup>-3</sup> 9.3 x 10 <sup>-4</sup> .012         12.5         830           3-4         14.4         4.4         .26         35.5            13-14         7.5 x 10 <sup>-1</sup> 2.3 x 10 <sup>-1</sup> .20         28.3         1.6           23-24         1.3         3.9 x 10 <sup>-1</sup> .21         21.8         1.6           33-34         1.5 x 10 <sup>-6</sup> 4.7 x 10 <sup>-7</sup> >.001         12.4

coefficient of permeability" in gallons per day per square foot introduced by Meinzer and Wenzel (1942) can be obtained by multiplying the hydraulic conductivity value in feet per day by 7.48 gallons per cubic foot.

In unconsolidated granular material the hydraulic conductivity is largely a function of the size and shape of the component grains, their degree of sorting, and the amount of interconnected pore space available for fluid transmission. Commonly, median diameter is used as an index of particle size; effective porosity, of pore space available for fluid transmission; and sorting coefficient, the degree of sorting in a sample. Hydraulic conductivity generally decreases with a decrease in median diameter and effective porosity and an increase in sorting coefficient. Median diameter and effective porosity decrease with increasing clay content. This was true for the core samples of wells 8 and 10, as shown in table 3. For core samples collected at wells 8 and 10 (see fig. 5 for location) the hydraulic conductivity was lowest for clay, next lowest for limestone, and highest for sand. The median diameter of 10 core samples collected at wells 8 and 10 ranged from less than 0.001 mm for dense clay to 0.26 mm for sand (table 3). The median diameter is the particle size that is larger than 50 percent of the sample and smaller than the other 50 percent. The average of all the median diameters was 0.13 mm, and for samples without an appreciable silt and clay fraction, 0.21 mm. These sizes are in the fine sand range (0.125 to 0.25 mm) of the Wentworth classification (Krumbein and Sloss, 1955). Sand particles of medium size (0.25 to 0.5 mm) constituted from O to 51 percent of the samples. Sand particles in the coarse (0.5 to 1 mm) and very coarse (1 to 2 mm) sizes constituted from 0 to 9.3 percent and 0 to 0.2 percent, respectively. None of the core samples contained gravel-size particles (greater than 2 mm).

Effective porosity (table 3) is expressed as a percentage of the total volume occupied by the interconnected interstices having entrance diameters larger than  $10^{-7}$  m. As pointed out by Lohman and others (1972), the present definition of effective porosity differs from that of Meinzer (1923b). The effective porosity is roughly 2 to 7 times greater for the samples without an appreciable silt or clay fraction than for those containing appreciable clay.

The sorting coefficient is a statistical parameter developed by Trask (1932) and defined as the extent to which the grains spread on either side of the average and is equal to the square root of the ratio of the particle diameter that is larger than 75 percent of the sample to the particle diameter that is larger than 25 percent of the sample. According to Trask, well-sorted sediments (which allow water to move through them rapidly) have sorting coefficients less than 2.5; moderately-sorted sediments, 2.5 to 4.0; and poorly-sorted sediments, greater than 4.0. Except for core samples of clay or limestone, the sorting coefficients listed in table 3 are less than 2.5, indicating the sand overlying the clay and limestone is well sorted.

# Evaluation of Regional Ground-Water Flow Pattern

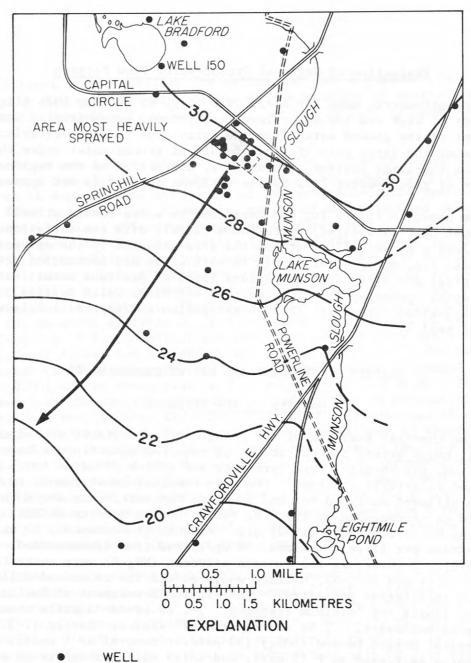
Potentiometric maps for April 1973 (fig. 8) and May 1974 (fig. 9) (periods of high and low water-level conditions, respectively) indicate movement of the ground water from the spray area is southwesterly. Because of the large rate of flow of natural ground water under the site and the large aquifer permeability, distortion of the regional pattern of ground-water flow by the effluent applied is not appreciable.

As shown in figure 10, the variation in water levels at well 1 (the area most heavily sprayed) corresponds closely with the variation in water levels at well 150 (outside the area affected by the sprayed effluent, fig. 8). Water levels in both wells are controlled primarily by rainfall and antecedent conditions (such as previous water levels). Water-level hydrographs (not shown) for the other wells drilled by the U.S. Geological Survey for this investigation correspond closely with that of well 1.

## QUALITY OF EFFLUENT AND NATIVE GROUND WATER

## Quality of the Effluent

The chemical analyses of the sewage effluent (table 4) indicate that although actual concentrations of chemical constituents have some variation, the chemical type (presence and ratios of major ions) has remained relatively constant. The only chemical constituents in the sewage effluent analyzed for and found in concentrations above those allowed or recommended by the U.S. Public Health Service (USPHS) Drinking Water Standards (1962) are: Arsenic (recommended, 10 ug/1 (micrograms per litre); maximum, 50 ug/l) and iron (recommended, 300 ug/1). However, if the ammonia-nitrogen (NH4-N) were converted to nitrate-nitrogen ( $NO_3$ -N), nitrate would exceed the recommended limit of 10 mg/l (milligrams per litre). The Florida Department of Pollution Control limits for "minimum treatment. . . or advanced waste treatment as deemed necessary. . . by the Department" (Rules, Chapter 17-3.04) for Biochemical Oxygen Demand (BOD<sub>5</sub>) (90 percent removal or 5 mg/1), total phosphorus expressed as P (1 mg/l) and total nitrogen expressed as N (3 mg/l) are exceeded. In addition, fecal coliform organisms have been detected in the effluent in concentrations from 4,000 to 80,000 colonies per 100 millilitres (ml) (William Leseman, Laboratory Director, Smith Plant, oral commun., December 1974), greatly exceeding the 1 colony per 100 ml recommended limit for drinking water (USPHS, 1962). No evaluation was made of the occurrence of viruses in this investigation.



DIRECTION OF GROUND WATER FLOW

POTENTIOMETRIC CONTOUR. SHOWS ALTITUDE AT WHICH WATER LEVEL WOULD HAVE STOOD IN TIGHTLY CASED WELLS, APRIL 1973. DASHED WHERE APPROXIMATELY LOCATED. CONTOUR INTERVAL IS 2 FEET. DATUM IS MEAN SEA LEVEL.

THOMAS P. SMITH WASTEWATER RENOVATION PLANT

FIGURE 8.--Potentiometric surface of upper part of Floridan aquifer in April 1973 (high-water period), Tallahassee vicinity.

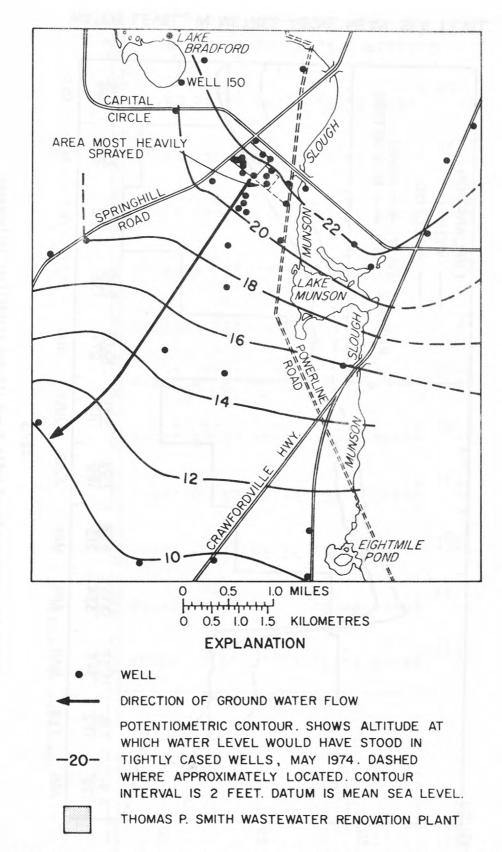


FIGURE 9.--Potentiometric surface of upper part of Floridan aquifer in May 1974 (low-water period), Tallahassee vicinity.

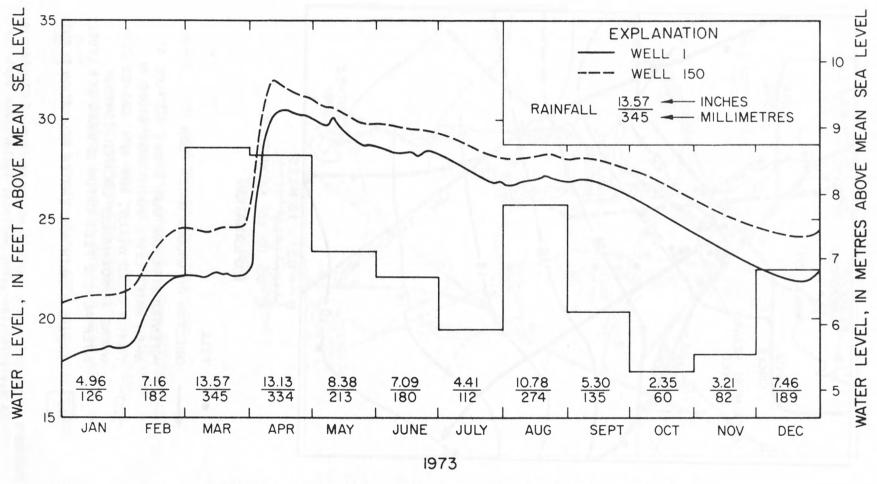


FIGURE 10.--Hydrographs of wells 1 and 150 and rainfall at Tallahassee.

Table 4.--Analyses of sewage effluent.

(Concentrations are in milligrams per litre except for constituents indicated by \* which are in micrograms per litre.)

Date of Collection	09-20-72	10-31-72	12-28-72	03-05-73	04-24-73	06-28-73	08-30-73	11-07-73	12-20-73	02-13-74	04-30-74	06-27-7
Color (units)	50	65	55	30	50	60						
Turbidity (JTU)	20	100	10	25	4	35			20		27	24
Silica	18	17	15	16	15	18						
Calcium	39	42	39	38	33	33		39	37	36	39	40
Magnesium	11	12	12	11	9.2	8.8		11	14	10	15	20
Sodium	51	56	46	51	41	40		54	47	50	50	45
Potassium	9.4	11	8.8	7.2	6.7	6.8		3.0	16	13	12	10
pH (units)	7.3	7.8	8.2	7.6	7.2	7.6	7.1	7.9	7.4	7.3	7.8	
Bicarbonate	240		150	180	190	170	190	220	72	270	270	
Carbonate	0	0	0	0	0	0	0	0	0	0	0	
												100
Sulfate	25	30	25	34	26	18				7.7	21	20
Chloride	52	80	100	92	57	58	40	62	42	48	51	50
Fluoride	1.4	.7	1.7	2.9	1.2	1.0			77			
Alkalinity	200		140	160	160	150	160	190	68	220	230	
Total Hardness	140		150	130	120	120		140	150	130	160	180
Noncarbonate Hardness	0		25	0	0	0		0	91	0	0	0
Dissolved SolidsResidue	355	351	390	329	282	270					314	278
Dissolved SolidsCalculated Sum	330		360	360	300	290						
Biochemical Oxygen Demand (5 Day)	7.1	30	20	51	62	30		7.8	8.0	38	8.4	45
Specific Conductance (umhos at 25°C)	650	728	718	702	560	570		698	649	541	700	
Inorganic Carbon	65	40	22	42	30	35		47	47	50	59	53
Total Organic Carbon	8	43	14	56	47	42		47	28	17	101	39
Total Carbon	73	83	36	98	77	77		94	75	67	160	92
AmmoniaNH <sub>4</sub> as N	20	22	21	20	17	15	11	22	22	23	25	21
Nitrogen, Total Organic as N	14	10	4.4	5.0	2.2	4.9	2.7	3.0	1.8	5.8	,85	2.9
NitrateNO <sub>3</sub> as N	.00	.00	2.6	.00	.04	.01	1.2	.00	.03	.09	.07	.06
NitriteNO <sub>2</sub> as N	.06	.06	.33	.06	.04	.55	.17	.01	.01	.06	.03	.11
Phosphorus, Ortho as P	8.2	10	9.0	8.8	6.5	5.9	6.1	9.5	9.4	10	11	9.2
Phosphorus, Total as P	9.9	12	10	9.0	7.4	5.9	6.4	9.5	9.6	10	12	9.2

Table 4.--Analyses of sewage effluent.--Continued

Date of Collection	09-20-72	10-31-72	12-28-72	03-05-73	04-24-73	06-28-73	08-30-73	11-07-73	12-20-73	02-13-74	04-30-74	06-27-74
*ArsenicDissolved	1										0	
*CadmiumDissolved	0				0							
*ChromiumDissolved Hexavalent	0				0	0					0	
*ChromiumDissolved	10		10	30								
*CobaltDissolved	0											
*CopperDissolved	30				40							
*IronDissolved	70				130						80	
*LeadDissolved	6				6						15	
*LithiumDissolved	10											
*ManganeseDissolved	20				50						25	
*StrontiumDissolved	300	400	320	140	40	300						
*ZincDissolved	70				40						20	
*AluminumTotal Recoverable	350		80	0		260						90
*ArsenicTotal Recoverable	20		10	6	5	7			1		0	10
*CadmiumTotal Recoverable	1		1	1		2			9		1	1
*ChromiumTotal Recoverable	30					60						0
*CobaltTotal Recoverable	0		1	1		0						10
*CopperTotal Recoverable	30		40	60		40			5			28
*IronTotal Recoverable	390		310	470		630			630			620
*LeadTotal Recoverable	18	**	12	17		19			6			18
*LithiumTotal Recoverable	10		0	10		0						8
*ManganeseTotal Recoverable	30		40	40		50			67			38
*MercuryTotal Recoverable	.5			.5		.6					. 2	.0
*NickelTotal Recoverable	4		8	1		6						13
*ZincTotal Recoverable	120		120	220		130			120			30

<sup>\*</sup>Micrograms per litre

## Quality of Native Ground Water

The natural quality of the ground water in the upper part of the Floridan aquifer immediately beneath the Thomas P. Smith Wastewater Renovation Plant was assumed to be the same as that at well 14 (fig. 4), approximately 4,000 ft (1,200 m) southwest of the area most heavily sprayed. (No samples were collected prior to spraying.) The quality of the ground water at well 14 (tables 5 and 6) remained practically constant throughout the investigation and was unaffected by the activities at the Plant, as evidenced by the chloride concentrations at well 14 being as low or lower than those observed at wells upgradient of the plant (wells A, B, C. D, E, Basic Data A). Natural chloride and nitrate-N concentrations are 2 and 0.05 mg/l, respectively. Table 6 compares averages of concentrations of major ions and nutrients present in the effluent and in ground water at wells 1, 5, 9, and 14.

## CHANGES IN QUALITY OF EFFLUENT

The quality of effluent is modified in several ways as it percolates downward, mixes with the underlying ground water, and eventually leaves the area. Major among these are: nitrification—denitrification, vegetative response, phosphate fixation, ion exchange, and dilution. Although dilution is normally the last of these mechansims to occur, it will be discussed first because of its importance in evaluating the others.

## Dilution

The effluent contains higher concentrations of major chemical constituents than the native ground water. Consequently, when effluent and native ground water are mixed, the concentrations of major chemical constituents of the resultant ground water fall between those of the effluent and the native ground water.

From the principles of dilution we know that for a well-mixed fluid:  $C_e N_e + C_b N_b = (100) N_w$ , where  $C_e$  and  $C_b$  represent the percent by volume of effluent and natural ground water ("background"), respectively, which combined to form the ground water at any given well. For cases of simple dilution of conservative parameters such as chloride (those not lost through adsorption, fixation, denitrification, precipitation, plant uptake, or ion exchange)  $N_e$ ,  $N_b$ , and  $N_w$  represent the concentration, in milligrams per litre, of the particular chemical constituent present in the effluent, "background" water, and the ground water at well w, respectively.

To solve for the percent of effluent-percolate in water from a given well, such as well 1, substitute the average chloride concentrations (table 6) present in the effluent and in water from well 14

Table 5.--Analyses of ground water at well 14. (Concentrations in milligrams per litre)

Date of Collection	12-20-72	02-01-73	03-05-73	04-12-73	05-30-73	07-25-73
Color (units)				10	5	0
Turbidity (JTU)		U2.				
Silica	6.5	6.3		6.3	5.0	7.0
Calcium			28	16	25	
Magnesium			1.2	•9	1.0	
Sodium			1.6	1.7	1.4	
Potassium			.6	.6	.6	
pH (units)				7.4	7.7	
Bicarbonate				56	70	56
Carbonate				0	0	0
Sulfate	5.8			3.2	13	
Chloride	2.0	site- glild		3.0	2.1	2.0
Fluoride	.2			.2	.2	
Alkalinity				46	57	46
Total Hardness			75	1414	67	
Noncarbonate Hardness			200	0	9	
Dissolved SolidsResidue		100	122	68	89	
Dissolved SolidsCalculated Sum		av-Lime		60	83	
Biochemical Oxygen Demand (5 Day)	of the mo	in the to	net prom	rac Sitz w	•5	.6
Specific Conductance (umhos at 25°C)	164	My HIC	010 4 4	108	145	129
Inorganic Carbon	Dog 200	dr.wol	et kants	7.5	19	8.0
Total Organic Carbon		0 77 10 sb	a officeral	8.0	5.0	2.0
AmmoniaNH4 as N	.04	.03	To Jakan	.04	.04	.01
Nitrogen, Total Organic as N	.60	.38	49 au 16	.65	.06	•37
NitrateNO3 as N	.00	.03		.07	.06	.00
NitriteNO <sub>2</sub> as N	.01	.01		.00	.00	.00
Phosphorus, Ortho as P		10.227		B 640 m)		.00
Phosphorus, Total as P	.00	.00		.00	.01	.00

Table 5.--Analyses of ground water at well 14.--Continued

08-29-73	10-04-73	11-06-73	01-24-74	02-13-74	03-13-74	04-25-74	05-15-74	06-27-74
				<del>-11</del> - 12			- 5" e.e.	7
7.0	6.7							
	20	16	14	13	14	14	12	13
	.8	1.1	.9	.7	•7	.7	.7	.7
	1.5	1.3	1.6	1.3	1.3	1.2	1.2	•9
	•5	.7	•3	.4	.4	.6	. 4	•3
	7.8			7.9	7.4	7.0	1	
	74			38	74	42		
	0			0	0	0		
							.4	2.1
	2.5	2.2	1.3	1.4	1.6	2.3	1.6	1.7
			9					.0
	61			31	61	34		ĕ
	53	44	39	35	<b>3</b> 8	38	33	35
	0			4	0	4	1	
	in the same					8	36	48
					1	18 18		
	2.2			1.4	1.3	.6		.4
	131	87	87	76	86	81	100	130
		11	- 20	8.0	10	7.0	9.0	16
		()		.0	.0	2.0	.0	
.00	.03			.03	.02	.03	.06	.05
.54	.63		'	.71	.06	•32	.17	.01
.00	.09			.04	.07	.06	.04	.09
.00	•00			.00	.00	.00	.03	.03
.00	.00	No. 10 510	1 22	.00	.00	.00	Ten Liel	
.00	.00			.00	.00	.00	.02	.05

			Average	concen	trations,	milligrams	s per litr	e*		
Identifier	Ca	Mg	Na	<u>K</u>	<u>C1</u>	NH <sub>4</sub> -N	NO <sub>2</sub> -N	NO3-N	Org-N	Total P
Effluent	38	12	48	9.4	62	20	0.12	0.34	4.8	9.2
Well 1	62	7.3	35	2.9	55	.04	.01	18	.24	.02
Well 5	31	5.6	7.3	.5	16	.02	.01	3.8	.18	.03
Well 9		2-1	36	1.8	56	. 34	.07	19	.22	.01
Well 14**	17	.8	1.4	.5	2.0	.03	. 01	.05	.38	.01

<sup>\*</sup> Average of all analyses for the 2-year study period (July 1972 to June 1974).

<sup>\*\*</sup> Chemical concentrations at well 14 are assumed to represent natural "background" conditions.

(taken as representing the "background" concentration), and from well 1 into the previous equation as follows:

$$C_e$$
 (62) +  $C_b$  (2) = (100)(55). And since  $C_e$  +  $C_b$  = 100 percent,  $C_e$  (62) + (100-  $C_e$ ) (2) = (100)(55). Rearranging and combining forms, 60  $C_e$  = 5,300. Thus,

 $C_e = 88 \text{ percent.}$  Rounding off,  $C_e = 90 \text{ percent.}$ 

Based on chloride concentrations, then, the water sampled at well 1 was approximately a mixture of 90 percent effluent and 10 percent natural water.

However, the calcium concentration at well 1, instead of falling between the concentration of that of the effluent and that at well 14, is actually 63 percent higher than that of undiluted effluent. Furthermore, the concentrations of sodium, potassium, and magnesium present at well 1 are all much lower than the predicted concentrations based on 10-percent dilution. This observation can be explained in part by cation exchange.

# Ion Exchange

The quality of the ground water at well 1 in the upper Floridan aquifer immediately beneath the 4-Big-Gun Area, as compared with the quality of the effluent and of the ground water at well 14 (table 6), indicates some important changes are occurring as the effluent percolates through the soil and rock.

The physical and chemical activity that takes place on the surface of particles within the soil column is largely determined by particle size. Ion exchange refers to the replacement of adsorbed ions (electrically charged atoms or groups of atoms) by ions in solution. Although nearly all soil and rock minerals have some ion-exchange capacity (Hem, 1970), the ion-exchange property of a soil is due almost entirely to its clay and silt fractions and organic matter (Winklander, 1964). Goldstein and Moberg (1973) attributed the increasing ion-exchange capacity of a soil with increasing clay content to the increasing surface area with decreasing particle size.

Cation exchange refers to the replacement of adsorbed cations (positively charged ions) by cations in solution. Inasmuch as the effluent contains relatively high amounts of sodium, potassium, and magnesium, and calcareous sand and clay are abundant above the limestone (see section on Basic Data, lithologic logs), the potential for cation exchange exists at the plant.

If cation exchange is occurring, the dilution equation still is valid for conservative parameters. However, for the major cations:  $\rm N_{\rm e}$ ,  $\rm N_{\rm b}$ , and  $\rm N_{\rm w}$  become the sum of calcium + magnesium + sodium + potassium, expressed in milliequivalents per litre, in the effluent, "background" water, and the ground water at well w, respectively. Converting the concentrations listed in table 6 for wells 1 and 14 and the effluent to milliequivalents per litre, substituting into the equation and solving it yields  $\rm C_{\rm e} = 100$  percent effluent at well 1. Apparently, then cation exchange is occurring with calcium adsorbed to the clay being replaced by sodium, potassium, and magnesium. The difference between  $\rm C_{\rm e}$  values obtained by the two methods (calculated on the basis of chlorides versus sums of milliequivalents of cations) is relatively small and could be explained by the fact that we are using average concentrations rather than single specific concentrations.

The correctness of this interpretation (cation exchange) is corroborated by similar calculations based on average concentrations at well 5. Based on the summation method, water at well 5 has averaged 21 percent effluent. Based on the conservative-parameter method and substituting for chloride, water at well 5 has averaged 23 percent effluent. The difference is negligible.

# Nitrification-Denitrification

Nitrification-denitrification is the process by which effluent-nitrogen is converted to different forms and in some cases removed from the ground-water system (Broadbent, 1973). The nitrogen cycle is an extremely complex biochemical phenomenon which is in a constant state of interaction. It is not the intent of this report to review the nitrogen cycle in detail, but to give in general terms a brief discussion of how it is involved in changing the chemical character of the effluent. The effluent nitrogen exists primarily as ammonia-nitrogen (NH4-N) and organic nitrogen when the effluent is sprayed (table 4). The "ammonia-nitrogen" determination as performed by the U.S. Geological Survey is a method which determines the total concentration of gaseous ammonia (NH3) and the ammonium ion (NH4) present in an aqueous solution and does not differentiate between the two species (Brown and others, 1970).

Nitrification and denitrification occurred in each of the sprayed areas, as indicated by the nitrifying and denitrifying bacteria numbers being much higher in the sprayed than the unsprayed areas. This is consistent with the results of a similar study (Cherry, and others, 1973) in St. Petersburg, Florida. Table 7 shows nitrifying and denitrifying bacteria concentrations determined on soil samples collected in July 1973 from selected sites in and near the area sprayed. The locations of these sites are indicated on figure 2. The most probable number per gram (MPN/g) of dry soil of Nitrosomonas (nitrifying bacteria which convert NH<sub>4</sub>-N to nitrite-nitrogen (NO<sub>2</sub>-N)), was from 23 to 580 times

Table 7.--Numbers of nitrifying and denitrifying bacteria in soil samples collected in the project area.

		Nitrif	iers	Denitrifiers
Site	Date collected	Nitrosomonas	Nitrobacter	All types
B-1	07-19-73	990	240	990
B-2	07-19-73	980	240	240
B-2		980	240	- , -
B-3	07-19-73	96	990	2,600
	te instruction of T			
B-4	07-19-73	46,000	2,600,000	2,600,000
B-5	07-19-73	100,000	480,000	1,700,000
B-6	07-19-73	580,000	13,000,000	3,100,000
B-7	07-19-73	270,000	2,700,000	2,600,000
B-8	07-19-73	260,000	260,000	460,000
B-9	07-19-73	23,000	2,600,000	12,000,000

higher in the area sprayed than outside the area. The Nitrobacter (nitrifying bacteria which convert  $\mathrm{NO}_2\mathrm{-N}$  to nitrate-nitrogen ( $\mathrm{NO}_3\mathrm{-N}$ )) concentration was from 1,000 to 54,000 times greater in the area sprayed than outside the area. Concentration of denitrifiers (bacteria which convert  $\mathrm{NO}_3\mathrm{-N}$  to gaseous nitrogen ( $\mathrm{N}_2$ ) which is then lost to the soil atmosphere (Broadbent, 1973)) was from 460 to 12,000 times greater in the area sprayed than outside the area. This indicates the bacteria are feeding on the nutrients in the effluent.

Almost complete nitrification occurred in the effluent-percolate in both the Irrigation-Fields Area and the 4-Big-Gun Area, but appreciable nitrogen uptake by plants occurred only in the Irrigation-Fields Area. The NH<sub>4</sub>-N, which is the most significant form of inorganic nitrogen in the effluent, was almost completely converted to NO<sub>3</sub>-N, as evidenced by analyses of ground water from well 1 (table 6). Table 8 compares the January to June 1974 chloride and total nitrogen (organic-N + NH4-N + NO2-N + NO3-N) data for ground water at well 5, downgradient of the 4-Big-Gun Area, and well 21, downgradient of the irrigation fields. From the chloride data, the ground water at both wells is estimated to consist of 25 percent effluent-percolate. If no nitrogen had been removed, the ground water at both wells should contain 0.25 times 25 mg/l (the average concentration of total nitrogen in the effluent, and obtained by summing and averaging the individual nitrogen species shown on table 4) or 6.2 mg/l. However, the average total nitrogen concentrations at wells 5 and 21 are 5.0 and 3.3 mg/1, respectively. Based on average data approximately 12 mg/1 ((6.2-3.3)/0.25) of the total nitrogen was removed from the effluent which was applied in the Irrigation-Fields Area and reached well 21 as effluent-percolate, and approximately 5 mg/l of the total nitrogen was removed from the effluent which was applied in the 4-Big-Gun Area and eventually reached well 5. However, the three-dimensional distribution of sampling points is insufficient to show the percentage of nitrogen being removed throughout the effluent plume. Of prime importance in the continuation study is definition of the plume with depth.

#### Phosphate-Fixation

The low phosphorus concentrations in the ground water at well 1 and well 9 (table 6) indicate that almost all the phosphorus is removed. Since Overman and Smith (1973) have already established that the soil at the Smith Plant has a large capacity to fix phosphorus, it appears likely that the phosphorus removal is due to fixation and adsorption before the effluent reaches the Floridan aquifer. The soil at the plant is Lakeland sand (U.S. Dept. of Agri., written commun., 1975). Hortenstine (1966) summarized the conclusions of other researchers in Florida by saying the phosphate-fixing capacity of the Lakeland sand increases progressively with increases in phosphorus concentration as a result of continuous breakdown of the clay materials and subsequent

Table 8.--Comparison of chloride and total nitrogen concentrations in ground water at wells 5 and 21.

<u>Date</u>	C1	ll 5 Total N * s per litre)	C1	1 21 Total N s per litre)
01-22-74	17	4.8	17	3.2
02-12-74	16	3.0	16	1.3
03-14-74	16	6.2	16	4.1
04-03-74	19	5.3	16	3.9
05-09-74	18	5.1	f nitrageo, ph	0 (38Y)
06-20-74	<u>17</u>	5.8	16	4.2
Average	17	5.0	16	3.3

<sup>\*</sup> Total N = Organic N +  $NH_4$ -N +  $NO_2$ -N +  $NO_3$ -N

release predominantly of aluminum and also iron. Similar studies by Hook and others (1973) and Kardos and Sopper (1973) in Pennsylvania have determined that more than 90 percent of the phosphorus was removed after water percolated through only 6 in (150 mm) of soil.

## Vegetative Response

Vegetative response to the applied sewage effluent at the plant has been reported in detail (Overman and Smith, 1973). For composite samples of pearl millet irrigated at 2, 4, 6, and 8 in per week during April 5, 1972 to September 27, 1972, the nutrients removed in the vegetation, expressed as percent of those in effluent were: nitrogen, 75 to 30; phosphorus, 93 to 24; potassium, 280 to 110; calcium, 18 to 7; magnesium, 73 to 27; and sodium, 1.1 to 0.9. Overman and Smith (1973, p. 43-44) suggested the effluent is "slightly deficient in potassium, since more potassium was harvested at all rates than was applied."

Some lateral movement of the effluent-percolate to the west of the general downgradient (southwesterly) flow from the 4-Big-Gun Area has taken place in the vicinity of well 9 (fig. 2). The average concentrations of nitrogen, phosphorus, chloride, and sodium at well 9 (25 ft (7.6 m) west of the area heavily sprayed) are nearly the same as those at well 1 (table 4). Since Overman and Smith (1973) have reported up to 75 percent "nitrogen recovery" by harvested pearl millet grown in the irrigation fields, it appears that the high-nitrogen ground water at well 9 had its origins in the heavily sprayed forest area.

#### Other Important Effluent Changes

BOD concentrations were reduced before the effluent mixed with the underlying ground water. The median 5-day BOD values at well 1 and well 9 were 0.3~mg/1 and 1.0~mg/1, respectively. The median 5-day BOD value for the effluent was 30~mg/1. This is consistent with the results of the Flushing Meadows Project (Bouwer, 1973).

No fecal coliform organisms were detected in the ground water at any of the test wells used in this investigation. As previously mentioned, the concentration of fecal coliform organisms in the effluent ranged from 4,000 to 80,000 colonies per 100 ml. Evidently the fecal coliform organisms were removed or died before the effluent reached the underlying ground water.

Pesticide analyses were performed on one water sample collected from the effluent and from the ground water at wells 4, 5, 6, 17, 18, 19, and 22. No aldrin, DDD, DDE, DDT, dieldrin, endrin, heptachlor, lindane, chlordane, PCB, PCN, 2,4-D, 2,4,5-T, or silvex were present in any of the samples.

#### CHANGES IN QUALITY OF GROUND WATER

# Variations of Ground-Water Quality with Time and Areal Extent of Effluent Plume

Variations in chemical quality of ground water with time have been most noticeable at well 5. Well 5 (fig. 4) is approximately 1,800 ft (550 m) downgradient of the 4-Big-Gun Area. It took the effluent plume approximately 3/4 year (from the beginning of spray-effluent application upgradient of well 5 in March 1972 until January 1973) to reach well 5. This indicates that the effluent-percolate is migrating at an apparent rate of about 2,400 ft (730 m) per year within the aquifer. By January 1974, the chloride concentration at well 5 had increased to 17 mg/l. During the next 6 months, the chloride concentrations remained relatively constant, still averaging 17 mg/l. This indicates that perhaps the effluent percolate has reached equilibrium with the underlying ground water reaching well 5 and has stabilized at a mixing ratio of ground water to effluent-percolate of about 1 to 4 (based on chlorides). Although variations have occurred, nitrate-nitrogen concentrations have been increasing at well 5, as shown by figure 11. This trend is sharpest during the first 6 months after the effluent arrived at well 5 and appear to be diminishing with time.

The downgradient edge of the effluent plume at shallow depths progressed from between well 10 and well 5 in December 1972 (fig. 12) to between well 5 and well 14 in June 1974 (fig. 13). The outlines of the effluent plumes are based on chloride concentrations in ground water in November and December 1972, and June 1974, as determined from water samples from wells 27 to 270 ft (8.2 to 82 m) deep. It should be noted that differences in well depths and the limited number of control points make it impossible to define the exact limits of the plume. Chloride concentrations greater than 6 mg/l were assumed to indicate the presence of effluent in ground water. As of June 1974, the effluent plume extended between 1,800 and 4,000 ft (550 and 1,200 m) downgradient from the plant site and encompassed an area of about 140 acres  $(570,000 \text{ m}^2)$ . The NO<sub>2</sub>-N concentrations in ground water throughout the 140-acre area exceeded the Florida DPC limit (advanced wastewater treatment) of 3 mg/1; however, only the ground waters in areas beneath or downgradient of the heavily sprayed areas exceeded the 10 mg/l limit for drinking water (U.S. Public Health Service, 1962) (fig. 14).

The applied effluent has not resulted in higher concentrations of trace metals in the ground-water sampling network downgradient of the plant. This is due, at least in part, to the low trace-element concentrations in the effluent. Table 9 compares U.S. Public Health Service (1962) recommended upper limits for trace elements in drinking water with data for samples collected from the effluent and ground water at representative wells and springs (see figs. 3 and 4). The chloride concentrations in wells 5 and 17 indicate they are affected by the

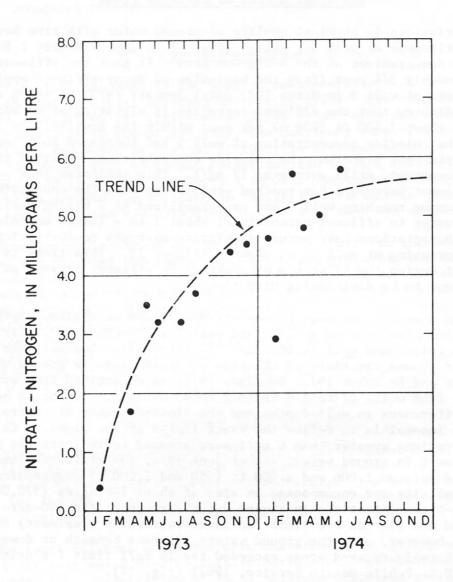


FIGURE 11. -- Variation of nitrate-nitrogen with time at well 5.

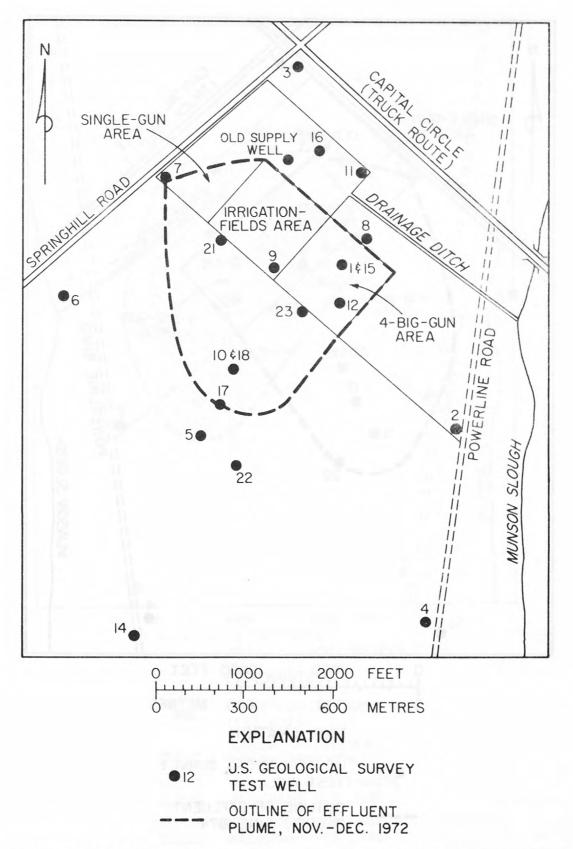


FIGURE 12.--Outline of the effluent plume based on chloride concentrations in ground water in November and December 1972.

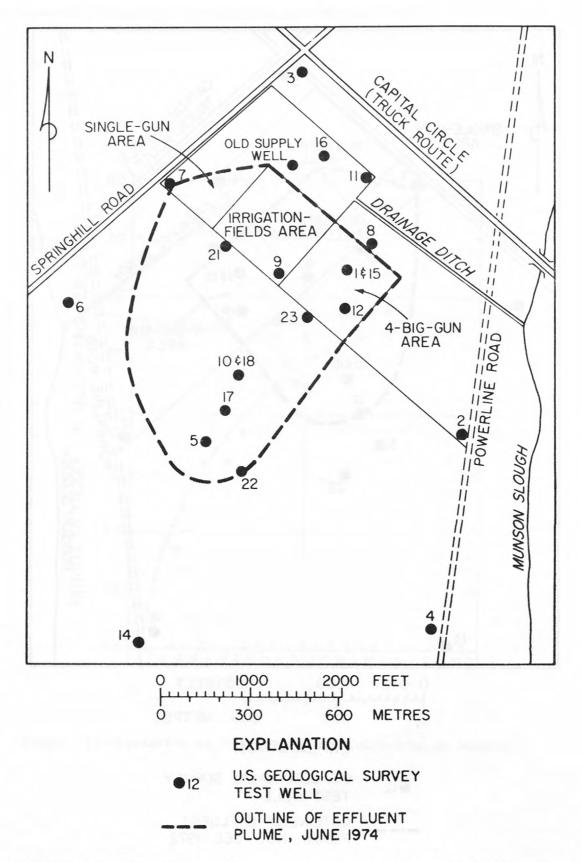


FIGURE 13.--Outline of the effluent plume based on chloride concentrations in ground water in June 1974.

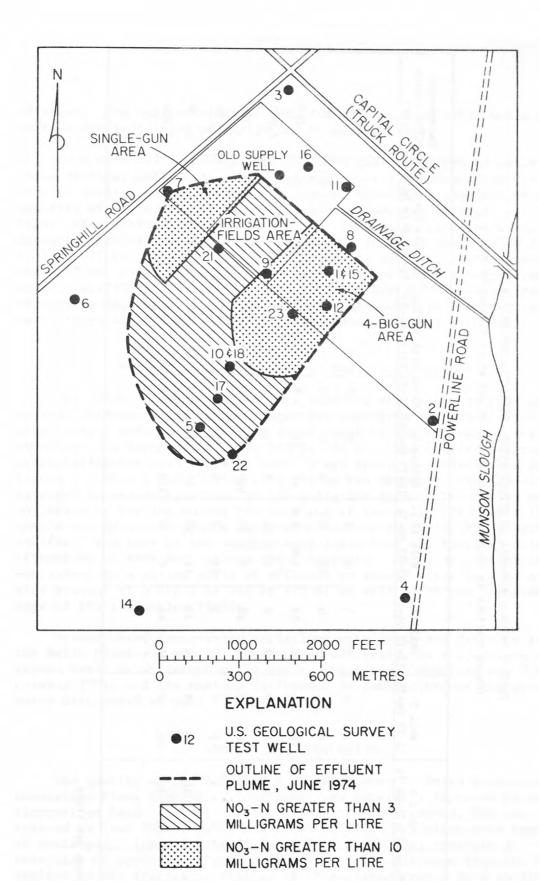


FIGURE 14. -- June 1974 nitrate-nitrogen concentrations.

Table 9.--Trace elements in ground water, effluent, and spring water.

(Concentrations in micrograms per litre, except where flagged by \*\*)

Element Date	Well 12-73			1 5	1	1 17 04-74	Well 12-73		Eff1 12-73	uent 04-74	River 01-73			Wakul 01-73	la Spr 12-73		Recommended USPHS Limits 1962
Arsenic	0	3	1	0	0	3	4	2	1	0	10	6	2	0	0	3	50
Cadmium	9	0	8	5	9	0	10	0	9	1	0		0	0		0	10
Chloride**	1.8	2.3	16	19	24	23	1.8	2.6	42	51	8.0	7.5	7.8	8.0	4.0	6.5	250
Copper	1		1	2	1	0	1	1	5		0	1	2	0	1	1	1,000
Iron	0	10	0	10	0	40	0	0	630	80	80	0	40	0	0	60	300
Lead	4	5	5	9	8	1	28	7	6	15	0	8	8	1	25	11	50
Manganese	0	10	0	20	14	10	14	50	67	25	10	14	10	0	14	10	50
Zinc	0	20	40	0	50	40	0	0	120	20	40	0	40	10	20	5	5,000

<sup>\*\*</sup>Milligrams per litre

effluent. The quality-of-water data for samples from other wells and springs are indicative of background chemical levels.

From March 1972 through June 1974 the quality of ground water at three springs, major points of discharge generally downgradient from the area of applied sewage effluent, remained unchanged regardless of the quantity of effluent applied at the plant or whether rainfall was low or high. (See tables 10-12 and figure 3.) This is as expected since the distances from the plant to the discharge points are large (8 to 13 mi or 13 to 21 km) and since the recharge to the aquifer, represented by the effluent magnitude, is a small percentage of the discharge from the aquifer system by the springs. The combined average discharge from the springs in southern Leon and northern Wakulla Counties has been estimated to exceed 400 Mgal/d (17.5 m $^3$ /s) (Hendry and Sproul, 1966).

### Variations of Ground-Water Quality with Depth

The chloride and nitrate—N data indicate that although some of the sprayed effluent tends to move laterally away from the application area upon contact with the underlying water, most of the effluent moves downward to a depth of at least 270 ft (82 m). The quality-of-ground—water variations with depth at well 23 are shown in table 13 and in figure 7. Well 23 is located (fig. 4) on the property line immediately adjacent to and downgradient of the 4-Big-Gun Area. The samples were collected by bailing during the drilling of the well. The 42-ft (13-m) sample was collected from a sandy-clay aquifer overlying the limestone aquifer. The rest of the samples were taken from the Floridan aquifer (limestone or sand body within the limestone). Chloride concentrations equivalent to a mixing ratio of effluent to ground water of 1 to 4 were also present at a depth of 248 ft (75 m) in well 21 at the downgradient edge of the irrigation fields.

Ground-water temperature variations with depth and distance from the Smith Plant are small. The maximum difference in temperature of the ground water at different wells during the monthly sampling was  $2.0^{\circ}$  Celsius (°C), and the maximum difference in temperature of the ground water with depth at well 23 was  $2.0^{\circ}$ C.

#### SUMMARY AND CONCLUSIONS

The quality of the effluent from the Thomas P. Smith Wastewater Renovation Plant (secondary treatment) was generally improved by spray disposal on land. Almost all the phosphorus was removed, BOD was reduced to less than 5 mg/l, and fecal coliform organisms were removed or destroyed. Average data (July 1972 to June 1974) indicate a reduction of approximately 12 mg/l of the total nitrogen from the effluent applied to the irrigation fields, which received from 2 to 8 in (50 to

Table 10. -- Analyses of water at Kini Spring. (Concentrations are in milligrams per litre except for constituents indicated by \* which are in micrograms per litre.)

Date of Collection	03-21-72	01-21-73	04-25-73	06-27-73	09-27-73	12-21-73	04-04-74	06-26-74
Color (units)		60	60	100				
Turbidity (JTU)		2	1	1	2			
Silica	6.0	7.2	5.8	7.0	6.6			
Calcium	25	23	26	25	27	24	26	30
Magnesium	5.8	5.1	5.6	5.3	6.3	5.6	5.5	7.2
Sodium	5.3	4.4	4.9	4.9	5.0	5.0	5.3	5.0
Potassium	.6	. 4	. 4	.5	. 4	. 7	.7	. 4
pH (units)	7.0	7.4	7.7	7.5	7.2	8.0	7.7	
Bicarbonate	90	83	88	83		93	91	
Carbonate	0	0	0	0		0	0	
Sulfate	12	12	13	12				13
Chloride	9.0	8.0	7.9	7.5	9.0	7.9	8.1	8.5
Fluoride	.2	.2	. 2	.3				. 2
Alkalinity	74	68	72	68		76	75	
Total Hardness	87	79	88	84		83	88	100
Noncarbonate Hardness	13	11	16	16		7		
Dissolved SolidsResidue	122	115	127	126				
Dissolved SolidsCalculated Sum	110	100	110	100				
Biochemical Oxygen Demand (5 Day)		. 4	.3	1.2		.6	.7	. 4
Specific Conductance (umhos at 25°C)	200	180	190	189	205	205	189	350
Inorganic Carbon	1	14	14	13	20	20	19	20
Total Organic Carbon		6	6	8	4	10	7	3
Total Carbon		20	20	21	24	30	26	23
AmmoniaNH as N		.03	.02	.02	.05	.02	.03	.04
Nitrogen, Total Organic as N	-	.24	.29	.39	.48	.15	.35	.07
NitrateNO as N	.03	.01	.08	.05	.06	.00	.03	.10
NitriteNO <sub>2</sub> as N	.00	.00	.00	.01	.00	.00	.00	.01
Phosphorus, Ortho as P		.02	.02	.02	.01	.02	.01	.02
Phosphorus, Total as P	:	.02	.02	.02	.06	.02	.02	.02

Table 10.--Analyses of water at Kini Spring.--Continued

*ArsenicDissolved	 			 7	0	0
*CadmiumDissolved	 0	0		 		
*ChromiumDissolved Hexavalent	 0	0	0	 		0
*ChromiumDissolved	 			 		
*CobaltDissolved	 			 		
*CopperDissolved	 0	0		 1	1	1
*IronDissolved	 80	50		 50	90	50
*LeadDissolved	 0	1		 5	7	0
*LithiumDissolved	 			 		
*ManganeseDissolved	 0	10		 14	20	0
*StrontiumDissolved	 180	130	100	 	12	
*ZincDissolved	 10	40		 8	9	3
*AluminumTotal Recoverable	 		220	 		
*ArsenicTotal Recoverable	 10	8	14	 		
*CadmiumTotal Recoverable	 		1	 	0	0
*ChromiumTotal Recoverable	 		10	 		
*CobaltTotal Recoverable	 		0	 		
*CopperTotal Recoverable	 		20	 		
*IronTotal Recoverable	 		160	 		
*LeadTotal Recoverable	 		4	 		
*LithiumTotal Recoverable	 		0	 		
*ManganeseTotal Recoverable	 		10	 		
*MercuryTotal Recoverable	 .0	.0	. 2	 	.0	
*NickelTotal Recoverable	 		4	 		
*ZincTotal Recoverable	 		8	 		

Table 11.--Analyses of water at Wakulla Spring.

(Concentrations are in milligrams per litre except for constituents indicated by \* which are in micrograms per litre.)

Date of Collection	03-21-72	01-21-73	04-25-73	06-27-73	09-27-73	12-21-73	04-04-74	06-26-7
Color (units)		10	20	10				
Turbidity (JTU)	1	1	4	1	2			
Silica	10	11	8.2	10	10			
Calcium	39	38	38	37	38	32	37	39
Magnesium	8.7	8.6	7.6	8.4	8.8	12	8.9	9.6
Sodium	3.7	4.1	4.3	4.2	3.9	4.0	4.5	3.7
Potassium	.3	.6	.6	1.2	.5	.6	.8	.5
pH (units)	7.3	7.9	8.0	7.8	7.6	8.1	8.0	
Bicarbonate	150	150	140	150		143	143	
Carbonate	0	0	0	0		0	0	
Sulfate	17	11	10	9.2				9.9
Chloride	3.4	8.0	7.0	6.0	5.0	4.0	6.5	5.9
Fluoride	.3	.3	.3	.3				.3
Alkalinity	130	120	120	120		117	117	
Total Hardness	130	130	130	130		130	130	140
Noncarbonate Hardness	8	9	15	9		12		
Dissolved SolidsResidue	162	154	161	159				178
Dissolved SolidsCalculated Sum	160	160	140	150				
Biochemical Oxygen Demand (5 Day)	. 4	. 2	. 2	1.8		.5	. 2	.5
Specific Conductance (umhos at 25°C)	279	260	255	265	270	270	269	340
Inorganic Carbon		22	14	19	31	27	30	29
Total Organic Carbon		5	6	9	1	10	0	3
Total Carbon		27	20	28	32	37	30	32
AmmoniaNH <sub>4</sub> as N	.00	.02	.01	.01	.00	.02	.04	.03
Nitrogen, Total Organic as N	.14	.40	.55	.26	.46	.38	.06	.00
NitrateNO as N	.25	.23	.10	.25	.22	.21	.23	.31
NitriteNO <sub>2</sub> as N	.00	.00	.00	.01	.00	.00	.00	.01
Phosphorus, Ortho as P	.03	.04	.05	.06	.05	.04	.05	.06
Phosphorus, Total as P	.04	.05	.07	.08	.06	.05	.05	.07

Table 11. -- Analyses of water at Wakulla Spring. -- Continued

*ArsenicDissolved				19	 		
*CadmiumDissolved	0	0	0		 		
*ChromiumDissolved Hexavalent	0	0	0	0	 		0
*ChromiumDissolved					 		
*CobaltDissolved					 		
*CopperDissolved		0	0		 1	1	0
*IronDissolved	30	0	30		 0	60	0
*LeadDissolved	2	1	1		 25	11	0
*LithiumDissolved					 		
*ManganeseDissolved	10	0	10		 14	10	0
*StrontiumDissolved	110	150	120	100	 		
*ZincDissolved		10	20		 20	5	0
*AluminumTotal Recoverable				110	 		
*ArsenicTotal Recoverable	0	0	6	5	 		
*CadmiumTotal Recoverable				2	 	0	10
*ChromiumTotal Recoverable				2	 		
*CobaltTotal Recoverable	0			0	 		
*CopperTotal Recoverable				2	 		
*IronTotal Recoverable				5	 		
*LeadTotal Recoverable				3	 		
*LithiumTotal Recoverable				0	 		
*ManganeseTotal Recoverable				0	 		
*MercuryTotal Recoverable	.0	.0	.0	.0	 .0	.0	
*NickelTotal Recoverable				5	 		
*ZincTotal Recoverable	20			10	 		

Table 12.--Analyses of water at River Sink Spring.

(Concentrations are in milligrams per litre except for constituents indicated by \* which are in micrograms per litre.)

F	Date of Collection	03-21-72	01-21-73	04-25-73	06-27-73	09-27-73	12-21-73	04-04-74	06-26-74
	Color (units)	50	60	30	100				
	Turbidity (JTU)		2	1	2	2			
	Silica	6.1	7.2	5.7	7.0	6.7			
1	Calcium	25	23	26	25	27	23	25	30
	Magnesium	5.8	5.1	5.6	5.3	6.3	8.0	5.5	7.2
	Sodium	4.8	4.5	4.6	4.7	5.0	5.0	5.3	5.1
	Potassium	.6	.4	.4	.5	.4	.7	.9	.4
		7.4	7.4	7.5	7.5	7.2	7.6	7.7	
2.1	pH (units)	90		88	90	7.2	94	89	300
	Bicarbonate		83					0	
1	Carbonate	0	0	0	0		0	0	
-	Sulfate	12	12	11	12				13
1	Chloride	8.0	8.0	7.7	6.0	9.0	7.9	7.8	8.7
	Fluoride	.2	.2	.3	.3				. 2
1	Alkalinity	74	68	72	74		77	73	
	Total Hardness	87	79	88	84		90	85	100
	Noncarbonate Hardness	13	11	16	10		12		
	Dissolved SolidsResidue	121	129	123	132				146
	Dissolved SolidsCalculated Sum	110	100	100	110				
	Biochemical Oxygen Demand (5 Day)		.5	.3	.7		.9		.8
	Specific Conductance (umhos at 25°C)	195	180	189	188	205	207	193	350
1	Inorganic Carbon	0	14	15	13	20	20	18	20
	Total Organic Carbon		6	4	10	2	8	7	5
1	Total Carbon		20	19	23	22	28	25	25
	AmmoniaNH <sub>4</sub> as N		.03	.02	.01	.02	.02	.01	.02
	Nitrogen, Total Organic as N		.52	.28	.24	.32	.19	.81	.03
	NitrateNO as N	.04	.03	.06	.07	.05	.03	.04	.10
	NitriteNO3 as N	.00	.00	.00	.01	.01	.00	.00	.01
1	Phosphorus, <sup>2</sup> Ortho as P		.01	.01	.02	.01	.02	.02	.02
	Phosphorus, Total as P		.01	.02	.04	.24	.03	.02	.05

Table 12.--Analyses of water at River Sink Spring.--Continued.

	1						
*ArsenicDissolved					 		
*CadmiumDissolved		0	0		 		
*ChromiumDissolved Hexavalent		0	0	0	 		0
*ChromiumDissolved			***		 		
*CobaltDissolved					 		
*CopperDissolved		0	0		 1	2	1
*IronDissolved		80	60		 0	100	190
*LeadDissolved		0	1		 8	8	0
*LithiumDissolved					 		
*ManganeseDissolved		10	10		 14	10	17
*StrontiumDissolved	d	60	140	100	 		
*ZincDissolved		40	0		 0	40	0
*AluminumTotal Recoverable				220	 		
*ArsenicTotal Recoverable		10	8	13	 		
*CadmiumTotal Recoverable				1	 	0	1
*ChromiumTotal Recoverable				0	 		
*CobaltTotal Recoverable				0	 		
*CopperTotal Recoverable				10	 		
*IronTotal Recoverable				140	 		
*LeadTotal Recoverable			wood full's	2	 		
*LithiumTotal Recoverable				0	 		
*ManganeseTotal Recoverable				10	 		
*MercuryTotal Recoverable		.0	.0	.1	 	.1	.1
*NickelTotal Recoverable				4	 		
*ZincTotal Recoverable				10	 		

4

Table 13.--Chemical analyses of water from well 23.

Date	(ft) Depth	(°Celsius) Temp.	(umhos) Specific Conductance	Chloride	(milli <u>NH,-N</u>	grams per	litro NO
06-13-74	42		235	38	0.01	0.01	20
06-14-74	65		150	14	.03	.01	4
06-14-74	75		148	16	.06	.01	3
06-14-74	85		210	26	.02	.01	9
06-14-74	95	23.5	188	22	.01	.01	8
06-14-74	105	22.0	210	27	.04	.01	10
06-14-74	115	22.0	215	27	.02	.01	11
06-14-74	125	22.0	220	30	.02	.01	12
06-14-74	135	22.5	230	30	.06	.01	12
06-14-74	137		210	22	.12	.10	-
06-15-74	150	22.5	390	51	.08	.09	20
06-16-74	165	22.0	360	39	.06	.01	19
06-17-74	185	21.5	420	46	.84	.43	32
06-17-74	187	22.0					-
06-17-74	190	22.5	370	44	.12	.07	20
06-17-74	200	22.0	420	46	.49	.49	21
06-18-74	215	22.0	350	44	2.6	1.7	18
06-18-74	220	22.0	400	45	.61	.74	2'
06-19-74	240		450	47	.14	.11	24
06-20-74	250		470	48	.16	.06	2(
06-20-74	260		470	48	.26	.11	26
06-20-74	270		470	50	.24	.27	2!

200 mm) of effluent per week. On the same basis there was a reduction of approximately 5 mg/l of the total nitrogen from the effluent that had been applied at a rate of 14 in (350 mm) per week to the undisturbed forest (the 4-Big-Gun Area). This effluent-percolate appeared at shallow depths 1,800 ft downgradient. However, it was not possible to define the vertical extent of the effluent plume. Nitrification occurred in both the irrigation fields and the undisturbed forest. The NH $_4$ -N, the most abundant form of nitrogen in the effluent, was almost completely converted to NO $_3$ -N before dilution by the underlying ground water.

As a result of the application of about 140 ft (43 m) of effluent to the forest area from March 1972 to June 1974, chloride and nitrogen concentrations equivalent to undiluted effluent were found from depths of 150 to 270 ft (46 to 82 m) in ground water at the downgradient edge of this heavily-sprayed area. Chloride concentrations indicated a mixing ratio of effluent to native ground water of 1 to 4 at a depth of 248 ft (75 m) at the downgradient edge of the irrigation fields.

Effluent-percolate has moved from the plant at about 2,400 ft (730 m) per year. It took the effluent 3/4 year to reach a well about 1,800 ft (550 m) downgradient of the heavily-sprayed area, and less than 2 years for the effluent to mix with ground water in a ratio of 1 to 4 at the same well. No changes in the quality of the ground water were observed beyond that point. As of June 1974, the water in about 140 acres  $(570,000 \text{ m}^2)$  of the underlying aquifer had increased NO3-N and chloride concentrations.

Of prime importance in the continuation of this study of the environmental effects of spray irrigation of effluent on the ground-water system is the establishment of a comprehensive well network to define the three-dimensional extent of the plume.

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BASIC DATA

A. Chemical data for water-supply wells.

Well (see fig. 3)	Date	Chloride* (C1)	pH (units)	Specific conductance (umhos/cm)	Bicarbonate* (HCO <sub>3</sub> )
A**	09-19-72	7.5	7.4	335	200
B**	09-18-72	5.0	7.6	220	130
C**	09-18-72	4.0	7.6	240	150
D**	09-23-72	3.5	8.1	160	80
E**	09-23-72	5.0	7.8	255	150
F	09-23-72	4.5	8.0	240	140
G	09-21-72	5.0	8.2	130	70
H	03-15-73	4.0	8.3	160	104
I	03-15-73	5.0	8.0	140	80
J	03-15-73	5.0	8.0	140	87
K	03-15-73	5.0	8.0	175	60
L	03-15-73	5.0	8.0	170	96
M	03-15-72	2.0	7.8	115	70
N	09-20-72	3.0	8.2	158	88

<sup>\*</sup> Milligrams per litre.

<sup>\*\*</sup> Wells upgradient from area of applied sewage effluent.

B. Lithologic logs and natural gamma logs of selected wells.

Depth (ft)	Description
1 3 18 28 30 34 38 43 52	SAND, brown, fine to medium SAND, light brown, fine to medium SAND, light brown, fine to medium SAND, yellow-brown, fine CLAY, SANDY, orange-brown, fine; with black organic matter CLAY, SANDY, orange-brown, fine; with fragments of limestone do LIMESTONE, white, fine do
	Well 2
6 12 19 25 30 36 44 49 50 51	SAND, light brown, medium to coarse SAND, light brown, fine to coarse do do SAND, very light brown, fine to medium do SAND, brown, fine to coarse do SAND, almost black, fine to medium LIMESTONE, white, fine SAND, black, fine to medium
	Well 3
0 3 8	SAND, light brown, fine to medium SAND, light brown, slightly orange, fine to medium do
24 30	SAND, orange-brown, fine to coarse
39	SAND, light brown, fine to coarse SAND, white, fine to coarse
50	do
56	do
70	SAND, light brown, fine to medium
76	do
82	SAND, CLAYEY, orange-brown, fine to coarse
85	do
90	SAND, calcareous, brown, fine to coarse, with white limestone particles
91	SAND, calcareous, white-brown, fine to coarse
95	SAND, calcareous, white-brown, fine to coarse
100	CLAY, SANDY, calcareous, white-brown, fine to coarse

Depth (ft)	Description
4 20 30	SAND, light brown, fine to medium SAND, tan, fine to medium CLAY, SANDY, brown, fine SAND, calcareous, light brown, fine
41 52	SAND, calcareous, light brown, fine to coarse
	Well 5
3 9 12 15	SAND, gray-pink, fine to medium SAND, light orange, fine to medium SAND, pale orange, fine to medium do
22 27 30 33 42	SAND, dark yellow-orange, fine to medium CLAY, SANDY, calcareous, with reddish brown layers SAND, calcareous, dark yellow-orange, fine to medium SAND, gray, fine LIMESTONE, powdery
43	LIMESTONE, hard Well 7
0 10 20 22	SAND, light tan, fine, medium, and coarse SAND, light tan, very fine and fine to coarse do do
25 28 30 33	do CLAY, SANDY, very fine with limestone particles CLAY, SANDY, very fine and LIMESTONE, white, fine LIMESTONE, white, fine
	Well 8
9 14 15 25 29 33 35	SAND, light reddish-orange, very fine and fine to coarse SAND, light reddish-orange, very fine and fine to medium CLAY, SANDY, gray, very fine and fine to medium CLAY, SANDY, red, brown, and white streaks, very fine CLAY, SANDY, brown, very fine LIMESTONE
49	do do

Depth (ft)	Description				
10	SAND, white and tan, very fine and fine to coarse, with				
	fossils				
12	do				
15	SAND, brownish-orange, very fine to coarse				
19	CLAY, SANDY, brown, very fine to coarse				
20	SAND, light yellow, very fine to coarse				
22	SAND, CLAYEY, yellow-brown, very fine to fine				
27	CLAY, SANDY, purple, white, and orange-brown streaks				
32	SAND, CLAYEY, orange-brown, very fine, formed balls with				
32	white limestone particles				
35	LIMESTONE				
	Well 10				
1	SAND, brown-gray, very fine to coarse				
4	SAND, light brown, fine to medium				
5	SAND, light brown, fine to medium				
14	SAND, orange-brown, fine to medium				
24	SAND, light brown, fine to medium				
34	CLAY, SANDY, white, gray, and black; very fine				
42	LIMESTONE				
	Well 11				
2	SAND, dark brown, fine to medium				
7	do				
12	SAND, orange-brown, fine to medium				
17	SAND, mixed shades of brown, fine to coarse				
22	SAND, red-brown, fine to coarse				
27	SAND, CLAYEY, red-brown, fine to medium				
37	SAND, light brown, fine to medium, with fragments of				
37	limestone				
40	LIMESTONE				
40	LIMESTONE				
	Well 12				
2	SAND, brown, fine to medium				
12	SAND, light brown, fine to medium				
17	do				
22	do				
27	SAND, yellow-brown, fine				
32	SAND, calcareous, brown and red, with blue clay layers				
37	SAND, calcareous, dark brown, fine to medium				
42	SAND, calcareous, light brown, fine to medium				
47	CLAY, SANDY, with LIMESTONE streaks				

Depth (ft)	Description
2 7 12 17	SAND, calcareous, dark brown, fine to coarse SAND, light brown, fine to coarse SAND, calcareous, brown, fine to medium
20 22 27	SAND, calcareous, white, fine; and LIMESTONE SAND, orange-brown, fine to medium do
30 32	LIMESTONE, thin (2") layer SAND, calcareous, orange-brown, fine
	Well 14
2 6 12 14 17 22 25 27 32 37 42 52	SAND, dark brown, fine to medium SAND, light brown, fine to medium SAND, tan, fine to medium do do SAND, light brown, fine to medium CLAY, SANDY, red-brown layers SAND, calcareous, orange-brown, fine to medium do do do LIMESTONE, white
	Well 15
2 7 12 17 22 27	SAND, brown, fine to coarse SAND, orange-brown SAND, darker, partially calcareous do do SAND, more orange, partially calcareous
32 34	SAND, white, fine grain, calcareous LIMESTONE

Depth (ft)	Description					
2	SAND, light brown, fine to medium					
7	SAND, light brown, fine to coarse					
12	SAND, calcareous, grayish orange, fine to coarse					
17	do					
22	SAND, grayish-orange, fine to medium					
27	SAND, grayish-orange, line to medium SAND, yellow-orange, fine to coarse					
32						
37	SAND, pale orange, fine to medium					
42	SAND, salarrous gravish grange fire to saves					
47	SAND, calcareous, grayish-orange, fine to coarse					
52	SAND, calcareous, dark yellow-orange, fine to coarse					
	SAND, calcareous, grayish-orange, fine to coarse					
55	SAND, calcareous, dark yellow-orange, fine to medium					
57	SAND, calcareous, dark yellow-orange, fine to coarse;					
()	limestone specks; saturated zone (water)					
62	SAND, calcareous, dark yellow-orange, fine to medium					
67	LIMESTONE, SANDY					
	Well 17					
17	SAND, yellow-orange; with gravel-size limestone and carbon particles					
19	LIMESTONE, gray; with orange sand and carbon and fossil					
17	particles					
35	SAND, calcareous, gray and pale orange, fine to medium					
40	do					
50	LIMESTONE, gray, fine to coarse gravel					
55	do					
61	LIMESTONE, gray and pale orange; with sand and carbon					
01	and fossil particles					
65	do					
73	LIMESTONE, gray and tan, fine to gravel; with turquoise					
70	particles of fine gravel to coarse gravel sizes					
78	LIMESTONE, gray, fine to coarse gravel					
84	LIMESTONE, hard, white; with fossils and sand streaks					
90	LIMESTONE, gray, fine to coarse gravel					
96	LIMESTONE, hard, white; with fossils and sand streaks					
100	LIMESTONE, gray; with orange sand and carbon and fossil particles					
105	SAND, calcareous, gray and pale orange, fine to medium					
110	do					
115	LIMESTONE, gray, fine to coarse gravel					
120	SAND, calcareous, gray and pale orange, fine to medium					
125	SAND, calcareous, gray, fine to medium					
130	do					
135	do					
140	do					
145	LIMESTONE, hard, white; with fossils and sand streaks					

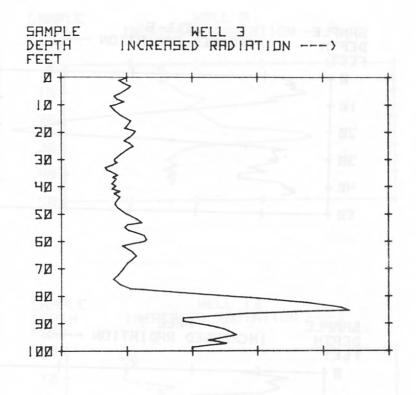
Depth (ft)	Description				
65	SAND, calcareous, orange-tan, very fine to coarse				
70	do				
75	do				
85	do				
90	do				
95	do				
100	SAND, calcareous, tan and white, very fine to medium				
105	do				
110	SAND, calcareous, tan, very fine to medium; with gravel-size limestone flakes				
115	SAND, calcareous, tan and white, very fine to medium				
120	SAND, tan, very fine to medium; with fine to medium quartz particles and limestone particles				
125	SAND, calcareous, pale orange, tan, very fine				
130	do				
135	SAND, calcareous, pale orange, fine to gravel				
140	do				
145	do				
150	do				
155	do				
160	do do				
	Well 19				
15	SAND, tan, fine				
20	SAND, tan to light brown, fine				
25	SAND, calcareous, tan to light brown, fine				
30	CLAY, SANDY, calcareous, light brown to tan, fine				
35	CLAY, SANDY, calcareous, light brown to tan, fine; with black (carbon) streaks				
45	LIMESTONE, gray, powdery; with fine tan sand				
55	do				
65	LIMESTONE, gray, fine gravel to coarse gravel				
70	do				
75	do -				
80	do d				

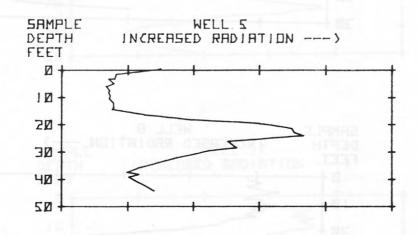
Depth	Description
(ft)	Depth
20	SAND, reddish-orange, fine to medium
25	do
30	do
35	do
40	LIMESTONE, gray; with red-orange sand streaks
45	do
50	do
55	do
60	LIMESTONE, gray
65	SAND, calcareous, tan, medium to coarse; with particles of limestone
70	do
75	SAND, calcareous, gray, fine to coarse
80	SAND, calcareous, gray, fine to medium gravel
85-250	do
(5-ft increme	nts)
	Well 22
20	CAND calconous mad areas fine to madium
20 30	SAND, calcareous, red-orange, fine to medium do
35	do
40	do
45	SAND, calcareous, red-orange, fine to medium; with fine
43	to medium gravel particles of white limerock with black streaks
75	LIMESTONE, gray-white, fine to coarse gravel
80	SAND, calcareous, tan, very fine to medium
95	SAND, calcareous, tan, fine to fine gravel
120	SAND, calcareous, tan, very fine to medium
165	do
175	do
185	do
200	LIMESTONE, tan and gray, fine to medium gravel
225	do
235	do
240	LIMESTONE, gray and white, fine to coarse gravel
245	SAND, calcareous, tan and gray, fine to coarse gravel; with black fossils
250	do
255	do
260	do

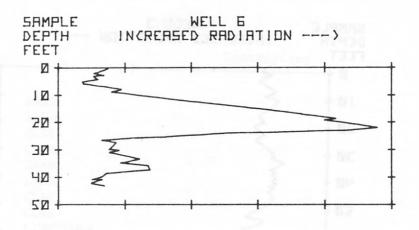
Well 23

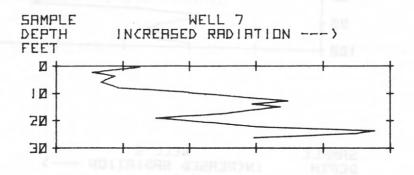
Depth (ft)			
0	SAND		
39	SAND		
40	CLAY		
50	CLAY		
51	SAND		
55	SAND		
56	CLAY		
59	CLAY		
60*	LIMESTONE		

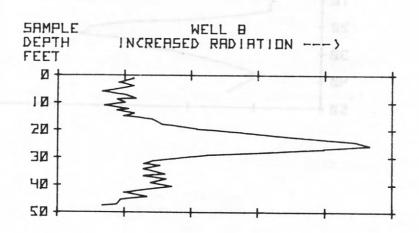
<sup>\*</sup>Drilling problems precluded certainty in knowing the origin of the well cuttings beyond a depth of 60 feet.

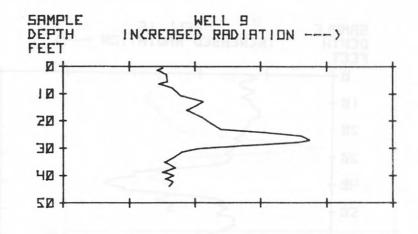


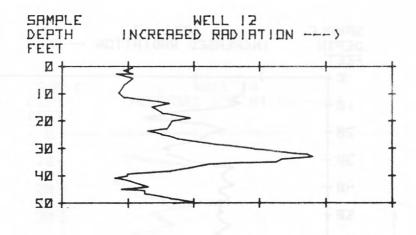


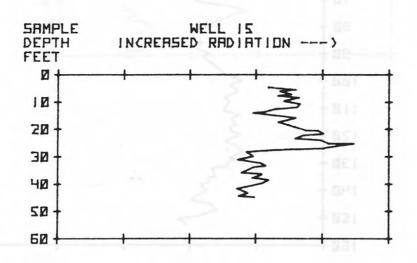


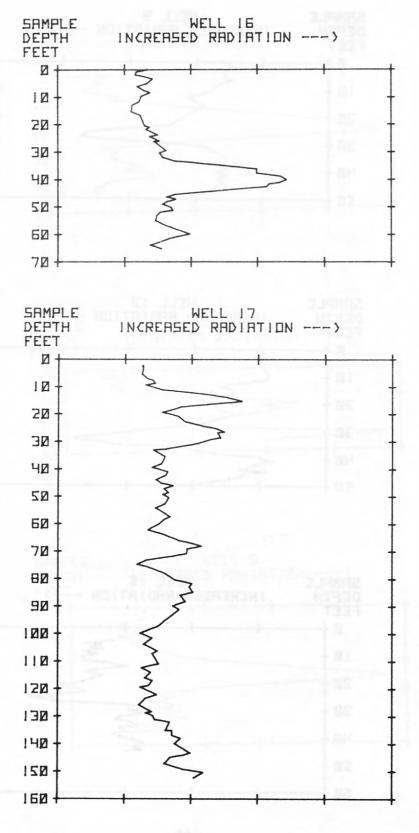


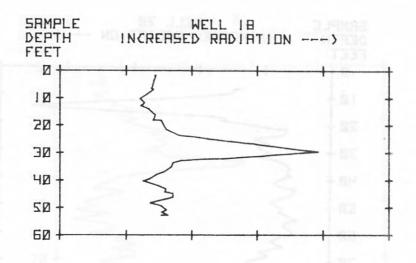


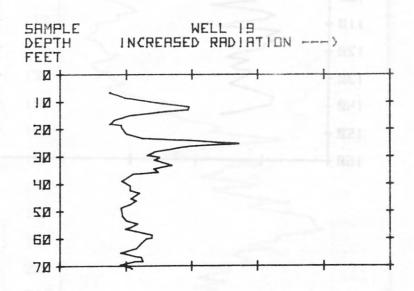


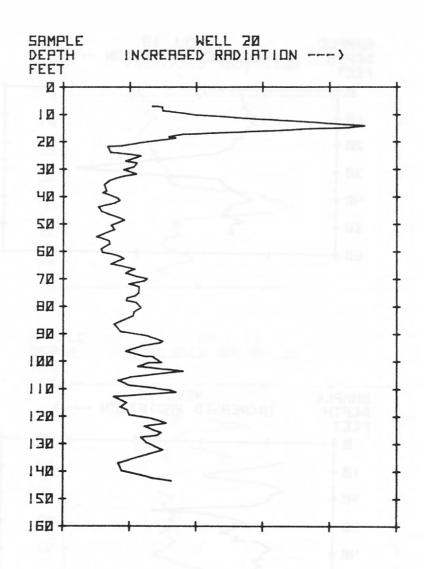


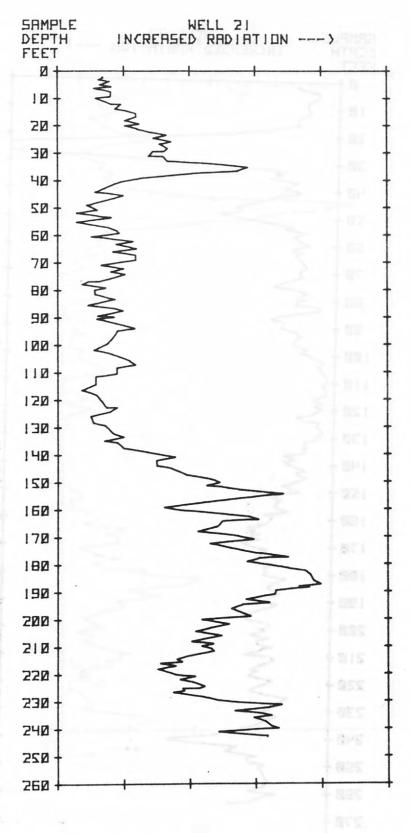


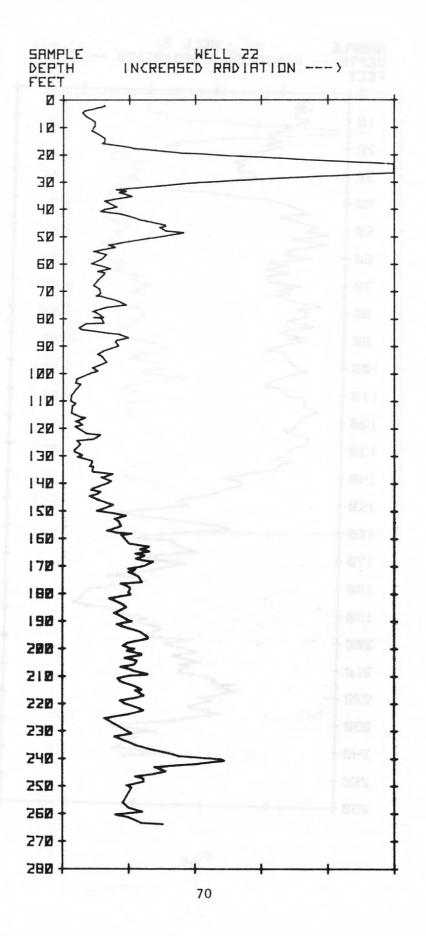


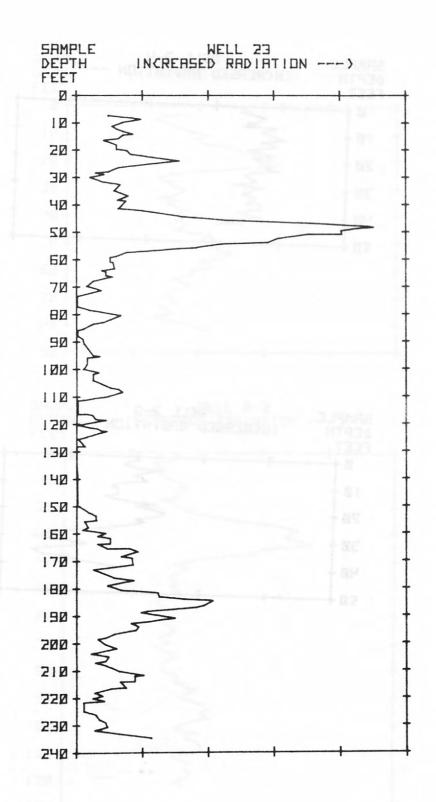


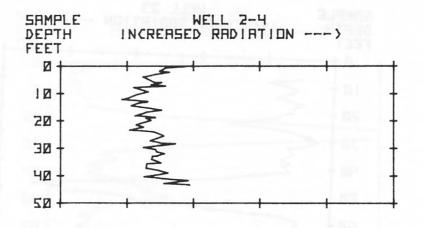


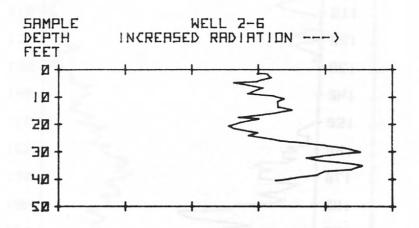


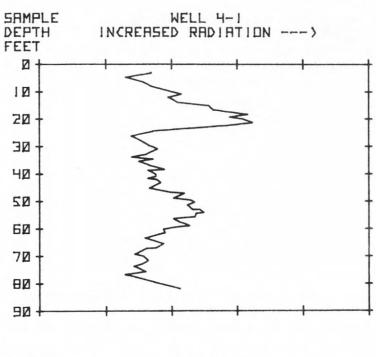


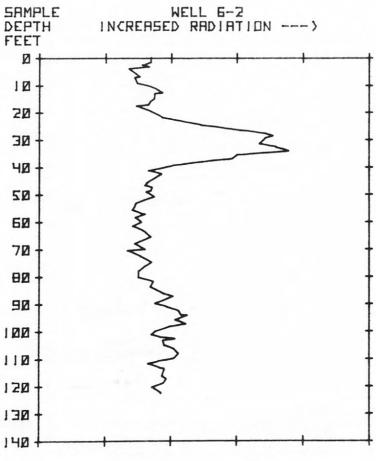












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