

A NUMERICAL MODEL TO EVALUATE PROPOSED  
GROUND-WATER ALLOCATIONS IN SOUTHWEST KANSAS

By

D. G. Jorgensen, H. F. Grubb, C. H. Baker, Jr.,  
G. E. Hilmes, and E. D. Jenkins

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SOUTHWEST KANSAS GROUNDWATER  
MANAGEMENT DISTRICT NO. 3



Lawrence, Kansas

1982

UNITED STATES DEPARTMENT OF THE INTERIOR

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GEOLOGICAL SURVEY

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## CONVERSION FACTORS

For those readers who would prefer to use the International System of Units (SI) rather than the inch-pound units given in this report, the following conversion factors are presented:

<u>Multiply inch-pound unit</u>	By	<u>To obtain SI unit</u>
inch	25.40	millimeter
foot	0.3048	meter
mile	1.609	kilometer
acre	0.4047	square hectometer
square mile	2.590	square kilometer
acre-foot	1,233	cubic meter
foot per second	30.48	centimeter per second
gallon per minute (gal/min)	0.06309	liter per second
gallon per day per square foot [(gal/d)/ft <sup>2</sup> ]	0.04074	meter per day

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ABSTRACT

A computer model was developed to calculate the drawdown, due to a proposed well, at all existing wells in the section of the proposed well and at all wells in the adjacent eight sections. The depletion expected in the 9-square-mile area due to all existing wells and the proposed well is computed and compared with allowable limits defined by the Southwest Kansas Groundwater Management District No. 3. An optional program permits the evaluation of allowable depletion for one or more townships. All options are designed to run interactively, thus allowing for immediate evaluation of proposed ground-water withdrawals.

INTRODUCTION

The Ogallala aquifer in the Ogallala Formation of late Tertiary age is the principal source of water for much of western Kansas. Irrigation from wells in Kansas was practiced on a limited scale until about 1960. During the 1960's, rapid development of ground water resulted in the significant lowering of the water table in the Ogallala. By 1970, the saturated thickness of the aquifer at some sites had been decreased to the extent that some wells would not yield water at an adequate rate for irrigation.

Concern by many residents in western Kansas was expressed through Kansas legislative action in 1972 when a bill was enacted that allowed the creation of ground-water management districts. Between 1972 and 1978, five ground-water management districts were formed. The Southwest Kansas Groundwater Management District No. 3 manages the ground water in the area shown in figure 1.

---

1 U.S. Geological Survey, Lawrence, Kansas.

2 U.S. Geological Survey, Austin, Texas.

3 Kansas State Board of Agriculture, Topeka, Kansas.

4 Southwest Kansas Groundwater Management District No. 3,  
Garden City, Kansas.

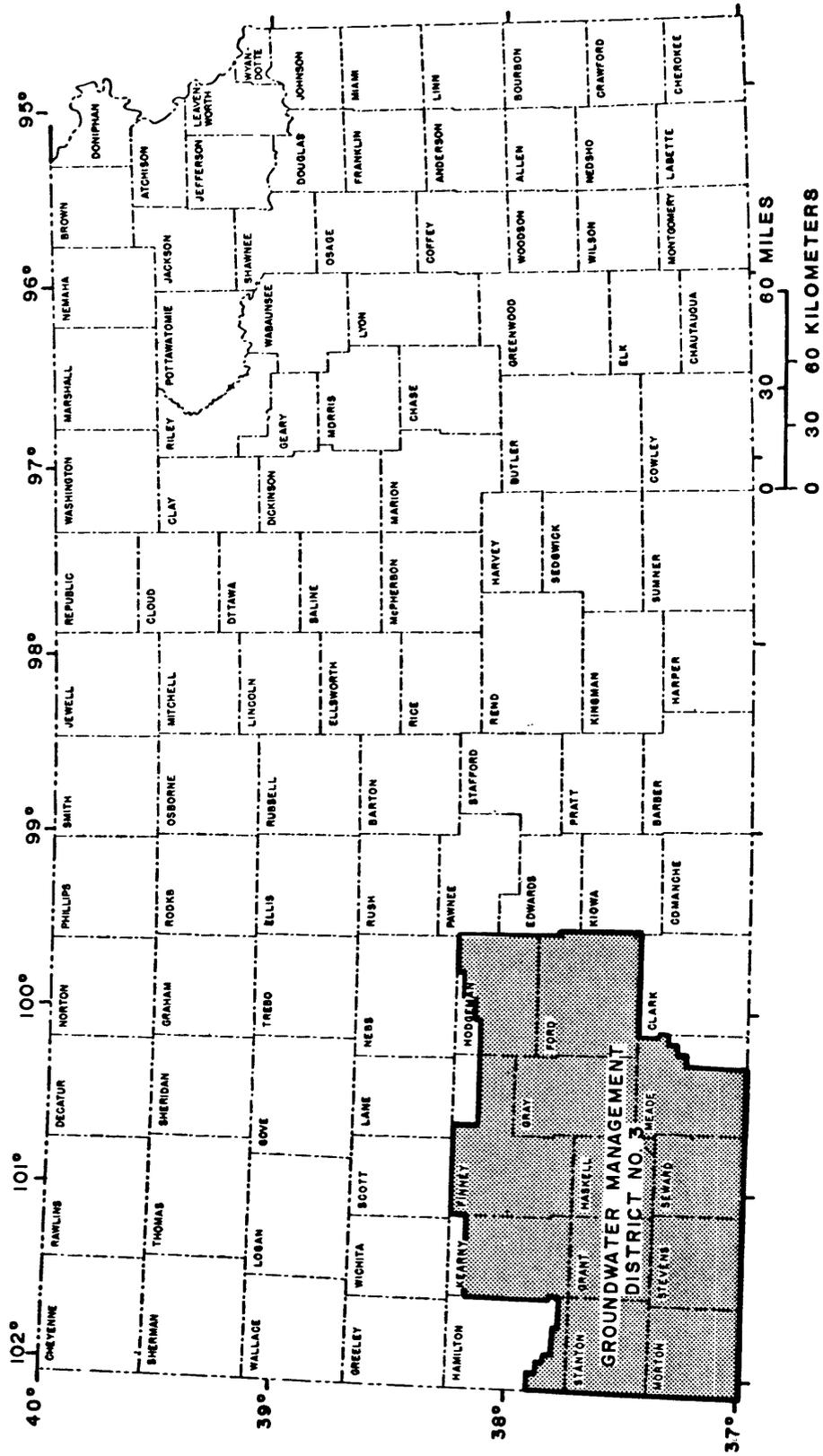


Figure 1.--Location of Southwest Kansas Groundwater Management District No. 3.

Regulations for the appropriation of ground water in Management District No. 3 are the joint responsibility of the management district and the Chief Engineer of the Division of Water Resources, Kansas State Board of Agriculture. Regulations written by the district are approved by the Chief Engineer, who then administers the appropriation of water rights. The Chief Engineer must decide if a proposed ground-water appropriation would interfere with a prior ground-water or surface-water appropriation. The Chief Engineer also must determine whether a new appropriation would cause a significant decrease in saturated thickness at existing well sites. Frequently, an appropriation may be proposed at a diversion rate greater than the aquifer can sustain.

The regulations adopted by the management district and approved by the Chief Engineer include:

- "1. Well spacing -- All wells included on an application for permit to appropriate water for beneficial use, except those for domestic uses and wells for which the maximum diversion rate is to be less than fifty (50) gallons per minute, shall meet the following criteria unless approval is authorized in accordance with exceptions defined in the management program.
  - (a) The minimum spacing of wells with a diversion rate of from fifty-one (51) to four hundred (400) gallons per minute and a maximum pump column diameter of six (6) inches shall be thirteen hundred (1,300) feet. The maximum diversion rate shall not exceed eight hundred (800) gallons per minute when combined with the authorized rate of any other wells within a standard legal one-quarter section of land containing approximately one hundred sixty (160) acres. Larger or smaller tracts shall be considered on an individual basis.
  - (b) The minimum spacing of wells with a diversion rate in excess of four hundred (400) gallons per minute or a pump column diameter in excess of six (6) inches shall be twenty-three hundred (2,300) feet.
  - (c) The location of a well or wells on an application for approval to change the point of diversion under an existing water right shall be no more than thirteen hundred twenty (1,320) feet from the authorized point of diversion and shall not decrease the existing spacing to other wells by more than three hundred (300) feet, unless the minimum spacing requirements for new applications are met or authorization is given in accordance with the exceptions defined by the management program. The three-hundred- (300-) foot limit shall not apply to proposed changes in points of diversion under an approved application for which the well has not been drilled.

2. Aquifer depletion--The approval of all applications for permit to appropriate water for beneficial use, except as noted below, shall be subject to the following criteria. The proposed appropriation, when added to the vested rights and prior rights, shall not cause more than forty- (40-) percent depletion in twenty-five (25) years of the saturated materials underlying the area included within a nominal nine- (9-) square-mile area about the location of the proposed well or wells. It shall be assumed, for the purpose of analysis, that all water rights are being fully exercised. The area of consideration shall include the legal section of land, referred to as one (1) square mile, nominally six hundred forty (640) acres, of the proposed well or wells and the eight (8) adjacent sections. If the boundary of the aquifer or of the district falls within the nine- (9-) square-mile area, the area of consideration shall include only the area overlying the aquifer or within the district.

Saturated thickness shall be determined from logs of test holes and wells within the area and from maps available from the U.S. Geological Survey and the Kansas Geological Survey and such refinements as have been accomplished by the district.

The coefficient of storage (specific yield) used in the analysis shall be assumed to be twenty (20) percent, unless hydrological information indicates a smaller value. A value of two (2) inches per year shall be assumed for recharge from natural sources and irrigation return flow."

In general, the procedure used by Groundwater Management District No. 3 for approving an allocation for a new water right is to determine if the new well meets all the existing criteria with respect to diversion rate, pump-column diameter, and minimum spacing to wells under vested or approved appropriation rights. The proposed well site also must meet the depletion criteria. To determine whether aquifer-depletion criteria are met, it is necessary to search the files for all ground-water rights (appropriations) in the 9-square-mile area (fig. 2). This procedure is lengthy and tedious and, therefore, subject to error. If the proposed well site and appropriation meet both the well-spacing and aquifer-depletion criteria, the question of impairment to existing water rights and to surface-water flow is evaluated by the Chief Engineer.

Groundwater Management District No. 3 has proposed that a management model be developed to determine if the proposed well site and appropriation meet the required spacing and depletion criteria and to calculate the drawdown at existing wells caused by pumping the proposed well. The model would use information from other programs now being used in relation to hydrology and water rights of the management district.

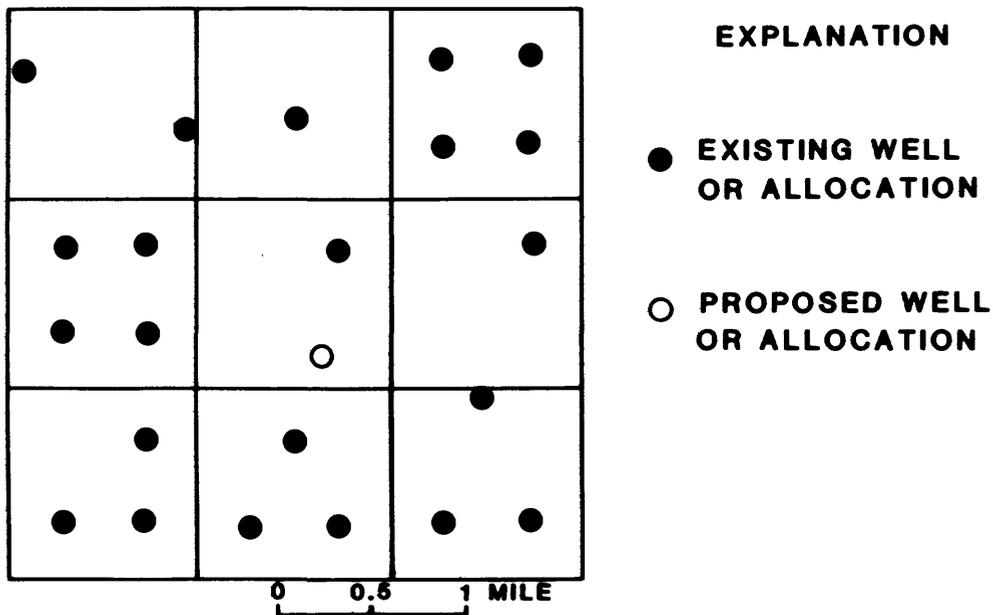


Figure 2.--Existing and proposed wells in 9-square-mile area.

The Southwest Kansas Groundwater Management District, in cooperation with the U.S. Geological Survey, periodically prepares and publishes maps showing the saturated thickness of the Ogallala aquifer in and near the boundaries of the district. These maps are used to determine the saturated thickness at a proposed well site. Values for saturated thickness, area, and specific yield are used to calculate the volume of water in storage. A value of 0.2 is assigned to specific yield, as designated in the regulations adopted by the management district.

The Division of Water Resources of the Kansas State Board of Agriculture is currently (1982) converting its existing "water-rights" file to a computer-managed data base. The U.S. Geological Survey is assisting the Division in this effort because the water-rights file contains valuable information on water use.

#### GROUND-WATER MODEL

The U.S. Geological Survey, in cooperation with Southwest Kansas Groundwater Management District No. 3, designed a ground-water model that will use the automated water-rights file on the computer to evaluate well-spacing and depletion requirements and to calculate the drawdown in all nearby wells.

## Numerical Method for Calculating Drawdown

The drawdown resulting from pumping a well is calculated by numerical techniques. The Ogallala aquifer is treated as an isotropic aquifer under unconfined (water-table) conditions. It is recognized that the aquifer is not truly isotropic and that drawdowns resulting from a very short pumping period may be more representative of confined (artesian) conditions. However, for pumping periods ranging from 2 weeks to 1 year or more, the aquifer will respond in the manner of an unconfined aquifer; thus, unconfined formulation is appropriate for the problem to be solved.

Radial flow to a fully penetrating well in an unconfined isotropic aquifer of infinite extent using the Dupuit-Forcheimer assumption is expressed mathematically as

$$\frac{1}{r} \frac{\partial}{\partial r} \left( \frac{rK}{2} \frac{\partial h^2}{\partial r} \right) = S' \frac{\partial h}{\partial t} , \quad (1)$$

where

$r$  = radial distance from pumping well, in units of length;

$K$  = hydraulic conductivity, in units of length per time;

$h$  = altitude of the water table above the base of the aquifer (saturated thickness), in units of length;

$S'$  = specific yield, dimensionless; and

$t$  = pumping time, in units of time.

The relationship between transmissivity ( $T$ ), hydraulic conductivity ( $K$ ), and saturated thickness ( $h$ ) is

$$T = Kh . \quad (2)$$

A schematic of a well pumping in an unconfined system is shown in figure 3.

Examining the differential within the parentheses of equation 1:

$$\frac{\partial h^2}{\partial r} = 2h \frac{\partial h}{\partial r} . \quad (3)$$

Substituting equation 3 into equation 1 and simplifying results in

$$\frac{1}{r} \frac{\partial}{\partial r} (rKh \frac{\partial h}{\partial r}) = S' \frac{\partial h}{\partial t} . \quad (4)$$

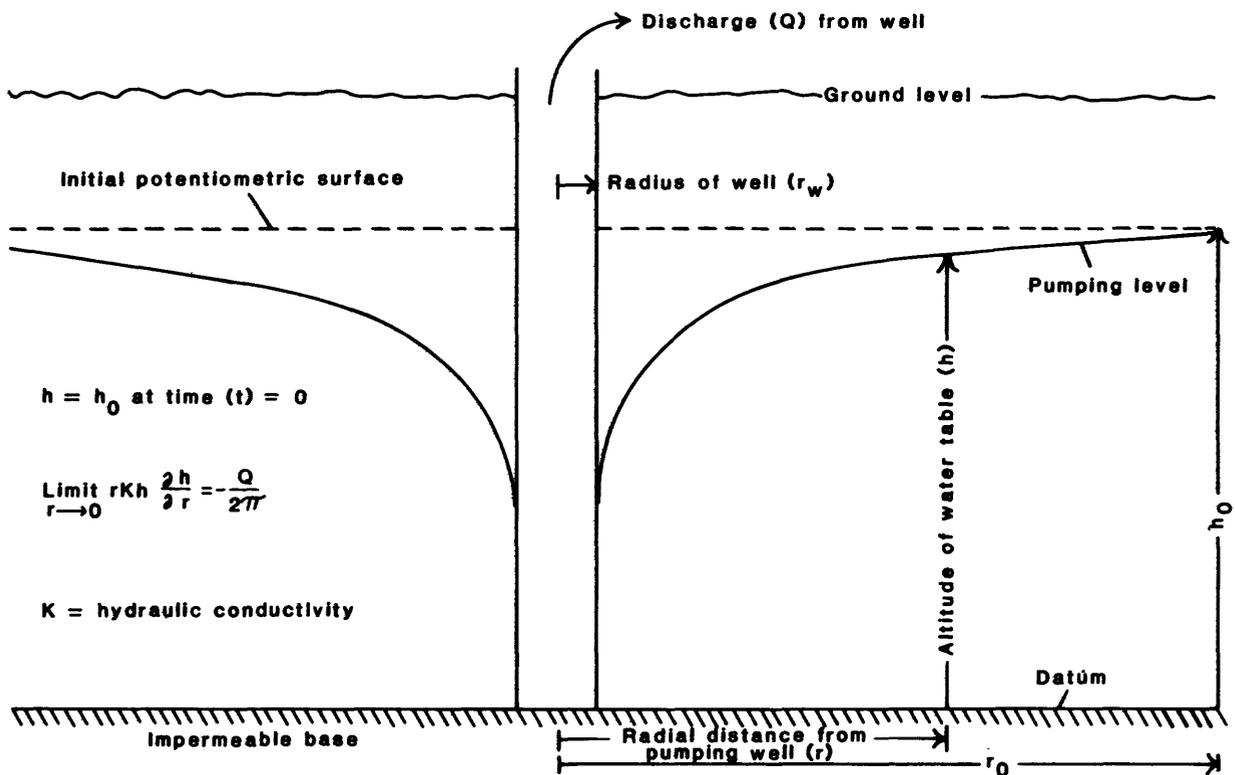


Figure 3.--Effects of pumping a well completed in an unconfined aquifer.

Equation 4 describes radial flow to a well in an unconfined aquifer. To obtain an equation for one-dimensional flow, equation 4 can be transformed using finite-difference approximations (see Supplemental Information, p. 30). The solution of the finite-difference equations was programmed into a numerical model. A flow diagram for the numerical-model program is shown in figure 4.

A comparison of the analytical solution, assuming a constant transmissivity, with the numerical solution to conditions where the drawdown is large (more than 5 percent of the saturated thickness) indicates the need for modeling the unconfined condition. However, the two solutions yielded nearly identical results when the drawdown was less than 5 percent of the original saturated thickness (fig. 5).

Additionally, the model was tested against the analytical steady-state solution of

$$h = \left( \frac{Q}{\pi K} \ln \frac{r}{r_w} + \frac{h^2}{n} \right)^{\frac{1}{2}}, \quad (5)$$

where

$Q$  is negative for a pumping well.

Equation 5 is similar to an equation derived by Jacob (1950, p. 79) or DeWeist

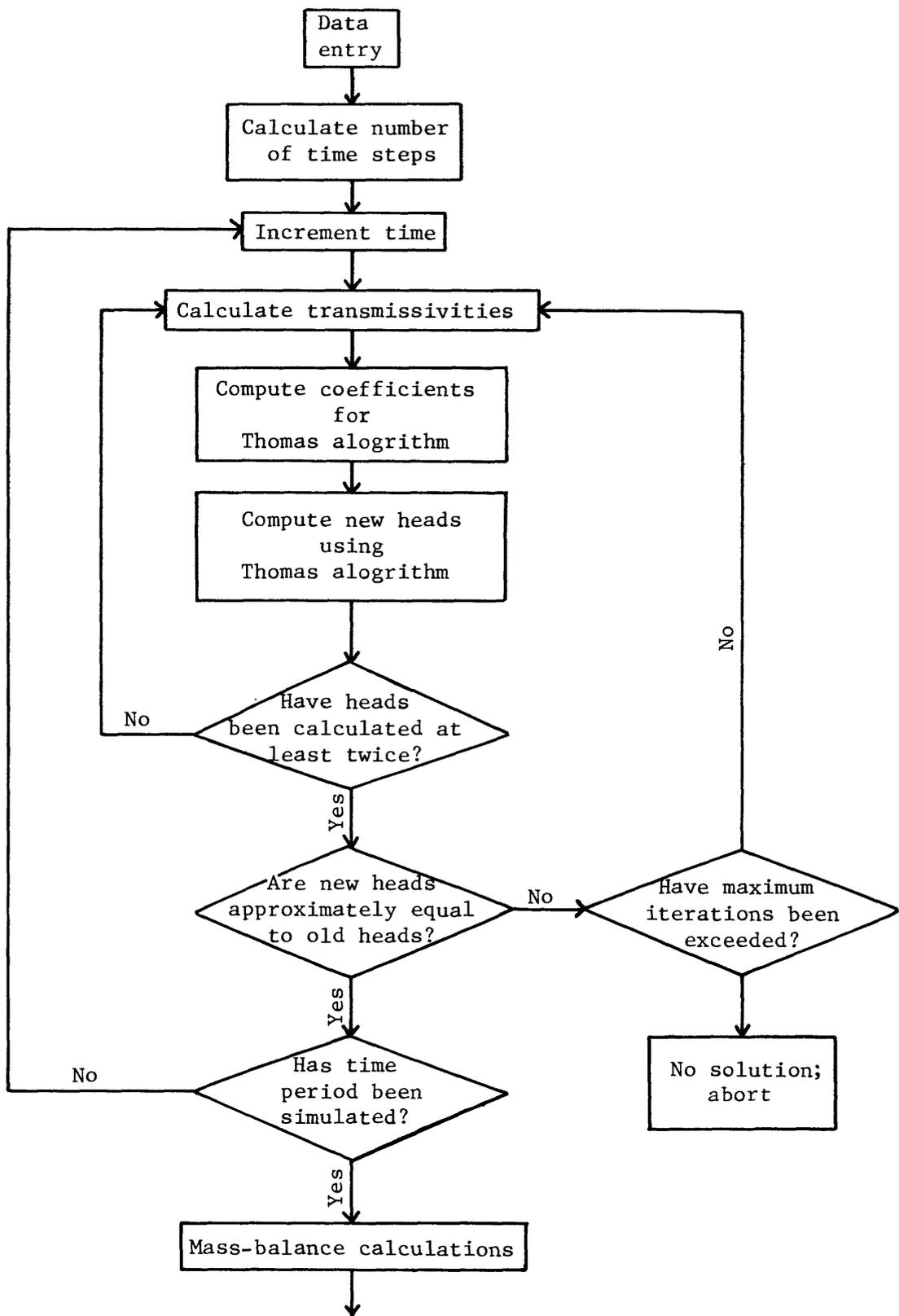


Figure 4.--Flow diagram for numerical-model program.

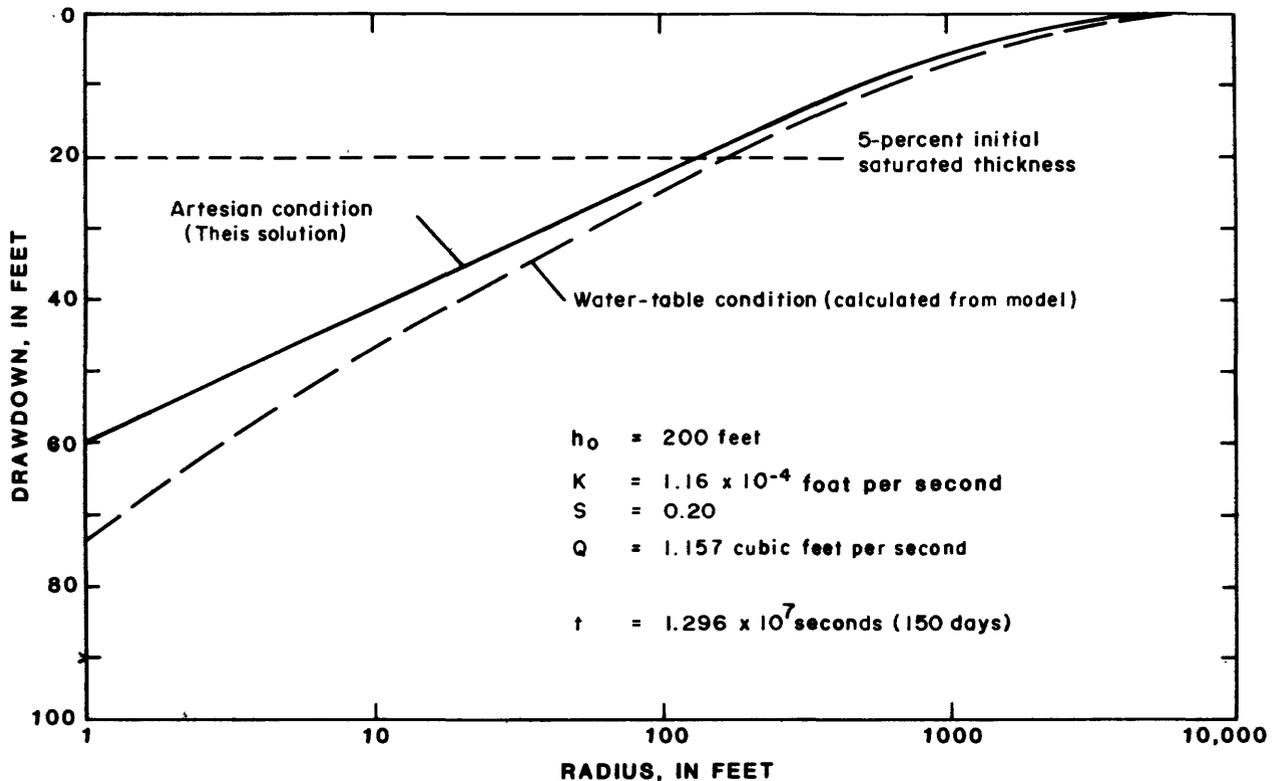


Figure 5.--Drawdown versus distance for transient conditions.

(1965, p. 243). (See figure 6.) The numerical technique was stable if a small  $\Delta t$  was used. An algorithm similar to that used by Trescott, Pinder, and Larson (1976, p. 85-86) was modified to calculate  $\Delta t$ . The modified algorithm calculates the correct  $\Delta t$  and number of time steps required to reach a designated pumping period. To decrease the number of time steps required, an acceleration factor (A) was used in the form  $\Delta t_{\text{new}} = A \Delta t_{\text{old}}$ . Generally, an acceleration factor of 1.5 was used. The model solutions were stable with an acceleration factor of 3; however, acceleration factors greater than 2 resulted in "hard starts." Hard-start oscillation occurred only for simulation of pumping of a few seconds. Actual problems required simulation of pumping durations greater than 1 week.

The program also contains a mass balance algorithm that calculates the water removed from the cone of depression and compares this to the product  $Q$  times the duration of pumping. The model does not solve the combined water-table-artesian problem, as was treated by Moench and Prickett (1972), or the delayed yield and vertical leakage, as treated by Ehlig and Halepaska (1976). The model calculates drawdown at any given radius; thus, knowing drawdown, impairment can be evaluated at the existing wells.

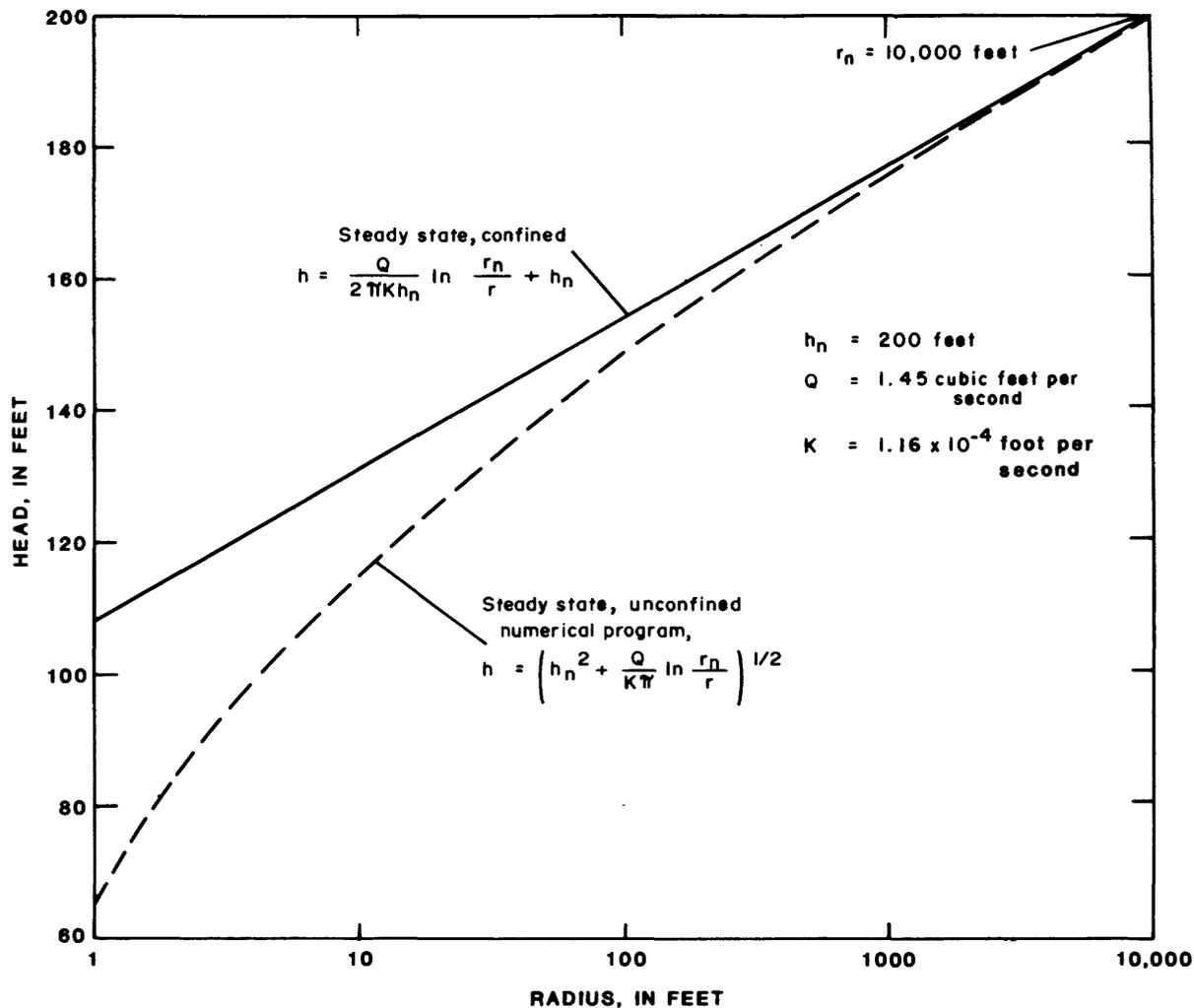


Figure 6.--Head versus distance for steady-state conditions.

### Depletion Calculations

One option for evaluation of ground-water depletion was adopted by the management district on July 12, 1978. It allows an evaluation of the depletion due to the addition of a single new well to all existing ground-water rights in the section of the proposed new well and the eight adjacent sections and commonly is described as within a 9-square-mile area. Another option for use in examining larger areas, although not adopted by the management district, allows an evaluation of depletion due to all existing water rights in a township, or a 36-square-mile area. The equations used to make depletion calculations are presented first, followed by examples of both the single-well option and the township option. Options identifying computers, program languages, data requirements, and summaries of possible results are shown by the flow diagram in figure 7.

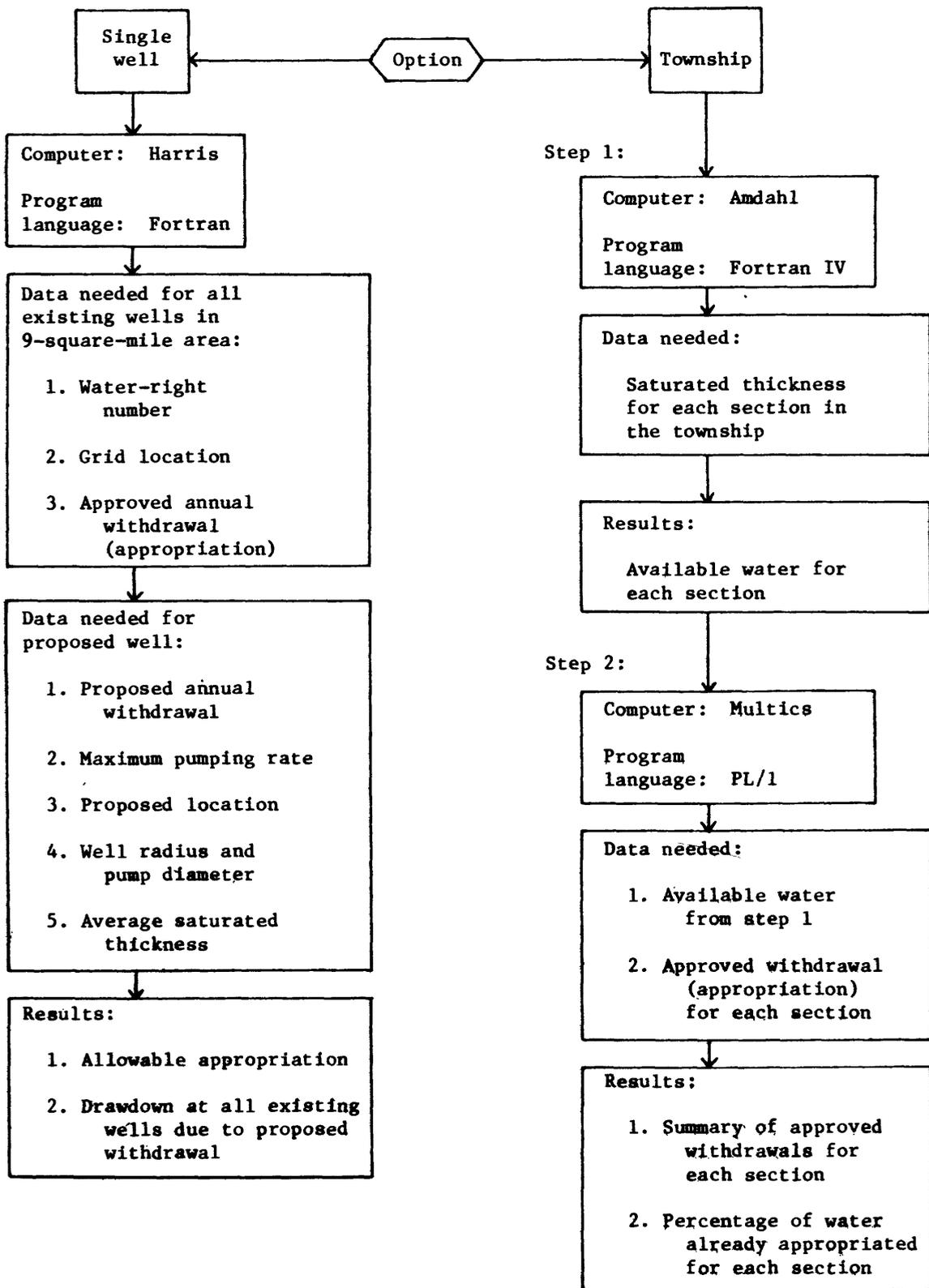


Figure 7.--Options identifying computers, program languages, principal data requirements, and program results.

The volume of ground-water available for appropriation ( $V_{av}$ ) is limited to 40 percent of that volume of water in ground-water storage plus that volume of water recharged to the aquifer from precipitation and irrigation return flow and is given by:

$$V_{av} = (V_{gws} + V_{rchg}) (0.40), \quad (6)$$

where

- $V_{av}$  = volume of ground water available for appropriation;
- $V_{gws}$  = volume of ground water in storage at time of analysis (specific yield assumed to be 20 percent); and
- $V_{rchg}$  = volume of recharge for 25 years (assumed to be 2 inches per year).

The total volume appropriated in 25 years is computed by adding all of the approved appropriations in the 9-square-mile area:

$$V_{app} = V_1 + V_2 + V_3 \dots V_n, \quad (7)$$

where

- $V_{app}$  = total volume appropriated in 9 square miles in 25 years; and
- $V_{1,2,\dots,n}$  = volume appropriated for an individual well in 25 years.

The volume of ground water appropriated must not exceed the allowable volume of depletion:

$$V_{app} \leq V_{av}. \quad (8)$$

If  $V_{app}/V_{av} > 1$ , then no further development of the aquifer is allowed.

#### Single-Well Option

Upon receipt of an application for a new well, the Southwest Kansas Groundwater Management District can use this option to evaluate the impact of the proposed ground-water withdrawal on the aquifer. The Fortran program (listed in Supplemental Information, p. 34) is designed for use in an interactive mode. The user provides necessary information about the proposed new well, existing wells, and aquifer properties. This program is written using Fortran IV (1966 ANSI) on a Harris S125<sup>1/</sup> computer system.

<sup>1/</sup> The use of brand names in this report is for identification purposes only and does not constitute endorsement by the U.S. Geological Survey.

### Data Requirements

Required information for the single-well option includes county name, land-line location, proposed annual appropriation, maximum pumping rate, well radius, and pump diameter. The location of the proposed well also is indicated by a row and column designation (fig. 8) representing one of the 10-acre tracts of the section in which the proposed well is to be drilled.

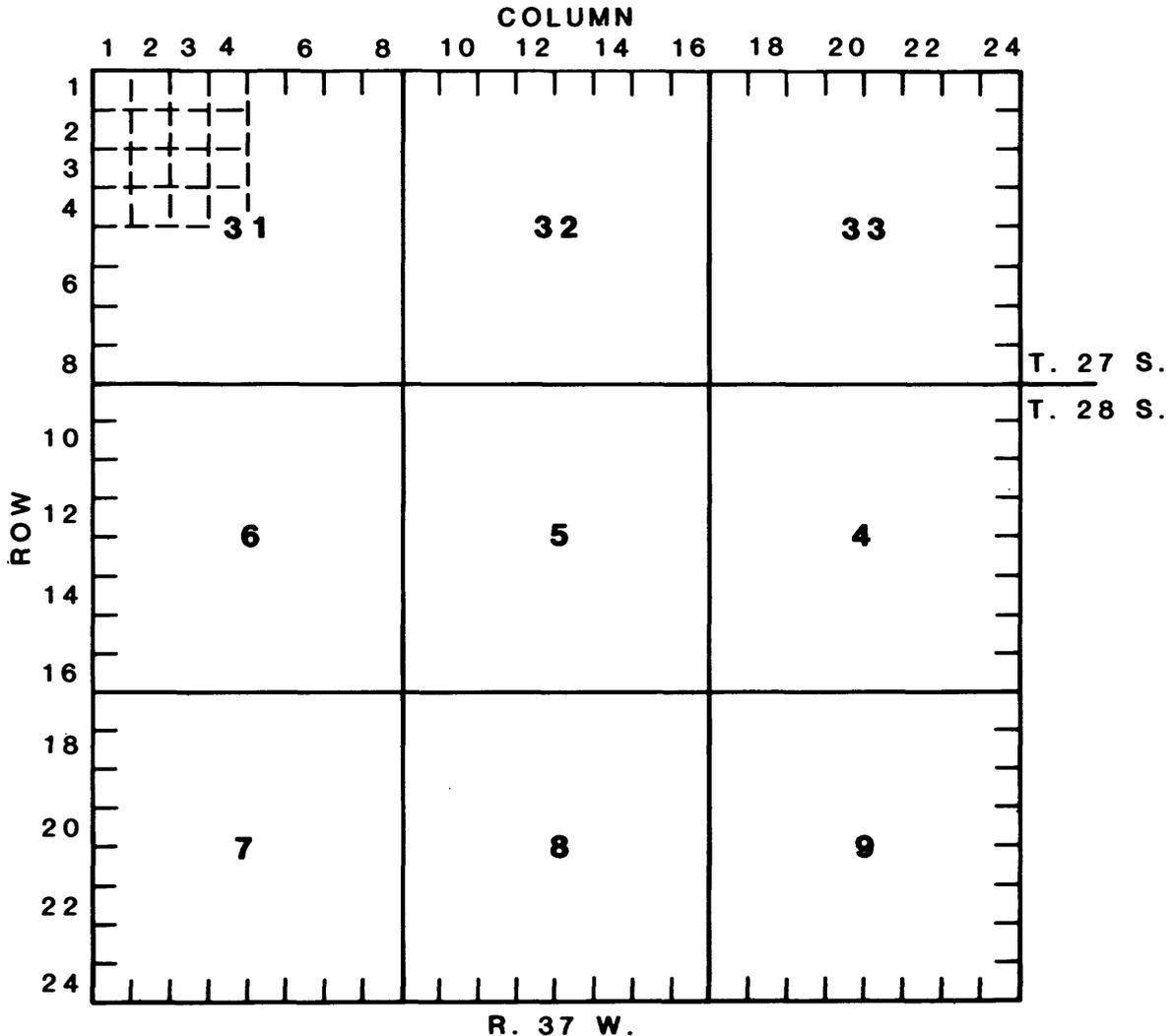


Figure 8.--Indexing system used to compute distance from proposed well to existing wells.

Information required for all existing wells in the section where the new well is proposed and in the adjacent eight sections include: (1) The water-right number, (2) the row and column location in the 9-square-mile area, and (3) the quantity of ground-water withdrawal approved for appropriation on an annual basis. The row and column location represents the 10-acre tract in which the well is located, based on the division of the 9-square-mile area shown in figure 8. This location is used to approximate the distance between the existing wells and the proposed well. The error

introduced by this approximation can be as great as 933 feet if two wells are located in the opposite corners of their respective 10-acre tracts. This accuracy suffices for the purpose of this model because the location of an existing well generally is not specified in the historical record to an accuracy greater than the 10-acre tract. If this degree of accuracy is inadequate to ascertain if well-spacing requirements are being met, an onsite investigation by the proposed water appropriator or management district personnel is necessary.

In some instances two or more water rights have been approved for the same parcel of land. This is called overlapping of water rights. It will be necessary, for management district personnel and those who use this program, to review such appropriations for limitations or corrections.

The aquifer properties necessary for the depletion calculation are saturated thickness and specific yield. The average saturated thickness for the 9-square-mile area is determined from the latest published saturated-thickness map, which recently has been prepared every third year (Pabst and Jenkins, 1976; Pabst, 1978). The specific yield is assumed to be 20 percent if other information for the 9 square miles in question is not available. An average aquifer hydraulic conductivity for the 9 square miles is needed for the drawdown computations and is determined by dividing the transmissivity by the saturated thickness.

### Results

The printed output for the single-well option includes a summary of the data plus the results of the depletion calculations and the computed drawdown in the aquifer due to the proposed well. The volume of water available considering all existing wells in the 9-square-mile area is printed for both 25 years and 1 year as the last two lines of the depletion information.

There are two basic forms of printed results depending upon the percentage of ground-water depletion with the proposed well. An example of the first form, where the percentage appropriation is equal to or less than the 40-percent allowable depletion for 25 years, is given in figure 9. However, the proposed diversion rate of 810 gal/min exceeded the maximum allowable diversion rate as defined by regulation. Thus, a message "MAXIMUM DIVERSION RATE EXCEEDED" was written.

The second form is characterized by a printed warning that depletion will exceed the 40-percent limit, and, if water is available, the volume requested for the proposed well will be adjusted downward to meet the maximum depletion limit. For the example problem shown in figure 10, a proposed annual appropriation of 500 acre-feet was requested. As this request for appropriation exceeded the allowable depletion, the proposed annual appropriation was reduced to 110 acre-feet. Because the requested appropriation exceeded the volume of water available for appropriation, the message "WITH PROPOSED WELL APPROPRIATION WILL EXCEED 40%" was printed.

SOUTHWEST KANSAS GROUND WATER MANAGEMENT DISTRICT  
GROUND WATER APPROPRIATION ANALYSIS  
WELL PROPOSED FOR  
HODGMAN COUNTY  
T 22E. R. 22E. SEC 22  
SE. SE. SE

DIVISION OF WATER RESOURCES NUMBER, 12343  
LOCATION OF 10 ACRE TRACT IN NINE SQUARE MILE AREA  
ROW, 12 COLUMN, 12

PROPOSED ANN APPROP., 500 ACRE- FEET  
PROPOSED DIVERSION RATE, 810 GALLONS PER MINUTE  
PUMP DIAMETER, 8 INCHES  
PROPOSED WELL RADIUS, 24 INCHES

-----  
AQUIFER PROPERTIES  
HYDRAULIC CONDUCTIVITY, 390 GAL/DAY/SQ FT  
SATURATED THICKNESS, 175 FEET  
-----

DWR NO.	LOCATION	NUMBER OF EXISTING WELLS IN NINE SQUARE MILE AREA- 1	ANNUAL APPROPRIATION	ACRE FEET	DISTANCE FROM PROPOSED WELL, IN FEET
23454	ROW COLUMN 13 20			600.	5321.

\*\*\*\*\*  
MAXIMUM DIVERSION RATE EXCEEDED  
\*\*\*\*\*

ACRE-FEET IN STORAGE, ACRE-FEET APPROPRIATED, PERCENT APPROPRIATED, ACRE-FEET AVAILABLE FOR APPROPRIATION, ACRE-FEET AVAILABLE FOR APPROPRIATION ANNUALLY,	WITHOUT PROPOSED WELL	WITH PROPOSED WELL
	235,600.	27,500.
	15,000.	12.
	7.	62,740.
	78240.	
	3010.	2310.

DRAWDOWN AFTER ONE YEAR DUE TO PUMPING PROPOSED WELL CONTINUOUSLY AT ANNUAL APPROPRIATION RATE

DISTANCE	DRAWDOWN
2.00	10.35
3.61	9.70
6.52	9.05
11.77	8.40
21.25	7.75
38.37	7.11
69.27	6.47
125.04	5.83
225.78	5.20
407.63	4.57
735.96	3.94
1328.72	3.31
2398.92	2.68
4331.10	2.06
7819.53	1.45
14117.66	0.87
25488.53	0.37
46017.92	0.07
83082.42	0.00
150000.01	0.00

EXISTING WELLS  
 5321.09 1.85  
 CALCULATED DELT AND NUMT, 0.9606E-05 66

DRAWDOWN DUE TO PUMPING ANNUAL APPROPRIATION FROM PROPOSED WELL CONTINUOUSLY AT PROPOSED DIVERSION RATE

DISTANCE	DRAWDOWN
2.00	21.92
3.61	20.09
6.52	18.29
11.77	16.50
21.25	14.74
38.37	12.99
69.27	11.27
125.04	9.56
225.78	7.87
407.63	6.20
735.96	4.56
1328.72	2.98
2398.92	1.53
4331.10	0.45
7819.53	0.04
14117.66	0.00
25488.53	0.00
46017.92	0.00
83082.42	0.00
150000.01	0.00

EXISTING WELLS  
 5321.09 0.30  
 139.69DAYS PUMP AT DIV.RATE;SHOULD NOT EXCEED 365

MASS BALANCE,  
 VOLUME OF CONE = 105810407.5000  
 VOL. PUMPED= 0.2178E+08 VOL. WATER FROM CONE= 0.2116E+08  
 IT,NUMDT,TIME, 3 66 0.12E+08

Figure 9.--Example of depletion and drawdown calculations when depletion is equal to or less than maximum allowed.

SOUTHWEST KANSAS GROUND WATER MANAGEMENT DISTRICT  
GROUND WATER APPROPRIATION ANALYSIS  
WELL PROPOSED FOR  
STEVENS COUNTY  
T.34S. R.10E. SEC.24  
SE. SE. SE

DIVISION OF WATER RESOURCES NUMBER, 20000  
LOCATION OF 10 ACRE TRACT IN NINE SQUARE MILE AREA  
ROW.12 COLUMN.13

PROPOSED ANN. APPROP., 110. ACRE- FEET  
PROPOSED DIVERSION RATE, 600. GALLONS PER MINUTE  
PUMP DIAMETER, 8. INCHES  
PROPOSED WELL RADIUS, 24. INCHES

-----  
AQUIFER PROPERTIES  
HYDRAULIC CONDUCTIVITY, 390 GAL/DAY/SQ FT  
SATURATED THICKNESS, 175 FEET  
-----

DWR NO.	NUMBER OF EXISTING WELLS IN NINE SQUARE MILE AREA- 7	ROW COLUMN	LOCATION	ANNUAL APPROPRIATION	DISTANCE FROM PROPOSED WELL, IN FEET
100	8	17	500.	500.	3734.
200	9	8	500.	500.	3848.
300	11	17	500.	500.	2721.
400	18	17	500.	500.	4759.
500	20	17	500.	500.	5903.
600	21	8	500.	500.	6795.
700	24	17	500.	500.	8348.

\*\*\*\*\*

WITH PROPOSED WELL APPROPRIATION WILL EXCEED 40%

\*\*\*\*\*

ACRE- FEET IN STORAGE,	WITHOUT PROPOSED WELL	WITH PROPOSED WELL
ACRE- FEET APPROPRIATED,	225600.	92500.
PERCENT APPROPRIATED,	87500.	41.
ACRE- FEET AVAILABLE FOR APPROPRIATION,	39.	-2260.
ACRE- FEET AVAILABLE FOR APPROPRIATION,	2740	
ANNUALLY,	110.	-90.

APPROPRIATION ADJUSTED TO, 110 ACRE FEET

DRAWDOWN AFTER ONE YEAR DUE TO PUMPING PROPOSED WELL CONTINUOUSLY AT ANNUAL APPROPRIATION RATE

DISTANCE	DRAWDOWN
2.00	2.22
3.61	2.08
6.32	1.94
11.77	1.81
21.25	1.67
36.37	1.53
67.27	1.40
125.06	1.26
225.78	1.13
407.63	0.99
735.96	0.86
1328.72	0.72
2398.92	0.59
4331.10	0.45
7819.53	0.32
14117.66	0.19
25488.55	0.08
46017.92	0.01
83082.42	0.00
150806.01	0.00

EXISTING WELLS

3733.52	0.48
3848.43	0.48
2721.25	0.56
4759.33	0.43
5903.22	0.38
6795.12	0.35
8348.41	0.30

CALCULATED DELT AND NUMT. 0.9594E-05 63

DRAWDOWN DUE TO PUMPING ANNUAL APPROPRIATION FROM PROPOSED WELL CONTINUOUSLY AT PROPOSED DIVERSION RATE

DISTANCE	DRAWDOWN
2.00	14.61
3.61	13.31
6.52	12.03
11.77	10.76
21.25	9.49
38.37	8.24
69.27	6.99
125.06	5.76
225.78	4.54
407.63	3.33
735.96	2.17
1328.72	1.11
2398.92	0.31
4331.10	0.03
7819.53	0.00
14117.66	0.00
25488.53	0.00
46017.92	0.00
83082.42	0.00
150000.01	0.00

41 34DAYS PUMP AT DIV. RATE; SHOULD NOT EXCEED 345

EXISTING WELLS

3733.52	0.10
3848.43	0.09
2721.25	0.25
4759.33	0.02
5903.22	0.01
6795.12	0.01
8348.41	0.00

MASS BALANCE.

VOLUME OF CONE = 2315224.5520

VOL. PUMPED= 0.4774E+07 VOL. WATER FROM CONE= 0.4639E+07

IT.NUMDT.TIME, 3 63 0.36E+07

Figure 10.--Example of depletion and drawdown calculations when depletion exceeds maximum allowed.

## Township Option

### Data Requirements

Evaluation of ground-water depletion for an entire township was accomplished in an interactive mode by using a remote terminal and programs and data on disk files of a Honeywell Multics Computer in Reston, Virginia. Although the computations are the same as the single-well evaluation for any given section, the method of data entry is different. First, equation 6 was solved using a FORTRAN computer program (Supplemental Information, p. 40) and the average saturated thickness in each square mile of the township. This program is written in Fortran IV (IBM "G" Compiler) and runs on an Amdahl V7 computer. An area of as many as four townships on a side can be analyzed in a single run of the program. This program calculates the volume of ground water available for appropriation on an annual basis in the 9-square-mile area centered around each section in the township (table 1). The ground water available for appropriation on an annual basis must be written on a disk file, which becomes one part of the data required by the PL/1 computer program used to determine  $V_{app}/V_{av}$ . The other part of the data required by this PL/1 interactive program is a file of water rights that was prepared from a tape file of the Kansas Water Office containing applications for water rights in Kansas up through 1974. Because this PL/1 program is primarily a file-management routine written specifically for Kansas water-rights data and has little or no general application, a listing is not included in this report.

### Results

Results from the computer program used to evaluate ground-water depletion on a township basis are a summary of the water rights for each section and the eight adjacent sections (fig. 11) and a tabular listing of the percentage of ground water appropriated for each section in the township, considering all available water and water rights in each 9-square-mile area (fig. 12). A withdrawal is "certified" (fig. 11) if the volume of water withdrawn has been certified by onsite measurements.

The procedure was tested using the 16 townships in northwestern Finney County. The available water in each 9-square-mile area, based on the management district's rules and regulations for 1978-2003, ranges from less than 1,000 to more than 5,000 acre-feet per year. The two areas where the available water generally is less than 1,000 acre-feet per year are located in the northeastern and northwestern townships where the January 1978 saturated thickness was less than 50 feet (fig. 13). The two southwestern townships have the most available water (fig. 13), with a few 9-square-mile areas averaging as much as 7,000 acre-feet of ground water available annually for 1978-2003.

Table 1.--Example of ground-water-availability calculations  
for an entire township

GROUND WATER AVAILABILITY  
BASED ON NINE SQUARE MILE DEPLETION MODEL

SOUTHWESTERN KANSAS GROUNDWATER  
MANAGEMENT DISTRICT NO. 3

U.S. GEOLOGICAL SURVEY

TOWNSHIP 24W RANGE 31S

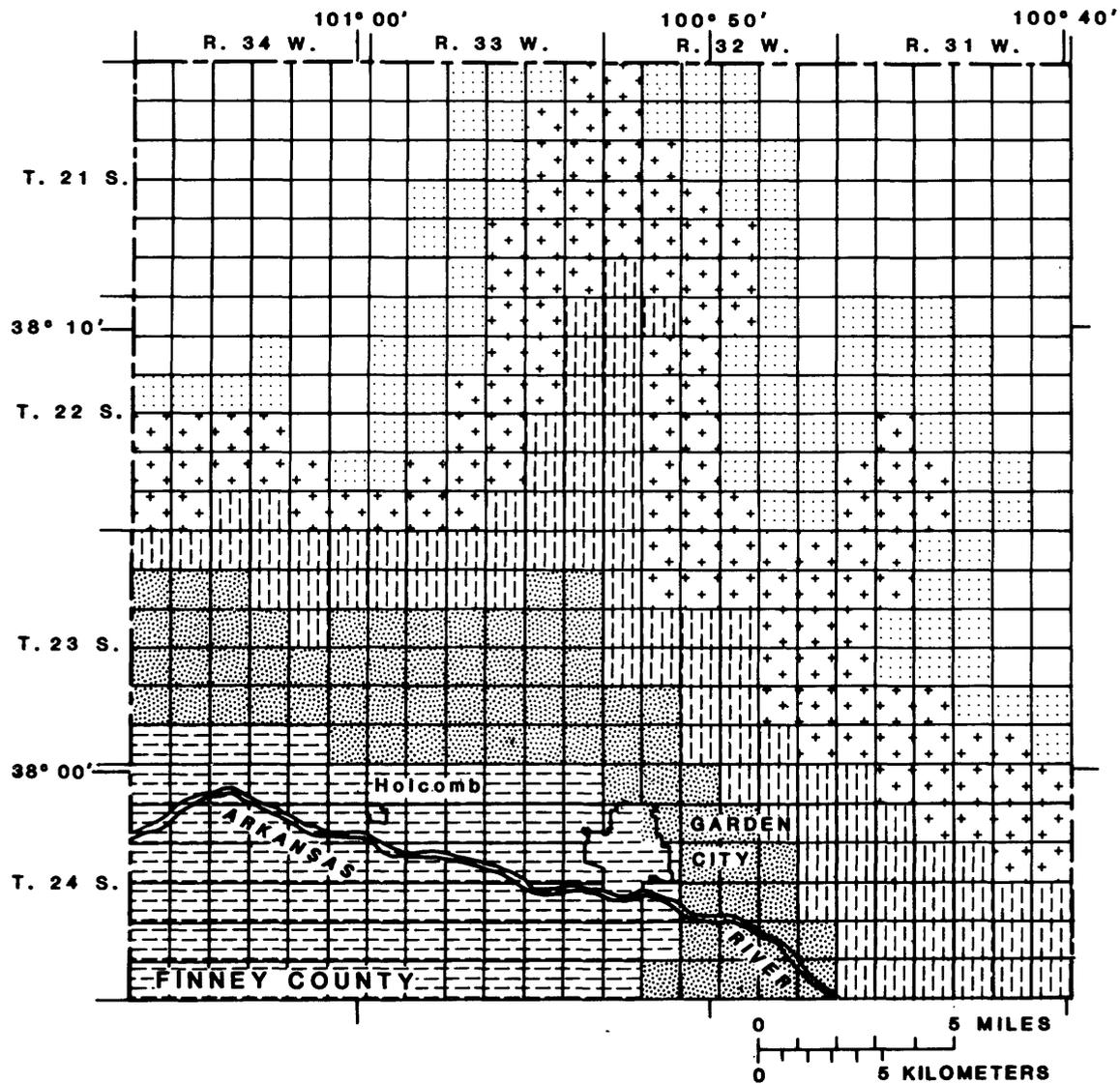
SECTION	AVERAGE SATURATED THICKNESS (FEET)	VOLUME IN STORAGE (ACRE- FEET)	VOLUME AVAILABLE IN 25 YEARS (ACRE- FEET)	ANNUAL VOLUME AVAILABLE (ACRE- FEET)
1	94.3	108671.9	53068.7	2122.7
2	100.7	115967.9	55987.1	2239.5
3	112.9	130047.9	61619.2	2464.8
4	122.2	140799.9	65919.9	2636.8
5	133.3	153599.9	71039.9	2841.6
6	145.0	167039.9	76415.9	3056.6
7	159.4	183679.9	83071.9	3322.9
8	146.7	168959.9	77183.9	3087.4
9	135.6	156159.9	72063.9	2882.6
10	126.8	146047.9	68019.1	2720.8
11	116.2	133887.9	63155.1	2526.2
12	111.0	127871.9	60748.8	2430.0
13	130.2	149951.9	69580.7	2783.2
14	134.6	155007.9	71603.1	2864.1
15	143.4	165247.9	75699.1	3028.0
16	148.9	171519.9	78207.9	3128.3
17	157.2	181119.9	82047.9	3281.9
18	167.8	193279.9	86911.9	3476.5
19	177.2	204159.9	91263.9	3650.6
20	168.3	193919.9	87167.9	3486.7
21	162.8	187519.9	84607.9	3384.3
22	157.8	181759.9	82303.9	3292.2
23	150.6	173439.9	78975.9	3159.0
24	146.7	168959.9	77183.9	3087.4
25	155.8	179519.9	81407.9	3256.3
26	157.8	181759.9	82303.9	3292.2
27	162.2	186879.9	84351.9	3374.1
28	166.7	191999.9	86399.9	3456.0
29	173.3	199679.9	89471.9	3578.9
30	183.9	211839.9	94335.9	3773.4
31	188.3	216959.9	96383.9	3855.4
32	175.8	202559.9	90623.9	3625.0
33	167.5	192959.9	86783.9	3471.4
34	161.7	186239.9	84095.9	3363.8
35	159.2	183359.9	82943.9	3317.8
36	158.7	182879.9	82751.9	3310.1

Section 31 Twp 14S Ran 42W	
Total withdrawal approved	6361 acre-feet
Total withdrawal certified	1516 acre-feet
9 Water rights in or near section 31	
Section 32 Twp 14S Ran 42W	
Total withdrawal approved	5516 acre-feet
Total withdrawal certified	1516 acre-feet
7 Water rights in or near section 32	
Section 33 Twp 14S Ran 42W	
Total withdrawal approved	7828 acre-feet
Total withdrawal certified	1280 acre-feet
10 Water rights in or near section 33	

Figure 11.--Example of water-rights summary for a section and eight adjacent sections within an entire township.

Percentage of water already appropriated in					
Township 14S Range 42W					
156	176	137	181	193	140
166	236	212	272	259	177
236	309	316	329	324	235
240	289	377	365	367	219
204	203	319	307	362	240
165	148	237	197	254	135

Figure 12.--Ground-water depletion evaluation for an entire township. Values for each section are in the same position as they would be found on a map, with section 1 being the top value on the right side.



**EXPLANATION**

**AVAILABLE GROUND WATER, IN ACRE-FEET**

- |   |  |   |             |   |                    |
|---|--|---|-------------|---|--------------------|
|  | No data -- Saturated thickness less than 50 feet, January 1978 |  | 2,000-2,999 |  | 4,000-4,999        |
|  | Less than 2,000  |  | 3,000-3,999 |  | Greater than 5,000 |

Figure 13.--Volume of ground water available for appropriation annually for each section and the adjacent eight sections, 1978-2003.

The percentage of water available during 1978, which was already appropriated during 1974, is shown in figure 14. There were only about 6 square miles in the southern part of the area where the percentage of ground water available for appropriation was less than the 40-percent limit set by the management district for new development. More than 100 percent of the ground water available during 1978, as defined by the management district's regulations, has been appropriated in all but 61 square miles of the 16 townships evaluated.

### CONCLUSIONS

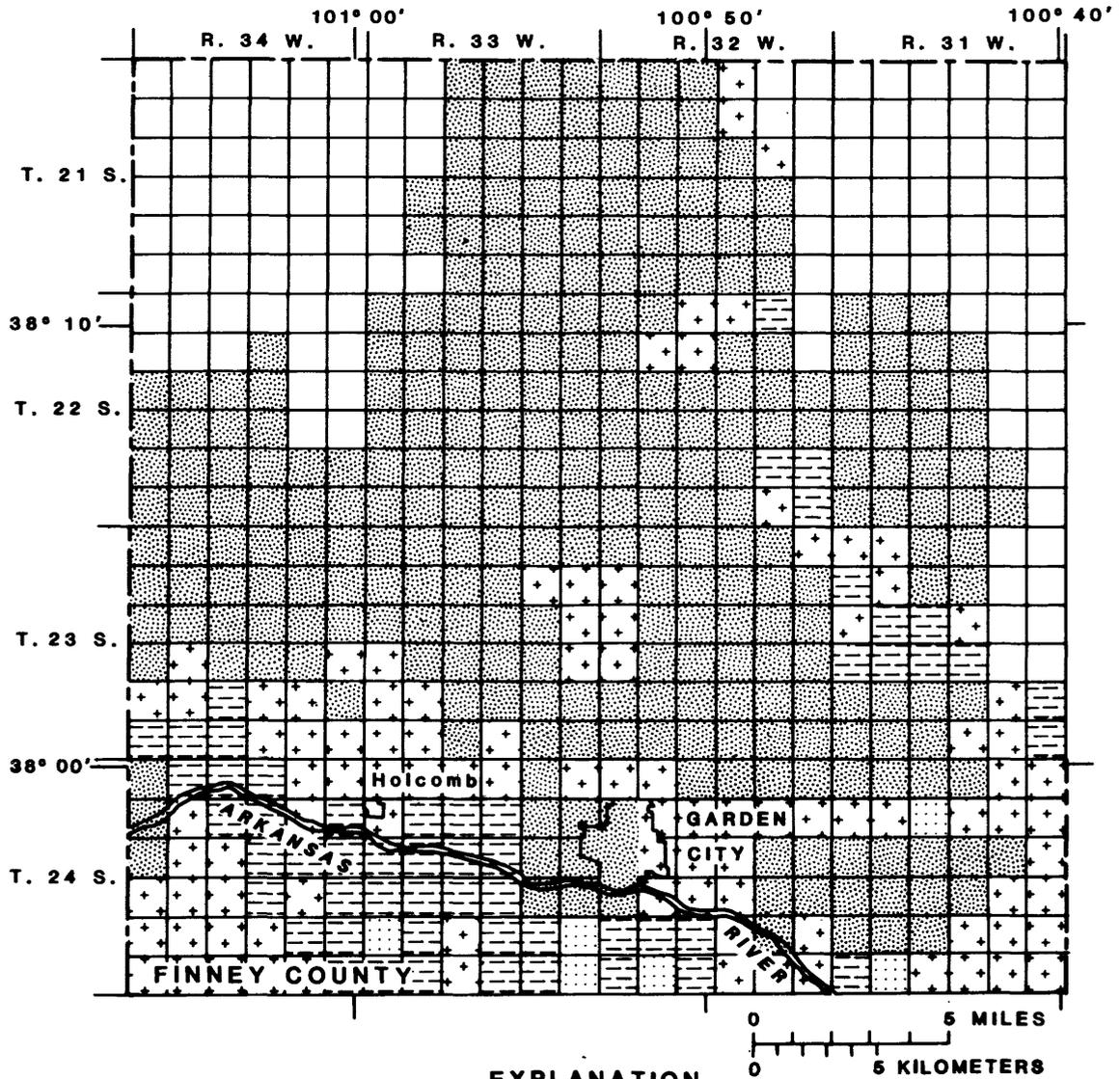
A computer model was developed to calculate the drawdown, due to a proposed well, at all existing wells in the section of the proposed well and at all wells in the adjacent eight sections. The depletion expected in the 9-square-mile area due to all existing wells and the proposed well is computed and compared with allowable limits defined by the Southwest Kansas Groundwater Management District No. 3. An optional program permits the evaluation of allowable depletion for one or more townships. All options are designed to run interactively, thus allowing for immediate evaluation of proposed ground-water withdrawals.

### ACKNOWLEDGMENTS

The assistance of the staff of the Southwest Kansas Groundwater Management District No. 3 in collating allocation data, and the advice and assistance of Dr. Carl McElwee of the Kansas Geological Survey in developing the finite-difference approximations used in the model are appreciated.

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- Ehlig, Christine, and Halepaska, J. C., 1976, A numerical study of confined-unconfined aquifers, including effects of delayed yield and leakage: *Water Resources Research*, v. 12, no. 6, p. 1175-1183.
- Jacob, C. E., 1950, *The flow of groundwater*, in Rouse, H. ed., *Engineering Hydraulics*, Chapter 5: New York, John Wiley, p. 321-385.
- Moench, A. F., and Prickett, T. A., 1972, Radial flow in an infinite aquifer undergoing conversion from artesian to water-table conditions: *Water Resources Research*, v. 8, no. 2, p. 494-499.
- Pabst, M. E., 1978, Map showing saturated thickness of the unconsolidated aquifer in southwestern Kansas, January 1978: U.S. Geological Survey Open-File Report 78-969, 4 pl.



**EXPLANATION**

**PERCENTAGE OF GROUND WATER APPROPRIATED**

- |   |  |   |         |   |                  |
|---|--|---|---------|---|------------------|
|  | No data -- Saturated thickness less than 50 feet, January 1978 |  | 40-99   |  | Greater than 200 |
|  | Less than 40   |  | 100-199 |   |                  |

Figure 14.--Percentage of ground water available for appropriation during 1978 that was already appropriated during 1974.

Pabst, M. E., and Jenkins, E. D., 1976, Water-level changes in southwestern Kansas, 1940-75: Kansas Geological Survey Journal, May 1976, 26 p.

Trescott, P. C., Pinder, G. F., and Larson, S. P., 1976, Finite-difference model for aquifer simulation in two dimensions with results of numerical experiments: U.S. Geological Survey Techniques of Water-Resources Investigations, Book 7, Chapter 1, 116 p.

Van Rosenberg, D. V., 1969, Methods for numerical solution of partial differential equations: New York, Elsevier, 113 p.

#### SUPPLEMENTAL INFORMATION

##### Finite-Difference Approximations of Radial-Flow Equation

$$\frac{1}{r} \frac{\partial}{\partial r} (rKh \frac{\partial h}{\partial r}) = S' \frac{\partial h}{\partial t} \quad (1)$$

Equation 1 in radial coordinates can be transformed to cartesian coordinates by a change of variables. Let

$$x = \ln \left( \frac{r}{r_w} \right), \quad (a)$$

where  $0 \leq x \leq R$ .

R is a very large distance from the origin, and  $r_w$  is the radius of the well. From equation a, it can be stated

$$r = r_w e^x \quad (b)$$

Differentiating equation a yields

$$\frac{\partial x}{\partial r} = \frac{1}{r} \quad (c)$$

Substituting equations b and c into equation 1 yields

$$\frac{1}{(r_w e^x)^2} \frac{\partial}{\partial x} (rKh \frac{\partial h}{\partial r}) = S' \frac{\partial h}{\partial t} \quad (d)$$

Applying the "chain rule" of differentiation to equation d results in

$$\frac{1}{r_w^2 e^{2x}} \frac{\partial}{\partial x} (Kh \frac{\partial h}{\partial x}) = S' \frac{\partial h}{\partial t} \quad (e)$$

Equation e is a nonlinear differential equation and can be solved numerically using finite-difference techniques.

Let  $x = i \Delta x$ , where  $i$  indexes the nodes at which equation e is to be evaluated. The finite-difference equation has a constant spacing. At the radius of the well where  $r = r_w$ :

$$x = \ln \frac{r}{r_w} = 0, \text{ at } i = 1.$$

Using a numerical method, noting equation 1, and using a technique similar to that used by Von Rosenberg (1969, p. 63), equation e has the form

$$\begin{aligned} & T_{i+1/2}^{m+1/2} h_{i+1}^{m+1} - T_{i+1/2}^{m+1/2} h_i^{m+1} - T_{i-1/2}^{m+1/2} h_i^{m+1} + T_{i-1/2}^{m+1/2} h_{i-1}^{m+1} - F h_i^{m+1} = \\ & -F h_i^m - T_{i+1/2}^{m+1/2} h_{i+1}^m + T_{i+1/2}^{m+1/2} h_i^m + T_{i-1/2}^{m+1/2} h_i^m - T_{i-1/2}^{m+1/2} h_{i-1}^m, \end{aligned} \quad (f)$$

where  $m$  is a superscript indicating time level, and

$$F = \frac{S' 2r_w^2 \Delta x^2}{e^{2i} \Delta x \Delta t}.$$

$F$  is an arbitrary variable created to simplify equation f and others. Equation f is for the interior nodes.

At the well face ( $r_w$ ), at  $i = 1$ , is a constant-flux boundary

$$\frac{-Q}{2\pi} = rhk \left. \frac{\partial h}{\partial r} \right|_{r=r_w} \quad (g)$$

Noting that  $r \frac{\partial h}{\partial r} = \frac{\partial h}{\partial x}$ , equation g changes to

$$\frac{-Q}{2\pi} = hk \left. \frac{\partial h}{\partial x} \right|_{r=r_w} \quad (h)$$

At an imaginary node inside the well at  $i = 0$ , let

$$\frac{\partial h}{\partial x} \approx \frac{h_2 - h_0}{2 \Delta x}, \text{ then} \quad (i)$$

$$h_0^m = h_2^m + \frac{\Delta x Q}{\pi T_1}.$$

Evaluating at  $i = 1$ , using equation i yields

$$-1 - \left( \frac{T_1^{m+1/2} + F}{T_{3/2}^{m+1/2}} \right) h_1^{m+1} + \left( 1 + \frac{T_{1/2}^{m+1/2}}{T_{3/2}^{m+1/2}} \right) h_2^{m+1/2} = \quad (j)$$

$$\frac{-Fh^m}{T_{3/2}^{m+1/2}} + (h_1^m - h_2^m) + \frac{T_{1/2}^{m+1/2}}{T_{3/2}^{m+1/2}} (h_1^m - h_2^m - \frac{\Delta x Q}{\pi T_1^m} - \frac{\Delta x Q}{\pi T_1^{m+1}}).$$

At the outside boundary,  $i = n$ , a constant head is chosen, or  $h_n^{m+1} = h_n^m$ .  
The equation at  $n$  becomes

$$\frac{T_{n-3/2}^{m+1/2}}{T_{n-1/2}^{m+1/2}} h_{n-2}^{m+1} + \left\{ -1 - \left( \frac{T_{n-1/2}^{m+1/2} + F}{T_{n-1/2}^{m+1/2}} \right) h_n^{m+1} \right\} =$$

$$F \frac{h_{n-1}^m}{T_{n-1/2}^{m+1/2}} + h_{n-1}^m - 2h_n^m + \left( \frac{T_{n-3/2}^{m+1/2}}{T_{n-1/2}^{m+1/2}} \right) (h_{n-1}^m - h_{n-2}^m). \quad (k)$$

The various forms of  $T$  can be reduced by using the following relations:

$$T_1^m = K h_1^m \quad ; \quad (l)$$

$$T_1^{m+1} = K h_1^{m+1} \quad ; \quad (m)$$

$$T_{i+1/2}^{m+1/2} = \frac{1}{2} (T_i^{m+1/2} + T_{i+1}^{m+1/2}) \quad ; \quad (n)$$

$$T_{i-1/2}^{m+1/2} = \frac{1}{2} (T_i^{m+1/2} + T_{i-1}^{m+1/2}) \quad ; \quad (o)$$

$$T_{3/2}^{m+1/2} = \frac{1}{2} (T_1^{m+1/2} + T_2^{m+1/2}) \quad ; \quad (p)$$

$$T_{1/2}^{m+1/2} = \frac{1}{2} (T_1^{m+1/2} + T_2^{m+1/2} + \frac{\Delta x Q}{\pi h_1^{m+1/2}}) \quad ; \quad \text{and} \quad (q)$$

$$h_1^{m+1/2} = \frac{1}{2} (h_1^{m+1} + h_1^m). \quad (r)$$

Equations f, j, and k are of the form that can be solved using the Thomas algorithm. The procedure for solution is to iterate the head values until little change occurs between  $h^m$  and  $h^{m+1}$ . This is accomplished by updating the T values after each iteration using

$$T_1^{m+1/2} = \frac{1}{2}K (h_i^{m+1} + h_i^m) . \quad (s)$$

The first estimate of T at a new time step assumes

$$T_1^{m+1} = K h_i^{m+1} \approx K h_i^m . \quad (t)$$

# Program Listing for Single-Well Option

27 OCT 81 10 21 01 MODULE: \*MAIN\* OPTIONS: SE SU WA LO SAPP.S  
 61516-02 ENHANCED SAU FORTRAN COMPILER REVISION LEVEL 08.01 PRE SOURCE INPUT:

```

3 C GWMD NO 3--APPROPRIATION OF GROUND WATER 00000
4 C 00000
5 C POLICY-REGULATIONS 00000
6 C AND 00000
7 C PREDICTION OF WATERTABLE DRAWDOWNS 00000
8 C ----- 00000
9 INTEGER DWRNO 00000
10 DIMENSION BCTA(20),HO(20),HN(20),T(20),GAMMA(20),R(20), 00000
11 & DWRNO(20),RTONWL(20),IR(20),IC(20),APPROP(20),CO(8),A(20),BB(1), 00000
12 & C(1) 00000
13 C CALL FPARAM (1,131) 00000
14 IPRINT=0 00000
15 PRINT 1000 00006
16 1000 FORMAT('COUNTY?') 00013
17 READ (33,1001) CO 00024
18 1001 FORMAT (8A1) 00034
19 PRINT 1007 00043
20 1002 FORMAT ('TOWNSHIP,RANGE,SECTION?') 00050
21 READ(33,-)ITNS,IRNG,ISEC 00066
22 PRINT 1111 00103
23 1111 FORMAT ('1/4,1/4,1/4?') 00110
24 READ (33,1112) A 00122
25 1112 FORMAT (20A1) 00132
26 C INPUT WELL DATA 00132
27 PRINT 650 00141
28 650 FORMAT('DIVISION OF WATER RESOURCES NUMBER?') 00146
29 READ (33,655)NDWRNO 00170
30 655 FORMAT (I8) 00200
31 PRINT 660 00206
32 660 FORMAT('ROW,COLUMN, AND APPROPRIATION IN AC FT./YR ?') 00213
33 READ(33,-) IRPW,ICFW,PROFAP 00240
34 WRITE(44,665) NDWRNO, (RPW,ICPW,PROFAP 00253
35 665 FORMAT('DWR NO ROW,COL,& APPROPRIATION','I8,I4,I4,F9.2) 00276
36 PRINT 670 00323
37 670 FORMAT('DIV RATE IN GPM &PUMP COL DIAM IN INCHES ARE?') 00330
38 READ(33,-)QINST,PUMDIA 00356
39 RMIN=1300 00370
40 IF(QINST GT 400 ) RMIN=2300 00373
41 IF(PUMDIA GT 6.) RMIN=2300 00402
42 PRINT 680 00411
43 680 FORMAT('RADIUS OF NEW WELL IN FEET ?') 00416
44 READ(33,-)RW 00436
45 PRINT 700 00445
46 RWI=RW*12.0 00452
47 700 FORMAT('NUMBER OF EXISTING WELLS ?') 00456
48 READ(33,-)NW 00475
49 IF(NW EQ 0) GO TO 22 00504
50 APPR=0 00511
51 PRINT 710 00515
52 710 FORMAT('DWR NO ON 1ST LINE ROW, COL, & APPROP ON 2ND LINE') 00522
53 DO 20 IW=1,NW 00532
54 READ(33,655) DWRNO(IW) 00562
55 READ(32,-)IROWI,ICOLWI,APPROP 00576
56 APPR=APPR+APPROP 00613
57 IR(IW)=IROWWI 00617
58 IC(IW)=ICOLWI 00623
59 APPROP(IW)=APPROP 00626
    
```

Program Listing for Single-Well Option--Continued

```

60. ASQ=((660.)*(IRPW-IROWL))**2 00633
61. BSO=((660.)*(JCPW-ICOLWL))**2 00642
62. RTONWL(IW)=SORT(ASQ+BSO) 00651
63. WRITE(44,730)DWRNO(IW),IROWWL,ICOLWL,APPROP,RTONWL(IW) 00664
64. 730 FORMAT('DWR NO,ROW,COL ,APPROP ,SRADIUS, ,TR,2I4,F6.0,F8.0) 00720
65. 20 CONTINUE 00746
66. WRITE(44,740) RMIN 00755
67. 740 FORMAT('DISTANCE BETWEEN WELLS SHOULD EXCEED',F7.0,' FT ') 00765
68. C HYDROLOGIC DATA 00765
69. 22 PRINT 760 01013
70. 760 FORMAT('HYDRAULIC CONDUCTIVITY IN GAL/DAY/50 FT IS ',) 01021
71. READ(33,*)P 01046
72. PM=P 01055
73. P=P/(7.481*86400.) 01060
74. S=0.20 01067
75. RECHAR=(2./12.)*9.*640.*25 01072
76. PRINT 765 01101
77. 765 FORMAT('SATURATED THICKNESS IN FEET ',) 01106
78. READ(33,*)H 01126
79. VOLSTR=S*H*9.*640.+RECHAR 01135
80. VOLAPR=APR*25 01144
81. VOLFLT=(4*VOLSTR)-VOLAPR 01150
82. PERCNT=(VOLAPR/VOLSTR)*100 01155
83. WRITE(44,770)VOLSTR,VOLAPR,PERCNT,VOLFLT 01162
84. 770 FORMAT('VOL STORED,VOL APPROP ,% APPROP ,VOL LEFT',4F12.0) 01203
85. TOTVAP=VOLAPR/PROPAP*25 01231
86. PERCT=(TOTVAP/VOLSTR)*100 01235
87. WRITE(44,800)PERCT,VOLFT 01243
88. VOLFT=(4*VOLSTR)-TOTVAP 01256
89. WRITE(44,800)PERCNT,VOLFLT 01263
90. AVLLFT=VOLFLT/25 01276
91. AVLFT=VOLFT/25 01302
92. 800 FORMAT('% APPROP INCLUDING NEW WELL AND VOL LEFT',2F12.2) 01306
93. IF(PERCT.LT.40.)GO TO 50 01334
94. ADJAPR=((VOLSTR*.4)-VOLAPR)/25 01340
95. WRITE(44,810)ADJAPR 01347
96. 810 FORMAT('THE ADJUSTED APPROPRIATION IS',F12.0,' IN AC.FT /YR.') 01357
97. PROPAP=ADJAPR 01406
98. GO TO 50 01411
99. 815 FORMAT('OVERAPPROP OR MAX DIV RATE EXCEEDED,CALC CONT ') 01412
100. C DATA FOR SOLUTION SCHEME 01412
101. 50 JCNT=0 01441
102. RO=150000 01444
103. N=20 01447
104. Q=(-1.)*(PROPAP*43560.)/(365.*86400.) 01452
105. TMAX=7.89E08 01464
106. DELT=0.00001 01467
107. IPRNT=IPRNT+1 01473
108. NUMT=150 01477
109. COLT=15 01502
110. JCNT=JCNT+1 01505
111. HDIFF=0.01 01511
112. ITHAX=20 01514
113. DX=ALOG(RO/RW)/(N-1) 01517
114. C COMPUTE ACTUAL DELT AND NUMT 01517
115. FACT=1 01533
116. IF(DELT.LT.1.)FACT=1./DELT 01536

```

Program Listing for Single-Well Option--Continued

27 OCT 81 10:21:01 MODULE: \*MAIN\* REVISION LEVEL 08.01 PRE SOURCE INPUT: SAPP: S

```

117 DELT=DELT*FACT
118 DT=DELT
119 TMAX=TMAX*FACT
120 TM=0
121 DO 131 I=1,NUMT
122 DT=CDLT*DT
123 TM=TM+DT
124 IF(TM GE TMAX) GO TO 132
125 131 CONTINUE
126 GO TO 133
127 132 DELT=TMAX/(TM*DELT)
128 NUMT=1
129 133 DELT=DELT/FACT
130 TMAX=TMAX/FACT
131 WRITE(44,530) DELT,NUMT
132 530 FORMAT(1X,'CALCULATED DELT AND NUMT.',E15.4,14)
133 C INITIALIZATION
134 TIME=0
135 NUMDT=0
136 PI=3.1416
137 HO(N)=H
138 T(N)=H*P
139 DO 140 I=1,N
140 HN(I)=H
141 140 CONTINUE
142 145 IT=0
143 NUMDT=NUMDT+1
144 DELT=DELT*CDLT
145 TIME=TIME+DELT
146 DO 150 I=1,N-1
147 150 HO(I)=HN(I)
148 C COMPUTING COEFFICIENTS FOR 1ST EQUATION
149 AI=0
150 151 RI=0
151 C COMPUTING T(1) THRU T(N-1) AT TIME M+1/2
152 DO 155 I=1,N-1
153 155 T(I)=5*P*(HO(I)+HN(I))
154 IT=IT+1
155 ARG=2*PI*DX
156 COEF2=1/(2*RW*RW*DX*DX*EXP(ARG))
157 COEF2=S/(COEF2*DELT)
158 T1HAF=5*(T(1)+T(2)+(DX*Q)/(PI*5*(HO(1)+HN(1))))
159 T3HAF=5*(T(1)+T(2))
160 TIM=P*HO(1)
161 TIMPL=P*HN(1)
162 BI=(-1-(T1HAF+COEF2)/T3HAF)
163 CI=1+T1HAF/T3HAF
164 DI=-COEF2*HO(1)/T3HAF+HO(1)
165 5*(HO(1)-HO(2)-(DX*Q)/(PI*TIM)-(DX*Q)/(PI*TIMPL))
166 BETA(1)=BI
167 GAMMA(1)=DI/BETA(1)
168 C COMPUTING COEF FOR INTERIOR AND LAST EQUATIONS
169 DO 160 I=2,N-1
170 TIMIN=(T(I-1)+T(I))/2
171 TIFLUS=(T(I)+T(I+1))/2
172 C ALL CI FROM C2 TO CN-1 ARE 1
173 RI=I-1

```

01546  
01552  
01555  
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01741  
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01753  
01757  
01763  
01767  
02002  
02002  
02015  
02020  
02020  
02024  
02037  
02061  
02061  
02065  
02074  
02114  
02123  
02150  
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02303

Program Listing for Single-Well Option--Continued

27 OCT 81 10:21:01 MODULE \*MAIN\* OPTIONS: SE SU WA I/O REVISION LEVEL 08 01 PRE SOURCE INPUT: SAPP:8

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174: ARG=2 *RI*DX
175: COEF2=1./((2.*RW*RW*DX*DX*EXP(ARG))
176: COEF2=B/(COEF2*DELT)
177: AI=TIMIN/TIPLUS
178: BI=(-1-(TIMIN+COEF2)/TIPLUS)
179: DI=(-COEF2*HO(I))/TIPLUS+HO(I)-HO(I+1)+AI*(HO(I)-HO(I+1))
180: IF(I.EQ.(N-1)) DI=DI-HN(N)
181: BETA(I)=BI-AI/BETA(I-1)
182: IF(I.EQ.2) BETA(2)=BI-AI*CI/BETA(1)
183: GAMMA(I)=(DI-AI*GAMMA(I-1))/BETA(I)
184: C TEMPORARILY STORING HEAD VALUES IN T MATRIX
185: DO 170 I=1,N-1
186: T(I)=HN(I)
187: HN(N-1)=GAMMA(N-1)
188: DO 180 J=1,N-3
189: I=(N-1)-J
190: HN(I)=GAMMA(I)-HN(I+1)/BETA(I)
191: HN(I)=GAMMA(I)-(CI*HN(2))/BETA(I)
192: HMAXDF=0
193: DO 190 I=1,N-1
194: DIFF=ABS(T(I)-HN(I))
195: IF(DIFF.GT.HMAXDF) HMAXDF=DIFF
196:
197: IF(IT.LT.3) GO TO 151
198: IF(HMAXDF.LT.HDIFF) GO TO 210
199: IF(IT.LT.ITMAX) GO TO 151
200:
201: CONTINUE
202: IF(HN(1).LT.0.) GO TO 215
203: IF(NUMDT.LT.NUMT) GO TO 143
204: GO TO 220
205: WRITE(44,597)
206: CONTINUE
207: C CALCULATING RADII
208: DO 240 I=1,N
209: Z=I-1
210: X=Z*DX
211: R(I)=RW*EXP(X)
212: CONTINUE
213: VOLCO=0.
214: DO 245 I=1,N-1
215: RAD=Z *R(I+1)*R(I)/(R(I+1)+R(I))
216: DELH=HN(I+1)-HN(I)
217: IF(DELH.LT.0.00001) DELH=0.0
218: VOLCO=VOLCO+PI*RAD*DELH
219: IF (IPRNT.GT.1) GO TO 1120
220: WRITE (44,1008)
221: FORMAT(1H1)
222: WRITE(44,1003) GO, ITNS,IRNG,ISEC,A,NDWRNO,FRPW,ICFW
223: FORMAT ("1",39X,"SOUTHWEST KANSAS GROUND WATER MANAGEMENT DISTRICT
224: & FOR"/56X,8A1,"COUNTY"/57X,"T",12,"S",12,"E",12,"/
225: 658X,20A1,"/1H0,1X,"DIVISION OF WATER RESOURCES NUMBER,"/1X,
226: 6"LOCATION OF 10 ACRE TRACT IN NINE SQUARE MILE AREA"/8X,"ROW",12,
227: 65X,"COLUMN",12)
228:
229: WRITE (44,1004) PROPAP.GINST,FUMDIA,RWT
230: FORMAT (1H0,2X,"PROPOSED ANN APPROP",1F9.0,"ACRE FEET"/1X,

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# Program Listing for Single-Well Option--Continued

27 OCT 81 10 21 01 MODULE \*MAIN\* OPTIONS SE SU WA LO SOURCE INPUT: SAPP.S  
 61516-02 ENHANCED SAU FORTRAN COMPILER REVISION LEVEL 08.01 PRE

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231:      &'PROPOSED DIVERSION RATE',F9.0,' GALLONS PER MINUTE',I1X,'PUMP DI
232:      &AMETER',F9.0,' INCHES',I3X,'PROPOSED WELL RADIUS',F9.0,' INCHES')
233:      WRITE (44,1005) PM,H,NW
234:      1005 FORMAT (1H0,10(' ')/4X,'AQUIFER PROPERTIES',/6X,'HYDRAULIC COND
235:      &UCTIVITY',F5.0,' GAL/DAY/SQ.FT',/6X,'SATURATED THICKNESS',F5.0,' F
236:      &EET',/10(' ')/1H0,20X,'NUMBER OF EXISTING WELLS IN NINE SQUARE MILE
237:      & AREA=',I2,I4X,'DWR NO.',7X,'LOCATION',7X,'ANNUAL APPROPRIATION',
238:      &4X,'DISTANCE FROM PROPOSED',/25X,'ROW',7X,'COLUMN',10X,'ACRE FEET',
239:      &15X,'WELL, IN FEET')
240:      DO 1007 I=1,NW
241:      WRITE (44,1006) DWRNO(I),IR(I),IC(I),AFROP(I),RTONWL(I)
242:      1007 CONTINUE
243:      1006 FORMAT (13X,I8.5X,I2.5X,I2.12X,F6.0,20X,F8.0)
244:      IF (QINST GT 800.) WRITE (44,8816)
245:      8816 FORMAT(1H0,100('*'),/1H0,10X,'MAXIMUM DIVERSION RATE EXCEEDED')
246:      IF (PERCT GT 10.) WRITE (44,8815)
247:      8815 FORMAT(1H0,100('*'),/1H0,10X,'WITH PROPOSED WELL APPROPRIATION WIL
248:      &L EXCEED 40%')
249:      WRITE (44,1010) VOLSTR,VOLAPR,TOTVAP,PERCT,PERCT,VOLLEF,VOLFT,
250:      &VLEFF,AVLEF
251:      1010 FORMAT(1H0,100('*'),
252:      &/1H0,45X,'WITHOUT PROPOSED WELL',
253:      &12X,'WITH PROPOSED WELL',/18X,'ACRE FEET IN STORAGE',/10X,F12.0/
254:      &16X,'ACRE FEET APPROPRIATED',/10X,F12.0,20X,F12.0/18X,
255:      &'PERCENT APPROPRIATED',/10X,F12.0,20X,F12.0/1X,
256:      &'ACRE FEET AVAILABLE FOR APPROPRIATION,'
257:      &/10X,F12.0,20X,F12.0/1X,'ACRE FEET AVAILABLE FOR APPROPRIATION',
258:      &/30X,'ANNUALLY',/10X,F12.0,20X,F12.0)
259:      IF (PERCT GT 40.) WRITE (44,8817) ADJAPR
260:      8817 FORMAT(1H0,20X,'APPROPRIATION ADJUSTED TO',F12.0,'ACRE FEET')
261:      1120 IF (IPRNT EQ 1) WRITE (44,1131)
262:      IF (IPRNT GT 1) WRITE(44,1130)
263:      1131 FORMAT (1H1,2X,'DRAWDOWN AFTER ONE YEAR DUE TO PUMPING PROPOSED WE
264:      &LL CONTINUOUSLY AT ANNUAL APPROPRIATION RATE')
265:      1130 FORMAT (1H1,2X,'DRAWDOWN DUE TO PUMPING ANNUAL APPROPRIATION FROM
266:      &PROPOSED WELL CONTINUOUSLY AT PROPOSED DIVERSION RATE')
267:      WRITE (44,1121)
268:      1121 FORMAT (1H0,6X,'DISTANCE',7X,'DRAWDOWN')
269:      DO 242 I=1,N
270:      DD=H-HN(I)
271:      WRITE(44,620) R(I),DD
272:      620 FORMAT(2F15.2)
273:      242 CONTINUE
274:      WRITE(44,653)
275:      653 FORMAT (1H0,13X,'EXISTING WELLS')
276:      DO 259 J=1,NW
277:      DO 255 I=2,N
278:      IF(R(I).GT RTONWL(J)) GO TO 257
279:      255 CONTINUE
280:      257 B=(HN(I)-HN(I-1))/ALOG(R(I)/R(I-1))
281:      DD=H-(HN(I)-B*ALOG(R(I)/RTONWL(J)))
282:      WRITE(44,620) RTONWL(J),DD
283:      259 CONTINUE
284:      Q=-QINST/(60.*7.481)
285:      TMAX=PROPAP*43560 /(-Q)
286:      THAXDA=TMAX/86400
287:      IF (IPRNT GT 1) WRITE (44,840) THAXDA
    
```

Program Listing for Single-Well Option--Continued

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27 OCT 81 10.21.01      MODULE: *MAIN*  OPTIONS: SE SU WA LO
61516-02 ENHANCED SAU FORTRAN COMPILER  REVISION LEVEL 08.01 PRE  SOURCE INPUT:  SAPP.S
288      840 FORMAT(F7.2,'DAYS PUMP AT DIV RATE: SHOULD NOT EXCEED 365.')
289      IF (1PRNT EQ 1) GO TO 888
290      WRITE (44,1122)
291      1122 FORMAT (1H0,1X,'MASS BALANCE,')
292      WRITE(44,610) VOLCO
293      610 FORMAT(' VOLUME OF CONE =',E15.4)
294      VOLVEL=PI*RW*RW*(H-HN(1))
295      VWATCO=S*(VOLCO--VOLWEL)
296      VOLWAT=VWATCO+VOLWEL
297      VOLPUM=-Q*TIME
298      WRITE(44,625) VOLPUM, VOLWAT
299      625 FORMAT(' VOL. PUMPED=',E15.4,' VOL. WATER FROM CONE=',E15.4)
300      WRITE (44,600) IT,NUMDT,TIME
301      600 FORMAT (' IT,NUMDT,TIME',,2I5,E12.2)
302      WRITE(44,8899)
303      8899 FORMAT (1H1)
304      8888 CONTINUE
305      IF(JCNT GT 1) GO TO 270
306      IF(TMAXDA GT 365 ) GO TO 270
307      GO TO 53
308      270 STOP
309      END
05072
05120
05126
05134
05152
05162
05177
05210
05215
05221
05227
05242
05270
05306
05324
05331
05340
05342
05350
05355
05357
05365

```

Program Listing for Township Option

```

/** *****
//STEP1 EXEC FTG1CG
//FORT.SYSIN DD *
    DOUBLE PRECISION TITLE
    DIMENSION AVSTK(24,24), VINST(24,24), VALAPP(24,24),
1 AVAAP(24,24),SATHK(24,24), DELX(24), DELY(24), TITLE(16)
    DATA DELX/24*5280./,DELY/24*5280./
    DATA TITLE/'AVERAGE ', 'SATURATE', 'D THICKN', 'ESS,FEET', 'VOLUME
1A', 'AVAILABLE', ' FOR APP', 'ROPRIATI', 'ON-25YRS', ', AC-FT ',
2 'ANNUAL V', 'OLUME AV', 'AILABLE ', 'FOR APPR', 'OPRIATIO',
3 'N, AC-FT'/
C*****
C          NUMBER OF COLUMNS IN INPUT ARRAY - JMX      (I2)
C          NUMBER OF ROWS IN INPUT ARRAY - IMX      (I2)
C          NORTHWEST MOST TOWNSHIP - IT      (I2)
C          NORTHWEST MOST RANGE - IR      (I2)
C          INITIALIZE MATRICES AND READ IN CONSTANTS FOR:
C          SPECIFIC YIELD - SY      (F5.0)
C          AREA IN NINE SQ MILES - A9SQMI (F5.0)
C          NUMBER YEARS FOR DEPLETION - TIME      (F5.0)
C          NO. YRS TIMES ANNUAL RECHARGE RATE - RCHGV (F5.0)
C          OPTIONAL PRINT OF OUTPUT IN MAP FORM - IOP      (I2)
C*****
C
    READ (5,100) IMX,JMX, IT, IR, SY, A9SQMI, TIME, RCHGV,IOP
    DO 10 I=1, IMX
    DO 11 J=1, JMX
    SATHK(I,J) = 0.0
    AVSTK(I,J) = 0.0
    VINST(I,J) = 0.0
    VALAPP(I,J) = 0.0
11 AVAAP(I,J) = 0.0
10 CONTINUE
    IMAP = 0
    DO 12 K=1, IMX
    READ (5,101) (SATHK(K,L), L=1,JMX)
12 CONTINUE
C***** COMPUTE SATURATED THICKNESS(AVE 9SQ.MI.) AND WATER AVAILABLE
    DO 13 I=1, IMX
    DO 14 J=1, JMX
    IF (I .EQ. 1 .OR. J.EQ.1 .OR. I.EQ.IMX .OR. J.EQ.JMX) GO TO 200
C***** AVERAGE SATURATED THICKNESS 9 SQ. MILES
    AVSTK(I,J) = (SATHK(I,J) + SATHK(I-1,J) + SATHK(I+1,J) +
1 SATHK(I-1,J-1) + SATHK(I,J-1) + SATHK(I+1,J-1) + SATHK(I-1,J+1) +
2 SATHK(I,J+1) + SATHK(I+1,J+1))/ 9.0
    GO TO 220
C
C
C*****
C
200 IF (I.EQ.1 .AND. J.EQ.1) GO TO 401
    IF (I.EQ.1 .AND. J.EQ.JMX) GO TO 402
    IF (I.EQ.IMX .AND. J.EQ.1) GO TO 403
    IF (I.EQ.IMX .AND. J.EQ.JMX) GO TO 404

```

Program Listing for Township Option--Continued

```

GO TO 445
C***      COMPUTATION FOR NW CORNER
C
401 AVSTK(I,J)=(SATHK(I,J) + SATHK(I,J+1) + SATHK(I+1,J) +
1 SATHK(I+1,J+1))/4.0
GO TO 220

C
C**       COMPUTATION FOR NE CORNER
C
402 AVSTK(I,J) = (SATHK(I,J) + SATHK(I,J-1) + SATHK(I+1,J-1) +
1 SATHK(I+1,J))/4.0
GO TO 220

C
C**       COMPUTATION FOR SW CORNER
C
403 AVSTK(I,J) = (SATHK(I,J) + SATHK(I-1,J) + SATHK(I-1,J+1) +
1 SATHK(I,J+1))/4.0
GO TO 220

C
C***      COMPUTATION OF SE CORNER
C
404 AVSTK(I,J) = (SATHK(I,J) + SATHK(I,J-1) + SATHK(I-1,J-1) +
1 SATHK(I-1,J))/4.0
GO TO 220

C
445 IF (I.EQ.1) GO TO 301
    IF (I.EQ.IMX) GO TO 302
    IF (J.EQ.1) GO TO 303
    IF (J.EQ.JMX) GO TO 304
GO TO 444

C
      COMPUTATION FOR NORTH SIDE OF MATRIX
301 AVSTK(I,J) = (SATHK(I,J) + SATHK(I,J-1) + SATHK(I,J+1) +
1 SATHK(I+1,J-1) + SATHK(I+1,J) + SATHK(I+1,J+1))/6.0
GO TO 220

C
      COMPUTATION FOR EAST SIDE OF MATRIX
304 AVSTK(I,J) = (SATHK(I,J) + SATHK(I-1,J) + SATHK(I+1,J) +
1 SATHK(I-1,J-1) + SATHK(I,J-1) + SATHK(I+1,J-1))/6.0
GO TO 220

C
      COMPUTATION FOR WEST SIDE OF MATRIX
303 AVSTK(I,J) = (SATHK(I,J) + SATHK(I-1,J) + SATHK(I+1,J) +
1 SATHK(I-1,J+1) + SATHK(I,J+1) + SATHK(I+1,J+1))/6.0
GO TO 220

C
C*****
C
C
      COMPUTATION FOR SOUTH SIDE OF MATRIX
302 AVSTK(I,J)=(SATHK(I,J) + SATHK(I-1,J) + SATHK(I,J-1) +
1 SATHK(I-1,J+1) + SATHK(I,J+1) + SATHK(I-1,J-1)) / 6.0

C
GO TO 220
210 AVSTK(I,J) = (SATHK(I,J) + SATHK(I+1,J) + SATHK(I,J+1) +
1 SATHK(I+1,J+1)) / 4.0
C***      WATER AVAILABLE
220 VINST(I,J) = AVSTK(I,J) * SY * A9SQMI
    VAVLB = VINST(I,J) + RCHGV

```

Program Listing for Township Option--Continued

```

      VALAPP(I,J) = VAVLB * 0.40
14  AVAAP(I,J) = VALAPP(I,J) / TIME
13  CONTINUE
C*****  MAP OUTPUT OPTION
      IF (IOP) 15,15,16
16  WRITE (6,102) IT,IR
C      DO 333 I=1,IMX
C      WRITE(6,335) (AVAAP(I,J), J=1,JMX)
CD 333 CONTINUE
C 335 FORMAT (1H0,6(3X,F6.0)/6(3X,F6.0)/6(3X,F6.0)/6(3X,F6.0))
      IMAP=IMAP + 1
      WRITE (6,102) IT,IR
      CALL VGMAP (AVSTK,IMX,JMX,DELX,DELY, 5280., TITLE,IMAP)
      IMAP=IMAP + 1
      WRITE (6, 102) IT, IR
      CALL VGMAP (VALAPP, IMX, JMX, DELX, DELY, 5280., TITLE, IMAP)
      WRITE (6, 102) IT, IR
      IMAP=IMAP + 1
      CALL VGMAP (AVAAP, IMX, JMX, DELX, DELY, 5280., TITLE, IMAP)
15  CONTINUE
      CALL REORD (AVSTK,VINST,VALAPP,AVAAP,IMX,JMX,IT,IR)
      GO TO 443
444 WRITE(6,103) I,J
443 CONTINUE
      STOP
100 FORMAT (4I2,4F5.0, I2)
101 FORMAT (20F4.0)
102 FORMAT (1H1, 'NORTHWEST MOST TOWNSHIP ',I2,1X,I2,'W')
103 FORMAT (1H1, 'MATRIX SELETION ERROR ', 2I2)
      END
      SUBROUTINE VGMAP(A,IMAX,JMAX,DELX,DELY,BASE,TITLE,IMAP)
C
C THIS SUBROUTINE PRINTS A VARIABLE GRID MAP
C
C ARGUMENTS:
C  A      - ARRAY TO BE PLOTTED
C  IMAX   - Y DIMENSION
C  JMAX   - X DIMENSION
C  DELY   - ARRAY OF DELTA Y'S
C  DELX   - ARRAY OF DELTA X'S
C  BASE   - DISTANCE WHICH CORRESPONDS TO 5X3 BOX ON PRINTER
C          (UNITS PER HALF INCH)
      DOUBLE PRECISION TITLE
      DIMENSION A(IMAX,JMAX),IA(25)
      DIMENSION DELX(JMAX),DELY(IMAX),TITLE(16)
      DIMENSION FMT0(9)
      DIMENSION FMT1(127),FMT2(11),FMT3(53),FMT4( 9),FMT5(104)
      DIMENSION LX1(25),LX2 (25),LX3 (25),LX4 (25)
      DATA FMT0/4H(4H(,4H4X ,4H 25,4H(2H,,4HI,I4,4H,1H,,4H,I4,,4H1HX),
      . 4H1H))/
      DATA FMT2/4H(4H(,4H 4X,,4H4H1H,4H+ , ,4H 25,4H(1H,,4H,I2,,4H1HX,,
      . 4H4H,1,4HH+),,4H1H))/
      DATA FMT4/4H(4H(,4H1XI,,4H4H2,,4H1X, ,4H 25,4H(2H,,4HI,I4,4H),1H,
      . 4H)) /

```

Program Listing for Township Option--Continued

```

DATA BLNK/'  '/
C
CALL JULDAT(NYR,MO,NDA)
C
C SCALE ARRAY VALUES TO 4 SIGNIFICANT DIGITS
C
Z=0.
DO 10 I=1,IMAX
DO 10 J=1,JMAX
10 Z = AMAX1(Z,ABS(A(I,J)))
S=1.
IF(Z.EQ.0.) GO TO 70
30 IF(Z-1000.) 40,70,50
40 S=S*10.
Z=Z*10.
GO TO 30
50 IF(10000.-Z) 60,60,70
60 S=S/10.
Z=Z/10.
GO TO 50
70 CONTINUE
C
C COMPUTE NODE SPACING PARAMETERS
C
KDX=0
200 CONTINUE
, SUMDX=0.
LENGTH=5
LX=0
N=0
JL=KDX+1
210 CONTINUE
SUMDX=SUMDX+DELX(KDX+1)
LDX =5.*SUMDX/BASE + 5.5 - LENGTH
LENGTH=LENGTH+LDX
IF(LENGTH.GT.130) GO TO 220
KDX=KDX+1
N=N+1
LX1(N)=LDX-1
LX3(N)=(LDX-2)/2
LX2(N)=LDX-LX3(N)
LX4(N)=(LDX+6)/2+LX
LX=(LDX-5)/2
IF(KDX.LT.JMAX) GO TO 210
220 CONTINUE
C
C CONSTRUCT FORMAT STATEMENTS
C
CALL CORE(FMT0(3),4)
WRITE(1,1010)N
1010 FORMAT(I4)
CALL CORE(FMT1,508)
WRITE(1,FMT0)(LX2(J),LX3(J),J=1,N)
CALL CORE(FMT2(5),4)
WRITE(1,1010)N

```

Program Listing for Township Option--Continued

```
CALL CORE(FMT3,212)
WRITE(1,FMT2)(LX1(J),J=1,N)
CALL CORE(FMT4(5),4)
WRITE(1,1010)N
CALL CORE(FMT5,416)
WRITE(1,FMT4)(LX4(J),J=1,N)
```

```
C
C PRINT ONE PAGE WIDTH OF MAP
```

```
C
SUMDY=0.
LENGTH=0
IF (IMAP .EQ.1) GO TO 9000
IF (IMAP .EQ.2) GO TO 9001
IF (IMAP .EQ.3) GO TO 9002
9000 WRITE(6,9010) (TITLE(I),I=1,4),S,MO,NDA,NYR
GO TO 9003
9001 WRITE(6,9011) (TITLE(I),I=5,10),S,MO,NDA,NYR
GO TO 9003
9002 WRITE(6,9011) (TITLE(I),I=11,16),S,MO,NDA,NYR
9010 FORMAT ( 5X, 4A8,5X,'(VALUES HAVE BEEN MULTIPLIED BY',1PE8.1,')',
.10X,2(I2,' - '),I2//)
9011 FORMAT ( 5X, 6A8,5X,'(VALUES HAVE BEEN MULTIPLIED BY',1PE8.1,')',
.10X,2(I2,' - '),I2//)
9003 WRITE(6,FMT1)(J,J=JL,KDX)
WRITE(6,FMT3)
DO 270 I=1,IMAX
SUMDY=SUMDY+DELY(I)
LDY =3.*SUMDY/BASE +.5 - LENGTH
LENGTH=LENGTH+LDY
ISP1=(LDY-1)/2
ISP2=LDY-2-ISP1
DO 240 K=1,ISP1
240 WRITE(6,6010)
6010 FORMAT()
L=0
DO 250 J=JL,KDX
L=L+1
250 IA(L)=A(I,J)*S+SIGN(.5,A(I,J))
WRITE(6,FMT5) I,(IA(J),J=1,L)
IF(ISP2.EQ.0) GO TO 270
DO 260 K=1,ISP2
260 WRITE(6,6010)
270 WRITE(6,FMT3)
WRITE(6,FMT1) (J,J=JL,KDX)
IF(KDX.LT.JMAX) GO TO 200
RETURN
END
SUBROUTINE REORD (BMRO,CMRO,DMRO,AMRO,IMX,JMX,IT,IR)
```

```
C
C***** THIS SUBROUTINE REORDERS A MATRIX OF COMPLETE TOWNSHIPS
C IN THE STANDARD GW DIGITAL MODEL FORMAT TO TOWNSHIP, RANGE
C AND SECTION ORDER
DIMENSION BMRO(24,24),CMRO(24,24),DMRO(24,24),AMRO(24,24),
1 RORDMX(5,5,36),BORDMX(5,5,36),CORDMX(5,5,36),DORDMX(5,5,36)
IRL = IMX/6
```

Program Listing for Township Option--Continued

```

ITL = JMX/6
N=1
DO 10 IRR=1, IRL
L=0
J=6
DO 11 JR = 1, ITL
L=L+1
IF(JR.NE.1)J=L*6
M=0
5 I=(N*6-6+1)
DO 12 KR =1, 36
RORDMX(IRR,JR,KR) = AMRO(I,J)
BORDMX(IRR,JR,KR)=BMRO(I,J)
CORDMX(IRR,JR,KR)=CMRO(I,J)
DORDMX(IRR,JR,KR)=DMRO(I,J)
M = M + 1
C-----ADDED 8-22-79-----
IF(KR.EQ.1 .OR. KR .EQ. 13 .OR. KR .EQ. 25) GO TO 23
IF(KR.EQ.7 .OR. KR.EQ.19 .OR. KR.EQ.31) GO TO 24
C-----
IF(J.EQ. 1 .OR. J.EQ.6)GO TO 22
IF(MOD(J,6).EQ.0)GO TO 22
IF(MOD(J-1,6) .EQ.0)GO TO 22
C-----LABLED 8-23-79-----
23 IF (MOD (I,2).NE.0) J=J-1
C-----
24 IF(MOD (I,2).EQ.0)J=J+1
22 IF(M.EQ.6 .OR.M .EQ.12 .OR. M .EQ.18 .OR. M.EQ.24 .OR. M.EQ.30)
1 I=I+1
12 CONTINUE
11 CONTINUE
10 N=N+1
C
C * WRITE REORDERED MATRIX
IRH= IR
DO 50 I=1, IRL
IR= IRH
IF (I.NE.1) IT= IT+1
DO 51 J=1, ITL
IF (J.NE.1) IR= IR-1
WRITE (6,40) IT,IR
DO 52 K=1,36
WRITE(6,41) K,BORDMX(I,J,K),CORDMX(I,J,K),DORDMX(I,J,K),
1 RORDMX(I,J,K)
IF(MOD(K,6).EQ.0)WRITE(6,43)
C WRITE(7,42) IRORD
52 CONTINUE
51 CONTINUE
50 CONTINUE
40 FORMAT (1H1,54X,'GROUND WATER AVAILABILITY'/46X,'BASED ON NINE SQU
1ARE MILE DEPLETION MODEL'/1H0,10X,'SOUTHWESTERN KANSAS GROUNDWATER
2',54X,'U.S.GEOLOGICAL SURVEY'/12X,'MANAGEMENT DISTRICT NO.3'///11X
3,'TOWNSHIP ',12,'S',' RANGE ',12,'W'/49X,'AVERAGE',11X,'VOLUME IN',
4,7X,'VOLUME AVAILABLE',4X,'ANNUAL VOLUME'/42X, 'SATURAT
5ED THICKNESS',6X,'STORAGE',9X,'IN 25 YEARS',10X,'AVAILABLE'/30X,

```

Program Listing for Township Option--Continued

```
6 'SECTION',11X,'(FEET)',11X,'(ACRE-FEET)',9X,'(ACRE-FEET)',  
77X,'(ACRE-FEET)'/)  
41 FORMAT (33X,I2,14X,F5.1,13X,F10.1,8X,F10.1,12X,F6.1)  
42 FORMAT (13I6)  
43 FORMAT (1H )  
RETURN  
END
```

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