

HYDROGEOLOGY OF WELL-FIELD AREAS NEAR TAMPA, FLORIDA, PHASE 2--  
DEVELOPMENT AND DOCUMENTATION OF A QUASI-THREE-DIMENSIONAL FINITE-  
DIFFERENCE MODEL FOR SIMULATION OF STEADY-STATE GROUND-WATER FLOW  
By C. B. Hutchinson

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U.S. GEOLOGICAL SURVEY

Water-Resources Investigations Report 84-4002

Prepared in cooperation with the  
SOUTHWEST FLORIDA WATER MANAGEMENT DISTRICT



Tallahassee, Florida

1984

UNITED STATES DEPARTMENT OF THE INTERIOR

WILLIAM P. CLARK, Secretary

GEOLOGICAL SURVEY

Dallas L. Peck, Director

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For additional information  
write to:

District Chief  
U.S. Geological Survey  
Suite 3015  
227 North Bronough Street  
Tallahassee, Florida 32301

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FOR SIMULATION OF STEADY-STATE GROUND-WATER FLOW

By C. B. Hutchinson

ABSTRACT

A quasi-three-dimensional finite-difference model was developed for simulation of steady-state ground-water flow in two aquifers throughout a 932-square-mile area that contains 10 municipal well fields. In the model, the surficial aquifer is unconfined and is hydraulically connected to the underlying Floridan aquifer by a leakage term that represents flow through a confining layer separating the two aquifers. Utilization of the head-controlled flux condition allows head and flow in the Floridan aquifer to vary at model-grid boundaries. The water table is held constant at model-grid lateral boundaries and at large surface-water bodies, but is allowed to fluctuate elsewhere in response to changes in evapotranspiration, recharge, and leakage.

Procedures are described to calibrate the model, test its sensitivity to input-parameter errors, and validate its accuracy for predictive purposes. Also included are attachments that describe operation of the model. Example model-interrogation runs simulate water-level and water-balance changes that can be expected as a result of pumping all 10 well fields simultaneously at annual average permitted rates totaling 186.9 million gallons per day from the Floridan aquifer with recharge varying 20 percent more and less than the long-term average rate. Maps are also presented that estimate the extent and depth of cones of depression in the water table and potentiometric surface around well fields as they are pumped individually.

Maximum drawdown in the Floridan aquifer simulated by the quasi-three-dimensional model is greater in every well field and averages about 4 feet more than maximum drawdown simulated by the two-dimensional model previously developed for the area. Under average recharge conditions, about 75 percent of the pumped water is derived by increasing downward leakage. The remaining 25 percent is gained by reducing natural upward leakage in swamp and marsh areas and by slightly reducing outflow along the model boundary. Ultimately, more than 95 percent of the pumped water is derived by reducing evapotranspiration and surface discharge from the water table.

When well fields are pumped individually, drawdown in the potentiometric surface is less than when all 10 well fields are pumped simultaneously. Drawdown is much greater in the center of the modeled area where well fields are in close proximity to one another, thus increasing interference effects. Interference effects range from about 0.1 foot of additional drawdown at Morris Bridge well field to 6.1 feet of additional drawdown at Northwest well field.

## INTRODUCTION

2 Ten municipal well fields have been established or are planned for a 932-mi<sup>2</sup> area north of Tampa, Fla. (fig. 1). Permits have been granted or are being considered for a combined average withdrawal rate of 186.9 Mgal/d (Southwest Florida Water Management District, written commun., 1982). In addition, several well fields for large housing subdivisions are being developed or are planned for development. Ground-water withdrawals from the Floridan aquifer in this area may eventually total several hundred million gallons per day.

A ground-water flow model that encompasses the well-field areas is needed to gain an understanding of the hydrology and to facilitate planning for the efficient utilization of water resources while conserving the environment. The model may be interrogated under various water-management alternatives to simulate water-level changes in the Floridan and surficial aquifers. The simulation runs may be used to assess adverse impacts of pumping, such as excessive draw-down, potential for saltwater encroachment, or destruction of wetlands.

The objective of this investigation is to evaluate the hydrogeology of an area encompassing the major well fields north of Tampa through development of a finite-difference digital ground-water flow model. The investigation includes two phases:

1. Develop and document a steady-state model with two-dimensional flow in one active layer.
2. Develop and document a steady-state quasi-three-dimensional model with two-dimensional flow in two active layers using the phase 1 model as a working base.

The phase 1 two-dimensional model is described by Hutchinson and others (1981). That model assisted in providing data for ground-water resource management decisions during development of the phase 2 model. Certain descriptions of the study area and modeling approach in this report are identical to or supercede those in the phase 1 report.

This report describes development of the phase 2 model and is intended as a guide for using the quasi-three-dimensional model. The aquifer system north of Tampa is conceptualized and formulated in the hydrologic model. The computer program and its application to a typical field problem in west-central Florida are described. The applicability of the model to the field problem is demonstrated through three interrogation runs.

The documentation assumes that the reader is familiar with the physics of ground-water flow, numerical methods of solving partial-differential equations, and the FORTRAN IV computer language. The report was prepared as part of a hydrogeologic investigation made by the U.S. Geological Survey in cooperation with the Southwest Florida Water Management District.



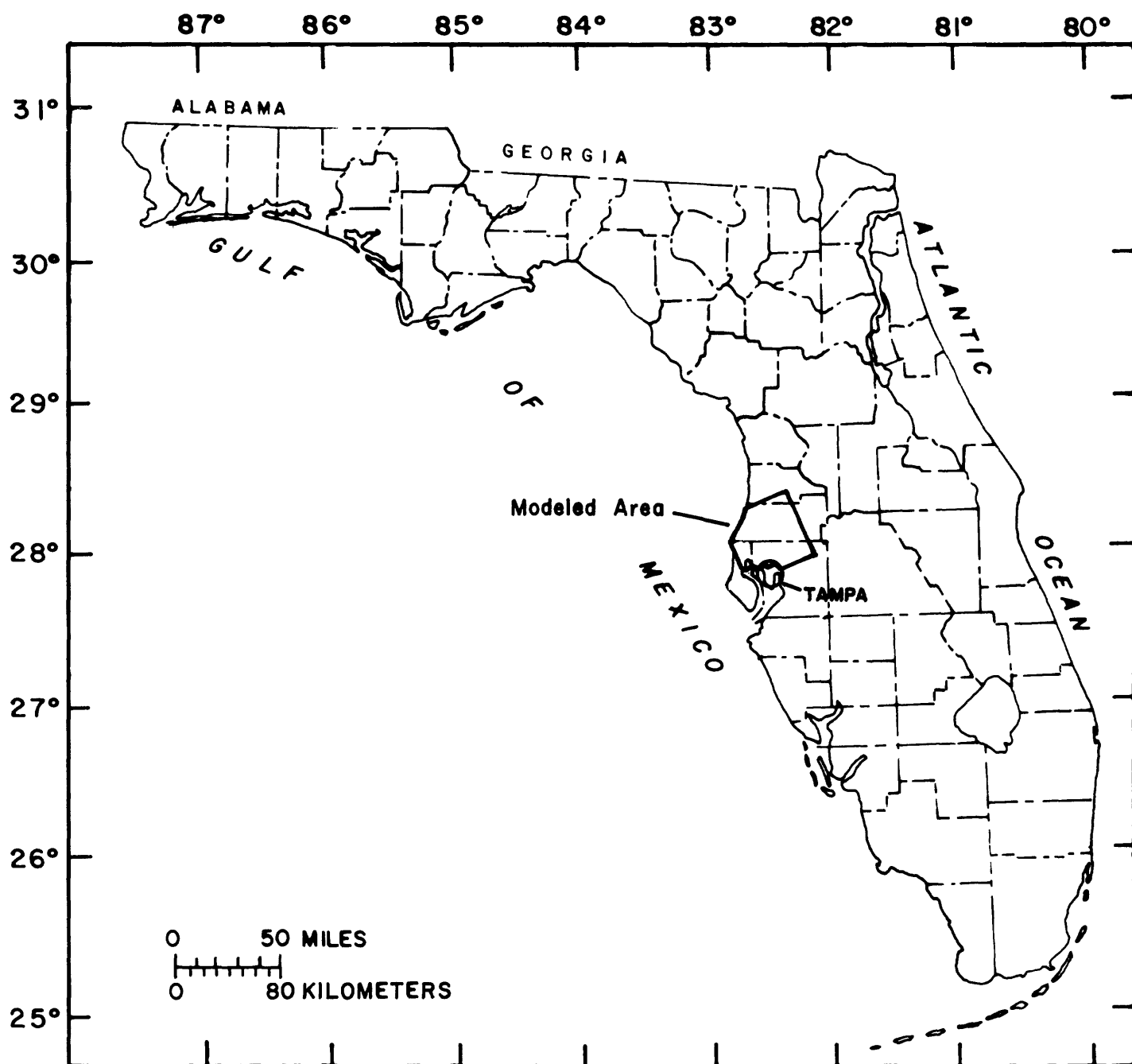


Figure 1.--Location of the modeled area near Tampa, Florida.

## PREVIOUS INVESTIGATIONS

Numerous ground-water flow models of the Floridan aquifer have been or are being constructed that include all or some of the well-field areas north of Tampa (fig. 2). Cherry and others (1970) developed a conceptual model of the ground-water flow regime in the middle Gulf area. Robertson and Mallory (1977) constructed a regional model for an 875-mi<sup>2</sup> area that included eight major well fields. Individual well-field models have been constructed for the Cypress Creek (Seaburn and Robertson, Inc., 1977; Ryder, 1978), Morris Bridge (Ryder and others, 1980), and Cross Bar Ranch (Leggette, Brashears, and Graham, Inc., 1979) well fields. A well-field model is being constructed by the U.S. Geological Survey for the Cross Bar Ranch well field. A regional model, with relatively large grid-spacing (4-mile centers), covering an area of about 10,000 mi<sup>2</sup> and including the study area, has been constructed by the U.S. Geological Survey (Ryder, 1982). A companion report to this one (Hutchinson and others, 1981) describes a two-dimensional flow model of the Floridan aquifer in the well-field areas near Tampa. All the above modeling reports give detailed information concerning the hydrogeology of the area.

The model documented herein expands the Robertson and Mallory (1977) model area, includes subsequent aquifer-test results, and incorporates information from the individual well-field models. The model grid is aligned with Ryder's (1982) coarsely gridded regional model so that they may be interfaced.

## HYDROLOGIC MODEL

### Description

The modeled area and its relation to the 10 municipal well fields are shown in figure 2. The model grid comprises an orthogonal array of 34 horizontal rows and 36 vertical columns; each grid block is 1 mile square. At the center of each grid block is a node through which data are input to or output from the model. Along the Gulf of Mexico and Tampa Bay coasts, the grid generally follows the shoreline.

The hydrologic setting is one of a coastal, karstic environment. The hydrologic system is represented by an unconfined surficial aquifer separated from the underlying Floridan aquifer by a relatively impermeable confining bed. The landscape is dotted with sinkhole depressions and the water table is near land surface. The general direction of ground-water movement is west toward the Gulf of Mexico and south toward Tampa Bay. The regional flow regime is modified by pumping from the well-field areas and by ground water discharging to streams.

### Hydrologic Cycle

The elements of the hydrologic cycle in west-central Florida are rainfall, surface and subsurface runoff, evapotranspiration (ET), leakage to or from the Floridan aquifer, pumpage, and changes in amounts of water in storage in the

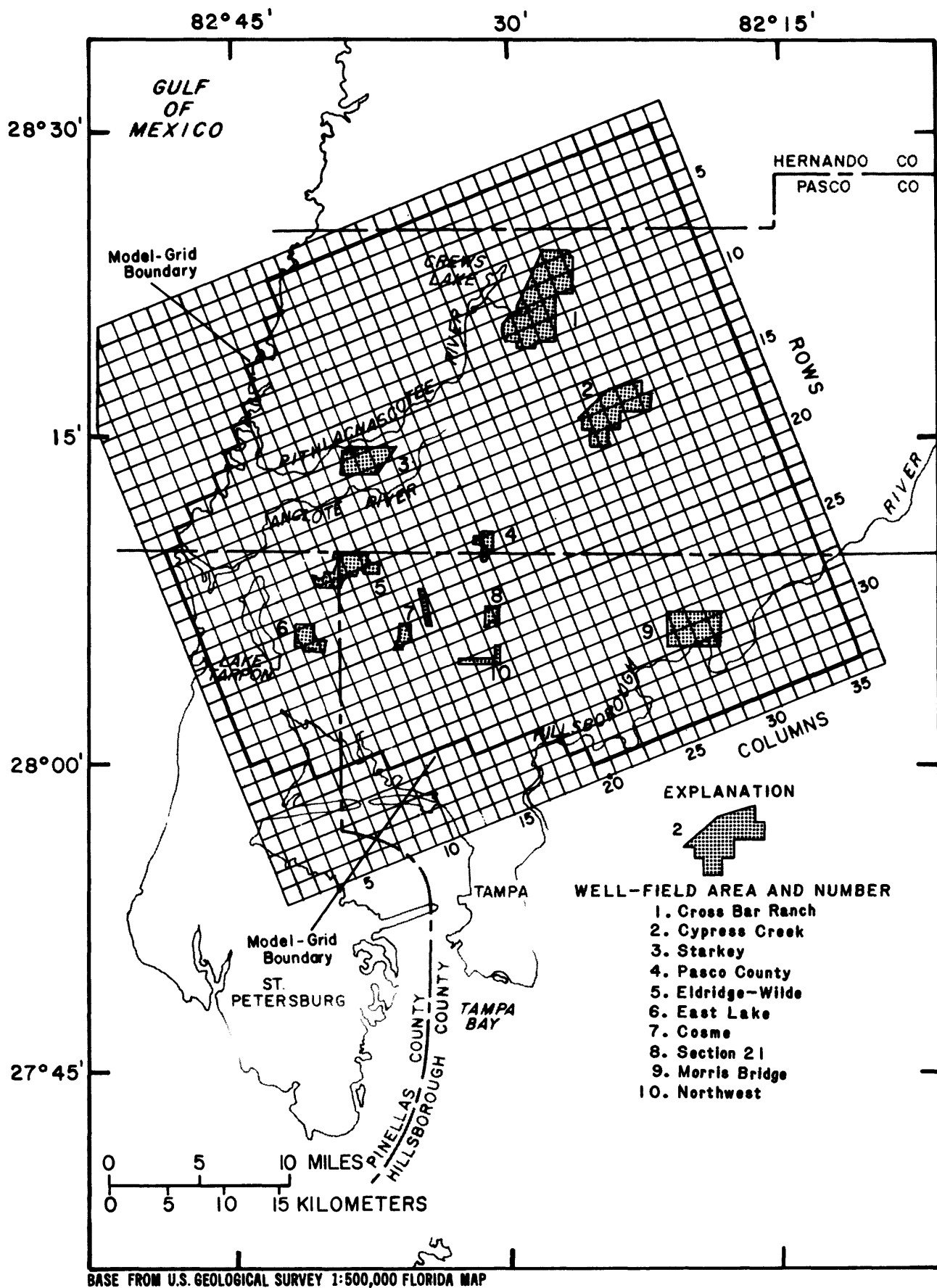


Figure 2.--Model grid and well-field areas.

surficial and Floridan aquifers. In this study, all time-dependent hydrologic parameters including ground-water levels are considered to be long-term averages, therefore, short-term fluctuations in amounts of water in storage in the surficial and Floridan aquifers are neglected. Pumpage from the surficial aquifer is so small that it is neglected.

In west-central Florida, mean annual rainfall is about 55 inches and is distributed unevenly as 7 inches in winter, 10 inches in spring, 25 inches in summer, and 13 inches in autumn (Hughes and others, 1971). At St. Leo, in east-central Pasco County, the extreme low rainfall of 36.61 inches fell in 1961, and the extreme high rainfall of 81.93 inches was recorded in 1945 (National Oceanic and Atmospheric Administration, 1958-81; and U.S. Department of Commerce, 1964).

Annual runoff ranges from near zero in internally drained areas to about 14 inches in the Anclote River basin and, in general, is directly proportional to rainfall. Under normal rainfall conditions, with no pumping, runoff from the modeled area probably averages about 10 inches per year. Five inches can be considered as overland runoff and 5 inches as contribution to base streamflow. Cherry and others (1970) indicate that up to 20 percent of the runoff from watersheds in the modeled area is derived from upward leakage from the Floridan aquifer. Parker (1975) estimated that average runoff from the Brooker Creek watershed, just east of Lake Tarpon, had declined substantially (possibly 50 percent) since the Eldridge-Wilde, Cosme, and Section 21 well fields were installed.

Evapotranspiration is a major item in the hydrologic cycle. It occurs in essentially three modes: (1) from plant surfaces and bare ground, (2) from the unsaturated zone (above the water table but beneath land surface), and (3) directly from the water table. The maximum potential evapotranspiration from a free water surface in west-central Florida is about 46 to 50 inches per year (Koehler and others, 1959; Dohrenwend, 1977). However, potential evapotranspiration is not maximum over all of west-central Florida because in much of the area the water table is below land surface and, in some areas, below plant root zones. In areas where the water table is far below land surface, water in the surficial aquifer is less subject to uptake by plants (transpiration) or direct evaporation from the water table than where the water table is at land surface and acts as a free water surface.

No matter how far below land surface the water table stands, there most likely is some minimum or base rate of evapotranspiration. This base rate is determined by evaporation and transpiration that takes place before any water can percolate to the water table. Estimates of this base rate of evapotranspiration range from 25 to 35 inches per year (Tibbals, 1978).

The actual evapotranspiration rate depends upon depth to water table, soil type, type of plant community, humidity, the amount of incoming energy (sunlight and wind), and the availability of water subject to evapotranspiration. On an areal and long-term annual basis, humidity, incoming energy, and available water can be regarded as fairly constant and uniformly distributed in west-central Florida. Soil types and plant communities are not uniformly distributed. For modeling purposes, these differences are not considered major factors in determining variability of actual evapotranspiration because depth to water table helps determine the plant community and the soil type. Therefore, depth to water table is used as the indicator of the actual rate of evapotranspiration.

The Floridan aquifer is recharged about 6 inches annually by downward leakage from the surficial aquifer. In swampy areas, about 1 inch of water discharges from the Floridan aquifer by leaking upward to the surficial aquifer. The net leakage is downward and is estimated to be roughly 5 inches per year under nonpumping conditions, based on a digital model of predevelopment flow developed by Ryder (1982). Pumping lowers the potentiometric surface, thereby inducing additional leakage from the surficial aquifer. For the calibration period, pumping 133 Mgal/d was estimated to have increased leakage to about 9 inches per year.

### Conceptual Model

A generalized conceptual model of the hydrologic system is shown schematically in figure 3. The Floridan aquifer is the principal source of ground-water supply. It is confined above and below and is overlain by the unconfined surficial aquifer.

Gross water budgets for each aquifer were conceptualized as a basis for modeling the hydrologic system. Inflows and outflows from each aquifer under steady-state conditions are equated as follows:

	<u>INFLOW</u>		<u>OUTFLOW</u>	
<u>SURFICIAL:</u>	R + UL	=	ETRO + DL	(1)
<u>FLORIDAN:</u>	DL + BI	=	UL + BO + P	(2)

where      R = recharge by seepage of rainfall;  
             UL = upward leakage through the upper confining bed;  
             ETRO = evapotranspiration plus runoff from the water table;  
             DL = downward leakage through the upper confining bed;  
             BI = boundary inflow;  
             BO = boundary outflow; and  
             P = pumpage.

Under normal climatological conditions, with no pumping, total inflow to the surficial aquifer averages about 26 inches per year. About 1 inch leaks upward from the Floridan aquifer, and about 25 inches is recharge computed as the residual of rainfall (55 inches) minus overland runoff (5 inches) and minimum evapotranspiration (25 inches from plant surfaces, bare land, and the unsaturated zone). About 6 inches leaks downward from the surficial aquifer to the Floridan aquifer and 5 inches seeps to streams. The remaining 15 inches of inflow is lost from the aquifer as evapotranspiration from the water table.

The Floridan aquifer receives inflow by downward leakage and across the boundary from outside the model grid. Under nonpumping conditions, downward leakage averages about 6 inches per year and boundary inflow about 1 inch per year. This water is lost through upward leakage at a rate of about 1 inch per year and boundary outflow of about 6 inches per year.

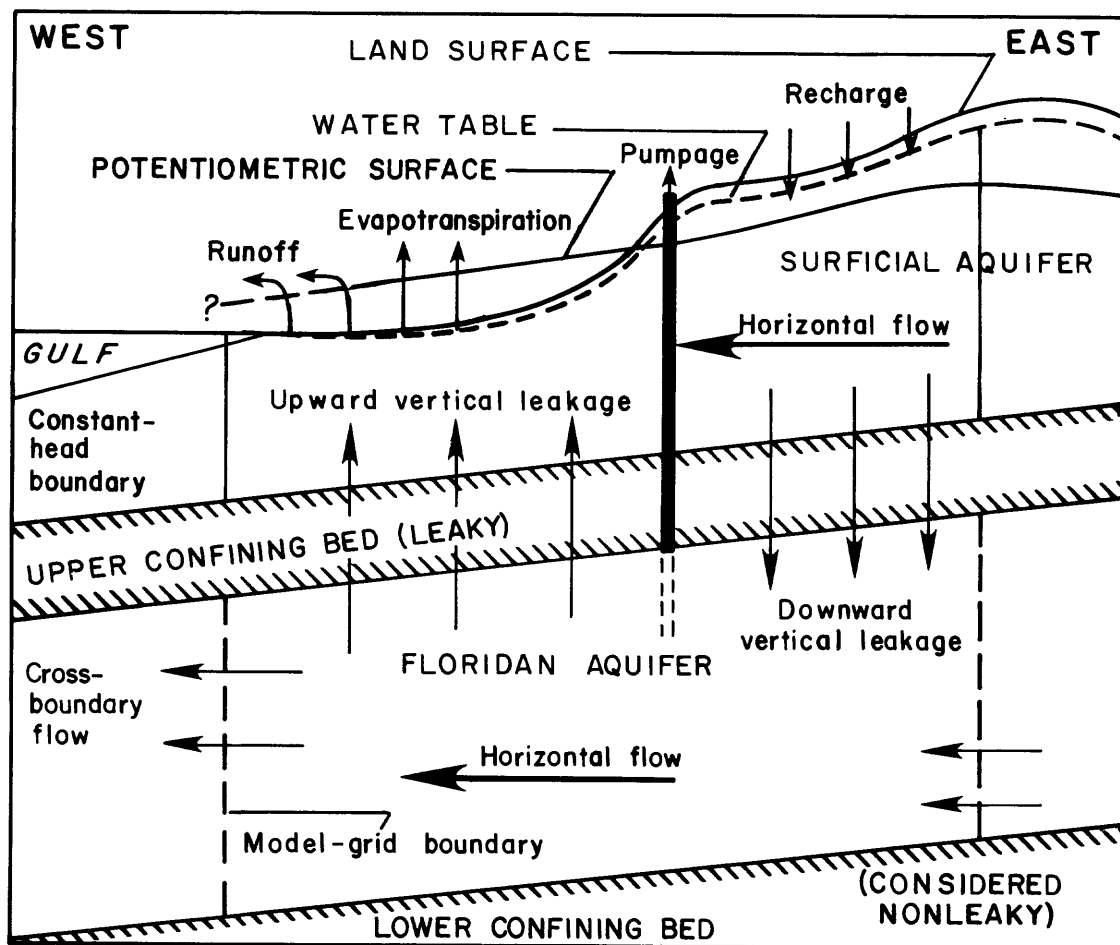


Figure 3.--Generalized conceptual model of the hydrogeologic system (modified from Wilson and Gerhart, 1980).

The water balance within the surficial aquifer may be altered significantly by pumping. Recharge may be increased, while surface-water runoff, ET, and ground-water discharge from the surficial aquifer (referred to hereafter as ET-runoff) are reduced. The mechanism in the model for handling these changes is the ET-runoff capture rate that relates the rate of capture to water-table depth. Based on the model conceptualization, all ET from the water table (15 inches) plus approximately half the runoff (6 inches) could be salvaged by lowering the water table from its average depth of about 4.5 feet to 10 feet. From a management standpoint, this may not be practical because widespread lowering of water levels could dry up lakes, alter the natural vegetation, cause pump failure in shallow wells, and induce sinkhole development. Although the ET-runoff capture rate has not been documented by field studies, results of this investigation indicate that for each foot of water-table decline about 3.8 inches of water may be salvaged.

The ET-runoff capture rate and depth probably vary within the modeled area, but for lack of validation, they were held constant. Instead, the model calibration was based on varying recharge to the surficial aquifer. Because ET-runoff capture is based on reducing water-table ET and runoff as the water table declines, recharge should approach maximum potential rates. In internally drained areas, recharge should not exceed rainfall (55 inches per year) minus minimum ET from plant surfaces and the unsaturated zone (25 inches per year), or about 30 inches per year.

The model apportions recharge to leakage and ET-runoff using the ET-runoff capture function, which is the quotient of the maximum capture rate divided by maximum capture depth. For example, if recharge is 20 inches per year and downward leakage is 5 inches per year under nonpumping conditions, the model will allocate 15 inches per year as ET-runoff. If pumping increases leakage from 5 inches per year to 12.6 inches per year, then the water table will drop 2 feet to capture the 7.6-inch-per-year leakage increase, and ET-runoff will be reduced from 15 inches per year to 7.4 inches per year. Should pumping capture all the 15-inch-per-year ET-runoff reserve, then the total recharge of 20 inches per year will leak down to the Floridan aquifer. Further pumping increases will not capture additional ET or runoff, with the result being accelerated water-table declines.

Six physiographic units were delineated (fig. 4) to conceptualize recharge to, evapotranspiration and leakage from, and transmissivity of the surficial aquifer:

<u>Physiographic unit</u>	<u>Area (mi<sup>2</sup>)</u>	<u>Recharge</u>	<u>Evapotrans- piration</u>	<u>Leakage</u>	<u>Transmis- sivity</u>
1. Coastal marsh	46	Low	High	Low	Low
2. Coastal sand ridge	19	Moderate	Low	High	High
3. Lowlands plain	561	Moderate	Moderate	Moderate	Moderate
4. Lakes terrace	156	High	Moderate	High	Moderate
5. Central swamp	84	Low	High	Low	Low
6. Brooksville ridge	66	Moderate	Low	High	High

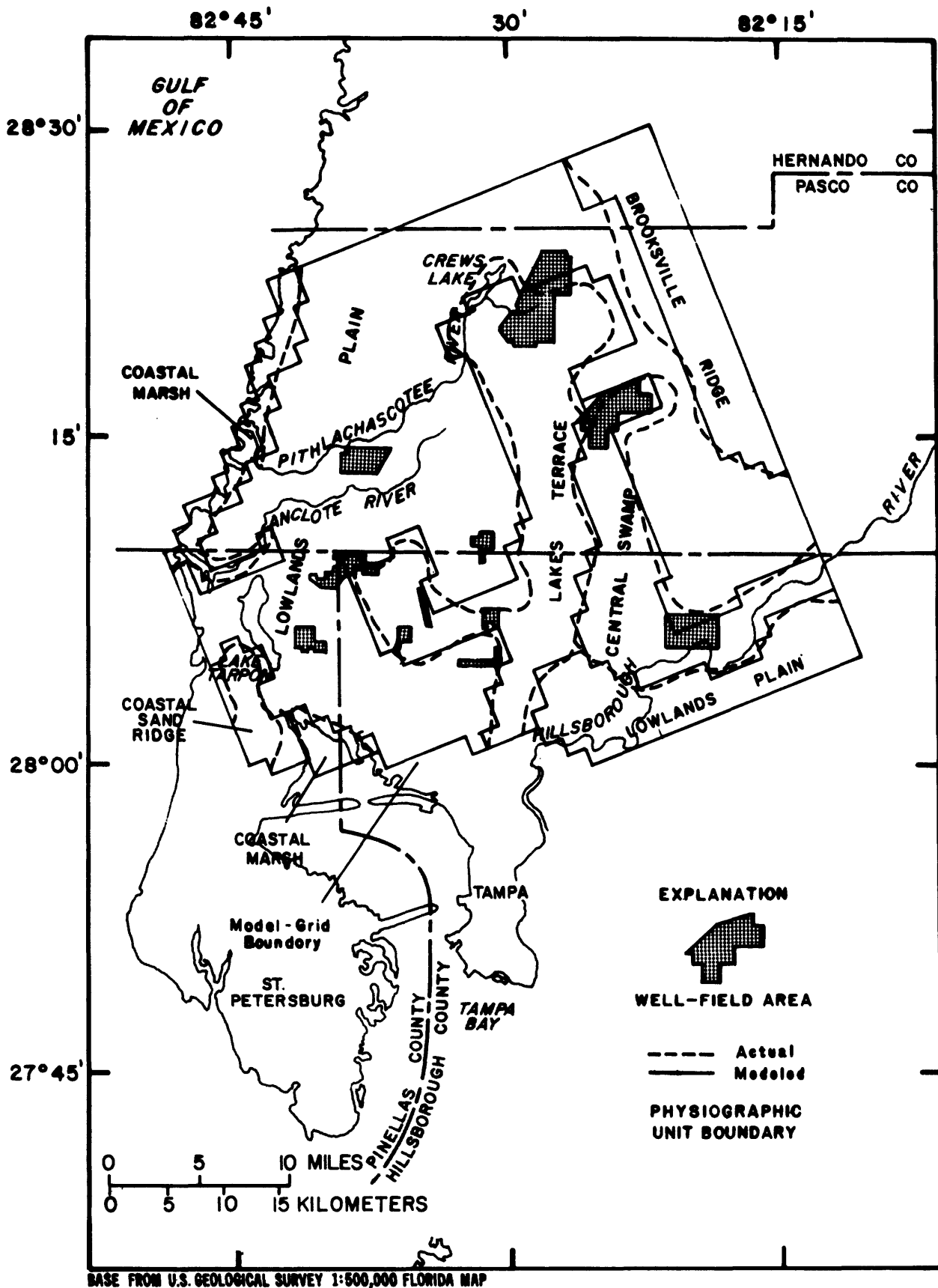


Figure 4.--Physiography of the modeled area.



Recharge to the surficial aquifer was considered to be low in marsh and swamp areas where the high water table is maintained by upward leakage from the Floridan aquifer; moderate in the moderately drained lowlands plain and the well-drained ridge areas; and high in the internally drained lakes terrace where hundreds of sinkhole lakes perforate the upper confining bed. Evapotranspiration from the water table was considered to be low along ridge areas where the water table is deep; moderate in the lowlands plain and lakes terrace where the water table is shallow; and high in marshy and swampy areas where the water table is at or near land surface. Leakage to the Floridan aquifer was considered to be low in the marsh and swamp areas where the water table generally lies below the potentiometric surface; moderate in the lowlands plain where the water table lies a few feet above the potentiometric surface; moderate in the ridge areas where the head difference between the water table and potentiometric surface is high, but the upper confining bed is relatively thick; and high in the lakes terrace where sinkholes increase the leakage rate. Transmissivity of the surficial aquifer is very low relative to transmissivity of the Floridan aquifer, but was considered to be lowest in the marsh and swamp areas where the sand is thin; moderate in the lowland plain and lakes terrace where the sand is moderately thick; and highest in the ridges where the saturated thickness is greatest.

### Modeling Procedure

Modeling procedures consisted of selecting representative input parameters, adjusting them within a reasonable range of values during calibration, testing the model's sensitivity to errors in the input parameters, and running the calibrated model under a separate set of pumping and climatological conditions to validate its accuracy. Prior to implementation of these modeling procedures, the adequacy of a steady-state model was evaluated and a time period was selected for the calibration.

Aquifer tests at the major well fields indicate that water levels in the Floridan aquifer rapidly stabilize. All the water pumped from the Floridan aquifer is soon accounted for by an increase in downward leakage from or a reduction in upward leakage to the surficial aquifer. Stewart (1968, p. 171 and 187) confirms that the potentiometric surface at the Section 21 and Eldridge-Wilde well fields approaches steady-state in less than 100 days of pumping.

Because the aquifer system approaches a steady-state condition shortly after pumping begins, a transient-state model of short-term water-level changes was deemed unnecessary. For a large region containing several well fields, a steady-state model representing long-term average conditions should adequately and simply portray the effects of pumping.

The time period selected for the steady-state calibration was May 1976-April 1977. For this period, average stream discharge of eight watersheds in the modeled area was 4.3 inches per year, or 70 percent of long-term average. Rainfall recorded by the National Oceanic and Atmospheric Administration at four sites (Tampa, Tarpon Springs, Cosme well field, and St. Leo) averaged 48.64 inches, or about 88 percent of the 55-inch long-term average. Average water levels in the Floridan and surficial aquifers were considered to be slightly below normal, based upon streamflow and climatological conditions.

Recharge to the surficial aquifer was considered to be directly proportional to rainfall. For the calibration period, recharge was assumed to be 91 percent of normal. The modeling procedure for subsequent predictive runs, representative of long-term average climatological conditions, included increasing recharge by 10 percent above that determined through calibration.

The hydrologic model assumes that:

1. Ground-water movement in the surficial and Floridan aquifers is horizontal.
2. Water moves vertically into and out of the Floridan aquifer through the upper confining bed.
3. The upper confining bed has negligible storage.
4. Changes in ground-water storage in the surficial and Floridan aquifers occur instantaneously with changes in hydraulic head.
5. Transmissivity of the Floridan aquifer and leakance coefficient of the upper confining bed do not change with time.
6. Head changes in the surficial and Floridan aquifers caused by an imposed stress will eventually stabilize; that is, a condition of steady state will be reached.
7. Constant-head conditions accurately represent the hydrologic conditions of the surficial aquifer at the model-grid boundary and Lake Tarpon.
8. Head-controlled flux (HCF) conditions accurately represent the hydrologic conditions of the Floridan aquifer near the model-grid boundary.
9. Recharge to and evapotranspiration from the surficial aquifer occur instantaneously.
10. Movement of the saltwater-freshwater interface has little or no effect on computed heads.

#### Input Parameters

The steady-state model requires input parameters for each grid block including:

1. Altitude of the observed potentiometric surface of the Floridan aquifer;
2. Altitude of the observed water table in the surficial aquifer;
3. Storage coefficient of the Floridan aquifer (defined as zero);
4. Storage coefficient (defined as zero) and constant-head nodes of the surficial aquifer;
5. Transmissivity of the Floridan aquifer;
6. Leakance coefficient of the upper confining bed;
7. Hydraulic conductivity of the surficial aquifer;
8. Altitude of the bottom of the surficial aquifer;
9. Recharge rate to the surficial aquifer;

10. HCF condition leakage factor for the Floridan aquifer;
11. Maximum evapotranspiration-runoff capture rate from the water table divided by maximum depth at which evapotranspiration-runoff capture occurs;
12. HCF condition head factor for the Floridan aquifer;
13. Altitude of the bottom of the zone in which evapotranspiration occurs;
14. Altitude of land surface;
15. Model-grid spacing; and
16. Pumping rate from the Floridan aquifer.

The model utilizes many input parameters directly in ground-water flow equations. Others are used indirectly to compute parameters that vary with head, such as transmissivity of the surficial aquifer, evapotranspiration rate, or boundary flux. Ranges for the model parameters are presented in table 1.

Since 1971, the U.S. Geological Survey has prepared maps showing the potentiometric surface of the well-field areas for each May and September that represent seasonal low and high water-level periods, respectively. Water-levels shown on these maps may be considered to represent levels of the potentiometric surface at the trough and peak of an annual water-level hydrograph. Of the available maps, those for September 1976 and May 1977 (Ryder and Mills, 1977a; 1977b) were considered to best represent high and low water-level conditions, respectively. The average potentiometric surface, derived from the two maps for input to the model, was considered to represent the average steady-state potentiometric surface for the calibration period.

The water table in the surficial aquifer was mapped using field measurements of wells and estimates from topographic maps. It was estimated to be at or a few feet below land surface in swampy areas and the lakes terrace and at depths greater than about 5 feet below land surface for the lowlands plain and ridge areas. The water table is lower than the potentiometric surface over a 177-mi<sup>2</sup> area (20 percent of the modeled area). In this primarily swampy area, upward flow occurs from the Floridan aquifer to the surficial aquifer and streams. Within the ridge areas, the water table is 20 to 100 feet higher than the potentiometric surface.

The storage coefficient for each aquifer was set at zero. Because the model represents steady-state, or stabilized aquifer conditions, inflows and outflows balance and there is no change in ground-water storage. Setting the storage coefficient matrices to zero in the model is for computational efficiency so that steady state can be reached in one time step.

The storage coefficient matrix is also used to assign constant-head values to nodes, such as at the model boundary, where water levels are not expected to change. Because the model will allow heads to change at the model-grid boundary in the Floridan aquifer (by means of the HCF condition), constant-head nodes are not designated in this modeled layer. Pumping from the Floridan aquifer is expected to have little impact on the water table in the surficial aquifer at the edges of the model; therefore, constant-head boundary conditions were assigned in the surficial aquifer. Even if head changes in grid blocks adjacent to the boundary are large, changes in lateral boundary flow would be negligible due to an aquifer transmissivity of only about 300 ft<sup>2</sup>/d. For example, if a 10-foot

Table 1.--Values for hydrologic parameters of the calibrated steady-state model

Parameter	Value	Source of data
Potentiometric-surface altitude.	1-87 ft	Ryder and Mills (1977a; 1977b).
Water-table altitude.	0-160 ft	Ryder and Mills (1977a; 1977b).
Storage coefficient, both aquifers.	0	----
Transmissivity of Floridan aquifer.	25,900-475,000 ft <sup>2</sup> /d	Published aquifer-test results.
Transmissivity of surficial aquifer.	74-359 ft <sup>2</sup> /d	Model computed, based on hydraulic conductivity measurements of Sinclair (1974).
Leakance coefficient of upper confining bed.	0.00015-0.0008 (ft/d)/ft	Published aquifer-test results.
Hydraulic conductivity of surficial aquifer.	10 ft/d	Sinclair (1974).
Altitude of the bottom of the surficial aquifer.	-10-(+130) ft	Wolansky and others (1979).
Saturated thickness of surficial aquifer.	7.4-35.9 ft	Model computed, based on difference between water table and estimated bottom of aquifer.
Recharge rate to surficial aquifer.	9-28 in/yr	Estimated by summing leakage and ET-runoff from water table.
Floridan aquifer boundary flux.		Model computed.
ET-runoff rate from water table.	0-38 in/yr	Model computed.
Altitude of land surface.	0-200 ft	USGS topographic maps.
Pumping rate from Floridan aquifer at individual nodes.	0-7,920,000 gal/d	SWFWMD water-use permits, pumping reports, and irrigation requirements.
Total pumping rate from Floridan aquifer.	133,400,000 gal/d	----

head change were to occur, by Darcy's formula, boundary flow would change by only 15 gal/min along the 1-mile-long face of the grid block adjacent to the boundary ( $\Delta Q = 300 \text{ ft}^2/\text{d} \times 10 \text{ ft}/\text{mi} \times 1 \text{ mi} = 3,000 \text{ ft}^3/\text{d} = 15 \text{ gal}/\text{min}$ ). The storage coefficient matrix for the surficial aquifer designates 118 constant-head nodes, 111 along the model-grid boundary, 5 at Lake Tarpon, and 2 at Tampa Bay. Crews Lake, the other large lake in the modeled area, fluctuates with the water table; therefore, constant-head nodes were not assigned there.

Transmissivity of the Floridan aquifer and leakance coefficient of the upper confining bed were based on analyses of aquifer tests (table 2) and on preliminary values derived from the phase 1 two-dimensional model calibration (Hutchinson and others, 1981). Transmissivity is high north of the Cross Bar Ranch well field and in the area of the Hillsborough River. Transmissivity is lowest beneath the lakes terrace physiographic unit (fig. 5).

Leakance coefficient of the upper confining bed is model-derived in areas outside the well fields (fig. 6). Leakance coefficient is lowest in the ridge areas and is based on a relatively large head difference between the water table and potentiometric surface. Leakance coefficient is highest in the lakes terrace, coastal marsh, and along the Hillsborough River where the confining bed has been thinned by erosion or breached by sinkholes. Figure 7 depicts leakage rates in each physiographic unit as derived in the calibrated model. Leakage is upward in the coastal marsh and central swamp physiographic units; elsewhere leakage is downward.

Hydraulic conductivity of the surficial aquifer was estimated at a uniform 10 ft/d. This estimate is based on laboratory measurements for surficial materials in northwest Hillsborough County (Sinclair, 1974). The model computes transmissivity of the surficial aquifer by multiplying hydraulic conductivity by saturated thickness, determined as the difference between the simulated water table and the bottom of the aquifer. A map showing distribution of transmissivity of the surficial aquifer is presented in figure 8.

The bottom of the surficial aquifer was mapped by subtracting the thickness of surficial deposits defined by Wolansky and others (1979) from land surface. The saturated thickness was then mapped. It was found to be 10 feet or less in the coastal marsh and central swamp and 15 to 30 feet elsewhere. Figure 9 shows the bottom configuration as input to the model and the saturated thickness simulated in the steady-state calibration.

Recharge to the surficial aquifer, considered to be derived from rainfall, irrigation return, and lake augmentation, was initially computed at each node as the sum of the phase 1 two-dimensional model-computed leakage rate (Hutchinson and others, 1981) and the estimated ET-runoff capture rate from the water table. The regional pattern of recharge was then modified to conform to controls assumed by the six physiographic units (fig. 10). For the calibration period, it was estimated that average recharge varies from a minimum of 15 inches per year in the central swamp to a maximum of 28 inches per year in the lakes terrace.

Table 2.--Aquifer-test results

Well field	Transmissivity from aquifer test (ft <sup>2</sup> /d)	Transmissivity in model (ft <sup>2</sup> /d)	Leakance coefficient from aquifer test [(ft/d)/ft]	Leakance coefficient in model [(ft/d)/ft]	Source of information
Cross Bar Ranch	47,500-115,000	25,900-194,400	0.0005-0.0027	0.0004-0.0008	Leggette, Brashears and Graham, Inc. (1978)
Cypress Creek	31,500-53,600 <sup>1/</sup>	25,900-41,500	.00003-.0009 <sup>1/</sup>	.00015-.0008	Ryder (1978)
Starkey	40,000 <sup>2/</sup>	57,000	.003	.0003	Robertson and Mallory (1977)
Pasco County	53,000	51,800-57,000	.0003	.0003-.0004	Robertson and Mallory (1977)
Eldridge-Wilde	33,000 <sup>2/</sup>	57,000	.0003	.0004-.0005	Robertson and Mallory (1977)
East Lake	40,000 <sup>2/</sup>	57,000	.001	.0003	Robertson and Mallory (1977)
Cosme	----	57,000	----	.0003	-----
Section 21	29,400-86,900	51,800	.0002-.0015	.0004	Stewart (1968)
Morris Bridge	53,500-130,000 <sup>1/</sup>	41,500-237,600	.0006-.001	.0003-.0006	Ryder and others (1980)
Northwest	----	25,900-51,800	----	.0003-.0004	----

<sup>1/</sup> Range includes values from aquifer tests near the well field.

<sup>2/</sup> Pumped well did not top the full thickness of the Floridan aquifer (partially penetrating).

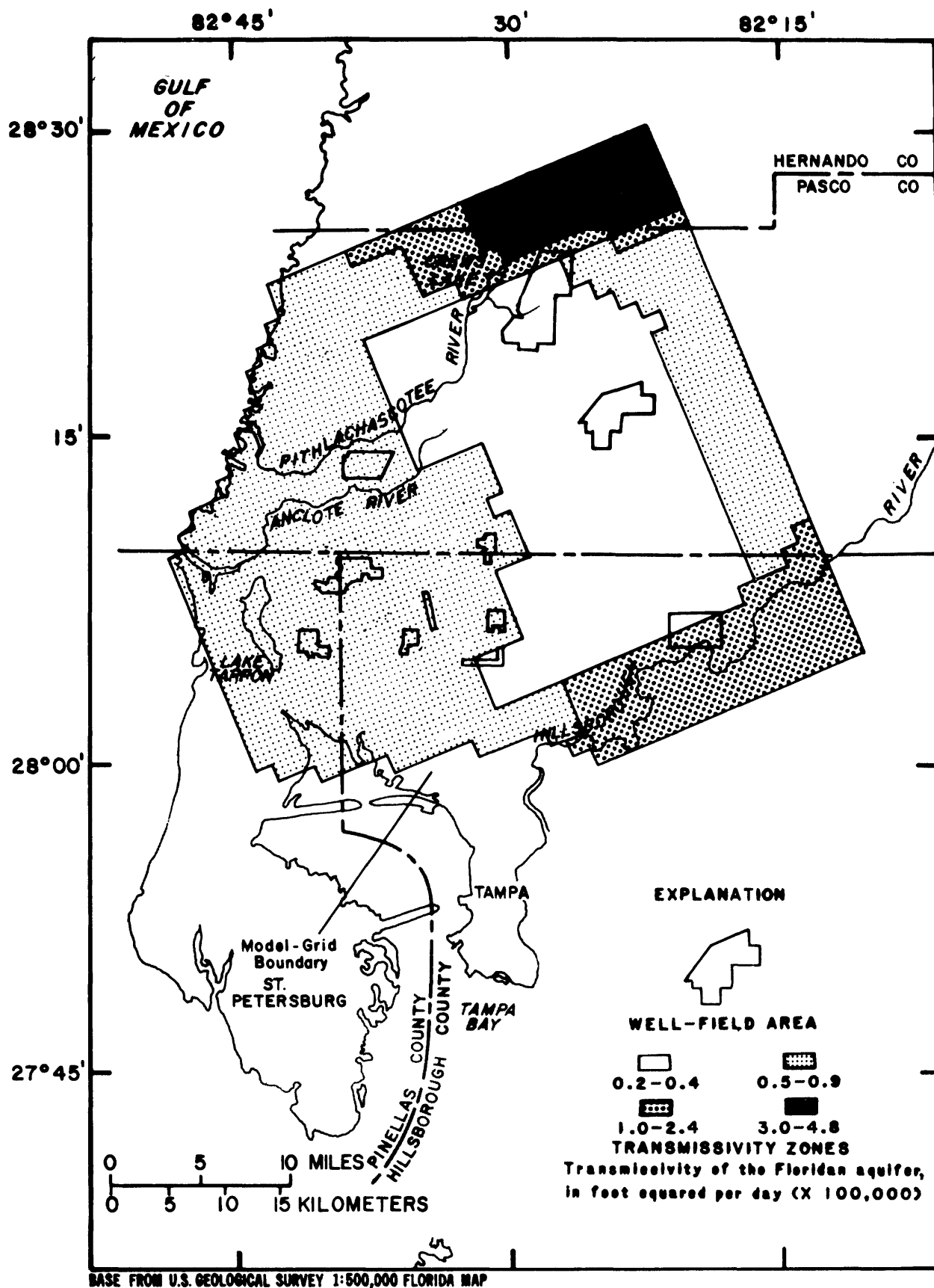


Figure 5.--Transmissivity of the Floridan aquifer.

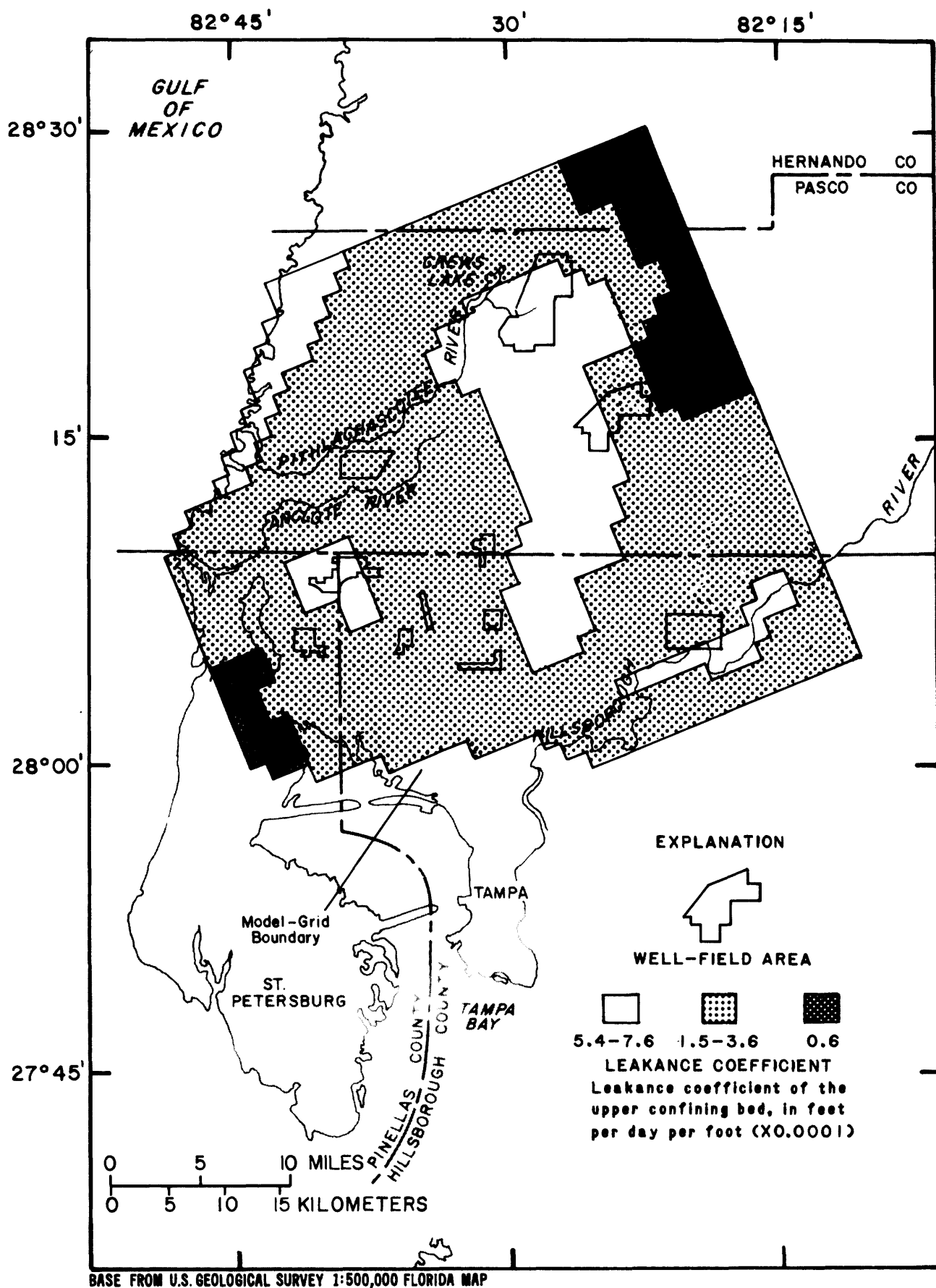


Figure 6.--Leakance coefficient of the upper confining bed.



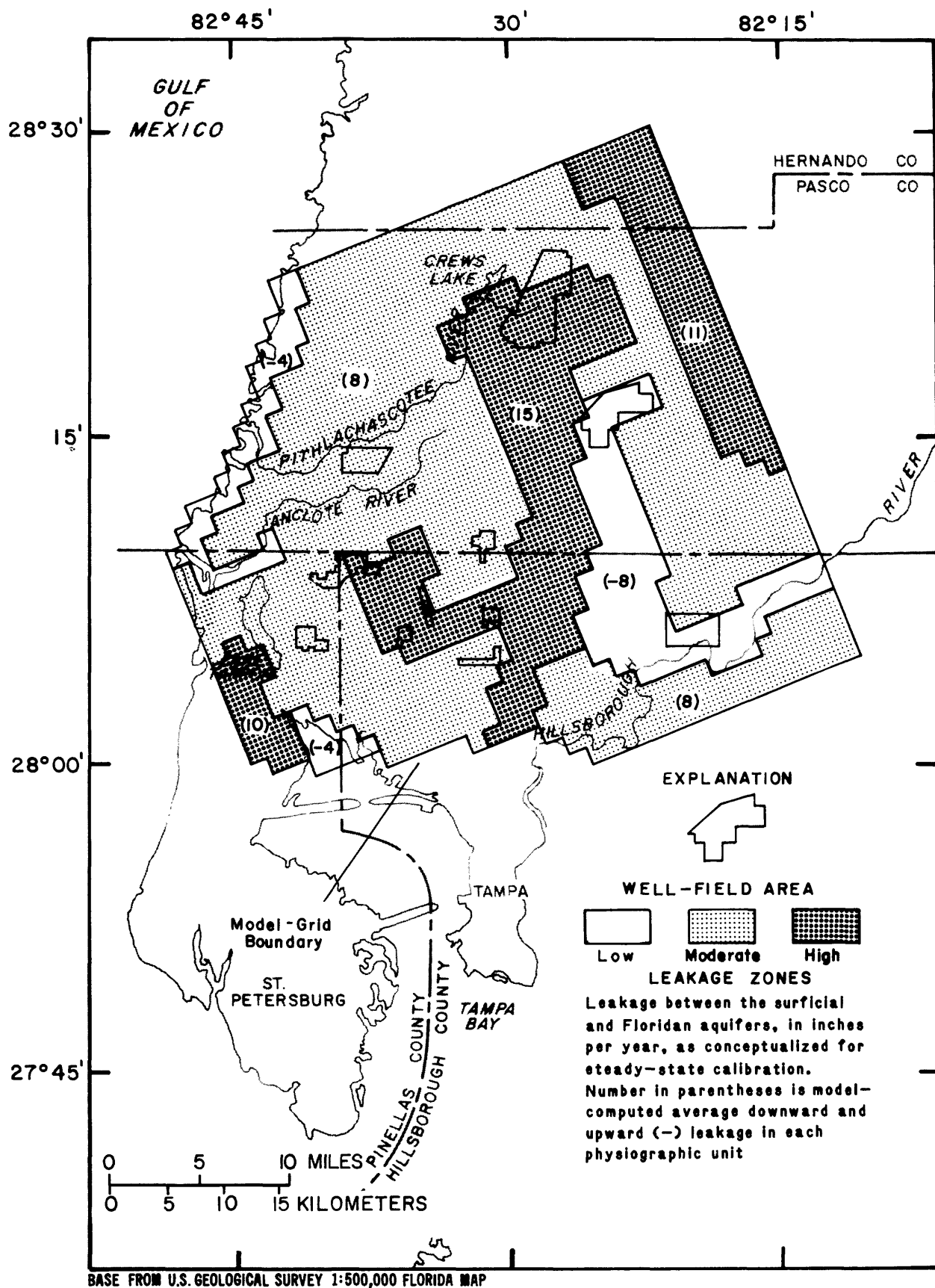


Figure 7.--Leakage rates through the upper confining bed, as computed in the steady-state model calibration.

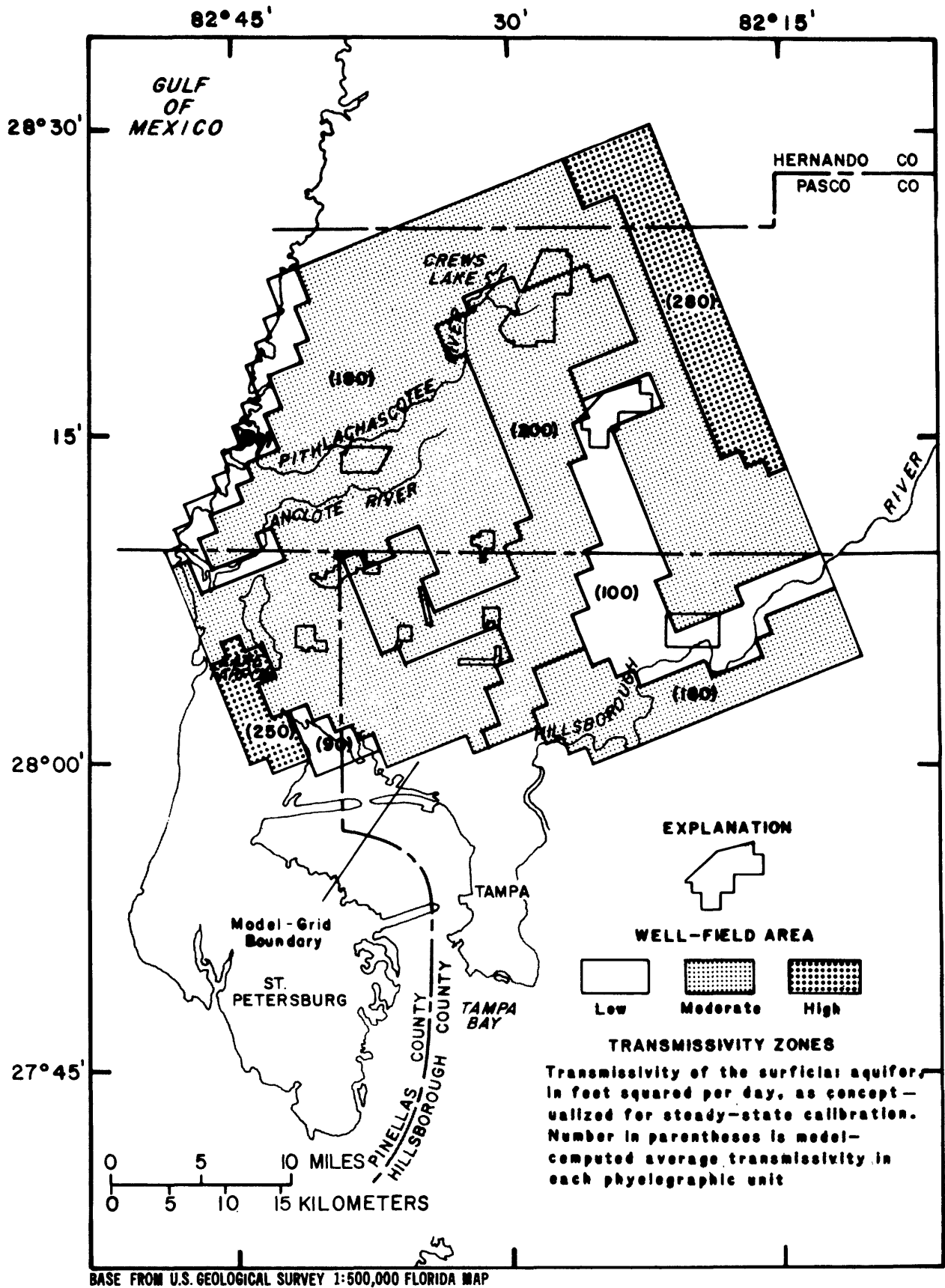


Figure 8.--Transmissivity of the surficial aquifer.

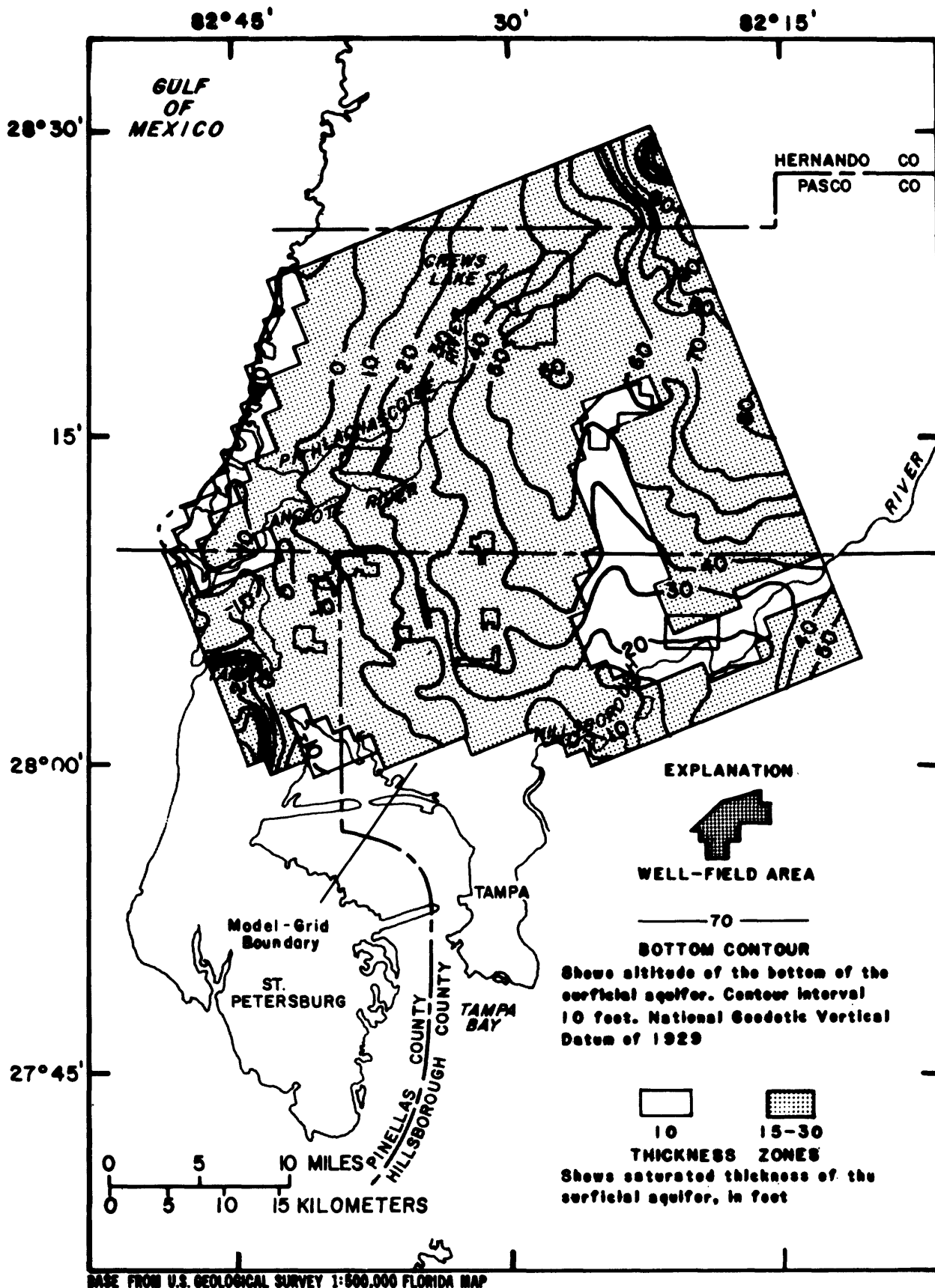


Figure 9.--Bottom configuration and saturated thickness of the surficial aquifer.

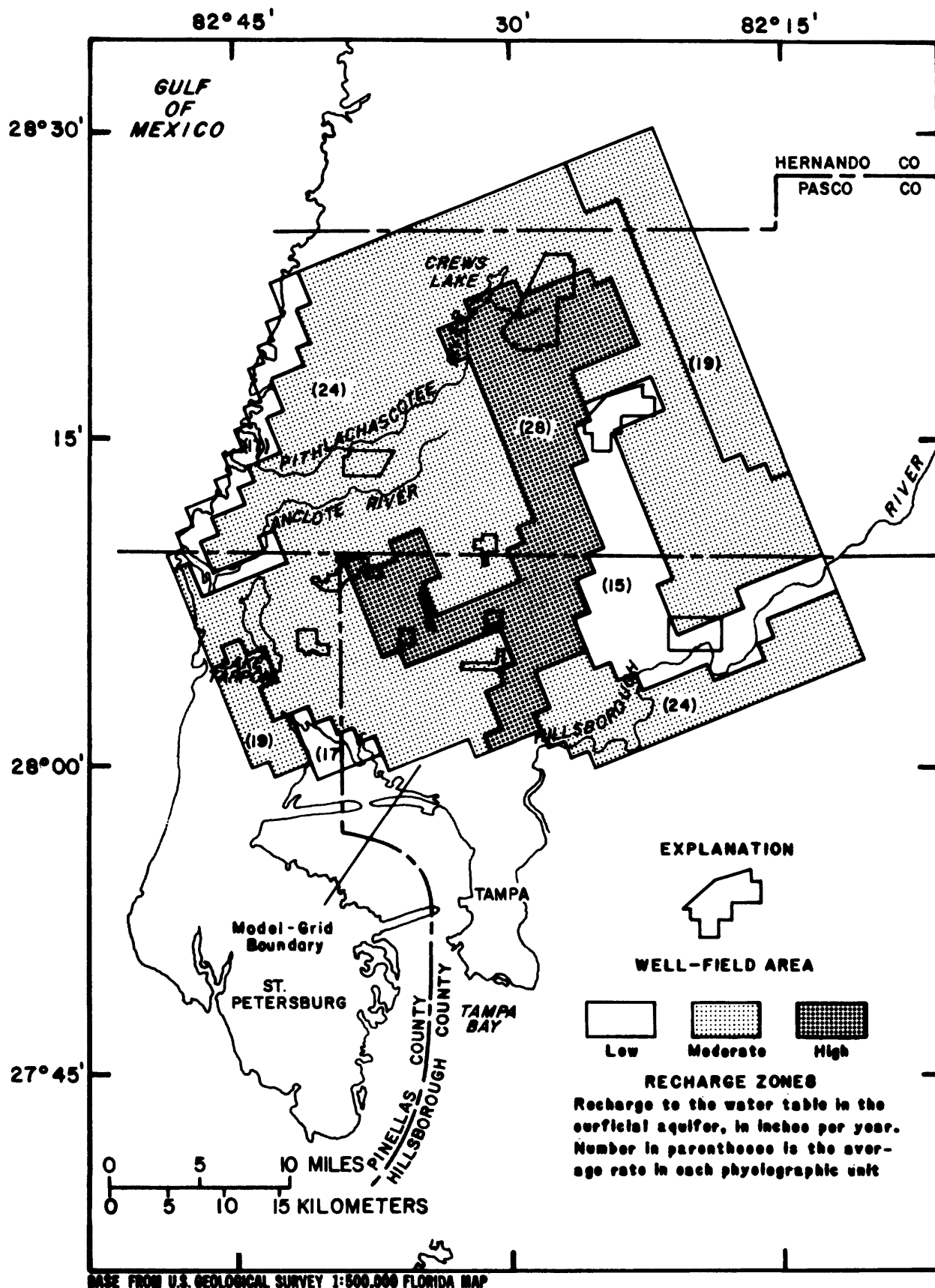


Figure 10.--Recharge to the water table in the surficial aquifer, as input for the steady-state calibration.

The HCF condition leakage factor for steady-state flow is computed analytically (using variable names that appear in the model code) by the equation:

$$CSS = \frac{T\sqrt{\lambda}}{\Delta X_i} \cdot \frac{(1 - e^{-2\lambda L\sqrt{\lambda}})}{(1 + e^{-2\lambda L\sqrt{\lambda}})}, \quad i = 1, 2 \quad (3)$$

where  
 CSS = HCF condition leakage factor (feet per second per foot);  
 T = transmissivity of the Floridan aquifer (feet squared per second);  
 $\lambda = TK/T$ ; TK is the leakance coefficient of the upper confining bed (feet per second per foot); k/b, where k is the hydraulic conductivity, and b is the thickness of the bed;  
 L = distance from model-grid boundary to constant-head point beyond (feet); and  
 $\Delta X_i$  = model-grid length parallel to boundary flow (feet), either DELX or DELY grid spacings.

The equation, formulated by S. P. Larson and J. V. Tracy (written commun., 1979) and derived later in this report, solves for the factor CSS. When CSS is multiplied by the difference between the head factor and potentiometric surface, flux at the model-grid boundary is computed.

The head factor in the HCF condition is the effective head in the surficial aquifer at an arbitrary distance of 15 miles from the model-grid boundary necessary to yield boundary flux calculated by the phase 1 two-dimensional model. A distance of 15 miles was selected as being beyond the effects of any stress that could be applied in the modeled area. For input to the quasi-three-dimensional model, the factor was computed using a form of Darcy's law:

$$HSS = \frac{XHCF}{CSS} + PHI \quad (4)$$

where  
 HSS = HCF condition head factor for the surficial aquifer (feet);  
 XHCF = boundary flux from two-dimensional model calibration (feet cubed per second per foot squared, or feet per second);  
 CSS = HCF condition leakage factor (feet per second per foot), computed from equation 1; and  
 PHI = head in Floridan aquifer at the model-grid boundary (feet).

The equation, rearranged to compute XHCF, is essentially that contained in the model for computing flux at the model-grid boundary (statement 11690 in computer program listing of Attachment A). Because this flux had already been computed in the phase 1 two-dimensional calibration, it was used to compute the head factor, which normally would be estimated.

Capture of evapotranspiration (ET) from the water table and runoff may be considered to be the variable source from which pumped water is derived since recharge is held constant in the model. Under normal climatological conditions, pumping will lower the water table, thereby creating the potential for extra recharge during the wet season and, subsequently, less runoff. Thus, the modeled ET-runoff capture parameter not only represents ET from the water table, but also changes in recharge and runoff.

Capture of ET from the water table and runoff was assumed to occur in the zone between land surface and a depth of 10 feet. The ET-runoff capture rate derived in the conceptual model is that for each foot of water-table decline, 3.8 inches per year of water can be captured. The maximum potential ET-runoff rate from the water table is 38 inches per year in areas where the water table is at land surface and zero where the water table is 10 feet or more below land surface. Regional patterns of ET-runoff were adjusted to conform to controls of the six physiographic units (fig. 11). It should be emphasized that ET is from the water table in the surficial aquifer and, thus, is only a component of the total ET found in standard hydrologic budget analyses (for example, Cherry and others, 1970). ET from land surface and the unsaturated zone and flood runoff are not represented in the model.

The average altitude of land surface in each grid block was obtained from U.S. Geological Survey 1:24,000 topographic maps. Topographic highs usually correspond to the ridge and terrace physiographic units, and closed basins or lows correspond to swamps and coastal marsh units.

Average withdrawals from the Floridan aquifer for the period September 1976 to May 1977 were estimated from records of the Southwest Florida Water Management District and from estimates of water requirements for citrus (University of Florida, 1977). These included withdrawals for municipal supply, miscellaneous municipal supply and treatment, citrus irrigation, and miscellaneous crop, pasture, and lake augmentation (table 3). Water pumped from private domestic wells was assumed to be small and was not considered for input to the model. The distribution of pumpage as input to the model is shown in figure 12. Pumping is distributed mainly along the Gulf Coast and in the southern part of the modeled area. The largest withdrawals occur in the well-field areas.

### Calibration

The model was calibrated by systematically adjusting input parameters until simulated heads in the Floridan and surficial aquifers matched long-term average steady-state levels. Leakage coefficient of the upper confining bed and transmissivity of the Floridan aquifer had been calibrated previously in the phase 1 two-dimensional model, and these parameters were readjusted during calibration of the quasi-three-dimensional model. Error limits for the steady-state quasi-three-dimensional model calibration were set at  $\pm 5$  feet of head in each aquifer. Statistics of the model calibration are listed in table 4.

The comparison of observed long-term average water levels with model-simulated values in both aquifers was good statistically, thus the model was considered to be adequately calibrated. Residuals were nearly within the  $\pm 5$ -foot limit and were normally distributed about means near zero. The standard deviation about the mean of the residuals for the water table was 1.3 feet. That is, the model-simulated water table matched the estimated long-term average levels within a range of 0.9 foot above to 1.7 feet below at about 68 percent of the nodes. Similarly, the model-simulated potentiometric surface matched the September 1976 to May 1977 average levels at 68 percent of the nodes within a range of 1.9 feet above to 2.3 feet below, based on a standard deviation of

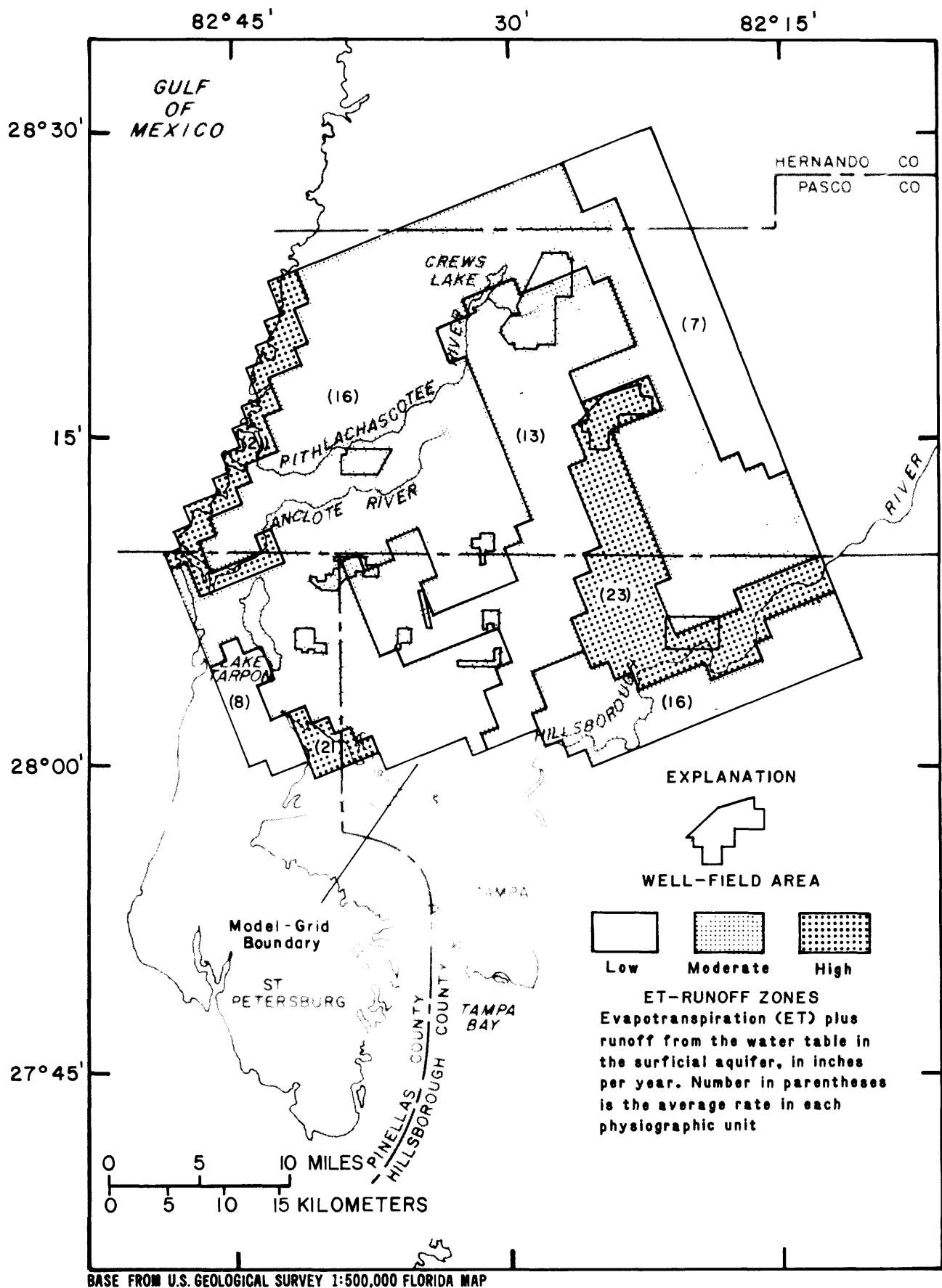


Figure 11.--Evapotranspiration plus runoff from the water table in the surficial aquifer, as computed in the steady-state calibration.

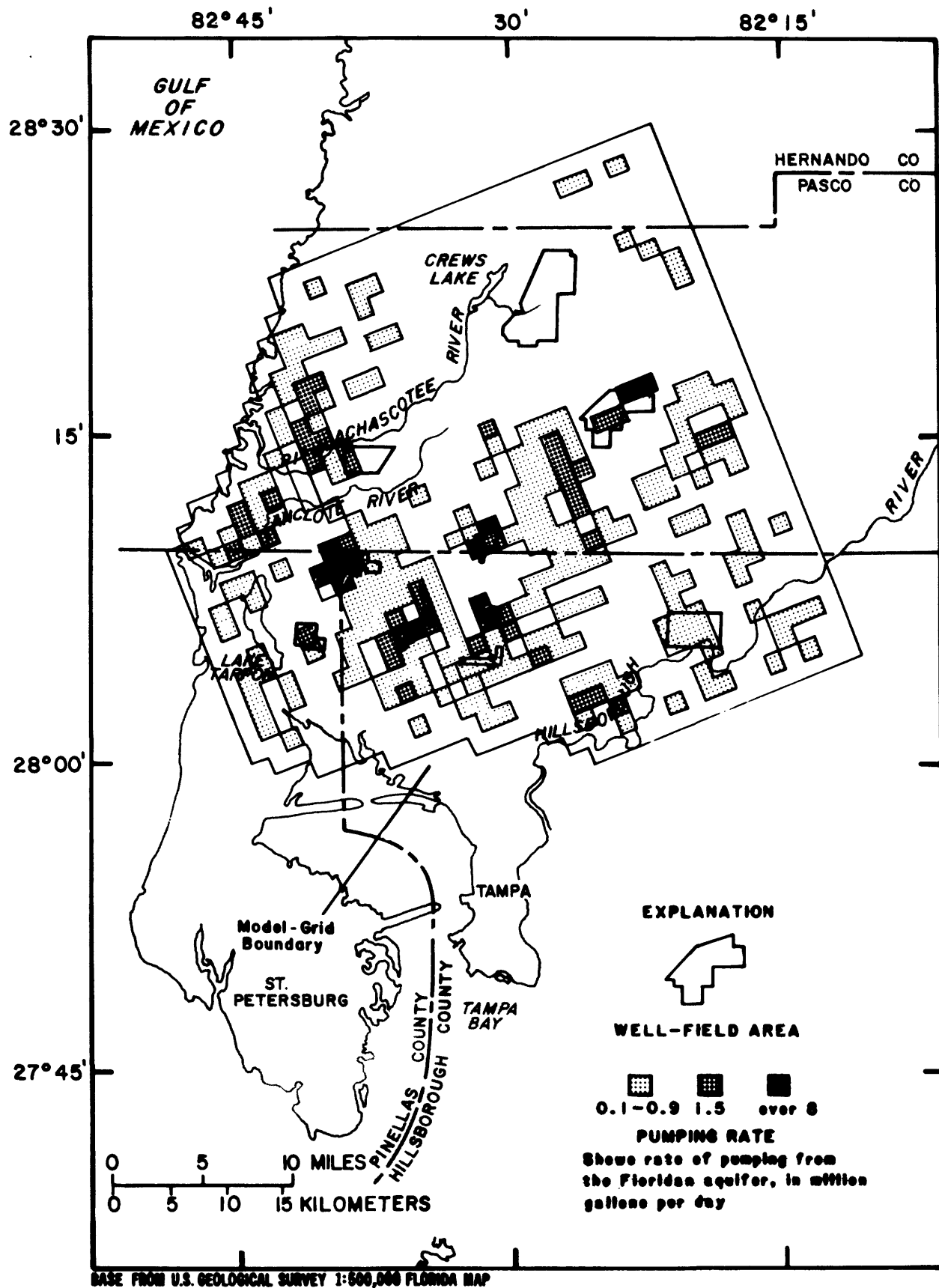


Figure 12.--Distribution of average pumpage in the modeled area, September 1976 to May 1977.



Table 3.--Average pumpage from the Floridan aquifer, September 1976 to May 1977

Use	Mgal/d	ft <sup>3</sup> /s
Municipal <sup>1/</sup>	87	134
Miscellaneous municipal and treatment <sup>2/</sup>	11	18
Citrus irrigation <sup>3/</sup>	24	37
Miscellaneous crop, pasture, and lake augmentation <sup>4/</sup>	11	17
Total	133	206

- <sup>1/</sup> Obtained from pumping records on file at Southwest Florida Water Management District.
- <sup>2/</sup> Computed as 75 percent of the daily pumpage permitted by Southwest Florida Water Management District.
- <sup>3/</sup> Computed by the method outlined by University of Florida (1977) using a 75-percent seepage efficiency.
- <sup>4/</sup> Computed as 50 percent of the daily pumpage permitted by Southwest Florida Water Management District.

2.1 feet about a residual mean of 0.2 foot below the average level. The correlation coefficients were near 1.000, indicating near-perfect association between the long-term average and model-simulated water levels in both aquifers. Comparisons between the long-term average and model-simulated water table and potentiometric surface are shown in figures 13 and 14, respectively. Water levels simulated by the steady-state calibration run were used as starting water levels for subsequent model-sensitivity runs.

#### Sensitivity Analysis

Sensitivity analysis tests model sensitivity to changes in input parameters. Separate model simulations are made with individual parameters varied in turn over a reasonable range of values within which they should occur. The model was not recalibrated each time parameter values were changed since this would be impractical in terms of time and cost. Exact values of head changes from sensitivity analyses should be viewed critically, but relative changes can provide insight as to the degree to which a change in any parameter may affect results of model simulation.



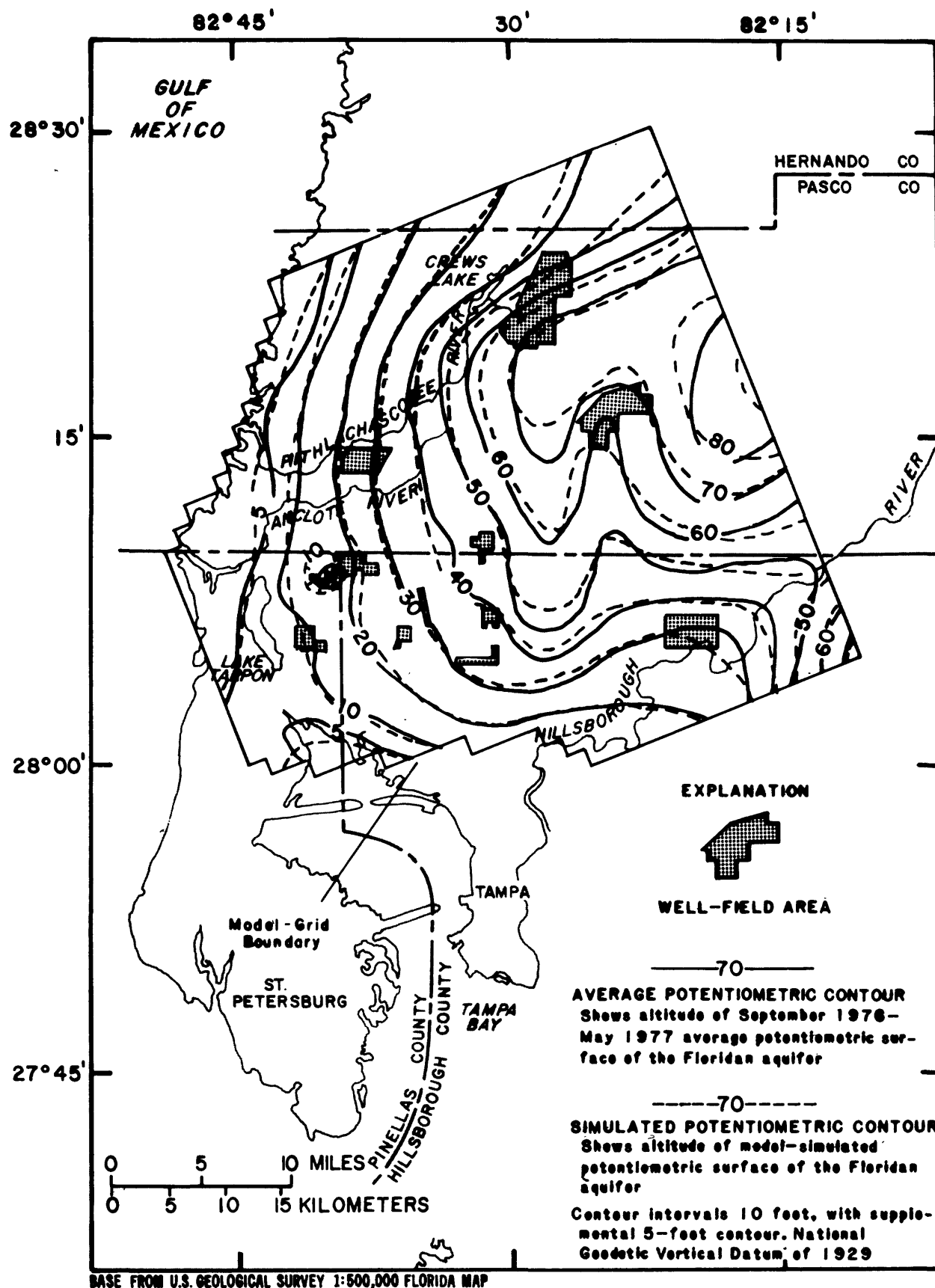


Figure 14.--Comparison of September 1976 to May 1977 average and model-simulated potentiometric surfaces, representing steady-state calibration.

Table 4.--Statistics of model calibration

Statistic	Long-term average versus model-simulated	
	Water table	Potentiometric surface
Number of active nodes	814	932
Maximum range in residuals <sup>1/</sup> (feet)	4.4 to (-5.0)	5.3 to (-4.9)
Mode of residuals (feet)	.1	.2
Median residual (feet)	.4	.3
Mean residual (feet)	.4	.2
Mean of absolute value of residuals (feet)	1.0	1.7
Standard deviation of residuals (feet)	1.3	2.1
Correlation coefficient	.9987	.9961

<sup>1/</sup> Residuals were computed by subtracting model-simulated water levels from the long-term average potentiometric surface and water table. A negative residual indicates that the model-simulated water level is higher than the long-term average water level, and the reverse is indicated by a positive residual.

Model sensitivity was tested by varying ET-runoff and recharge parameters and hydraulic parameters of the surficial and Floridan aquifers and the confining bed. Figure 15 shows deviations from the steady-state calibration water table and potentiometric surface by varying maximum ET-runoff capture depth (ETD) by  $\pm 5$  feet, recharge rate (QRE) by  $\pm 20$  percent, and maximum potential ET-runoff capture rate (ETR) by  $\pm 20$  percent. Figure 16 shows deviations due to varying hydraulic conductivity (PERM) of the surficial aquifer by a factor of 3, transmissivity (T) of the Floridan aquifer by factors of 2 and 0.5, and leakance coefficient (TK) of the upper confining bed by factors of 2 and 0.5. The cross sections in both figures depict model-simulated heads along row 20 of the model. This row, through the center of the model, intersects five of the six physiographic units. The two cross sections exemplify and were used in conjunction with maps that supply areal perspective to the sensitivity analysis.

# DEPARTURE OF COMPUTED HEAD FROM STEADY STATE CALIBRATION HEAD, IN FEET

WATER TABLE IN SURFICIAL AQUIFER

PHYSIO-  
GRAPHIC  
UNITS

POTENTIOMETRIC SURFACE  
OF FLORIDAN AQUIFER

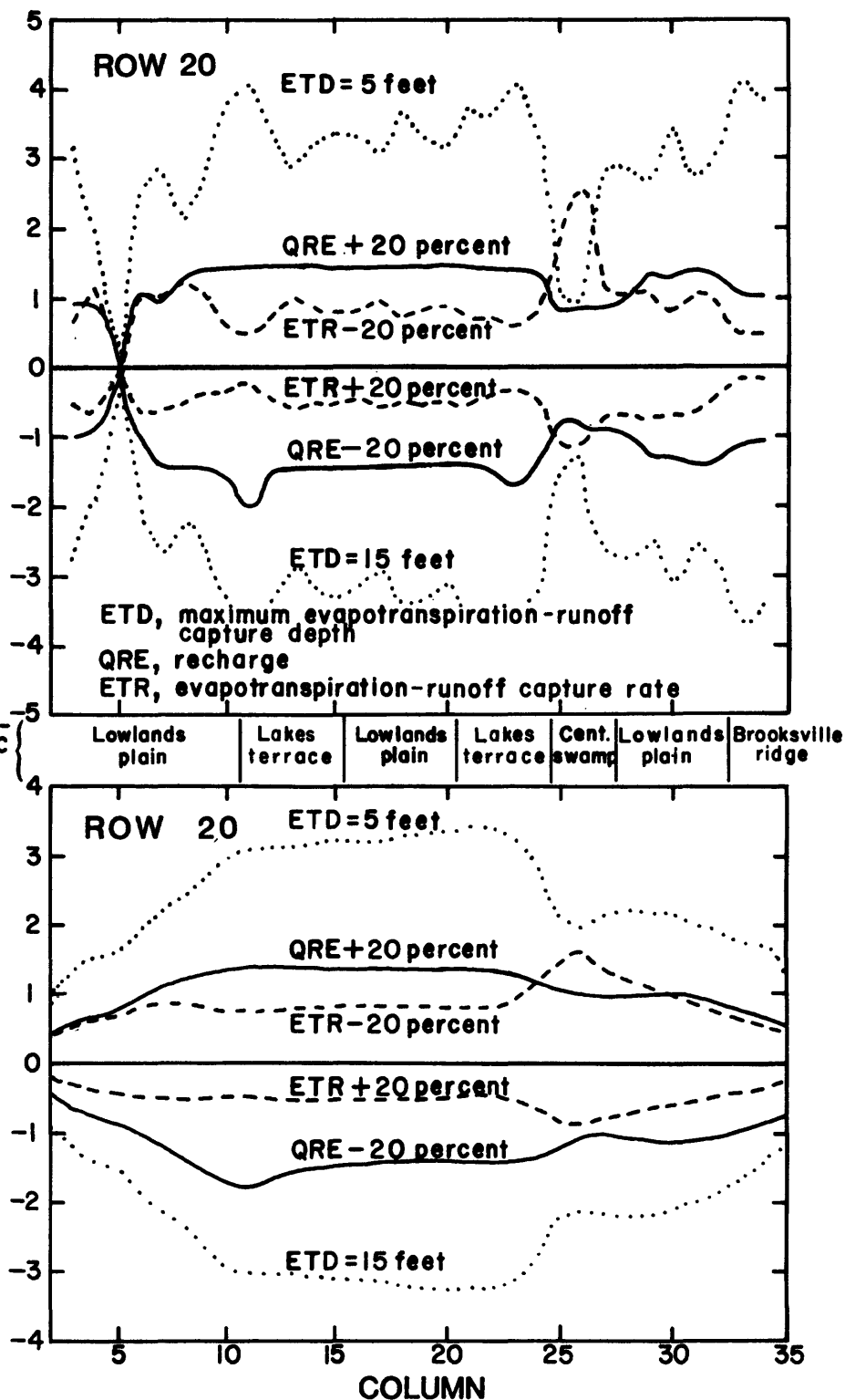


Figure 15.--Effects of varying evapotranspiration-runoff and recharge parameters on the steady-state calibration.

DEPARTURE OF COMPUTED HEAD FROM STEADY-STATE CALIBRATION HEAD, IN FEET

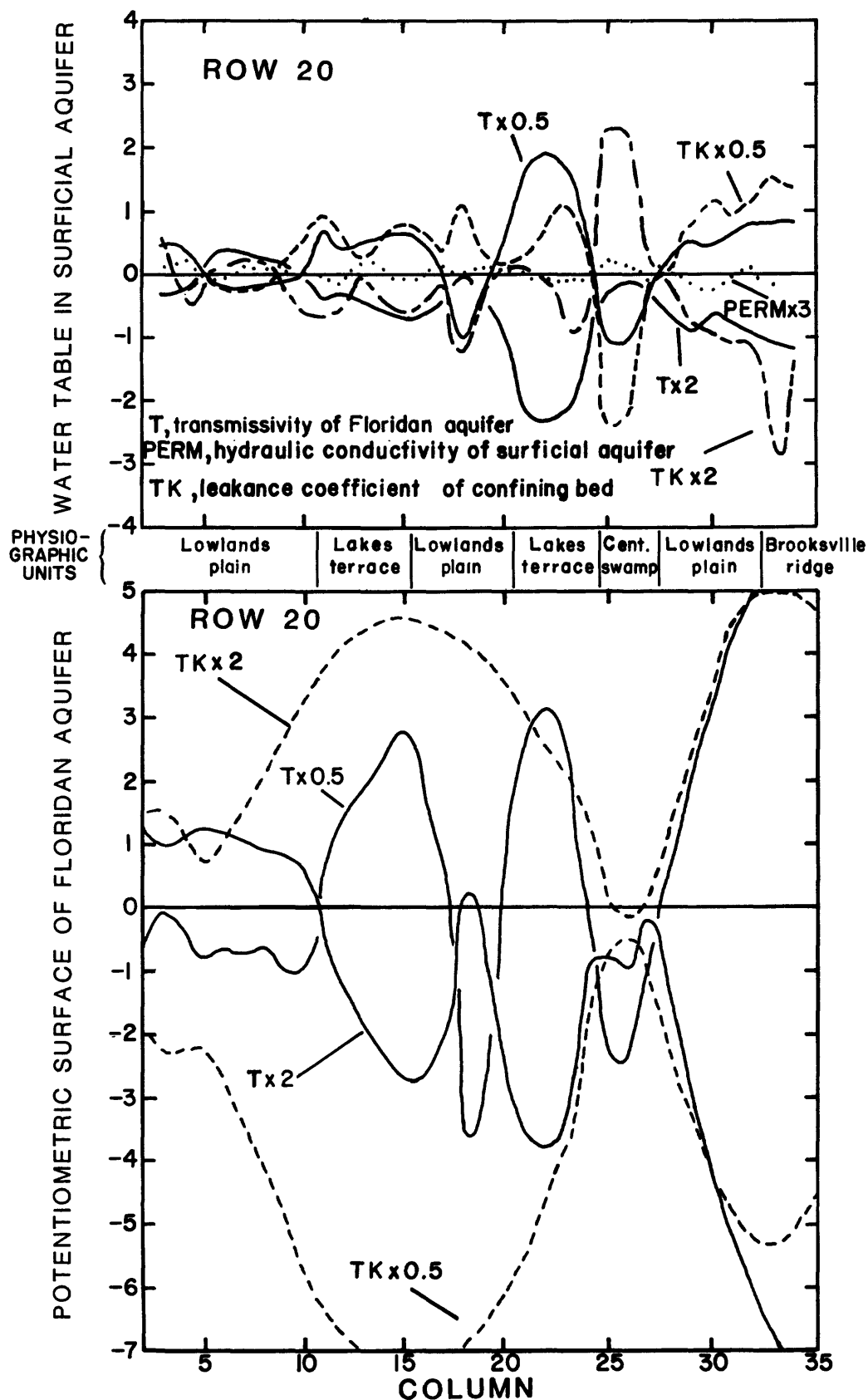


Figure 16.--Effects of varying aquifer and confining bed hydraulic parameters on the steady-state calibration.

Varying the ET-runoff capture rate and recharge has a slightly greater effect on the water table than the potentiometric surface. One might expect to see a much larger effect on the water table because these changes directly apply to inflow to and outflow from the surficial aquifer. But, due to the relatively high leakage rate from the surficial aquifer through the upper confining bed and the dampening effect on heads in this aquifer by the ET-runoff capture function, head deviations from the steady-state calibration are nearly the same in each aquifer. Although not depicted in figure 15, the ridge areas are sensitive to recharge. The water table in the ridges responds dramatically to small changes in recharge because the leakance coefficient is low and the water table generally lies 10 feet or more below land surface, thereby nullifying the dampening effect of ET-runoff capture. Other than in ridge areas, the effects of increasing or reducing recharge are dampened by increasing or reducing ET-runoff outflow. In the swampy areas ET-runoff is high and changing this parameter strongly influences the calibration, as can be seen at columns 25-26 of figure 15.

Because the model is sensitive to ET-runoff capture and recharge, better estimates of these input parameters would produce a more accurate model. A preliminary model was calibrated that assumed the water table would drop 1 foot for each 1.8 inches captured from ET and runoff. This preliminary calibration produced about twice the observed water-table fluctuations under different pumping conditions and, thus, was a poor validation. Subsequently, the model was recalibrated using a higher potential ET-runoff capture rate where the water table would drop 1 foot for each 3.8 inches of water captured. This resulted in more realistic fluctuations of the water table. Overall, the model indicates that better definition is needed of (1) recharge rate of various soil types and physiographic units, (2) relation between recharge and depth of water table, (3) potential evapotranspiration rate from the water table, and (4) relation between evapotranspiration and depth of water table.

Of the hydraulic parameters tested, the model is most sensitive to changes in leakance coefficient of the upper confining bed, moderately sensitive to transmissivity of the Floridan aquifer, and least sensitive to hydraulic conductivity of the surficial aquifer. Average deviations in the water table are relatively small compared to those in the potentiometric surface, showing again the dampening effect of ET-runoff. For example, when leakance was doubled, the potentiometric surface rose 4.6 feet at column 15 (fig. 16), but the water table declined only about 0.5 foot. The increased downward leakage in the center of the modeled area was composed entirely of captured ET-runoff. In ridge areas where the water table generally is 10 feet or more below land surface, evapotranspiration from the water table and the potential for capturing runoff are nil, and small changes in the potentiometric surface sometimes result in large fluctuations in water-table levels.

Tests of the model's sensitivity to boundary conditions were made during the predictive-modeling phase of the study. Ten well fields were pumped at a combined rate of 186.9 Mgal/d and drawdowns in the Floridan aquifer were observed under constant-head, constant-flow, and HCF boundary conditions. Average drawdown at the boundary was zero under constant-head conditions, 1.2 feet under HCF conditions, and 2.5 feet under constant-flow conditions. Average drawdown over the modeled area was 3.4, 3.6, and 4.2 feet, respectively, under these conditions. Had the distance from the model-grid boundary to the constant-head point beyond been less than 15 miles, boundary drawdowns would be equal to or less than 1.2 feet, observed using the 15-mile distance. Increasing distance greater than 15 miles would produce boundary drawdowns equal to or slightly greater than the HCF level.

The sensitivity analysis exemplifies an important limitation of the model whereby the water table can rise above land surface, when actually it can rise only to land surface and becomes surface water with additional rise. Unless ponding occurs, rises at these nodes are not possible because the water table already lies at land surface. Errors in the water-table levels will lead to errors in computed leakage rate through the upper confining bed and potentiometric head in the underlying Floridan aquifer. When using the model for predictive purposes, this limitation should be kept in mind and areas of water-table rise above land surface should be recognized. The model code was modified to flag these areas.

### Validation

Model validation is a technique for testing the accuracy of a calibrated model for predictive purposes. The test case chosen for validation involved removing all pumpage from the calibrated steady-state model; recharge in pumping nodes was reduced by subtracting 20 percent of the previous irrigation pumpage (because irrigation-return flow would cease) and 100 percent of the lake augmentation pumpage (all water pumped for lake augmentation was considered to be recharge, and hence, would cease). The natural recharge rate was increased by 10 percent, based on the assumption that rainfall, and subsequently recharge, was 10 percent below normal during the calibration period. The model-simulated water table was compared with the steady-state calibrated water table to check if water-level changes in the surficial aquifer were plausible. The model-simulated potentiometric surface was compared with an estimated potentiometric surface generally unaffected by pumping, mapped by Johnston and others (1980), that represents predevelopment conditions. Statistics of these comparisons are listed in table 5. Comparisons are good statistically; thus, the model was considered to be adequately validated.

The model-simulated predevelopment water table rose a maximum of 10.7 feet above the calibrated water table. The model indicated an average water-table rise of 1.4 feet. The standard deviation of 1.1 feet about the mean indicates that, in about 68 percent of the nodes, the model-simulated water table remained within a range of 0.3 foot lower and 2.5 feet higher than the steady-state calibrated level. The greatest rise occurred at the Eldridge-Wilde well field. In other well fields, the rise due to cessation of pumpage was 2 to 4 feet. Estimated average water levels for 1976-77 are assumed to represent stressed conditions. Figure 17 indicates that should all pumping cease, the water would recover in some areas. Although not discernible at the scale of figure 17, there was a rebound of water-table levels to smooth out depressions caused by pumping in all the well fields. The water table simulated by the validation run was used as the predevelopment starting water table upon which predictive model runs are based.

Comparison of predevelopment and model-simulated potentiometric surfaces was good statistically. Over the 932 nodes within the model-grid boundary, the simulated predevelopment potentiometric surface ranged from 9.4 feet higher than to 11.6 feet lower than the estimated level. The mean was 1.8 feet higher than the estimated level. The standard deviation about the mean of the residuals was 3.5 feet, which indicates that the model-simulated potentiometric surface matched within a range of 5.3 feet higher than to 1.7 feet lower than the estimated predevelopment level at about 68 percent of the nodes. A correlation coefficient of 0.9899 indicates a good correlation between the two surfaces.



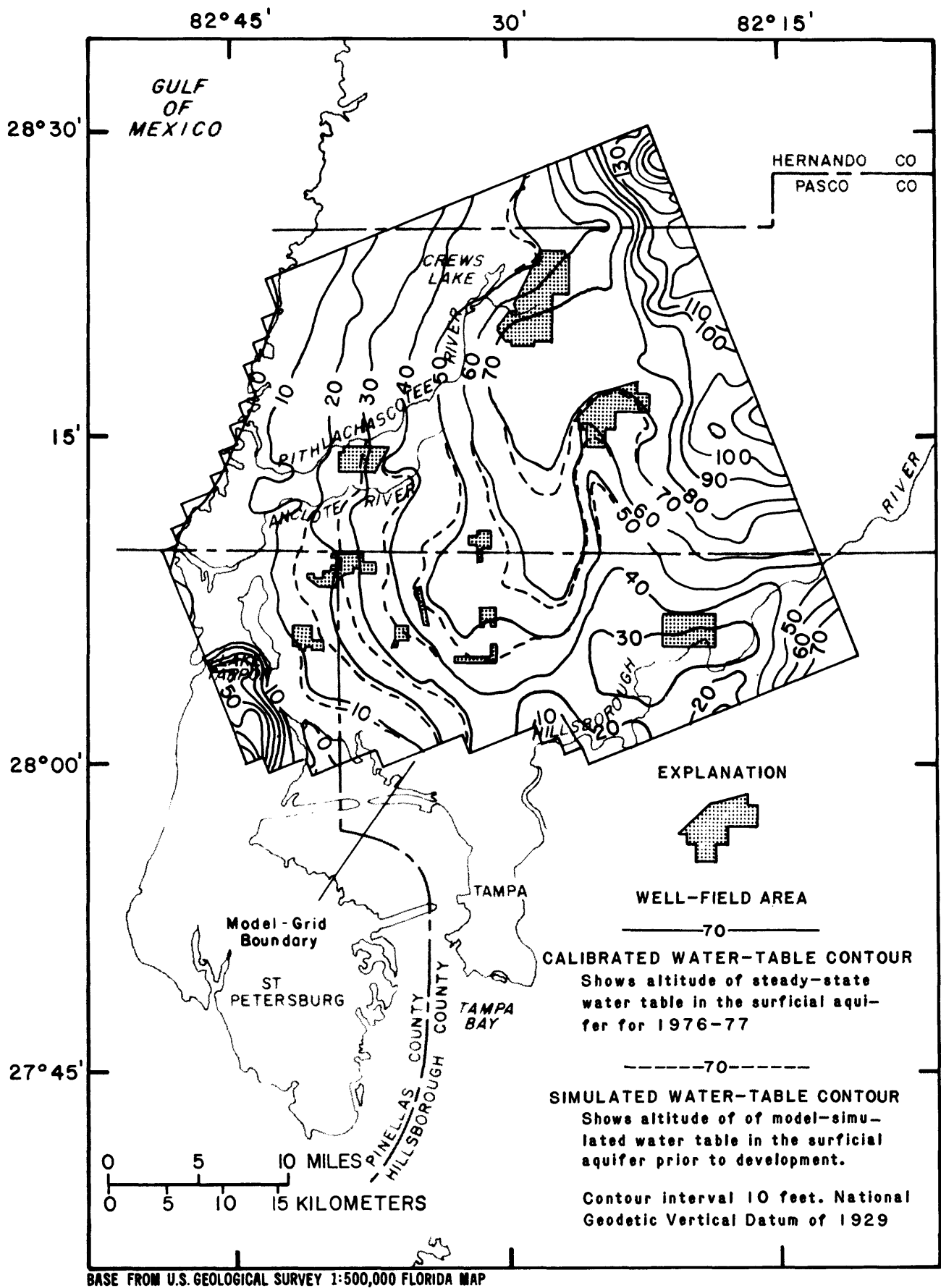


Figure 17.--Comparison of steady-state calibrated and model-simulated predevelopment water tables, representing model validation.

Table 5.--Statistics of model validation

Statistic	Calibrated steady-state versus model-simulated water table	Estimated prestressed versus model-simulated potentiometric surface
Number of active nodes	814	932
Maximum range in residuals <sup>1/</sup> (feet)	0 to (-10.7)	11.6 to (-9.4)
Mode of residuals (feet)	- .7	-3.7
Median residual (feet)	-1.1	-1.9
Mean residual (feet)	-1.4	-1.8
Standard deviation of residuals (feet)	1.1	3.5
Correlation coefficient	.9992	.9899

<sup>1/</sup> Residuals were computed by subtracting model-simulated water levels from the calibrated steady-state water table and estimated prestressed potentiometric surface, respectively. A negative residual indicates that the model-simulated water level is higher than the water level with which it is compared, and the reverse is indicated by a positive residual.

Comparison between the estimated and simulated potentiometric surfaces for predevelopment conditions is shown in figure 18. The wide range in model residuals between the two surfaces (21 feet) may be due to several factors. For example, the map of the estimated historic potentiometric surface may be in error where data are sparse and specifically in areas where the predevelopment head is lower than the average steady-state head. Errors could be greater along the coasts of the Gulf of Mexico and Tampa Bay where upwelling of freshwater results in upward flow in the aquifer and where channel dredging may have changed the hydraulic properties of the upper confining bed between predevelopment and September 1976 to May 1977 average maps. Although these errors may not represent model-calibration errors, they serve to increase the statistical errors of the model validation.

The model validation represents average hydrologic conditions prior to pumping. The water balance under these conditions will differ from that of the calibrated model in that recharge and pumpage have been altered. These alterations in turn affect ET-runoff from the water table and leakage. Table 6 lists the model-computed water balance for the surficial aquifer in each physiographic unit and for the surficial and Floridan aquifers over the total modeled area. It conforms to the conceptual model and is the water balance upon which all predictive runs of the model are based.

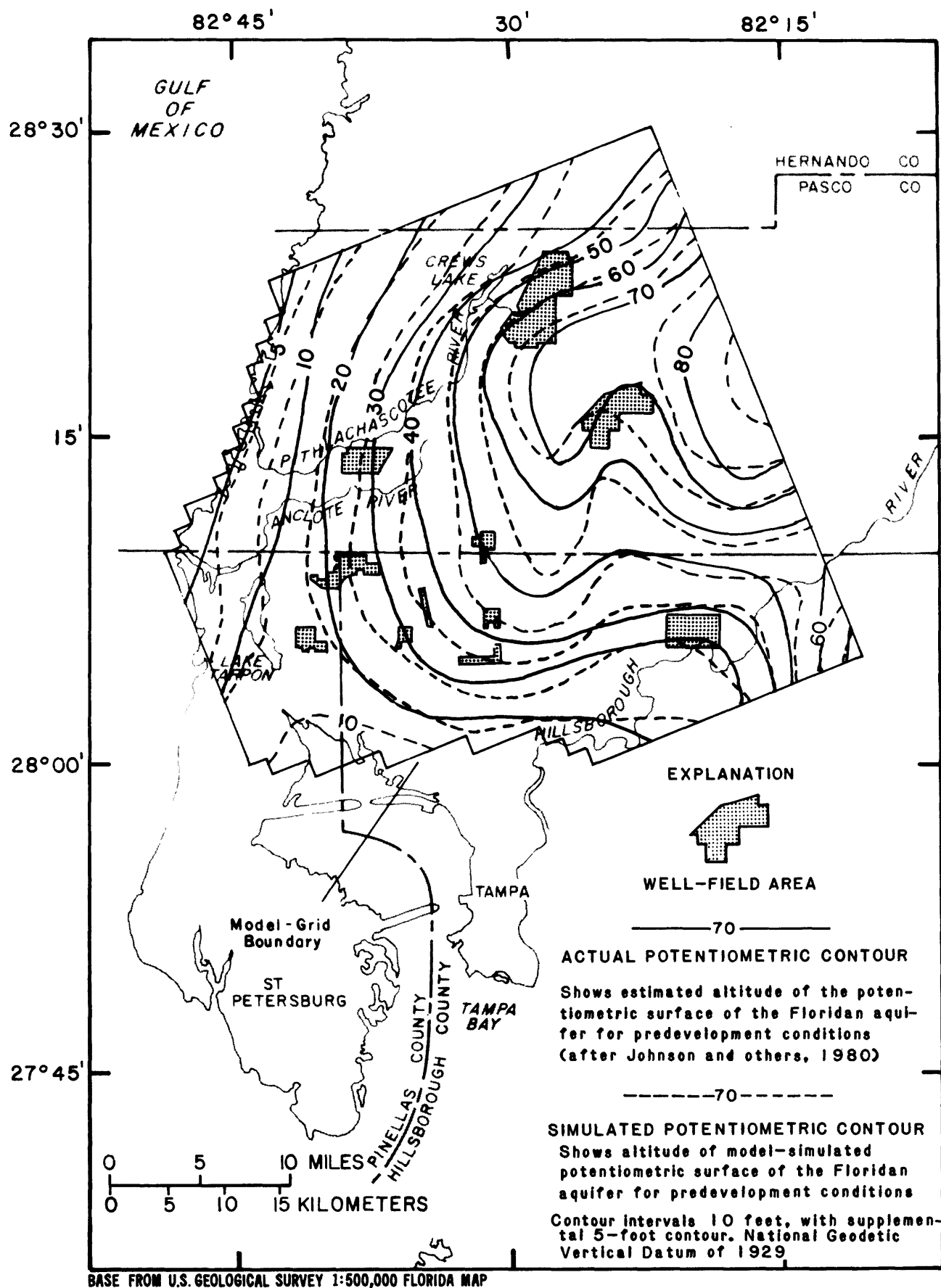


Figure 18.--Comparison of estimated and model-simulated potentiometric surfaces for predevelopment conditions, representing model validation.

Table 6.--Water balance for the surficial and Floridan aquifers simulated by the model under average hydrologic conditions prior to pumping

A. SURFICIAL AQUIFER BY PHYSIOGRAPHIC UNIT

Physiographic unit	Area (mi <sup>2</sup> )	Recharge (in/yr)	ET-runoff from water table (in/yr)	Net leakage up (-) or down (+) (in/yr)
Coastal marsh	25	18.6	25.0	-6.3
Lowlands plain	496	25.9	20.4	5.4
Brooksville ridge	41	20.6	8.9	11.3
Lakes terrace	159	30.0	19.4	10.5
Central swamp	82	16.3	26.9	-10.6
Coastal sand ridge	11	20.4	10.0	9.4

B. SURFICIAL AQUIFER

	Area (mi <sup>2</sup> )	QRE Recharge (in/yr)	ETRO ET-runoff from water table (in/yr)	UL Upward leakage (in/yr)	DL Downward leakage (in/yr)
Average for model	814	25.1	20.3	1.6	6.4
Water balance: QRE + UL = ETRO + DL					

C. FLORIDAN AQUIFER

	Area (mi <sup>2</sup> )	UL Upward leakage (in/yr)	DL Downward leakage (in/yr)	BI Boundary inflow (in/yr)	BO Boundary outflow (in/yr)
Average for model	932	1.6	6.7	1.1	6.2
Water balance: DL + BI = UL + BO					

The water table and potentiometric surface simulated by the validation run were used as predevelopment starting heads upon which predictive-model runs are based. This zeros out the drawdown array computed by the model and puts the system in equilibrium for the start of a predictive run. Had the estimated predevelopment potentiometric surface been input as the starting head, then the system would only be near equilibrium, and predicted drawdowns under an anticipated pumping condition would contain errors carried over from the validation run.

## PREDICTIVE MODELING

The quasi-three-dimensional model may be applied to various field problems. An example presented here illustrates options available in the program. An example field problem is presented, and results are compared to the phase 1 two-dimensional model results.

### Example Field Problem

The field problem involves determining water-balance and water-level changes that can be expected as a result of pumping all 10 well fields at their annual average permitted or proposed rates with recharge varying 20 percent above and below the average rate. The model simulates wet conditions when recharge is above average and dry conditions where recharge is below average. The well fields currently (1981) are permitted by the Southwest Florida Water Management District to pump 186.9 Mgal/d from 164 wells (table 7). Within each well field, all wells were assumed to pump at the same rate and were assigned to the square-mile grid block in which they occur. Thus, pumpage was distributed as a function of well location rather than evenly throughout each well field.

Attachment C illustrates the data deck for the model interrogation of average recharge conditions. Attachment D illustrates the model output generated by the data deck. Figures 19-22 compare anticipated drawdowns in the water table and potentiometric surface under high, low, and average recharge conditions. Table 8 summarizes water-balance and water-level data under the various simulation runs and compares these values with initial nonpumping conditions.

The drawdown maps show a series of coalescing cones of depression. Because the well fields are close together, the cones will overlap or interfere with one another as they spread out. It should be realized that drawdown in the proximity of a single well field is increased by pumping from nearby well fields. Attachment E shows the model-simulated limits of cones of depression that should develop in the surficial and Floridan aquifers as the 10 well fields are pumped individually. Attachment E also shows three-dimensional graphical plots of the water table and potentiometric surface before and after pumping 10 well fields simultaneously.

When all 10 well fields are pumped simultaneously under average recharge conditions, the water table can be expected to drop 2 feet or more from nonpumping conditions over an area of about 163 mi<sup>2</sup>. A 2-foot or greater decline in the potentiometric surface would occur over an area of about 505 mi<sup>2</sup>. The maximum drawdowns occur at the well fields. For the entire modeled area, average drawdowns of 1.3 feet and 3.6 feet would occur in the surficial aquifer and Floridan aquifer, respectively. Recharge accounts for about 90 percent of the

Table 7.--Annual average permitted pumping rates used for model simulations

[Southwest Florida Water Management District, written commun., 1982]

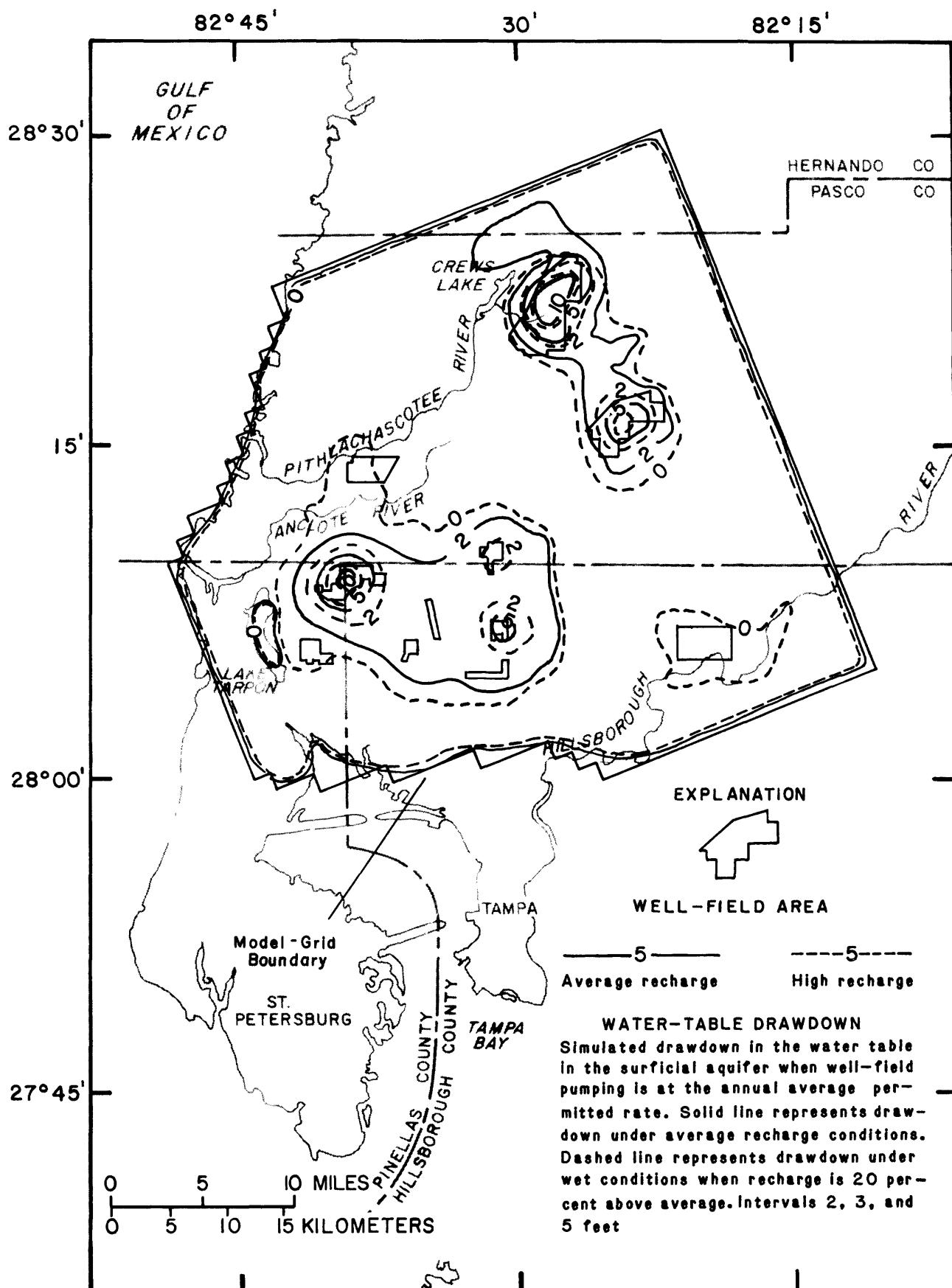
Well field	Annual average permitted pumping rate (Mgal/d)	Number of wells
Cross Bar Ranch	30	17
Cypress Creek	30	10
Starkey	8	5
Pasco County	16.9	8
Eldridge-Wilde	35.2	58
East Lake	3	8
Cosme	19	23
Section 21	18	7
Morris Bridge	18	20
Northwest <sup>1/</sup>	8.8	8
Total	186.9	164

<sup>1/</sup> Consumptive use permit No. 206676 is pending approval by the Southwest Florida Water Management District.

inflow to the system, and ET-runoff from the water table accounts for about 58 percent of the outflow. Of the 187 Mgal/d pumped, 159 Mgal/d were derived from reduced ET-runoff and the remainder from a combination of increased boundary inflow and downward leakage (table 8). Although the amount of water captured from ET-runoff seems high, it represents reduced ET and runoff from the water table and possibly increased recharge when the water table is lowered.

Under high recharge conditions, the 2-foot cone of depression decreases from 163 to 41 mi<sup>2</sup> in the water table and from 505 to 323 mi<sup>2</sup> in the Floridan aquifer. More than 90 percent (178 of 194 Mgal/d) of the increased recharge would be lost to evapotranspiration, however, leaving little of this increased quantity of water to leak to the Floridan aquifer.

Compared to average recharge conditions, inflow and outflow trends under low recharge conditions are reversed with respect to wet recharge conditions. The 2-foot cone of depression would expand from 163 mi<sup>2</sup> under average recharge conditions to 460 mi<sup>2</sup> under low recharge conditions in the water table and from 505 to 755 mi<sup>2</sup> in the Floridan aquifer. The water table would decline about 3.2 feet, or 1.9 feet more than under average recharge conditions. The average decline in the potentiometric surface would be less than the water-table decline, about 1.4 feet more than under average recharge conditions. In the ridge areas and north of the Cross Bar Ranch well field where there is little or no ET-runoff,



BASE FROM U.S. GEOLOGICAL SURVEY 1:500,000 FLORIDA MAP

Figure 19.--Model-simulated drawdown in the water table in the surficial aquifer under average and high recharge conditions with well fields pumping at the annual average permitted rates.

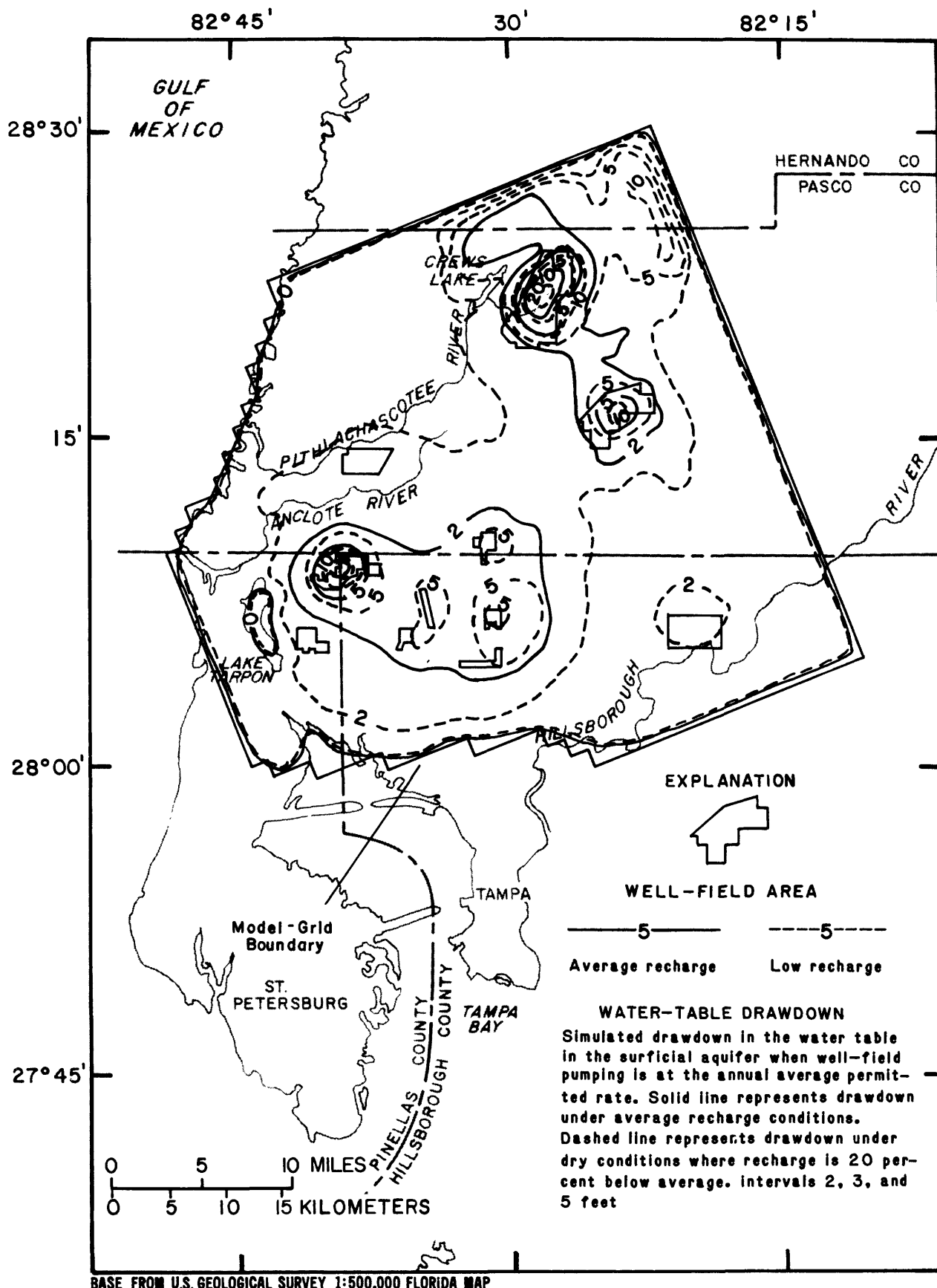


Figure 20.--Model-simulated drawdown in the water table in the surficial aquifer under average and low recharge conditions with well fields pumping at the annual average permitted rates.







Table 8.--Summary of water-balance and water-level data simulated by the model  
under varying conditions of recharge and pumping

A. WATER BALANCE FOR SURFICIAL AQUIFER (814 mi<sup>2</sup>)

	QRE Recharge (in/yr)	ETRO ET-runoff from water table (in/yr)	UL Upward leakage (in/yr)	DL Downward leakage (in/yr)
Initial nonpumping condition	25.1	20.3	1.6	6.4
Pumping with high recharge	30.1	20.8	1.0	10.3
Pumping with average recharge	25.1	16.2	1.0	9.9
Pumping with low recharge	20.0	11.7	1.0	9.4

Water balance: QRE + UL = ETRO + DL

B. WATER BALANCE FOR FLORIDAN AQUIFER (932 mi<sup>2</sup>)

	UL Upward leakage (in/yr)	DL Downward leakage (in/yr)	BI Boundary inflow (in/yr)	BO Boundary outflow (in/yr)	Pumpage (in/yr)
Initial nonpumping condition	1.6	6.7	1.1	6.2	0
Pumping with high recharge	1.1	10.1	1.2	6.0	4.2
Pumping with average recharge	1.1	9.8	1.2	5.7	4.2
Pumping with low recharge	1.0	9.4	1.2	5.4	4.2

Water balance: DL + BI = UL + BO + P

C. WATER-LEVEL CHANGE DATA

Recharge condition	Water table			Potentiometric surface		
	High	Aver- age	Low	High	Aver- age	Low
Area encompassing 2 feet or more drawdown (mi <sup>2</sup> )	41	163	460	323	505	755
Area encompassing 5 feet or more drawdown (mi <sup>2</sup> )	10	23	116	154	207	318
Maximum rise (feet)	10.2	0	0	.8	0	0
Maximum drawdown (feet)	9.9	14.9	19.3	21.9	23.2	26.0
Average drawdown (+) or rise (-) (feet)	1.0	1.3	3.2	2.5	3.6	5.0

relatively large declines develop in the water table. These declines resemble cones of depression, but probably result more from reduced recharge than from pumping. In areas where the water table is within 10 feet of land surface, a reduction in recharge results in a reduction in ET-runoff, and the water table declines moderately. The water-table decline is dampened by the capture of ET-runoff. In parts of the ridge areas, the water table is below the maximum ET-runoff capture depth (10 feet). This results in no dampening and an increased water-table decline.

Figures 19 through 22 illustrate water-level changes from predevelopment conditions due to the combined effects of pumping and varying recharge. Just the effects of pumping are illustrated under the average recharge condition. Under the high and low recharge conditions the effects of varying recharge could be negated by running the model with no pumping and varying recharge by 20 percent. The respective simulated water levels could then be input as starting heads for simulations under pumping conditions with high and low recharge. This modeling technique would filter out the effects of varying recharge and produce maps of drawdown caused only by pumping. A test of this type indicated that less than 3 feet of the water-table drawdown northeast of the Cross Bar Ranch well field was caused by pumping under the low recharge condition.

The constant-head boundary limits the accuracy for simulating changes in the water table near the perimeter of the model. For example, where a cone of depression reaches the boundary, the constant-head condition holds the water table constant, indicating that the boundary is an infinite source of water for the aquifer. Had a constant-flow boundary been used in the surficial aquifer, no change in boundary flow could be induced by pumping, and the water table would be lowered in excess of what might actually occur. Based on the sensitivity analysis of boundary conditions in the Floridan aquifer, the HCF condition would produce a water table somewhere near the extremes simulated under the constant-head and constant-flow conditions. However, the CSS and HSS arrays used for HCF in the Floridan aquifer were already dedicated to the ET-runoff capture function in the surficial aquifer, thus they were not available for HCF. The user should be aware that the model has a limited capability for predicting changes in hydrologic conditions within the surficial aquifer near the boundary.

#### Comparisons of Quasi-Three-Dimensional and Two-Dimensional Model Simulations

The phase 1 two-dimensional (2-D) model and phase 2 quasi-three-dimensional (Q-3-D) model produce different results when interrogated under the same pumping conditions. Because boundary conditions and hydraulic characteristics of the upper confining bed and Floridan aquifer are similar in both models, the differences may be attributed primarily to activating the water table in the Q-3-D model.

Maximum drawdowns in the potentiometric surface in the well fields simulated by each model are listed in table 9. Drawdown predicted by the Q-3-D model when the 10 well fields are pumped simultaneously at permitted capacities (table 7) is greater in every well field and averages about 4 feet more than that simulated by the 2-D model. In the 2-D model, the water table is held constant, but

Table 9.--Comparison of maximum drawdowns at 10 well fields simulated by the quasi-three-dimensional and two-dimensional models under various pumping conditions

Well field	Pumping rate (Mgal/d)	Maximum drawdown (feet)			
		Pump 10 well fields <sup>1/</sup>		Pump individual well fields	
		Potentiometric surface		Water table	Potentiometric surface
		Q-3-D	2-D	Q-3-D	Q-3-D
Cross Bar Ranch	30	18.3	7.8	14.6	17.9
Cypress Creek	30	23.2	19.8	7.2	22.8
Starkey	8	7.5	5.5	1.5	5.7
Pasco County	16.9	15.5	13.0	3.1	11.5
Eldridge-Wilde	35.2	21.3	14.6	9.1	19.3
East Lake	3	6.5	4.4	.6	2.5
Cosme	19	13.5	10.3	2.5	9.9
Section 21	18	19.5	11.2	4.1	14.0
Morris Bridge <sup>2/</sup>	18	7.7	6.3	1.9	7.6
Northwest <sup>2/</sup>	8.8	13.9	--	2.2	7.8

<sup>1/</sup> Because 10 well fields are pumped simultaneously, maximum drawdown in the proximity of a single well field is increased by pumping from nearby well fields.

<sup>2/</sup> Northwest well field was not proposed at time of 2-D model run.

in the Q-3-D model, it declines in response to the change in leakage induced by lowering the potentiometric surface at the well fields. Because leakage is proportional to head difference between the potentiometric surface and water table, the potentiometric surface must be drawn down more in the Q-3-D model than the 2-D model to induce a similar quantity of leakage. The additional drawdown results in a larger cone of depression around each well field and greater drawdown at the well-field boundaries than predicted by the 2-D model.

The comparison of maximum drawdowns in the well fields adds perspective to the 2-D and Q-3-D model simulations. The 2-D model minimizes drawdown, whereas the Q-3-D model depicts more realistic movement of the potentiometric surface. The two models may be used for comparing or bracketing expected drawdowns. However, the Q-3-D model, the final product of this two-phased investigation, is considered to more accurately represent the hydrogeologic system.

Comparison of model results between the Q-3-D model and the 2-D model of Robertson and Mallory (1977) could not be made as their model did not simulate drawdown in the Floridan aquifer. But, because leakance coefficient of the upper confining bed and transmissivity of the Floridan aquifer varies as much as 100 percent outside the well field areas where data are sparse, the models should not agree precisely.

At the Cypress Creek well field, a 2-D model by Ryder (1978) indicated that drawdown in the potentiometric surface would be at least 5 feet over an area of about 7 mi<sup>2</sup> and that maximum drawdown should be about 15 feet when the well field is pumped at 30 Mgal/d. Again, this model contains a fixed water table, so drawdown should be less than that simulated by the Q-3-D model. The map of drawdown at Cypress Creek well field presented in attachment E supports this contention in that the 5-foot cone of depression in the Floridan aquifer should expand over a 27-mi<sup>2</sup> area and maximum drawdown should be about 23 feet. Maximum drawdown in the water table should be about 7 feet. Note that the head difference between the water table and potentiometric surface in the grid block of maximum drawdown is 16 feet, which compares favorably with Ryder's (1978) 15-foot head difference.

At the Morris Bridge well field, the Q-3-D model by Ryder and others (1980) indicated that when the well field is pumped at 40 Mgal/d, drawdown in the potentiometric surface would be 5 feet or more over an area of 20 mi<sup>2</sup> and that maximum drawdown should be about 15 feet. In the current Q-3-D model study, the well field was pumped at its permitted average rate of 18 Mgal/d, thus drawdown and spread of the cone of depression should be about half those predicted by the model developed by Ryder and others (1980). The map of drawdown at Morris Bridge well field presented in attachment E indicates that drawdown of 5 feet or more should spread over 8 mi<sup>2</sup> and maximum drawdown in the potentiometric surface should be about 7.6 feet. Water-table declines simulated by the two models were similar.

#### LIMITATIONS OF MODEL APPLICATION

A conceptual approach to ground-water modeling was used in the application of this model. The hydrogeologic system was conceptualized, its parameters identified, and it was transformed to the mathematical analog. The mathematical model approximates the physical processes that control the conceptual model, but it is only an approximate representation of the prototype.

The hydrogeology has been simplified to the extent that an operational mathematical model could be constructed. The mathematical solution is an approximate solution to the differential equations that define the system. Because the model grid is on a coarse regional scale of 1 mi<sup>2</sup>, the localized impact of pumping small quantities of water will not be accurately depicted. Also, because of mathematical approximations associated with simulating boundary flow, the impact of pumping large quantities of water near the model-grid boundary may not be accurately depicted. Boundary assumptions also limit the accuracy for simulating drawdown in the water table near the perimeter of the model under other than average recharge conditions. Additional computational errors may be introduced in coastal areas, particularly in the East Lake, Eldridge-Wilde, and Starkey well fields, because the model does not consider movement of the freshwater-saltwater interface and the resultant displacement of a less dense fluid by another of greater density. A model limitation that could lead to significant errors occurs when the water table rises above land surface or falls to the base of the surficial aquifer. The model will flag areas where those phenomena occur. The model also only grossly accounts for changes in recharge and runoff that result from changes in the water table. Finally, because the model assumes a steady-state condition, the solution is not time dependent, and the time required for computed heads to reach these levels cannot be determined from this model.

The predictive-model runs exemplify the types of analyses possible with the Q-3-D model. Generally, it can be used to compute water-balance and regional water-level changes in response to various distributions of pumping and conditions of recharge. Local changes such as inflections in the water table near streams cannot be computed because any stream generally occupies less than one-fiftieth the area of a node. Because the model simulates long-term average water-level changes that should result from pumping, extreme high or low conditions could be significantly different from simulated conditions. Ideally, the model should represent all characteristics of the prototype, but realistically, it represents a few of the more important characteristics of the hydrologic system. The model simulates ground-water flow on a megascopic scale.

## COMPUTER PROGRAM

The computer program documented here is written for the AMDAHL<sup>1</sup> 470-V6 MVS system installed at the U.S. Geological Survey office in Reston, Va. The generic program is documented in Trescott (1975) with an expansion of the documentation in Trescott and Larson (1976). The program was modified for this study to (1) include the HCF condition in the lower layer, (2) compute ET-runoff capture from the upper layer, (3) produce parameter maps, (4) generate drawdown data in a format compatible with the CALCOMP (California Computer Products, Inc., 1971) contouring program, and (5) replace or add certain iteration parameters used in the strongly implicit procedure. Because the FORTRAN code of Trescott was modified extensively, the model may be considered as developed specifically for the well-field area. The model is not recommended for other applications unless the code is reviewed. A complete listing of the computer program is presented in attachment A.

Memory requirements and running time depend upon the size and complexity of the physical situation being simulated. For the field application documented herein, which utilized 1,224 nodes per layer over two layers, an average model run required 144,000 bytes of core memory on the FORTRAN G1 compiler, 300,000 bytes for executing the program, and about 15 seconds of Central Processing Unit time on the Geological Survey's computer.

## Head-Controlled Flux Condition

### Theory

In a recent modeling investigation (Wilson and Gerhart, 1980), a head-controlled flux (HCF) condition was introduced. The HCF condition allows head and flux to change at the model-grid boundary, thus adding flexibility to the Q-3-D model (Trescott, 1975) that previously incorporated only constant-head and constant-flux conditions. Under the HCF condition, flux across the model-grid boundary in the Floridan aquifer varies as a function of the potentiometric surface

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<sup>1</sup>The use of brand names in this report is for identification purposes only and does not imply endorsement by the U.S. Geological Survey.

at the HCF grid block. Because transmissivity of the surficial aquifer is low, boundary flux is not considered to be significant, and the HCF condition was not applied to the upper layer.

The HCF condition is useful in situations where simulated stresses spread to a model boundary (thus rendering constant head or constant flux unrealistic), and it is undesirable to increase the size of the modeled area by expanding the grid to a point where stress effects are negligible. Although the physical boundaries of the model grid remain stable, the HCF condition calculates a boundary-flow component based on a strip of aquifer as wide as the HCF grid-block edge and extending laterally a specified distance to a point of constant head. Thus, the HCF condition does not expand the model area or the grid upon which numerical solutions to the flow equation are calculated.

Figure 23 is a conceptualization of the HCF condition. The assumptions are made that (1) there is a point beyond the model grid (at distance L) where the water table (HSS) and the stressed potentiometric surface (PHI) will remain constant and are equal to the starting potentiometric surface (STRT); and (2) the transmissivity (T) and confining-bed leakance (TK) are constant in the aquifer strip between the model-grid boundary and the constant-head boundary. The assumptions allow reasonable finite boundaries to be placed on extensive aquifer systems that lack natural hydrologic boundaries.

Equations for solving boundary discharge under the HCF condition were developed at the U.S. Geological Survey's Northeast Region Research Project Office (J. V. Tracy and S. P. Larson, written commun., 1979). Names of variables used here conform to names of variables used in the model code. The governing equation for steady flow in the region  $0 \leq x \leq L$  outside the modeled area is:

$$\frac{\partial^2 \text{PHI}_x}{\partial x^2} - \frac{\text{TK}}{T} (\text{PHI}_x - \text{HSS}) = 0, \quad (5)$$

where  $\text{PHI}_x$  = altitude of the stressed potentiometric surface at distance x (feet);  
HSS = altitude of the water table (feet);  
TK = confining-bed leakance (feet per second per foot);  
T = transmissivity of the Floridan aquifer (feet squared per second);  
x = the distance from the model-grid boundary, for which the equation is to be solved (feet).

Under the assumption that the water table is constant in the aquifer strip:

$$\frac{\partial^2 \text{HSS}}{\partial x^2} = 0, \quad (6)$$

when subtracted from both sides of equation 5

$$\frac{\partial^2 \text{PHI}_x}{\partial x^2} - \frac{\partial^2 \text{HSS}}{\partial x^2} - \frac{\text{TK}}{T} (\text{PHI}_x - \text{HSS}) = 0. \quad (7)$$



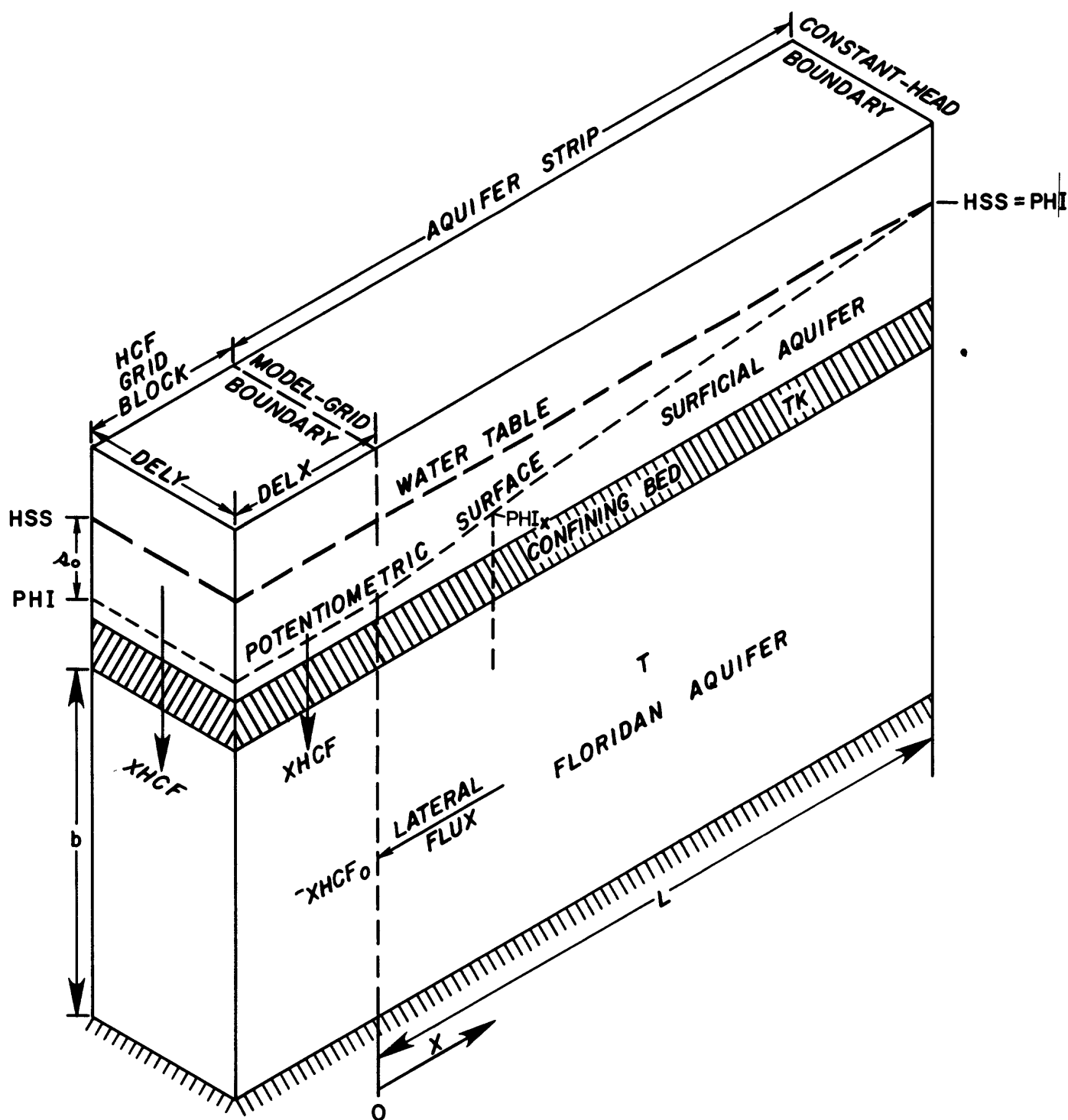


Figure 23.--Conceptualization of the head-controlled flux (HCF) condition, as applied to boundaries of the quasi-three-dimensional model.

Therefore

$$\frac{\partial^2 s}{\partial x^2} - \lambda s = 0, \quad (8)$$

where

$$s = \text{PHI}_x - \text{HSS}, \text{ and}$$

$$\lambda = \frac{\text{TK}}{T}.$$

Verruijt (1970, p. 30) showed the solution to be of the form:

$$s = c_1 e^{x\sqrt{\lambda}} + c_2 e^{-x\sqrt{\lambda}}. \quad (9)$$

Boundary conditions are

$$s = \text{PHI} - \text{HSS} = s_0 \text{ at } x = 0, \text{ and}$$

$$s = 0 \text{ at } x = L.$$

Therefore

$$s_0 = c_1 e^0 + c_2 e^{-0} = c_1 + c_2, \quad (10)$$

$$0 = c_1 e^{L\sqrt{\lambda}} + c_2 e^{-L\sqrt{\lambda}}. \quad (11)$$

Solving 10 and 11 simultaneously for  $c_1$  and  $c_2$

$$c_1 = \frac{-s_0 e^{-2L\sqrt{\lambda}}}{(1 - e^{-2L\sqrt{\lambda}})}, \quad (12)$$

$$c_2 = \frac{s_0}{(1 - e^{-2L\sqrt{\lambda}})}, \quad (13)$$

and

$$s = s_0 \frac{(e^{-x\sqrt{\lambda}} - e^{(x-2L)\sqrt{\lambda}})}{(1 - e^{-2L\sqrt{\lambda}})}. \quad (14)$$

If equation 8 is solved for  $s$ , then Darcy's law may be applied at the model-grid boundary ( $x = 0$ ) to solve for lateral boundary flux,  $XHCF_0$ :

$$XHCF_0 = - \frac{T \partial s}{\partial x} \Big|_{x=0}, \quad (15)$$

$$= -T \frac{-\sqrt{\lambda} e^{-x\sqrt{\lambda}} - \sqrt{\lambda} e^{(x-2L)\sqrt{\lambda}} s_0}{(1 - e^{-2L\sqrt{\lambda}})} \Big|_{x=0} \quad (16)$$

$$= -Ts_0 \frac{(-\sqrt{\lambda} - \sqrt{\lambda} e^{-2L\sqrt{\lambda}})}{(1 - e^{-2L\sqrt{\lambda}})} \quad (17)$$

$$= -Ts_0 \sqrt{\lambda} \frac{(1 + e^{-2L\sqrt{\lambda}})}{(1 - e^{-2L\sqrt{\lambda}})} \quad (18)$$

$$= -Ts_0 \sqrt{\lambda} \frac{(1 + e^{-2L\sqrt{\lambda}})}{(1 - e^{-2L\sqrt{\lambda}})} \quad (19)$$

$$XHCF_0 = -CSSs_0 = -CSS (HSS - PHI). \quad (20)$$

Inflow to the model area is  $-XHCF_0$ , or  $CSS(HSS-PHI)$ .  $XHCF_0$  is the volumetric flow per unit width and this must be multiplied by the grid-block width (DELY, in fig. 23) to obtain the horizontal volumetric flow rate  $Q$  across the grid face:

$$Q = -XHCF_0 \cdot DELY. \quad (21)$$

This volume of water is distributed in the model as vertical leakage over the HCF grid block by dividing  $Q$  by the grid-block area:

$$XHCF = \frac{-XHCF_0 \cdot DELY}{DELX \cdot DELY} \quad (22)$$

$XHCF$  is really only normalized to be consistent dimensionally with the other fluxes computed by the model. The true flux is the quotient of  $Q$  and the grid face area ( $DELY \cdot b$ ). The grid-block area is used as the divisor instead because  $DELX \cdot DELY$  is used as a multiplier in the CHECKI subroutine to convert all fluxes to volumetric flow rates.

Substituting equation 19 for  $XHCF_0$  results in the equation:

$$XHCF = (HSS - PHI) \cdot \frac{T\sqrt{\lambda}}{DELX} \cdot \frac{(1 + e^{-2L\sqrt{\lambda}})}{(1 - e^{-2L\sqrt{\lambda}})} \quad (23)$$

which is the product of the head difference in the boundary grid block ( $HSS-PHI$ ) and the HCF condition leakage factor,  $CSS$ , described earlier in equation 3. The resulting function for boundary flux:

$$XHCF = (HSS - PHI) \cdot CSS \quad (24)$$

is a form of equation 4 and is listed several times in the SOLVE and CHECKI sub-routines of the program code in Attachment A. CSS is the HCF condition leakage factor comprising transmissivity, leakance, length of the aquifer strip outside the model-grid boundary, and length of HCF grid block parallel to the boundary flux (see equation 3).

Equation 24 is a generalized expression for discharge as a function of head--hence the name "head-controlled flux." It is generalized in the sense that the constant CSS will be different for other system conceptualizations. For example, if the aquifer beyond the model-grid boundary were not leaky, CSS would be different, but equation 24 would still be valid.

The expression for lateral flux ( $XHCF_0$ ) at a boundary is exactly analogous to vertical leakage; the constant CSS can be considered as a "leakance." This constant is added to the vertical leakage in the HCF grid block(s). With this increased vertical leakage, the amount of water flowing into or out of the HCF grid block is the sum of true vertical leakage and lateral flow at the edge.

#### Assumptions and Restrictions

An important assumption in using the HCF condition is that of uniform aquifer properties beyond the model-grid boundary. Transmissivity, vertical hydraulic conductivity of the confining bed, confining bed thickness, and so forth, are all considered to be uniform and equal to their respective values in the HCF grid blocks from which they derive. If the data support this assumption, then the HCF condition can be a fair approximation; however, if the data show a wide range of values or an irregular distribution of values, then the use of the HCF condition should be qualified.

A source of error in the HCF condition is in the estimation of flow across the model-grid boundary at the corners of the model area. Figure 24 indicates that there is a substantial area at each corner in which the amount of boundary flow caused by a head change is ignored.

#### Use

The programming changes and additions that are necessary to include the HCF condition in the Q-3-D model are listed in attachment A. The data-deck instructions listed in attachment B include instructions used to specify the HCF condition in a model run.

An HCF grid block is defined as a grid block on the edge of the model grid that has an outside edge perpendicular to the main direction of flow that will be caused by a change in head in the HCF grid block (fig. 24). In irregularly shaped grids, there may be many grid blocks that could be designated corner grid blocks (grid blocks with two edges corresponding to the model-grid boundary). To avoid overlapping of the aquifer strips extending out from each boundary grid

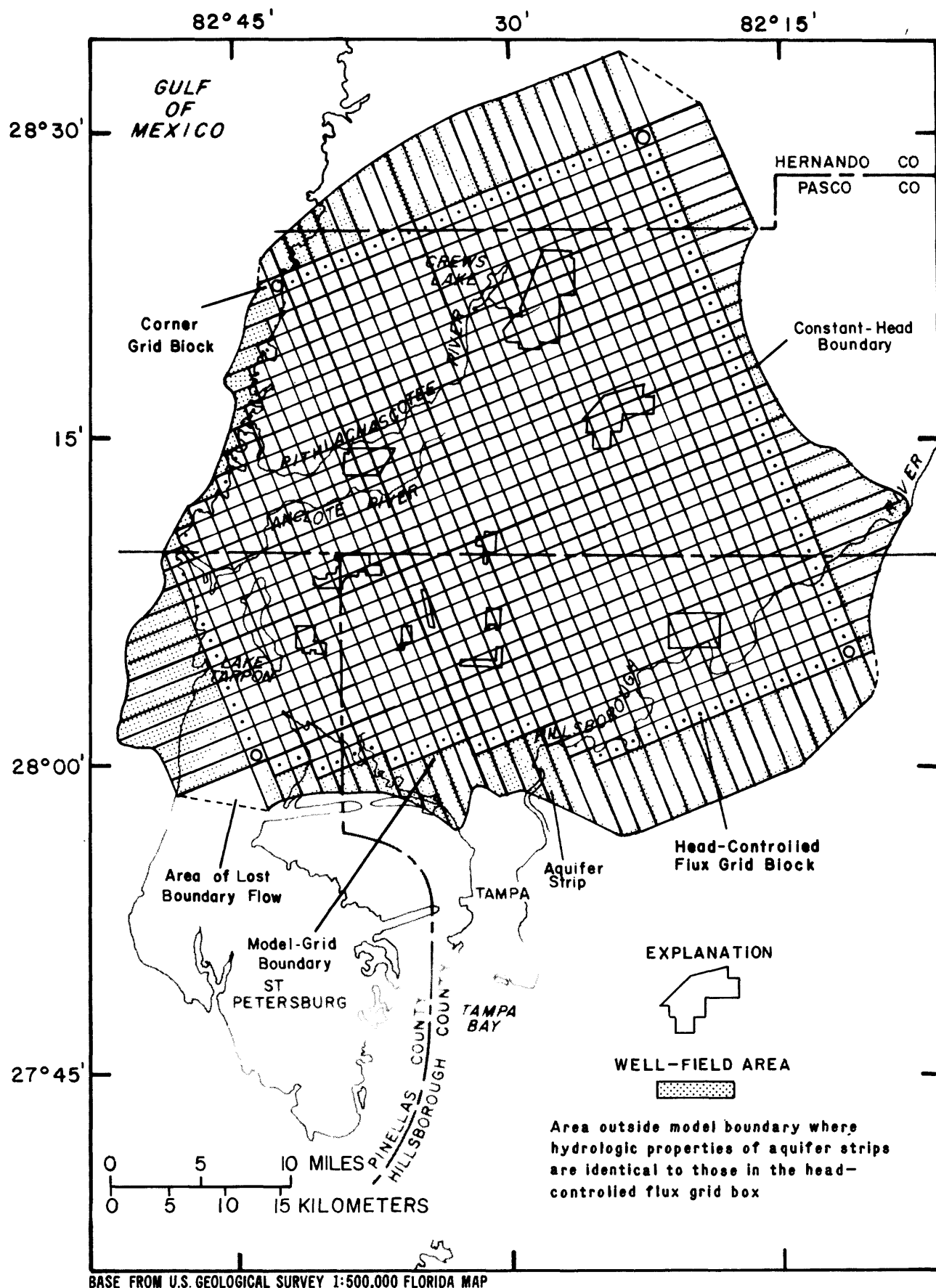


Figure 24.--Well-fields area model with head-controlled flux (HCF) condition, showing HCF and corner grid blocks, areas of lost boundary flow, and orientation of aquifer strips (modified from Hutchinson and others, 1981).

block, the program changes outlined in attachments A and B require that the user designate only four of these possible corners as corner grid blocks. An additional requirement is that between two corner grid blocks, a boundary must be straight or convex with respect to the model area. Between two corner grid blocks, all the aquifer strips extend out in the same direction from the model-grid boundary. The model area in figure 24 is a typical case that conforms to the above requirements specified in the program changes in attachment A.

Flow rates at each HCF grid block may be printed out by removing the "C" from column 1 of card 11710 of the program code (attachment A). The total leakage that is due to lateral HCF flow into or out of the model-grid boundary nodes is included as leakage in the mass balance printout.

Finally, if the HCF condition is to be used in a steady-state calibration, the water levels in the grid blocks adjacent to the HCF grid blocks and just across the boundary from them must be included in the STRT matrix. The transmissivities in these grid blocks must be zero, however, since they are beyond the model-grid boundary.

### Evapotranspiration-Runoff Capture

Evapotranspiration (ET) from the water table and runoff (RO) captured by lowering the water table are modeled together as a head-dependent outflow function. Coding changes for incorporating ET-runoff into the model (Trescott, 1975) were described by J. V. Tracy and S. P. Larson (written commun., in an advanced modeling seminar at the U.S. Geological Survey Training Center in Denver, Colo. The mathematical form of the equation for determining the volumetric ET-runoff rate in the model is equivalent to equation 24:

$$ETFLUX = CSS \cdot (HSS - PHI) \quad (25)$$

where       $ETFLUX$  = ET-runoff flux (feet per second);  
              $CSS$  = maximum ET-runoff rate (per unit area) divided by maximum  
                                 depth at which ET-runoff capture occurs (feet per second  
                                 per foot);  
              $HSS$  = altitude of the base of the ET-runoff capture zone (feet);  
                                 and  
              $PHI$  = altitude of the water table (feet).

Corrections to the computed ET-runoff flux must be made under two conditions:

1. The water table lies below the base of the ET-runoff capture zone--ET-runoff would become positive, representing inflow to the system.
2. The water table rises above land surface--ET-runoff would exceed the potential ET-runoff capture rate, representing excessive outflow from the system.

Program modifications were made to check for and correct these special cases. This was accomplished by first computing a total ET-runoff flux using equation 25. Then a check was made for conditions 1 and 2 above. The total ET-runoff flux was then adjusted by subtracting the extra inflow or adding back the excessive outflow. Thus, the same arrays are utilized in the model to compute head-controlled flux across the perimeter of the modeled area in the Floridan aquifer (layer 1) and ET-runoff flux from the surface of the water table in the surficial aquifer (layer 2).

## Parameter Maps

The original computer program of Trescott (1975) produced maps of head and drawdown and listed input parameters in tabular form. To expedite the calibration procedure, C. H. Tibbals (U.S. Geological Survey, written commun., 1981) modified the program to produce maps of transmissivity and leakage. This modification was expanded upon for the well-field areas model so that the program would produce additional maps of head difference between the water table and potentiometric surface, recharge rate to the water table, ET-runoff rate from the water table, leakage rate through the upper confining bed, and the distribution of pumpage.

The parameter maps provide a useful tool for assessing predictive model runs and simplifying the calibration procedure. For example, under predicted pumping conditions, areas of greatest change in ET-runoff, leakage, and reversal of head can be easily detected. The map of pumpage distribution allows correlation of pumping centers with the other parameter maps and helps locate errors in pumping rates input to the model for predictive runs.

## Contour Mapping

A program modification that proved useful during the modeling process was the ability to punch drawdown and hydraulic head in the format used by the CALCOMP (California Computer Products, 1971) contouring program. All contoured illustrations in this report were traced from machine-drawn maps produced by the CALCOMP contouring program.

To punch the model output in CALCOMP format, cards 6290 and 6500 of the program are activated manually. The punched cards are then combined with control cards described in the CALCOMP manual (1971) and submitted as a separate program to the Survey's computer. Contour maps output from the CALCOMP contouring program are displayed on a Tektronix 4014-1 terminal at the U.S. Geological Survey Tampa Subdistrict Office. The output was written to magnetic tape for processing on a flat-bed plotter that draws contours on translucent paper to overlay base maps of any scale.

SAS/GRAPH, a program published by the SAS Institute, Inc. (1980), was used to portray three-dimensional graphical representations of the water table and potentiometric surface under predevelopment and pumping conditions (attachment E). The procedure for 3-D plotting is the same as that for the CALCOMP plots. The 3-D plots exhibit depth perspective that cannot be perceived from contour maps.

## Iteration Parameters

The strongly implicit procedure (SIP) utilizes an iterative scheme to solve the flow matrix in the model. In this approach, a modifying matrix is added to the flow matrix thereby simplifying factorization. The iterative technique results in considerable savings in computer time and storage over the method of Gaussian elimination (Trescott, 1975, p. 12).

The original version of the model cycles a sequence of iteration parameters ranging from zero to 1 until convergence is achieved. The minimum parameter is not critical and zero is normally chosen. Trescott (1975, p. 25) recommends that,

"if the sequence of parameters computed by the equations in the program are all (except the first parameter) close to 1.0 and if this results in slow convergence or even divergence, bypass the computations in the model and insert  $WMAX \approx 0.99863$ ."

For expediency in the well-fields model, line 7430 was inserted to override computation of the maximum iteration parameter, setting it equal to the value recommended by Trescott.

Convergence may be achieved more rapidly by multiplying the finite-difference residual by an acceleration parameter, BETA, based on test results of Trescott and others (1976, p. 27) that emphasize the advantages of this extra SIP iteration parameter. BETA was introduced as a multiplier in lines 8300, 8430, 9210, and 9340 of the computer program. In the early stages of the well-field model development, several values were chosen for BETA based on an optimum range of 0.1 to 1.5 inferred by Trescott and others (1976). Early test runs indicated that this model is relatively insensitive to BETA values near 1.0, so it was held constant at unity during the calibration, validation, and prediction phases of the modeling process.

## SUMMARY AND CONCLUSIONS

Ten well fields north of Tampa are proposed or are currently supplying the city of Tampa and Gulf Coast municipalities with freshwater. A Q-3-D model of ground-water flow was developed to gain a better understanding of the hydrology and to facilitate water-resources planning and management. This report describes development of the Q-3-D model and its application in assessment of the impact of existing and proposed well fields. The study was preceded by a phase 1 two-dimensional model, used for planning and management during development of the more complex phase 2 model.

The Q-3-D model accounts for inflow to and outflow from two aquifers (surficial and Floridan) separated by a leaky confining bed. Water pumped from the Floridan aquifer at a well field at steady state is supplied by reducing outflow or increasing inflow laterally across the model boundary and by reducing upward leakage or increasing downward leakage through the overlying confining bed. Changes in the amount of vertical leakage induce declines in the water table of the surficial aquifer. Lowering the water table reduces the supply of water available for evapotranspiration by surface vegetation, reduces base streamflow and storm runoff, and increases the aquifer's capacity for accepting recharge. Ultimately, the source of the pumped water is by capture of water that would normally be subject to evapotranspiration or runoff. Thus, the model may be used to assess the effects of pumping one well field, or the interference effects of pumping multiple well fields, on drawdown in the aquifers, changes in leakage, ET-runoff, and lateral flow.



The Q-3-D model has advantages over the 2-D model in that the upper layer (surficial aquifer) is active and the water table may vary. Because the water table in the 2-D model was held constant, drawdown in the Floridan aquifer was minimized. The Q-3-D model more accurately simulates the physical system by allowing movement of the water table, thereby minimizing leakage changes and maximizing drawdowns.

Example model-interrogation runs simulate water-level and water-balance changes that can be expected as a result of pumping all 10 well fields simultaneously at annual average permitted rates totaling 186.9 Mgal/d from the Floridan aquifer with recharge varying 20 percent more and less than the long-term average rate. Under these conditions, water-level and water-balance changes with respect to nonpumping conditions should be as follows:

1. Under high recharge conditions, the water table should decline an average of 1.0 foot, and the potentiometric surface should decline an average of 2.5 feet. If average recharge increases from 25 to 30 inches per year, downward leakage should increase from 6.7 to 10.1 inches per year, and ET-runoff from the water table should increase from 20.3 to 20.8 inches per year.
2. Under average recharge conditions, the water table should decline an average of 1.3 feet, and the potentiometric surface should decline an average of 3.6 feet. If recharge remains at 25 inches per year, downward leakage should increase from 6.7 to 9.8 inches per year, and ET-runoff should decrease from 20.3 to 16.2 inches per year.
3. Under low recharge conditions, the water table should decline an average of 3.2 feet, and the potentiometric surface should decline an average of 5.0 feet. If recharge is reduced from 25 to 20 inches per year, downward leakage should increase from 6.7 to 9.4 inches per year, and ET-runoff should decrease from 20.3 to 11.7 inches per year.

Maximum drawdown in the Floridan aquifer simulated by the Q-3-D model is greater in every well field and averages about 4 feet more than maximum drawdown simulated by the 2-D model previously developed for the area. Under average recharge conditions, about 75 percent of the pumped water is derived by increasing downward leakage. The remaining 25 percent is gained by reducing natural upward leakage in swamp and marsh areas and by slightly reducing outflow along the model boundary. Ultimately, more than 95 percent of the pumped water is derived by reducing evapotranspiration from the water table and runoff.

When well fields are pumped individually, drawdown in the potentiometric surface is less than when all 10 well fields are pumped simultaneously. Drawdown is much greater in the center of the modeled area where well fields are in close proximity to one another, thus increasing interference effects. Interference effects range from about 0.1 foot of additional drawdown at Morris Bridge well field to 6.1 feet of additional drawdown at Northwest well field.

Conclusions drawn from this modeling study concerning hydrology of the aquifer system and impact of pumping include:

1. Transmissivity of the Floridan aquifer ranges from about 26,000 to 475,000 ft<sup>2</sup>/d.

2. Leakage coefficient of the upper confining bed ranges from about 0.00015 to 0.008 (ft/d)/ft.
3. Recharge rate to the surficial aquifer ranges from near zero in the coastal marsh and central swamp physiographic units to 30 inches per year in the lakes terrace physiographic unit.
4. Evapotranspiration plus runoff from the water table in the surficial aquifer ranges from zero in some ridge areas where the water table is deep to 38 inches per year in some swampy areas where the water table lies at land surface and is maintained by upward leakage from the Floridan aquifer.
5. The annual water balance for the surficial aquifer, representing long-term average hydrologic conditions prior to pumping, equates inflow and outflow as:

$$\begin{array}{rccccccc}
 \text{Recharge} & + & \text{Upward} & = & \text{Evapotranspiration} & + & \text{Downward} \\
 & & \text{leakage} & & \text{plus runoff} & & \text{leakage} \\
 & & & & \text{from water table} & & \\
 25.1 \text{ inches} & + & 1.6 \text{ inches} & = & 20.3 \text{ inches} & + & 6.4 \text{ inches}
 \end{array}$$

6. The annual water balance for the Floridan aquifer equates inflow and outflow as:

$$\begin{array}{rccccccc}
 \text{Downward} & + & \text{Boundary} & = & \text{Upward} & + & \text{Boundary} \\
 \text{leakage} & & \text{inflow} & & \text{leakage} & & \text{outflow} \\
 6.7 \text{ inches} & + & 1.1 \text{ inches} & = & 1.6 \text{ inches} & + & 6.2 \text{ inches}
 \end{array}$$

7. Nearly all of the water pumped from the Floridan aquifer is derived by increasing leakage.
8. Pumpage from the Floridan aquifer is ultimately derived by capturing water that would normally be subject to evapotranspiration or runoff.
9. Well-field interference significantly increases drawdown at the Starkey, Pasco County, Eldrige-Wilde, East Lake, Cosme, Section 21, and Northwest well fields, which are in close proximity to one another.
10. Concentrated pumping from the Floridan aquifer develops extensive cones of depression in the potentiometric surface and the water table.
11. Possible adverse impacts of pumping from the municipal well fields include upconing of deep saline water, saltwater encroachment along the coast, interference with existing private wells, lowering lake levels, inducing sinkhole collapse, and alteration of vegetal cover.

The well-field areas model results could be improved by increasing the accuracy of the input data. Additional data are needed to define the distribution of potential evapotranspiration and how it varies with depth. An evaluation of recharge potential based on soil type, topography, and depth to water table would allow the model to be programmed so that recharge varies in response to water-table fluctuations. The model could then forecast the decline in recharge as the water table rises to land surface, or conversely, the increase in recharge over former swampy areas that go dry.

The Q-3-D model has been developed for regional assessment of environmental impact and management of the ground-water reservoir to avoid potentially large

drawdowns. The model can also aid water resources managers, hydrologists, and the general public in the following:

1. Regulating drawdowns;
2. Evaluating well permits;
3. Considering economic constraints of power consumption and well spacing;
4. Assessing the potential for saltwater intrusion;
5. Assessing the potential impact of well-field development on lake levels;
6. Siting water-level and water-quality monitoring wells;
7. Relating water-table fluctuations to changes in vegetation; and
8. Designing aquifer tests.

Although the guidelines presented here are general, they provide a framework for the effective utilization of the model. The decisionmaker should be able to understand the model, recognize its limitations and predictive capabilities, and make knowledgeable evaluations of its output.

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## ATTACHMENT A: COMPUTER PROGRAM LISTING

The generic FORTRAN program by Trescott (1975) for simulating three-dimensional ground-water flow has been modified to a 1,576-card program. Major coding changes were made in the "SOLVE" subroutine to accommodate the HCF condition, ET-runoff capture, and iteration parameters. The "CHECKI" subroutine was modified to incorporate ET-runoff and boundary flux in the mass balance and to compute statistics. The "PRINTAI" subroutine was modified to produce parameter maps.

ATTACHMENT A: COMPUTER PROGRAM LISTING (modified from Trescott, 1975)

```

C -----00000010
C FINITE-DIFFERENCE MODEL FOR SIMULATION OF GROUND-WATER FLOW IN 00000020
C THREE DIMENSIONS, SEPTEMBER, 1975 BY P.C. TRESCOTT, U. S. G. S. 00000030
C WITH CONTRIBUTIONS TO MAIN, DATAI AND SOLVE BY S.P. LARSON 00000040
C MODIFIED BY C. TIBBALS AND C. HUTCHINSON, 1981 00000050
C -----00000060
C SPECIFICATIONS: 00000070
C REAL *8YSTR 00000080
C DIMENSION Y(52000) , L(25), HEADNG(33), NAME(42), INFT(2,2), IOFT(00000090
19,4), DUM(3) 00000100
C EQUIVALENCE (YSTR,Y(1)) 00000110
C COMMON /INTEGR/ IO,JO,KO,I1,J1,K1,I,J,K,NPER,KTH,ITMAX,LENGTH,KP,N00000120
1WEL,NUMT,IFINAL,IT,KT,IHEAD,IDRAW,IFLO,IERR,I2,J2,K2,IMAX,ITMX1,NC00000130
2H,IDK1,IDK2,IWATER,IQRE,IP,JP,IQ,JQ,IK,JK,K5,IPU1,IPU2,ITK,IEQN,KQ00000140
COMMON /SPARAM/ TMAX,CDLT,DELT,ERR,TEST,SUM,SUMP,QR 00000150
COMMON /SARRAY/ ICHK(14),LEVEL1(4),LEVEL2(4),LEVEL3(4),LEVEL4(4),L00000160
1LEVEL5(4),LEVEL6(4),LEVEL7(4) 00000170
C DATA NAME/2*4H ,4H S,4HTART,4HING ,4HHEAD,4H ,4H STO,4HRAG00000180
1E,4H COE,4HFFIC,4HIENT,2*4H ,4H TR,4HANSM,4HISSI,4HVITY,5*4H 00000190
2 ,4H TK,4H HY,4HDRAU,4HLIC ,4HCOND,4HUCTI,4HVITY,2*4H ,4HBOT00000200
3T,4HOM E,4HLEVA,4HTION,2*4H ,4H R,4HECHA,4HRGE ,4HRATE/ 00000210
C DATA INFT/4H(20F,4H4.0),4H(8F1,4H0.4)/ 00000220
C DATA IOFT/4H(1H0,4H,I2,,4H2X,2,4HOF6.,4H1/(5,4HX,20,4HF6.1,4H)) ,00000230
14H ,4H(1H0,4H,I5,,4H14F9,4H.5/(,4H1H ,4H5X,1,4H4F9.,4H5)) ,4H 00000240
2 ,4H(1H0,4H,I5,,4H10E1,4H2.5/,4H(1H ,4H,5X,,4H10E1,4H2.5),4H) 00000250
3,4H(1H0,4H,I5,,4H10E1,4H1.3/,4H(1H ,4H,5X,,4H10E1,4H1.3),4H) / 00000260
C DEFINE FILE 2(20,1200,U,KKK),3(1,50,U,KKK),4(1,24,U,KKK),8(3,1,U,K00000270
1KK),9(3,1200,U,KKK) 00000280
C .....00000290
C ---READ TITLE, PROGRAM SIZE AND OPTIONS--- 00000300
C READ (5,200) HEADNG 00000310
C WRITE (6,190) HEADNG 00000320
C READ (5,160) IO,JO,KO,ITMAX,NCH 00000330
C WRITE (6,180) IO,JO,KO,ITMAX,NCH 00000340
C READ (5,210) IDRAW,IHEAD,IFLO,IDK1,IDK2,IWATER,IQRE,IPU1,IPU2,ITK 00000350
1,IEQN 00000360
C WRITE (6,220) IDRAW,IHEAD,IFLO,IDK1,IDK2,IWATER,IQRE,IPU1,IPU2,ITK00000370
1,IEQN 00000380
C IERR=0 00000390
C ---COMPUTE DIMENSIONS FOR ARRAYS--- 00000400
C J1=JO-1 00000410
C I1=IO-1 00000420
C K1=KO-1 00000430
C I2=IO-2 00000440
C J2=JO-2 00000450
C K2=KO-2 00000460
C IMAX=MAX0(IO,JO) 00000470

```

# MAIN

NCD=MAX0(1,NCH)	00000480
ITMX1=ITMAX+1	00000490
ISIZ=I0*J0*K0	00000500
IK1=I0*J0	00000510
IK2=MAX0(IK1*K1,1)	00000520
ISUM=2*ISIZ+1	00000530
L(1)=1	00000540
DO 30 I=2,14	00000550
IF (I.NE.8) GO TO 20	00000560
L(8)=ISUM	00000570
ISUM=ISUM+IK2	00000580
IF (IK2.EQ.1) GO TO 10	00000590
IK=I0	00000600
JK=J0	00000610
K5=K1	00000620
GO TO 30	00000630
10 IK=1	00000640
JK=1	00000650
K5=1	00000660
GO TO 30	00000670
20 L(I)=ISUM	00000680
ISUM=ISUM+ISIZ	00000690
30 CONTINUE	00000700
L(15)=ISUM	00000710
ISUM=ISUM+J0	00000720
L(16)=ISUM	00000730
ISUM=ISUM+I0	00000740
L(17)=ISUM	00000750
ISUM=ISUM+K0	00000760
L(18)=ISUM	00000770
ISUM=ISUM+IMAX	00000780
L(19)=ISUM	00000790
ISUM=ISUM+K0*3	00000800
L(20)=ISUM	00000810
ISUM=ISUM+ITMX1	00000820
L(21)=ISUM	00000830
ISUM=ISUM+3*NCD	00000840
L(22)=ISUM	00000850
ISUM=ISUM+NCD	00000860
L(23)=ISUM	00000870
IF (IWATER.NE.ICHK(6)) GO TO 40	00000880
ISUM=ISUM+IK1	00000890
L(24)=ISUM	00000900
ISUM=ISUM+IK1	00000910
IP=I0	00000920
JP=J0	00000930
GO TO 50	00000940
40 ISUM=ISUM+1	00000950
L(24)=ISUM	00000960
ISUM=ISUM+1	00000970
IP=1	00000980
JP=1	00000990



# MAIN

50	L(25)=ISUM	00001000
	IF (IQRE.NE.ICHK(7)) GO TO 60	00001010
	ISUM=ISUM+ISIZ	00001020
	IQ=IO	00001030
	JQ=JO	00001040
	KQ=KO	00001050
	GO TO 70	00001060
60	ISUM=ISUM+1	00001070
	IQ=1	00001080
	JQ=1	00001090
	KQ=1	00001100
70	LCSS=ISUM	00001110
	ISUM=ISUM+ISIZ	00001120
	LHSS=ISUM	00001130
	ISUM=ISUM+ISIZ	00001140
	LHB=ISUM	00001150
	ISUM=ISUM+ISIZ	00001160
	LETRAT=ISUM	00001170
	ISUM=ISUM+ISIZ	00001180
	WRITE (6,170) ISUM	00001190
C	---PASS INITIAL ADDRESSES OF ARRAYS TO SUBROUTINES---	00001200
	CALL DATAI(Y(L(1)),Y(L(2)),Y(L(3)),Y(L(4)),Y(L(5)),Y(L(6)),Y(L(7)),	00001210
	1,Y(L(8)),Y(L(9)),Y(L(15)),Y(L(16)),Y(L(17)),Y(L(19)),Y(L(23)),Y(L(	00001220
	224)),Y(L(25)))	00001230
	CALL STEP(Y(L(1)),Y(L(2)),Y(L(3)),Y(L(4)),Y(L(5)),Y(L(6)),Y(L(7)),	00001240
	1Y(L(8)),Y(L(9)),Y(L(15)),Y(L(16)),Y(L(17)),Y(L(19)),Y(L(18)),Y(L(20	00001250
	20)))	00001260
	CALL SOLVE(Y(L(1)),Y(L(2)),Y(L(3)),Y(L(4)),Y(L(5)),Y(L(6)),Y(L(7)),	00001270
	1,Y(L(8)),Y(L(9)),Y(L(15)),Y(L(16)),Y(L(17)),Y(L(19)),Y(L(10)),Y(L(	00001280
	21)),Y(L(12)),Y(L(13)),Y(L(14)),Y(L(20)),Y(L(25)),Y(LCSS),Y(LHSS),	00001290
	3Y(LHB))	00001300
	CALL COEF(Y(L(1)),Y(L(2)),Y(L(3)),Y(L(4)),Y(L(5)),Y(L(6)),Y(L(7)),	00001310
	1Y(L(8)),Y(L(9)),Y(L(15)),Y(L(16)),Y(L(17)),Y(L(19)),Y(L(23)),Y(L(20	00001320
	24)),Y(L(25)))	00001330
	CALL CHECKI(Y(L(1)),Y(L(2)),Y(L(3)),Y(L(4)),Y(L(5)),Y(L(6)),Y(L(7)),	00001340
	1),Y(L(8)),Y(L(9)),Y(L(15)),Y(L(16)),Y(L(17)),Y(L(19)),Y(L(21)),Y(L(	00001350
	2(22)),Y(L(25)),Y(LCSS),Y(LHSS),Y(LHB),Y(LETRAT))	00001360
	CALL PRNTAI(Y(L(1)),Y(L(2)),Y(L(4)),Y(L(5)),Y(L(9)),Y(L(15)),Y(L(1	00001370
	16)),Y(L(25)),Y(L(8)),Y(LETRAT))	00001380
C	---START COMPUTATIONS---	00001390
C	*****	00001400
C	---READ AND WRITE DATA FOR GROUPS II AND III---	00001410
	CALL DATAIN	00001420
	IRN=1	00001430
	NIJ=IO*JO	00001440
	DO 80 K=1,KO	00001450
	LOC=L(2)+(K-1)*NIJ	00001460
80	CALL ARRAY(Y(LOC),INFT(1,2),IOFT(1,1),NAME(1),IRN,DUM)	00001470
	DO 90 K=1,KO	00001480
	LOC=L(5)+(K-1)*NIJ	00001490
90	CALL ARRAY(Y(LOC),INFT(1,1),IOFT(1,2),NAME(7),IRN,DUM)	00001500
	DO 100 K=1,KO	00001510

# MAIN

LOC=L(4)+(K-1)*NIJ	00001520
L1=L(19)+K-1	00001530
L2=L(19)+K0+K-1	00001540
L3=L(19)+2*K0+K-1	00001550
CALL ARRAY(Y(LOC),INFT(1,1),IOFT(1,2),NAME(13),IRN,DUM)	00001560
Y(L1)=DUM(1)	00001570
Y(L2)=DUM(2)	00001580
Y(L3)=DUM(3)	00001590
100 WRITE (6,230) K,Y(L1),Y(L2),Y(L3)	00001600
IF (ITK.NE.ICHK(10)) GO TO 120	00001610
DO 110 K=1,K1	00001620
LOC=L(8)+(K-1)*NIJ	00001630
110 CALL ARRAY(Y(LOC),INFT(1,1),IOFT(1,3),NAME(19),IRN,DUM)	00001640
120 IF (IWATER.NE.ICHK(6)) GO TO 130	00001650
K=K0	00001660
CALL ARRAY(Y(L(23)),INFT(1,1),IOFT(1,4),NAME(25),IRN,DUM)	00001670
CALL ARRAY(Y(L(24)),INFT(1,1),IOFT(1,1),NAME(31),IRN,DUM)	00001680
130 IF (IQRE.NE.ICHK(7)) GO TO 132	00001690
DO 131 K=1,K0	00001700
LOC=L(25)+(K-1)*NIJ	00001710
131 CALL ARRAY(Y(LOC),INFT(1,1),IOFT(1,4),NAME(37),IRN,DUM)	00001720
132 DO 135 K=1,K0	00001730
LOC=LCSS+(K-1)*NIJ	00001740
135 CALL ARRAY(Y(LOC),INFT(1,1),IOFT(1,3),	00001750
\$ 24H ET-RUNOFF/DEPTH ,IRN,DUM)	00001760
DO 136 K=1,K0	00001770
LOC=LHSS+(K-1)*NIJ	00001780
136 CALL ARRAY(Y(LOC),INFT(1,1),IOFT(1,1),	00001790
\$ 24H1=HCF HD 2=LSD-ET DPTH ,IRN,DUM)	00001800
DO 137 K=1,K0	00001810
LOC=LHB+(K-1)*NIJ	00001820
137 CALL ARRAY(Y(LOC),INFT(1,1),IOFT(1,1),	00001830
\$ 24H LAND SURFACE ,IRN,DUM)	00001840
CALL MDAT	00001850
C ---COMPUTE TRANSMISSIVITY FOR UNCONFINED LAYER---	00001860
IF (IWATER.EQ.ICHK(6)) CALL TRANS(1)	00001870
C ---COMPUTE T COEFFICIENTS---	00001880
CALL TCOF	00001890
C ---COMPUTE ITERATION PARAMETERS---	00001900
CALL ITER	00001910
C ---READ TIME PARAMETERS AND PUMPING DATA FOR A NEW PUMPING PERIOD---	00001920
140 CALL NEWPER	00001930
KT=0	00001940
IFINAL=0	00001950
C ---START NEW TIME STEP COMPUTATIONS---	00001960
150 CALL NEWSTP	00001970
C ---START NEW ITERATION IF MAXIMUM NO. ITERATIONS NOT EXCEEDED---	00001980
CALL NEWITA	00001990
C ---PRINT OUTPUT AT DESIGNATED TIME STEPS---	00002000
CALL OUTPUT	00002010
C ---LAST TIME STEP IN PUMPING PERIOD ?---	00002020
IF (IFINAL.NE.1) GO TO 150	00002030

# MAIN

```

C      ---CHECK FOR NEW PUMPING PERIOD---                                00002040
      IF (KP.LT.NPER) GO TO 140                                           00002050
      STOP                                                                  00002060
C      ---FORMATS---                                                    00002070
160 FORMAT (8I10)                                                         00002080
170 FORMAT ('0',54X,'WORDS OF VECTOR Y USED =',I7)                      00002090
180 FORMAT ('0',62X,'NUMBER OF ROWS =',I5/60X,'NUMBER OF COLUMNS =',I500002100
      1/61X,'NUMBER OF LAYERS =',I5//39X,'MAXIMUM PERMITTED NUMBER OF ITE00002110
      2RATIONS =',I5//48X,'NUMBER OF CONSTANT HEAD NODES =',I5)         00002120
190 FORMAT ('1',33A4)                                                     00002130
200 FORMAT (20A4)                                                         00002140
210 FORMAT (16(A4,1X))                                                    00002150
220 FORMAT ('-SIMULATION OPTIONS: ',13(A4,4X))                          00002160
230 FORMAT (1H0,44X,'DIRECTIONAL TRANSMISSIVITY MULTIPLICATION FACTORS00002170
      1 FOR LAYER',I3//,76X,'X =',G15.7/76X,'Y =',G15.7/76X,'Z =',G15.7) 00002180
      END                                                                  00002190

```

```

SUBROUTINE DATAI(PHI,STRT,OLD,T,S,TR,TC,TK,WELL,DELX,DELY,DELZ,FAC00002200
1T,PERM,BOTTOM,QRE)                                00002210
C -----00002220
C READ AND WRITE DATA                                00002230
C -----00002240
C SPECIFICATIONS:                                00002250
REAL *8PHI                                00002260
REAL *8XLABEL,YLABEL,TITLE,XN1,MESUR                                00002270
DIMENSION PHI(IO,JO,KO),STRT(IO,JO,KO),OLD(IO,JO,KO),T(IO,JO,KO00002280
1),S(IO,JO,KO),TR(IO,JO,KO),TC(IO,JO,KO),TK(IK,JK,K5),WELL(IO,00002290
2JO,KO),DELX(JO),DELY(IO),DELZ(KO),FACT(KO,3),PERM(IP,JP),BOT00002300
3TOM(IP,JP),QRE(IQ,JQ,KQ),TF(3),A(IO,JO),IN(6),IOFT(9),INFT(200002310
4),IWELLO(10)                                00002320
COMMON /INTEGR/ IO,JO,KO,I1,J1,K1,I,J,K,NPER,KTH,ITMAX,LENGTH,KP,N00002330
1WEL,NUMT,IFINAL,IT,KT,IHEAD,IDRAW,IFLO,IERR,I2,J2,K2,IMAX,ITMX1,NC00002340
2H,IDK1,IDK2,IWATER,IQRE,IP,JP,IQ,JQ,IK,JK,K5,IPU1,IPU2,ITK,IEQN,KQ00002350
COMMON /SPARAM/ TMAX,CDLT,DELT,ERR,TEST,SUM,SUMP,QR                                00002360
COMMON /SARRAY/ ICHK(14),LEVEL1(4),LEVEL2(4),LEVEL3(4),LEVEL4(4),L00002370
1LEVEL5(4),LEVEL6(4),LEVEL7(4)                                00002380
COMMON /CK/ ETFLXT,STORT,QRET,CHST,CHDT,FLUXT,PUMPT,CFLUXT,FLXNT 00002390
COMMON /PR/ XLABEL(3),YLABEL(6),TITLE(6),XN1,MESUR,PRNT(122),BLANK00002400
1(60),DIGIT(122),VF1(6),VF2(6),VF3(7),XSCALE,DINCH,SYM(17),XN(100),00002410
2YN(13),NA(4),N1,N2,N3,YSCALE,FACT1,FACT2,FACT3,FACT4,FACT5,FACT6,F00002420
3ACT7                                00002430
COMMON /B/ BETA                                00002440
RETURN                                00002450
C .....00002460
C *****00002470
ENTRY DATAIN                                00002480
C *****00002490
C ---READ AND WRITE SCALAR PARAMETERS---00002500
READ (5,330) NPER,KTH,ERR,LENGTH,BETA                                00002510
WRITE (6,340) NPER,KTH,ERR                                00002520
WRITE(6,346) BETA                                00002530
READ (5,460) XSCALE,YSCALE,DINCH,FACT1,(LEVEL1(I),I=1,4),FACT2,(LE00002540
1VEL2(I),I=1,4),FACT3,(LEVEL3(I),I=1,4),FACT4,(LEVEL4(I),I=1,4),FAC00002550
2T5,(LEVEL5(I),I=1,4),FACT6,(LEVEL6(I),I=1,4),FACT7,(LEVEL7(I),I=1,00002560
34),MESUR                                00002570
IF (XSCALE.NE.0.) WRITE (6,470) XSCALE,YSCALE,MESUR,MESUR,DINCH,FA00002580
1CT1,LEVEL1,FACT2,LEVEL2,FACT3,LEVEL3,FACT4,LEVEL4,FACT5,LEVEL5,FAC00002590
2T6,LEVEL6,FACT7,LEVEL7                                00002600
C ---READ CUMULATIVE MASS BALANCE PARAMETERS---00002610
READ (5,450) SUM,SUMP,PUMPT,CFLUXT,QRET,CHST,CHDT,FLUXT,STORT,ETFL00002620
1XT,FLXNT                                00002630
IF (IDK1.EQ.ICHK(4)) GO TO 20                                00002640
IF (IPU1.NE.ICHK(8)) GO TO 50                                00002650
C ---READ INITIAL HEAD VALUES FROM CARDS---00002660

```

# DATAI

	DO 10 K=1,K0	00002670
	DO 10 I=1,I0	00002680
	10 READ (5,360) (PHI(I,J,K),J=1,J0)	00002690
	GO TO 30	00002700
C	---READ INITIAL HEAD AND MASS BALANCE PARAMETERS FROM DISK---	00002710
	20 READ (4) PHI,SUM,SUMP,PUMPT,CFLUXT,QRET,CHST,CHDT,FLUXT,STORT,ETFL	00002720
	1XT,FLXNT	00002730
	REWIND 4	00002740
	30 WRITE (6,430) SUM	00002750
	DO 40 K=1,K0	00002760
	WRITE (6,440) K	00002770
	DO 40 I=1,I0	00002780
	40 WRITE (6,350) I,(PHI(I,J,K),J=1,J0)	00002790
	50 DO 60 K=1,K0	00002800
	DO 60 I=1,I0	00002810
	DO 60 J=1,J0	00002820
	WELL(I,J,K)=0.	00002830
	TR(I,J,K)=0.	00002840
	TC(I,J,K)=0.	00002850
	IF (K.NE.K0) TK(I,J,K)=0.	00002860
	60 CONTINUE	00002870
	RETURN	00002880
C	*****	00002890
	ENTRY ARRAY(A,INFT,IOFT,IN,IRN,TF)	00002900
C	*****	00002910
	READ (5,330) FAC,IVAR,IPRN,TF,IRECS,IRECD	00002920
	IPRN=1	00002930
	IC=4*IRECS+2*IVAR+IPRN+1	00002940
	GO TO (70,70,90,90,120,120), IC	00002950
	70 DO 80 I=1,I0	00002960
	DO 80 J=1,J0	00002970
	80 A(I,J)=FAC	00002980
	WRITE (6,280) IN,FAC,K	00002990
	GO TO 140	00003000
	90 IF (IC.EQ.3) WRITE (6,290) IN,K	00003010
	DO 110 I=1,I0	00003020
	READ (5,INFT) (A(I,J),J=1,J0)	00003030
	DO 100 J=1,J0	00003040
	100 A(I,J)=A(I,J)*FAC	00003050
	110 IF (IC.EQ.3) WRITE (6,IOFT) I,(A(I,J),J=1,J0)	00003060
	GO TO 140	00003070
	120 READ (2*IRN) A	00003080
	IF (IC.EQ.6) GO TO 140	00003090
	WRITE (6,290) IN,K	00003100
	DO 130 I=1,I0	00003110
	130 WRITE (6,IOFT) I,(A(I,J),J=1,J0)	00003120
	140 IF (IRECD.EQ.1) WRITE (2*IRN) A	00003130
	IRN=IRN+1	00003140
	RETURN	00003150
C	*****	00003160
	ENTRY MDAT	00003170
C	*****	00003180

# DATAI

	NCHCK=0	00003190
	DO 150 K=1,KO	00003200
	DO 150 I=1,IO	00003210
	DO 150 J=1,JO	00003220
	IF (I.EQ.1.OR.I.EQ.IO.OR.J.EQ.1.OR.J.EQ.JO) T(I,J,K)=0.	00003230
	IF (IDK1.NE.ICHK(4).AND.IPU1.NE.ICHK(8)) PHI(I,J,K)=STRT(I,J,K)	00003240
	IF (IWATER.EQ.ICHK(6).AND.K.EQ.KO) GO TO 147	00003250
	IF(T(I,J,K).EQ.0.OR.S(I,J,K).GE.0) GO TO 147	00003260
	NCHCK=NCHCK+1	00003270
147	IF (K.NE.KO.OR.IWATER.NE.ICHK(6)) GO TO 150	00003280
	IF (I.EQ.1.OR.I.EQ.IO.OR.J.EQ.1.OR.J.EQ.JO) PERM(I,J)=0.	00003290
	IF(PERM(I,J).EQ.0.OR.S(I,J,K).GE.0) GO TO 150	00003300
	NCHCK=NCHCK+1	00003310
150	CONTINUE	00003320
	IF(NCHCK.EQ.NCH)GO TO 152	00003330
	WRITE(6,475)NCHCK,NCH	00003340
C	..... DELX .....	00003350
152	IRN3=1	00003360
	READ (5,330) FAC,IVAR,IPRN,TF,IRECS,IRECD	00003370
	IF(IRECS.EQ.1) READ(3'IRN3) DELX	00003380
	IF(IRECS.EQ.1) GOTO 171	00003390
	IF (IVAR.EQ.1) READ (5,330) (DELX(J),J=1,JO)	00003400
	DO 170 J=1,JO	00003410
	IF (IVAR.NE.1) GO TO 160	00003420
	DELX(J)=DELX(J)*FAC	00003430
	GO TO 170	00003440
160	DELX(J)=FAC	00003450
170	CONTINUE	00003460
171	IF(IRECD.EQ.1) WRITE(3'IRN3) DELX	00003470
	IF (IVAR.EQ.1.AND.IPRN.NE.1) WRITE (6,370) (DELX(J),J=1,JO)	00003480
	IF (IVAR.EQ.0) WRITE (6,300) FAC	00003490
C	..... DELY .....	00003500
	IRN4=1	00003510
	READ (5,330) FAC,IVAR,IPRN,TF,IRECS,IRECD	00003520
	IF(IRECS.EQ.1) READ(4'IRN4) DELY	00003530
	IF(IRECS.EQ.1) GO TO 191	00003540
	IF (IVAR.EQ.1) READ (5,330) (DELY(I),I=1,IO)	00003550
	DO 190 I=1,IO	00003560
	IF (IVAR.NE.1) GO TO 180	00003570
	DELY(I)=DELY(I)*FAC	00003580
	GO TO 190	00003590
180	DELY(I)=FAC	00003600
190	CONTINUE	00003610
191	IF(IRECD.EQ.1) WRITE(4'IRN4) DELY	00003620
	IF (IVAR.EQ.1.AND.IPRN.NE.1) WRITE (6,380) (DELY(I),I=1,IO)	00003630
	IF (IVAR.EQ.0) WRITE (6,310) FAC	00003640
C	..... DELZ .....	00003650
	IRN8=1	00003660
	READ (5,330) FAC,IVAR,IPRN,TF,IRECS,IRECD	00003670
	IF(IRECS.EQ.1) READ(8'IRN8) DELZ	00003680
	IF(IRECS.EQ.1) GOTO 211	00003690
	IF (IVAR.EQ.1) READ (5,330) (DELZ(K),K=1,KO)	00003700

# DATAI

	DO 210 K=1,KO	00003710
211	IF(IRECD.EQ.1) WRITE(8'IRN8) DELZ	00003720
	IRN8=IRN8+1	00003730
	IF (IVAR.NE.1) GO TO 200	00003740
	DELZ(K)=DELZ(K)*FAC	00003750
	GO TO 210	00003760
200	DELZ(K)=FAC	00003770
210	CONTINUE	00003780
	IF (IVAR.EQ.1.AND.IPRN.NE.1) WRITE (6,390) (DELZ(K),K=1,KO)	00003790
	IF (IVAR.EQ.0) WRITE (6,320) FAC	00003800
C	---INITIALIZE VARIABLES---	00003810
	B=0.	00003820
	D=0.	00003830
	F=0.	00003840
	H=0.	00003850
	SU=0.	00003860
	Z=0.	00003870
	IF (XSCALE.NE.0.) CALL MAP	00003880
	RETURN	00003890
C	.....	00003900
C	---READ TIME PARAMETERS AND PUMPING DATA FOR A NEW PUMPING PERIOD---	00003910
C	*****	00003920
	ENTRY NEWPER	00003930
C	*****	00003940
	IRN9=1	00003950
	READ (5,330) KP,KPM1,NWEL,TMAX,NUMT,CDLT,DELT	00003960
C	---COMPUTE ACTUAL DELT AND NUMT---	00003970
	DT=DELT/24.	00003980
	TM=0.0	00003990
	DO 220 I=1,NUMT	00004000
	DT=CDLT*DT	00004010
	TM=TM+DT	00004020
	IF (TM.GE.TMAX) GO TO 230	00004030
220	CONTINUE	00004040
	GO TO 240	00004050
230	DELT=TMAX/TM*DELT	00004060
	NUMT=I	00004070
240	WRITE (6,400) KP,TMAX,NUMT,DELT,CDLT	00004080
	DELT=DELT*3600.	00004090
	TMAX=TMAX*36400.	00004100
	SUMP=0.0	00004110
C	---READ AND WRITE WELL PUMPING RATES---	00004120
	WRITE (6,410) NWEL	00004130
	IF (NWEL.EQ.0) GO TO 260	00004140
	IF(SUMP.EQ.0.0) GO TO 241	00004150
	DO 246 K=1,KO	00004160
	READ(5,330) FAC,IVAR,IPRN,TF,IRECS,IRECD	00004170
	IF(IRECS.EQ.1) READ(9'IRN9) A	00004180
	IF(IRECS.EQ.1)GOTO 242	00004190
	IF(IVAR.EQ.0)GO TO 244	00004200
	DO 247 I=1,IO	00004210
247	READ(5,331)(A(I,J),J=1,JO)	00004220

# DATAI

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DO 248 I=1,IO                                00004230
DO 248 J=1,JO                                00004240
A(I,J)=A(I,J)*FAC                            00004250
248 CONTINUE                                00004260
GO TO 242                                    00004270
244 DO 261 I=1,IO                            00004280
DO 261 J=1,JO                                00004290
261 A(I,J)=FAC                                00004300
242 DO 249 I=1,IO                            00004310
DO 249 J=1,JO                                00004320
WELL(I,J,K)=A(I,J)/(DELX(J)*DELY(I))        00004330
249 CONTINUE                                00004340
DO 243 I=1,IO                                00004350
IF(IVAR.EQ.1.AND.IPRN.NE.1) WRITE(6,332) I,(WELL(I,J,K),J=1,JO) 00004360
243 CONTINUE                                00004370
IF(IVAR.EQ.0) WRITE(6,333) K,FAC             00004380
IF(IRECD.EQ.1) WRITE(9'IRN9) A              00004390
IRN9=IRN9+1                                  00004400
246 CONTINUE                                00004410
GO TO 260                                    00004420
241 DO 245 K=1,KO                            00004430
DO 245 I=1,IO                                00004440
DO 245 J=1,JO                                00004450
245 WELL(I,J,K)=0.0                          00004460
DO 250 II=1,NWEL                             00004470
READ (5,335) K,I,J,WELL(I,J,K),(IWELLO(KK),KK=1,10) 00004480
WELMGD=WELL(I,J,K)*.646317                  00004490
WRITE (6,420) K,I,J,WELL(I,J,K),WELMGD,(IWELLO(KK),KK=1,10) 00004500
250 WELL(I,J,K)=WELL(I,J,K)/(DELX(J)*DELY(I)) 00004510
260 RETURN                                    00004520
C ----FORMATS-----                        00004530
280 FORMAT (1H0,52X,6A4,' =',G15.7,' FOR LAYER',I3) 00004540
290 FORMAT (1H1,45X,6A4,' MATRIX, LAYER',I3/46X,41('-')) 00004550
300 FORMAT ('0',72X,'DELX =',G15.7)          00004560
310 FORMAT ('0',72X,'DELY =',G15.7)          00004570
320 FORMAT ('0',72X,'DELZ =',G15.7)          00004580
330 FORMAT (8G10.0)                          00004590
331 FORMAT(20F4.1)                           00004600
332 FORMAT(' ',I2,10F8.1)                    00004610
333 FORMAT('0',72X,'PUMPING IN LAYER',2X,I2,2X,'=',G15.7) 00004620
335 FORMAT (4G10.0,10A4)                     00004630
340 FORMAT ('0',51X,'NUMBER OF PUMPING PERIODS =',I5/49X,'TIME STEPS BETWEEN PRINTOUTS =',I5//51X,'ERROR CRITERIA FOR CLOSURE =',G15.7//) 00004640
346 FORMAT('0',72X,'BETA= ',F4.2)            00004660
350 FORMAT ('0',I2,2X,20F6.1/(5X,20F6.1))    00004670
360 FORMAT (8F10.4)                          00004680
370 FORMAT (1H1,46X,40HGRID SPACING IN PROTOTYPE IN X DIRECTION/47X,4000004690
1('-')//('0',12F10.0))                      00004700
380 FORMAT (1H-,46X,40HGRID SPACING IN PROTOTYPE IN Y DIRECTION/47X,4000004710
1('-')//('0',12F10.0))                      00004720
390 FORMAT (1H-,46X,40HGRID SPACING IN PROTOTYPE IN Z DIRECTION/47X,4000004730
1('-')//('0',12F10.0))                      00004740

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# DATAI

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400 FORMAT ('-',50X,'PUMPING PERIOD NO.',I4,':',F10.2,' DAYS'/51X,38('00004750
1-')//53X,'NUMBER OF TIME STEPS=',I6//59X,'DELT IN HOURS =',F10.3//00004760
253X,'MULTIPLIER FOR DELT =',F10.3) 00004770
410 FORMAT ('-',53X,I4,' WELLS'/55X,9(' - ')/39X,'AQ',8X,'ROW',7X,'COL'00004780
1,8X,'CFS',10X,'MGD',/)) 00004790
420 FORMAT (31X,3I10,2F13.2,10A4) 00004800
430 FORMAT ('-',40X,' CONTINUATION - HEAD AFTER ',G20.7,' SEC PUMPING 00004810
1'/42X,58(' - ')) 00004820
440 FORMAT ('1',55X,'INITIAL HEAD MATRIX, LAYER',I3/56X,30(' - ')) 00004830
450 FORMAT (4G20.10) 00004840
460 FORMAT (3G7.0,7(G3.0,4I1),5X,A5) 00004850
470 FORMAT ('0',30X,'ON ALPHAMERIC MAP:'/40X,'MULTIPLICATION FACTOR F000004860
1R X DIMENSION =',G15.7/40X,'MULTIPLICATION FACTOR FOR Y DIMENSION 00004870
2=',G15.7/55X,' MAP SCALE IN UNITS OF',A12/49X,'NUMBER OF ',A8,' P 00004880
3ER INCH =',G15.7/43X,'MULTIPLICATION FACTOR FOR DRAWDOWN =',G15.7,00004890
4' PRINTED FOR LAYERS',4I2/47X,'MULTIPLICATION FACTOR FOR HEAD =',G00004900
515.7,' PRINTED FOR LAYERS',4I2/47X,'MULT FACTOR FOR HEAD DIFFERENC00004910
6 =',G15.7,' PRINTED FOR LAYERS',4I2/47X,'MULTIPLICATION FACTOR FOR00004920
7 RECH =',G15.7,' PRINTED FOR LAYERS',4I2/44X,'MULTIPLICATION FACT 00004930
8FOR ET-RUNOFF =',G15.7,' PRINTED FOR LAYERS',4I2/44X,'MULTIPLICATIO00004940
9ON FACTOR FOR LEAKAGE =',G15.7,' PRINTED FOR LAYERS',4I2/44X,'MULT00004950
1IPLICATION FACTOR FOR PUMPAGE =',G15.7,'PRINTED FOR LAYERS',4I2) 00004960
475 FORMAT(1H0,10X,11(1H*)/11X,1H*/9H WARNING ,1H*/11X,11(1H*)//5X, 00004970
281H THE NUMBER OF CONSTANT HEAD NODES CODED IN AND THE NUMBER COMPO00004980
3UTED DO NOT AGREE:/11X, 21HTHE NUMBER COMPUTED =,I5/11X,21HTHE NUM00004990
4BER CODED IN =,I5//2X,100(1H*)) 00005000
END 00005010

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SUBROUTINE STEP(PHI,STRT,OLD,T,S,TR,TC,TK,WELL,DELX,DELY,DELZ,FACT00005020
1,DDN,TEST3)00005030
C-----00005040
C INITIALIZE DATA FOR A NEW TIME STEP AND PRINT RESULTS00005050
C-----00005060
C SPECIFICATIONS:00005070
REAL *8PHI00005080
REAL *8XLABEL,YLABEL,TITLE,XN1,MESUR00005090
DIMENSION PHI(IO,J0,K0), STRT(IO,J0,K0), OLD(IO,J0,K0), T(IO,J0,K000005100
1), S(IO,J0,K0), TR(IO,J0,K0), TC(IO,J0,K0), TK(IK,JK,K5), WELL(IO,00005110
2J0,K0), DELX(J0), DELY(IO), DELZ(K0), FACT(K0,3), DDN(IMAX), TEST300005120
3(ITMX1), ITTO(50)00005130
COMMON /INTEGR/ IO,J0,K0,I1,J1,K1,I,J,K,NPER,KTH,ITMAX,LENGTH,KP,N00005140
1WEL,NUMT,IFINAL,IT,KT,IHEAD,IDRAW,IFLO,IERR,I2,J2,K2,IMAX,ITMX1,NC00005150
2H,IDK1,IDK2,IWATER,IQRE,IP,JP,IQ,JQ,IK,JK,K5,IPU1,IPU2,ITK,IEQN,KQ00005160
COMMON /SPARAM/ TMAX,CDLT,DELT,ERR,TEST,SUM,SUMP,QR00005170
COMMON /SARRAY/ ICHK(14),LEVEL1(4),LEVEL2(4),LEVEL3(4),LEVEL4(4),L00005180
1LEVEL5(4),LEVEL6(4),LEVEL7(4)00005190
COMMON /CK/ ETFLXT,STORT,QRET,CHST,CHDT,FLUXT,PUMPT,CFLUXT,FLXNT00005200
COMMON /PR/ XLABEL(3),YLABEL(6),TITLE(6),XN1,MESUR,PRNT(122),BLANK00005210
1(60),DIGIT(122),VF1(6),VF2(6),VF3(7),XSCALE,DINCH,SYM(17),XN(100),00005220
2YN(13),NA(4),N1,N2,N3,YSCALE,FACT1,FACT2,FACT3,FACT4,FACT5,FACT6,F00005230
3ACT7,IWELLO(10)00005240
RETURN00005250
C.....00005260
C *****00005270
C ENTRY NEWSTP00005280
C *****00005290
KT=KT+100005300
IT=00005310
DO 10 K=1,K00005320
DO 10 I=1,I00005330
DO 10 J=1,J00005340
10 OLD(I,J,K)=PHI(I,J,K)00005350
DELT=CDLT*DELT00005360
SUM=SUM+DELT00005370
SUMP=SUMP+DELT00005380
DAYSP=SUMP/86400.00005390
YRSP=DAYSP/365.00005400
HRS=SUM/3600.00005410
SMIN=HRS*60.00005420
DAYS=HRS/24.00005430
YRS=DAYS/365.00005440
RETURN00005450
C ---PRINT OUTPUT AT DESIGNATED TIME STEPS---00005460
C *****00005470
ENTRY OUTPUT00005480

```

# STEP

```

C *****
IF (KT.EQ.NUMT) IFINAL=1
ITTO(KT)=IT
IF (IT.LE.ITMAX) GO TO 20
IT=IT-1
ITTO(KT)=IT
IERR=2
C ---IF MAXIMUM ITERATIONS EXCEEDED,WRITE RESULTS ON DISK OR CARDS--
IF (IDK2.EQ.ICHK(5)) WRITE (4) PHI,SUM,SUMP,PUMPT,CFLUXT,QRET,CHST
1,CHDT,FLUXT,STORT,ETFLXT,FLXNT
IF (IPU2.EQ.ICHK(9)) WRITE (7,230) SUM,SUMP,PUMPT,CFLUXT,QRET,CHST
1,CHDT,FLUXT,STORT,ETFLXT,FLXNT
20 IF (IFLO.EQ.ICHK(3)) CALL CHECK
IF (IERR.EQ.2) GO TO 30
IF (MOD(KT,KTH).NE.0.AND.IFINAL.NE.1) RETURN
30 WRITE (6,210) KT,DELT,SUM,SMIN,HRS,DAYS,YRS,DAYSP,YRSP
IF (IFLO.EQ.ICHK(3)) CALL CWRITE
IT=IT+1
C ---REMOVE C FROM NEXT CARD TO PRINT HEAD CHANGE FOR ITERATIONS---
C WRITE (6,180) (TEST3(J),J=1,IT)
I3=1
I5=0
352 I5=I5+40
I4=MIN0(KT,I5)
WRITE (6,240) (I,I=I3,I4)
WRITE (6,260)
WRITE (6,250) (ITTO(I),I=I3,I4)
WRITE (6,260)
IF(KT.LE.I5) GO TO 353
I3=I3+40
GO TO 352
C --MAP DRAWDOWN,HEAD,HEAD DIFF,RECHARGE,ET-RUNOFF,LEAKAGE,PUMPAGE--
353 IF (XSCALE.EQ.0.) GO TO 38
IF (FACT1.EQ.0.) GO TO 50
DO 40 IA=1,4
II=LEVEL1(IA)
IF (II.EQ.0) GO TO 50
40 CALL PRNTA(1,II)
50 IF (FACT2.EQ.0.) GO TO 65
DO 60 IA=1,4
II=LEVEL2(IA)
IF (II.EQ.0) GO TO 65
60 CALL PRNTA(2,II)
65 IF(FACT3.EQ.0) GO TO 75
DO 67 IA=1,4
II=LEVEL3(IA)
IF(II.EQ.0) GO TO 75
67 CALL PRNTA(3,II)
75 IF(IQRE.NE.ICHK(7)) GO TO 79
IF(FACT4.EQ.0) GO TO 79
DO 77 IA=1,4
II=LEVEL4(IA)

```

# STEP

	IF(II.EQ.0) GO TO 79	00006010
77	CALL PRNTA(4,II)	00006020
79	IF(FACT5.EQ.0) GO TO 82	00006030
	DO 80 IA=1,4	00006040
	II=LEVEL5(IA)	00006050
	IF(II.EQ.0.) GO TO 82	00006060
80	CALL PRNTA(5,II)	00006070
82	IF(FACT6.EQ.0) GO TO 87	00006080
	IF(ITK.NE.ICHK(10)) GO TO 87	00006090
	DO 84 IA=1,4	00006100
	II=LEVEL6(IA)	00006110
	IF(II.EQ.0) GO TO 87	00006120
84	CALL PRNTA(6,II)	00006130
87	IF(FACT7.EQ.0)GO TO 88	00006140
	DO 99 IA=1,4	00006150
	II=LEVEL7(IA)	00006160
	IF(II.EQ.0)GO TO 88	00006170
99	CALL PRNTA(7,II)	00006180
88	IF (IDRAW.NE.ICHK(1)) GO TO 100	00006190
C	---PRINT DRAWDOWN---	00006200
	DO 90 K=1,K0	00006210
	WRITE (6,200) K	00006220
	DO 90 I=1,I0	00006230
	DO 89 J=1,J0	00006240
	DDN(J)=STRT(I,J,K)-PHI(I,J,K)	00006250
	IF(K.EQ.1.AND.T(I,J,K).EQ.0.0) GO TO 89	00006260
	IF(K.EQ.2.AND.T(I,J,K-1).EQ.0.0) GO TO 89	00006270
C	--REMOVE C FROM COL 1 OF NEXT CARD TO PUNCH DDN IN CALCOMP FORMAT--	00006280
	WRITE(7,171) I,J,DDN(J),K	00006290
89	CONTINUE	00006300
90	WRITE (6,170) I,(DDN(J),J=1,J0)	00006310
100	IF (IHEAD.NE.ICHK(2)) GO TO 120	00006320
C	---PRINT HEAD MATRIX---	00006330
	DO 110 K=1,K0	00006340
	WRITE (6,190) K	00006350
	DO 110 I=1,I0	00006360
110	WRITE (6,170) I,(PHI(I,J,K),J=1,J0)	00006370
C	---WRITE ON DISK---	00006380
120	IF (IERR.EQ.2) GO TO 130	00006390
	IF (KP.LT.NPER.OR.IFINAL.NE.1) RETURN	00006400
	IF (IDK2.EQ.ICHK(5)) WRITE (4) PHI,SUM,SUMP,PUMPT,CFLUXT,QRET,CHST	00006410
	1,CHDT,FLUXT,STORT,ETFLXT,FLXNT	00006420
C	---PUNCHED OUTPUT---	00006430
130	IF (IPU2.NE.ICHK(9)) GO TO 160	00006440
	IF (IERR.EQ.2) GO TO 140	00006450
	WRITE (7,230) SUM,SUMP,PUMPT,CFLUXT,QRET,CHST,CHDT,FLUXT,STORT,ETF	00006460
	1LXT,FLXNT	00006470
140	DO 150 K=1,K0	00006480
	DO 150 I=1,I0	00006490
150	WRITE (7,220) (PHI(I,J,K),J=1,J0)	00006500
160	IF (IERR.EQ.2) STOP	00006510
	RETURN	00006520

# STEP

```

C      ---FORMATS---
170  FORMAT ('0',I4,18F7.2/(5X,18F7.2))
171  FORMAT(2I5,F5.1,20X,'DDN IN LAYER',I5)
180  FORMAT ('0MAXIMUM HEAD CHANGE FOR EACH ITERATION:'/ ' ',39('-')/('000006530
      1',10F12.4))
190  FORMAT ('1',55X,'HEAD MATRIX, LAYER',I3/56X,21('-'))
200  FORMAT ('1',55X,'    DRAWDOWN, LAYER',I3/59X,18('-'))
210  FORMAT (1H1,44X,57('-')/45X,'|',14X,'TIME STEP NUMBER =',I9,14X,'|00006600
      1'/45X,57('-')//50X,29HSIZE OF TIME STEP IN SECONDS=,F14.2//55X,'T000006610
      2TAL SIMULATION TIME IN SECONDS=,F14.2/80X,8HMINUTES=,F14.2/82X,6H00006620
      3HOURS=,F14.2/83X,5H DAYS=,F14.2/82X,'YEARS=,F14.2///45X,'DURATION 00006630
      4OF CURRENT PUMPING PERIOD IN DAYS=,F14.2/82X,'YEARS=,F14.2//) 00006640
220  FORMAT (8F10.4)
230  FORMAT (4G20.10)
240  FORMAT ('0TIME STEP :',40I3)
250  FORMAT ('0ITERATIONS:',40I3)
260  FORMAT (' ',10('-'))
      END

```

```

SUBROUTINE SOLVE(PHI,STRT,OLD,T,S,TR,TC,TK,WELL,DELX,DELY,DELZ,FAC00006710
1T,EL,FL,GL,V,XI,TEST3,QRE,CSS,HSS,HB) 00006720
C -----00006730
C SOLUTION BY THE STRONGLY IMPLICIT PROCEDURE 00006740
C -----00006750
C SPECIFICATIONS: 00006760
REAL *8PHI,RHO,B,D,F,H,Z,SU,RHOP,W,WMIN,RHO1,RHO2,RHO3,XPART,YPART00006770
1,ZPART,DMIN1,WMAX,XT,YT,ZT,DABS,DMAX1,DEN,TXM,TYM,TZM 00006780
REAL *8E,AL,BL,CL,A,C,G,WU,TU,U,DL,RES,SUPH,GLXI,ZPHI 00006790
DIMENSION PHI(1),STRT(1),OLD(1),T(1),S(1),TR(1),TC(1),TK(1)00006800
1,WELL(1),DELX(1),DELY(1),DELZ(1),FACT(K0,3),RHOP(20),TEST3(00006810
21),EL(1),FL(1),GL(1),V(1),XI(1),QRE(1),CSS(1),HSS(1),HB(1) 00006820
COMMON /INTEGR/ IO,J0,K0,I1,J1,K1,I,J,K,NPER,KTH,ITMAX,LENGTH,KP,N00006830
1WEL,NUMT,IFINAL,IT,KT,IHEAD,IDRAW,IFLO,IERR,I2,J2,K2,IMAX,ITMX1,NC00006840
2H,IDK1,IDK2,IWATER,IQRE,IP,JP,IQ,JQ,IK,JK,K5,IPU1,IPU2,ITK,IEQN,KQ00006850
COMMON /SPARAM/ TMAX,CDLT,DELT,ERR,TEST,SUM,SUMP,QR 00006860
COMMON /SARRAY/ ICHK(14),LEVEL1(4),LEVEL2(4),LEVEL3(4),LEVEL4(4),L00006870
1LEVEL5(4),LEVEL6(4),LEVEL7(4) 00006880
COMMON /B/ BETA 00006890
RETURN 00006900
C .....00006910
C *****00006920
ENTRY ITER 00006930
C *****00006940
C ---COMPUTE AND PRINT ITERATION PARAMETERS--- 00006950
WRITE (6,240) 00006960
WMIN=1.00 00006970
DELT=1. 00006980
P2=LENGTH-1 00006990
NT=IO*J0*K0 00007000
NIJ=IO*J0 00007010
XT=3.141593**2/(2.*J2*J2) 00007020
YT=3.141593**2/(2.*I2*I2) 00007030
ZT=3.141593**2/(2.*K0*K0) 00007040
RH01=0.00 00007050
RH02=0.00 00007060
RH03=0.00 00007070
DO 40 K=1,K0 00007080
DO 40 I=2,I1 00007090
DO 40 J=2,J1 00007100
N=I+(J-1)*IO+(K-1)*NIJ 00007110
IF (T(N).EQ.0.) GO TO 40 00007120
D=TR(N-IO)/DELX(J) 00007130
F=TR(N)/DELX(J) 00007140
B=TC(N-1)/DELY(I) 00007150
H=TC(N)/DELY(I) 00007160
SU=0.00 00007170

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# SOLVE

Z=0.00	00007180
IF (K.NE.1) Z=TK(N-NIJ)/DELZ(K)	00007190
IF (K.NE.K0) SU=TK(N)/DELZ(K)	00007200
10 CONTINUE	00007210
TXM=DMAX1(D,F)	00007220
TYM=DMAX1(B,H)	00007230
TZM=DMAX1(SU,Z)	00007240
DEN=DMIN1(D,F)	00007250
IF (DEN.EQ.0.00) DEN=TXM	00007260
IF (DEN.EQ.0.00) GO TO 20	00007270
RHO1=DMAX1(RHO1,TYM/DEN)	00007280
20 DEN=DMIN1(B,H)	00007290
IF (DEN.EQ.0.00) DEN=TYM	00007300
IF (DEN.EQ.0.00) GO TO 30	00007310
RHO2=DMAX1(RHO2,TXM/DEN)	00007320
30 DEN=DMIN1(SU,Z)	00007330
IF (DEN.EQ.0.00) DEN=TZM	00007340
IF (DEN.EQ.0.00) GO TO 40	00007350
RHO3=DMAX1(RHO3,TXM/DEN)	00007360
40 CONTINUE	00007370
XPART=XT/(1.00+RHO1)	00007380
YPART=YT/(1.00+RHO2)	00007390
ZPART=ZT/(1.00+RHO3)	00007400
WMIN=DMIN1(WMIN,XPART,YPART,ZPART)	00007410
WMAX=1.00-WMIN	00007420
WMAX=.99863	00007430
PJ=-1.	00007440
DO 50 I=1,LENGTH	00007450
PJ=PJ+1.	00007460
50 RHOP(I)=1.00-(1.00-WMAX)**(PJ/P2)	00007470
---REMOVE C FROM NEXT CARD TO PRINT ITERATION PARAMETERS---	00007480
WRITE (6,230) LENGTH,(RHOP(J),J=1,LENGTH)	00007490
RETURN	00007500
.....	00007510
---INITIALIZE DATA FOR A NEW ITERATION---	00007520
60 IT=IT+1	00007530
IF (IT.LE.ITMAX) GO TO 70	00007540
WRITE (6,220)	00007550
CALL OUTPUT	00007560
70 IF (MOD(IT,LENGTH)) 80,80,90	00007570
*****	00007580
ENTRY NEWITA	00007590
*****	00007600
80 NTH=0	00007610
90 NTH=NTH+1	00007620
W=RHOP(NTH)	00007630
TEST3(IT+1)=0.	00007640
TEST=0.0	00007650
BIG=0.	00007660
DO 100 I=1,NT	00007670
EL(I)=0.	00007680
FL(I)=0.	00007690

# SOLVE

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      GL(I)=0.                                00007700
      V(I)=0.                                00007710
100  XI(I)=0.                                00007720
C    ---COMPUTE TRANSMISSIVITY AND T COEFFICIENTS FOR UPPER 00007730
C    HYDROLOGIC UNIT WHEN IT IS UNCONFINED---              00007740
      IF (IWATER.NE.ICHK(6)) GO TO 110          00007750
      CALL TRANS(0)                                00007760
C    ---CHOOSE SIP NORMAL OR REVERSE ALGORITHM---          00007770
110  IF (MOD(IT,2)) 120,120,170                00007780
120  DO 150 K=1,K0                                00007790
      DO 150 I=2,I1                                00007800
      DO 150 J=2,J1                                00007810
      N=I+(J-1)*IO+(K-1)*NIJ                    00007820
      NIA=N+1                                      00007830
      NIB=N-1                                      00007840
      NJA=N+IO                                      00007850
      NJB=N-IO                                      00007860
      NKA=N+NIJ                                    00007870
      NKB=N-NIJ                                    00007880
C    ---SKIP COMPUTATIONS IF NODE OUTSIDE MODEL---          00007890
      IF (T(N).EQ.0..OR.S(N).LT.0.) GO TO 150    00007900
C    ---COMPUTE COEFFICIENTS---                          00007910
      D=TR(NJB)/DELX(J)                          00007920
      F=TR(N)/OELX(J)                            00007930
      B=TC(NIB)/DELY(I)                          00007940
      H=TC(N)/DELY(I)                            00007950
      SU=0.00                                      00007960
      Z=0.00                                      00007970
      IF(K.EQ.1) GO TO 124                        00007980
      Z=TK(NKB)                                    00007990
      IF(IEQN.EQ.ICHK(11)) Z=Z/DELZ(K)            00008000
124  IF(K.EQ.K0) GO TO 125                        00008010
      SU=TK(N)                                    00008020
      IF(IEQN.EQ.ICHK(11))SU=SU/DELZ(K)           00008030
125  RHO=S(N)/DELT                                00008040
      QR=0.                                        00008050
      IF (IQRE.EQ.ICHK(7)) QR=QRE(N)              00008060
C    ---SIP NORMAL ALGORITHM---                          00008070
C    ---FORWARD SUBSTITUTE, COMPUTING INTERMEDIATE VECTOR V--- 00008080
130  E=-B-D-F-H-SU-Z-RHO-CSS(N)                  00008090
      BL=B/(1.+W*(EL(NIB)+GL(NIB)))              00008100
      CL=D/(1.+W*(FL(NJB)+GL(NJB)))              00008110
      C=BL*EL(NIB)                                00008120
      G=CL*FL(NJB)                                00008130
      WU=CL*GL(NJB)                                00008140
      U=BL*GL(NIB)                                00008150
      IF (K.EQ.1) GO TO 140                        00008160
      AL=Z/(1.+W*(EL(NKB)+FL(NKB)))              00008170
      A=AL*EL(NKB)                                00008180
      TU=AL*FL(NKB)                                00008190
      DL=E+W*(A+C+G+WU+TU+U)-CL*EL(NJB)-BL*FL(NIB)-AL*GL(NKB) 00008200
      EL(N)=(F-W*(A+C))/DL                        00008210

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# SOLVE

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FL(N)=(H-W*(G+TU))/DL                                00008220
GL(N)=(SU-W*(WU+U))/DL                                00008230
SUPH=0.00                                              00008240
IF (K.NE.KO) SUPH=SU*PHI(NKA)                          00008250
RES=-B*PHI(NIB)-D*PHI(NJB)-E*PHI(N)-F*PHI(NJA)-H*PHI(NIA)-SUPH-Z*P00008260
1HI(NKB)-WELL(N)-RHO*OLD(N)-QR-CSS(N)*HSS(N)          00008270
IF(PHI(N).LT.HSS(N).AND.K.EQ.2)RES=RES+CSS(N)*(HSS(N)-PHI(N)) 00008280
IF(PHI(N).GT.HB(N).AND.K.EQ.2)RES=RES+CSS(N)*(HB(N)-PHI(N)) 00008290
RES=BETA*RES                                           00008300
V(N)=(RES-AL*V(NKB)-BL*V(NIB)-CL*V(NJB))/DL          00008310
GO TO 150                                              00008320
140 DL=E+W*(C+G+WU+U)-CL*EL(NJB)-BL*FL(NIB)          00008330
EL(N)=(F-W*C)/DL                                       00008340
FL(N)=(H-W*G)/DL                                       00008350
GL(N)=(SU-W*(WU+U))/DL                                00008360
SUPH=0.00                                              00008370
IF (K.NE.KO) SUPH=SU*PHI(NKA)                          00008380
RES=-B*PHI(NIB)-D*PHI(NJB)-E*PHI(N)-F*PHI(NJA)-H*PHI(NIA)-SUPH-WEL00008390
1L(N)-RHO*OLD(N)-QR-CSS(N)*HSS(N)                    00008400
IF(PHI(N).LT.HSS(N).AND.K.EQ.2)RES=RES+CSS(N)*(HSS(N)-PHI(N)) 00008410
IF(PHI(N).GT.HB(N).AND.K.EQ.2)RES=RES+CSS(N)*(HB(N)-PHI(N)) 00008420
RES=BETA*RES                                           00008430
V(N)=(RES-BL*V(NIB)-CL*V(NJB))/DL                    00008440
150 CONTINUE                                           00008450
C ---BACK SUBSTITUTE FOR VECTOR XI---                 00008460
DO 160 K=1,KO                                         00008470
K3=KO-K+1                                             00008480
DO 160 I=1,I2                                         00008490
I3=IO-I                                               00008500
DO 160 J=1,J2                                         00008510
J3=JO-J                                               00008520
N=I3+(J3-1)*IO+(K3-1)*NIJ+I-I                       00008530
IF (T(N).EQ.0..OR.S(N).LT.0.) GO TO 160              00008540
GLXI=0.00                                             00008550
IF (K3.NE.KO) GLXI=GL(N)*XI(N+NIJ)                   00008560
XI(N)=V(N)-EL(N)*XI(N+IO)-FL(N)*XI(N+1)-GLXI        00008570
C ---COMPARE MAGNITUDE OF CHANGE WITH CLOSURE CRITERIA--- 00008580
TCHK=ABS(XI(N))                                       00008590
IF (TCHK.GT.BIG) BIG=TCHK                             00008600
PHI(N)=PHI(N)+XI(N)                                  00008610
160 CONTINUE                                           00008620
IF (BIG.GT.ERR) TEST=1.                               00008630
TEST3(IT+1)=BIG                                       00008640
IF (TEST.EQ.0.) RETURN                                00008650
GO TO 60                                              00008660
C .....00008670
170 DO 200 KK=1,KO                                    00008680
K=KO-KK+1                                             00008690
DO 200 II=1,I2                                         00008700
I=IO-II                                               00008710
DO 200 J=2,J1                                          00008720
N=I+(J-1)*IO+(K-1)*NIJ                               00008730

```

# SOLVE

	NIA=N+1	00008740
	NIB=N-1	00008750
	NJA=N+IO	00008760
	NJB=N-IO	00008770
	NKA=N+NIJ	00008780
	NKB=N-NIJ	00008790
C	---SKIP COMPUTATIONS IF NODE OUTSIDE AQUIFER---	00008800
	IF (T(N).EQ.0..OR.S(N).LT.0.) GO TO 200	00008810
C	---COMPUTE COEFFICIENTS---	00008820
	D=TR(NJB)/DELX(J)	00008830
	F=TR(N)/DELX(J)	00008840
	B=TC(NIB)/DELY(I)	00008850
	H=TC(N)/DELY(I)	00008860
	SU=0.DO	00008870
	Z=0.DO	00008880
	IF(K.EQ.1) GO TO 174	00008890
	Z=TK(NKB)	00008900
	IF(IEQN.EQ.ICHK(11)) Z=Z/DELZ(K)	00008910
174	IF(K.EQ.KO) GO TO 175	00008920
	SU=TK(N)	00008930
	IF(IEQN.EQ.ICHK(11))SU=SU/DELZ(K)	00008940
175	RHO=S(N)/DELT	00008950
	QR=0.	00008960
	IF (IQRE.EQ.ICHK(7)) QR=QRE(N)	00008970
C	---SIP REVERSE ALGORITHM---	00008980
C	---FORWARD SUBSTITUTE, COMPUTING INTERMEDIATE VECTOR V---	00008990
180	E=-B-D-F-H-SU-Z-RHO-CSS(N)	00009000
	BL=H/(1.+W*(EL(NIA)+GL(NIA)))	00009010
	CL=D/(1.+W*(FL(NJB)+GL(NJB)))	00009020
	C=BL*EL(NIA)	00009030
	G=CL*FL(NJB)	00009040
	WU=CL*GL(NJB)	00009050
	U=BL*GL(NIA)	00009060
	IF (K.EQ.KO) GO TO 190	00009070
	AL=SU/(1.+W*(EL(NKA)+FL(NKA)))	00009080
	A=AL*EL(NKA)	00009090
	TU=AL*FL(NKA)	00009100
	DL=E+W*(C+G+A+WU+TU+U)-AL*GL(NKA)-BL*FL(NIA)-CL*EL(NJB)	00009110
	EL(N)=(F-W*(C+A))/DL	00009120
	FL(N)=(B-W*(G+TU))/DL	00009130
	GL(N)=(Z-W*(WU+U))/DL	00009140
	ZPHI=0.DO	00009150
	IF (K.NE.1) ZPHI=Z*PHI(NKB)	00009160
	RES=-B*PHI(NIB)-D*PHI(NJB)-E*PHI(N)-F*PHI(NJA)-H*PHI(NIA)-SU*PHI(N	00009170
	1KA)-ZPHI-WELL(N)-RHO*OLD(N)-QR-CSS(N)*HSS(N)	00009180
	IF(PHI(N).LT.HSS(N).AND.K.EQ.2)RES=RES+CSS(N)*(HSS(N)-PHI(N))	00009190
	IF(PHI(N).GT.HB(N).AND.K.EQ.2)RES=RES+CSS(N)*(HB(N)-PHI(N))	00009200
	RES=BETA*RES	00009210
	V(N)=(RES-AL*V(NKA)-BL*V(NIA)-CL*V(NJB))/DL	00009220
	GO TO 200	00009230
190	DL=E+W*(C+G+WU+U)-BL*FL(NIA)-CL*EL(NJB)	00009240
	EL(N)=(F-W*C)/DL	00009250

# SOLVE

```

FL(N)=(B-W*G)/DL                                00009260
GL(N)=(Z-W*(WU+U))/DL                            00009270
ZPHI=0.00                                         00009280
IF (K.NE.1) ZPHI=Z*PHI(NKB)                      00009290
RES=-B*PHI(NIB)-D*PHI(NJB)-E*PHI(N)-F*PHI(NJA)-H*PHI(NIA)-ZPHI-WEL 00009300
1L(N)-RHO*OLD(N)-QR-CSS(N)*HSS(N)                00009310
IF(PHI(N).LT.HSS(N).AND.K.EQ.2)RES=RES+CSS(N)*(HSS(N)-PHI(N)) 00009320
IF(PHI(N).GT.HB(N).AND.K.EQ.2)RES=RES+CSS(N)*(HB(N)-PHI(N)) 00009330
RES=BETA*RES                                       00009340
V(N)=(RES-BL*V(NIA)-CL*V(NJB))/DL                 00009350
200 CONTINUE                                       00009360
C ---BACK SUBSTITUTE FOR VECTOR XI---              00009370
DO 210 K=1,K0                                     00009380
DO 210 I=2,I1                                       00009390
DO 210 J=1,J2                                       00009400
J3=J0-J                                             00009410
N=I+(J3-1)*I0+(K-1)*NIJ                           00009420
IF (T(N).EQ.0..OR.S(N).LT.0.) GO TO 210            00009430
GLXI=0.00                                           00009440
IF (K.NE.1) GLXI=GL(N)*XI(N-NIJ)                   00009450
XI(N)=V(N)-EL(N)*XI(N+I0)-FL(N)*XI(N-1)-GLXI      00009460
C ---COMPARE MAGNITUDE OF CHANGE WITH CLOSURE CRITERIA--- 00009470
TCHK=ABS(XI(N))                                     00009480
IF (TCHK.GT.BIG) BIG=TCHK                           00009490
PHI(N)=PHI(N)+XI(N)                                00009500
210 CONTINUE                                       00009510
IF (BIG.GT.ERR) TEST=1.                             00009520
TEST3(IT+1)=BIG                                     00009530
IF (TEST.EQ.0.) RETURN                             00009540
GO TO 60                                            00009550
C .....00009560
C ---FORMATS---00009570
220 FORMAT ('OEXCEEDED PERMITTED NUMBER OF ITERATIONS'/' ',39('*')) 00009580
230 FORMAT (///1H0,I5,22H ITERATION PARAMETERS:,6E15.7/(/28X,6E15.7/))00009590
240 FORMAT ('-',44X,'SOLUTION BY THE STRONGLY IMPLICIT PROCEDURE'/45X,00009600
143('_'))00009610
END00009620

```

```

SUBROUTINE COEF(PHI,STRT,OLD,T,S,TR,TC,TK,WELL,DELX,DELY,DELZ,FACT00009630
1,PERM,BOTTOM,QRE)                                00009640
-----00009650
C COMPUTE COEFFICIENTS                                00009660
C -----00009670
C SPECIFICATIONS:                                00009680
C REAL *8PHI                                00009690
C DIMENSION PHI(IO,J0,K0), STRT(IO,J0,K0), OLD(IO,J0,K0), T(IO,J0,K000009700
1), S(IO,J0,K0), TR(IO,J0,K0), TC(IO,J0,K0), TK(IK,JK,K5), WELL(IO,00009710
2J0,K0), DELX(J0), DELY(IO), DELZ(K0), FACT(K0,3), PERM(IP,JP), BOT00009720
3TOM(IP,JP), QRE(IQ,JQ,KQ)                                00009730
C COMMON /INTEGR/ IO,J0,K0,I1,J1,K1,I,J,K,NPER,KTH,ITMAX,LENGTH,KP,N00009740
1WEL,NUMT,IFINAL,IT,KT,IHEAD,IDRAW,IFLO,IERR,I2,J2,K2,IMAX,ITMX1,NC00009750
2H,IDK1,IDK2,IWATER,IQRE,IP,JP,IQ,JQ,IK,JK,K5,IPU1,IPU2,ITK,IEQN,KQ00009760
COMMON /SPARAM/ TMAX,CDLT,DELT,ERR,TEST,SUM,SUMP,QR                                00009770
COMMON /SARRAY/ ICHK(14),LEVEL1(4),LEVEL2(4),LEVEL3(4),LEVEL4(4),L00009780
1EVEL5(4),LEVEL6(4),LEVEL7(4)                                00009790
C DATA N3=1                                00009800
C RETURN                                00009810
C .....00009820
C ---COMPUTE TRANSMISSIVITY FOR UPPER HYDROLOGIC UNIT WHEN 00009830
C IT IS UNCONFINED---                                00009840
C *****00009850
C ENTRY TRANS(N3)                                00009860
C *****00009870
C DO 10 I=2,I1                                00009880
C DO 10 J=2,J1                                00009890
C IF (PERM(I,J).EQ.0.) GO TO 10                                00009900
C T(I,J,K0)=PERM(I,J)*(PHI(I,J,K0)-BOTTOM(I,J)) 00009910
C IF (T(I,J,K0).GT.0.) GO TO 10                                00009920
C IF (WELL(I,J,K0).LT.0.) WRITE (6,60) I,J,K0 00009930
C IF (WELL(I,J,K0).GE.0.) WRITE (6,70) I,J,K0 00009940
C PERM(I,J)=0.                                00009950
C T(I,J,K0)=0.                                00009960
C TR(I,J-1,K0)=0.                                00009970
C TR(I,J,K0)=0.                                00009980
C TC(I,J,K0)=0.                                00009990
C TC(I-1,J,K0)=0.                                00010000
C IF (K0.NE.1) TK(I,J,K1)=0.                                00010010
C PHI(I,J,K0)=1.D30                                00010020
10 CONTINUE                                00010030
C IF (N3.EQ.1) RETURN                                00010040
C N1=K0                                00010050
C N2=K0                                00010060
C N4=K1                                00010070
C GO TO 20                                00010080
C ---COMPUTE T COEFFICIENTS---                                00010090

```

# COEF

C	*****	00010100
	ENTRY TCOF	00010110
C	*****	00010120
	N1=1	00010130
	N2=K0	00010140
	N4=1	00010150
20	DO 40 K=N1,N2	00010160
	DO 40 I=1,I1	00010170
	DO 40 J=1,J1	00010180
	IF (T(I,J,K).EQ.0.) GO TO 40	00010190
	IF (T(I,J+1,K).EQ.0.) GO TO 30	00010200
	TR(I,J,K)=(2.*T(I,J+1,K)*T(I,J,K))/(T(I,J,K)*DELX(J+1)+T(I,J+1,K)*	00010210
	1DELX(J))*FACT(K,1)	00010220
30	IF (T(I+1,J,K).EQ.0.) GO TO 40	00010230
	TC(I,J,K)=(2.*T(I+1,J,K)*T(I,J,K))/(T(I,J,K)*DELY(I+1)+T(I+1,J,K)*	00010240
	1DELY(I))*FACT(K,2)	00010250
40	CONTINUE	00010260
	IF (K0.EQ.1.OR.ITK.EQ.ICHK(10).OR.N3.EQ.0) RETURN	00010270
	DO 50 K=N4,K1	00010280
	DO 50 I=2,I1	00010290
	DO 50 J=2,J1	00010300
	IF (T(I,J,K+1).EQ.0.) GO TO 50	00010310
	T1=T(I,J,K)*FACT(K,3)	00010320
	T2=T(I,J,K+1)*FACT(K+1,3)	00010330
	TK(I,J,K)=(2.*T2*T1)/(T1*DELZ(K+1)+T2*DELZ(K))	00010340
50	CONTINUE	00010350
	RETURN	00010360
60	FORMAT ('-',20('*'),'WELL',2I3,' IN LAYER',I3,' GOES DRY',20('*'))	00010370
70	FORMAT ('-',20('*'),'NODE',2I3,' IN LAYER',I3,' GOES DRY',20('*'))	00010380
	END	00010390

```

SUBROUTINE CHECKI(PHI,STRT,OLD,T,S,TR,TC,TK,WELL,DELX,DELY,DELZ,FA00010400
1CT,JFLO,FLOW,QRE,CSS,HSS,HB,ETRAT)                                00010410
C -----00010420
C COMPUTE A VOLUMETRIC BALANCE                                00010430
C -----00010440
C SPECIFICATIONS:                                00010450
REAL *8PHI                                00010460
DIMENSION PHI(IO,JO,KO), STRT(IO,JO,KO), OLD(IO,JO,KO), T(IO,JO,KO00010470
1), S(IO,JO,KO), TR(IO,JO,KO), TC(IO,JO,KO), TK(IK,JK,K5), WELL(IO,00010480
2JO,KO), DELX(JO), DELY(IO), DELZ(KO), FACT(KO,3), JFLO(NCH,3), FLO00010490
3W(NCH),QRE(IQ,JQ,KQ),CSS(IO,JO,KO),HSS(IO,JO,KO),HB(IO,JO,KO),    00010500
4ETRAT(IO,JO,KO)                                00010510
COMMON /INTEGR/ IO,JO,KO,I1,J1,K1,I,J,K,NPER,KTH,ITMAX,LENGTH,KP,N00010520
1WEL,NUMT,IFINAL,IT,KT,IHEAD,IDRAW,IFLO,IERR,I2,J2,K2,IMAX,ITMX1,NC00010530
2H,IDK1,IDK2,IWATER,IQRE,IP,JP,IQ,JQ,IK,JK,K5,IPU1,IPU2,ITK,IEQN,KQ00010540
COMMON /SPARAM/ TMAX,CDLT,DELT,ERR,TEST,SUM,SUMP,QR    00010550
COMMON /SARRAY/ ICHK(14),LEVEL1(4),LEVEL2(4),LEVEL3(4),LEVEL4(4),L00010560
1LEVEL5(4),LEVEL6(4),LEVEL7(4)                                00010570
COMMON /CK/ ETFLXT,STORT,QRET,CHST,CHDT,FLUXT,PUMPT,CFLUXT,FLXNT 00010580
RETURN                                00010590
C .....00010600
C *****00010610
ENTRY CHECK                                00010620
C *****00010630
C ---INITIALIZE VARIABLES---00010640
PUMP=0.                                00010650
STOR=0.                                00010660
FLUXS=0.0                                00010670
CHD1=0.0                                00010680
CHD2=0.0                                00010690
QREFLX=0.                                00010700
CFLUX=0.                                00010710
FLUX=0.                                00010720
ETFLUX=0.                                00010730
FLXN=0.0                                00010740
II=0                                00010750
C .....00010760
C ---COMPUTE RATES,STORAGE AND PUMPAGE FOR THIS STEP---00010770
DO 220 K=1,KO                                00010780
DO 220 I=2,I1                                00010790
DO 220 J=2,J1                                00010800
IF (T(I,J,K).EQ.0.) GO TO 220                00010810
AREA=DELX(J)*DELY(I)                        00010820
VOLUME=AREA*DELZ(K)                        00010830
IF (S(I,J,K).GE.0.) GO TO 180                00010840
C ---COMPUTE FLOW RATES TO AND FROM CONSTANT HEAD BOUNDARIES---00010850
II=II+1                                00010860

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# CHECKI

FLOW(II)=0.	00010870
JFLO(II,1)=K	00010880
JFLO(II,2)=I	00010890
JFLO(II,3)=J	00010900
IF (S(I,J-1,K).LT.0..OR.T(I,J-1,K).EQ.0.) GO TO 30	00010910
X=(PHI(I,J,K)-PHI(I,J-1,K))*TR(I,J-1,K)*DELY(I)	00010920
IF(IEQN.EQ.ICHK(11)) X=X*DELZ(K)	00010930
FLOW(II)=FLOW(II)+X	00010940
IF (X) 10,30,20	00010950
10 CHD1=CHD1+X	00010960
GO TO 30	00010970
20 CHD2=CHD2+X	00010980
30 IF (S(I,J+1,K).LT.0..OR.T(I,J+1,K).EQ.0.) GO TO 60	00010990
X=(PHI(I,J,K)-PHI(I,J+1,K))*DELY(I)*TR(I,J,K)	00011000
IF(IEQN.EQ.ICHK(11)) X=X*DELZ(K)	00011010
FLOW(II)=FLOW(II)+X	00011020
IF (X) 40,60,50	00011030
40 CHD1=CHD1+X	00011040
GO TO 60	00011050
50 CHD2=CHD2+X	00011060
60 IF (K.EQ.1) GO TO 90	00011070
IF (S(I,J,K-1).LT.0..OR.T(I,J,K-1).EQ.0.) GO TO 90	00011080
X=(PHI(I,J,K)-PHI(I,J,K-1))*TK(I,J,K-1)*AREA	00011090
FLOW(II)=FLOW(II)+X	00011100
IF (X) 70,90,80	00011110
70 CHD1=CHD1+X	00011120
GO TO 90	00011130
80 CHD2=CHD2+X	00011140
90 IF (K.EQ.K0) GO TO 120	00011150
IF (S(I,J,K+1).LT.0..OR.T(I,J,K+1).EQ.0.) GO TO 120	00011160
X=(PHI(I,J,K)-PHI(I,J,K+1))*TK(I,J,K)*AREA	00011170
FLOW(II)=FLOW(II)+X	00011180
IF (X) 100,120,110	00011190
100 CHD1=CHD1+X	00011200
GO TO 120	00011210
110 CHD2=CHD2+X	00011220
120 IF (S(I-1,J,K).LT.0..OR.T(I-1,J,K).EQ.0.) GO TO 150	00011230
X=(PHI(I,J,K)-PHI(I-1,J,K))*TC(I-1,J,K)*DELY(J)	00011240
IF(IEQN.EQ.ICHK(11)) X=X*DELZ(K)	00011250
FLOW(II)=FLOW(II)+X	00011260
IF (X) 130,150,140	00011270
130 CHD1=CHD1+X	00011280
GO TO 150	00011290
140 CHD2=CHD2+X	00011300
150 IF (S(I+1,J,K).LT.0..OR.T(I+1,J,K).EQ.0.) GO TO 220	00011310
X=(PHI(I,J,K)-PHI(I+1,J,K))*TC(I,J,K)*DELY(J)	00011320
IF(IEQN.EQ.ICHK(11)) X=X*DELZ(K)	00011330
FLOW(II)=FLOW(II)+X	00011340
IF (X) 160,220,170	00011350
160 CHD1=CHD1+X	00011360
GO TO 220	00011370
170 CHD2=CHD2+X	00011380

# CHECKI

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GO TO 220 00011390
C ---CHECK FOR EQUATION BEING SOLVED--- 00011400
180 IF(IEQN.EQ.ICHK(11)) GO TO 211 00011410
C ---EQUATION 4--- 00011420
C ---RECHARGE AND WELLS--- 00011430
IF(IQRE.EQ.ICHK(7))QREFLX=QREFLX+QRE(I,J,K)*AREA 00011440
IF (WELL(I,J,K)) 190,210,200 00011450
190 PUMP=PUMP+WELL(I,J,K)*AREA 00011460
GO TO 210 00011470
200 CFLUX=CFLUX+WELL(I,J,K)*AREA 00011480
C ---COMPUTE VOLUME FROM STORAGE--- 00011490
210 STOR=STOR+S(I,J,K)*(OLD(I,J,K)-PHI(I,J,K))*AREA 00011500
HDD=PHI(I,J,K) 00011510
IF(HDD.LT.HSS(I,J,K).AND.K.EQ.2) HDD=HSS(I,J,K) 00011520
IF(HDD.GT.HB(I,J,K).AND.K.EQ.2)HDD=HB(I,J,K) 00011530
XNET=(HSS(I,J,K)-HDD)*CSS(I,J,K)*AREA 00011540
IF(K.EQ.1) GO TO 261 00011550
ETFLUX=ETFLUX+XNET 00011560
GO TO 262 00011570
261 FLUXS=FLUXS+XNET 00011580
IF(XNET.LT.0) FLXN=FLXN-XNET 00011590
C ---COMPUTE ET-RUNOFF RATES IN INCHES PER YEAR-- 00011600
262 IF(K.EQ.1) GO TO 219 00011610
IF(HDD.GT.HB(I,J,K))ETRAT(I,J,K)=3.784E08*CSS(I,J,K)*(HB(I,J,K)- 00011620
1HSS(I,J,K)) 00011630
IF(HDD.LE.HB(I,J,K).AND.HDD.GT.HSS(I,J,K))ETRAT(I,J,K)= 00011640
13.784E08*CSS(I,J,K)*(HDD-HSS(I,J,K)) 00011650
IF(HDD.LE.HSS(I,J,K))ETRAT(I,J,K)=0.0 00011660
C ---PRINT HCF FLOW RATE AT MODEL-GRID BOUNDARY--- 00011670
219 IF (K.NE.1) GO TO 220 00011680
XHCF=(HSS(I,J,K)-PHI(I,J,K))*CSS(I,J,K) 00011690
IF(XHCF.EQ.0.0)GO TO 220 00011700
C WRITE(6,295)I,J,XHCF 00011710
WRITE(7,295)I,J,XHCF 00011720
GO TO 220 00011730
C ---EQUATION 3--- 00011740
C ---RECHARGE AND WELLS--- 00011750
211 IF(IQRE.EQ.ICHK(7))QREFLX=QREFLX+QRE(I,J,K)*VOLUME 00011760
IF (WELL(I,J,K)) 212,214,213 00011770
212 PUMP=PUMP+WELL(I,J,K)*VOLUME 00011780
GO TO 214 00011790
213 CFLUX=CFLUX+WELL(I,J,K)*VOLUME 00011800
C ---COMPUTE VOLUME FROM STORAGE--- 00011810
214 STOR=STOR+S(I,J,K)*(OLD(I,J,K)-PHI(I,J,K))*VOLUME 00011820
HDD=PHI(I,J,K) 00011830
IF(HDD.LT.HSS(I,J,K).AND.K.EQ.2)HDD=HSS(I,J,K) 00011840
IF(HDD.GT.HB(I,J,K).AND.K.EQ.2)HDD=HB(I,J,K) 00011850
XNET=(HSS(I,J,K)-HDD)*CSS(I,J,K)*VOLUME 00011860
IF(K.EQ.1) GO TO 271 00011870
ETFLUX=ETFLUX+XNET 00011880
GO TO 220 00011890
271 FLUXS=FLUXS+XNET 00011900

```



# CHECKI

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      IF(XNET.LT.0) FLXN=FLXN-XNET                                00011910
220  CONTINUE                                                    00011920
C    ---DETERMINE IF WATER TABLE RISES ABOVE LAND SURFACE--    00011930
      DO 221 K=2,2                                              00011940
      DO 221 I=1,IO                                             00011950
      DO 221 J=1,JO                                             00011960
      RISE=PHI(I,J,K)-HB(I,J,K)                                00011970
      -REMOVE C FROM NEXT CARD TO PREVENT WT RISE ABOVE LAND    00011980
C    IF(RISE.GT.0.00) PHI(I,J,K)=HB(I,J,K)                    00011990
221  IF(RISE.GT.0.0)WRITE(6,296)RISE,I,J                      00012000
C    ---REMOVE C FROM COL 1 OF NEXT 4 CARDS TO PUNCH ET-RUNOFF-- 00012010
      WRITE(7,298)                                              00012020
      DO 222 K=2,2                                              00012030
      DO 222 I=1,IO                                             00012040
C 222  WRITE(7,297)(ETRAT(I,J,K),J=1,JO)                      00012050
C    .....00012060
C    ---COMPUTE CUMULATIVE VOLUMES, TOTALS, AND DIFFERENCES--- 00012070
      FLXPT=0.0                                                00012080
      STORT=STORT+STOR                                          00012090
      STOR=STOR/DELT                                           00012100
      FLUXT=FLUXT+FLUXS*DELT                                    00012110
      FLXNT=FLXNT+FLXN*DELT                                    00012120
      FLXPT=FLUXT+FLXNT                                         00012130
      QRET=QRET+QREFLX*DELT                                    00012140
      ETFLXT=ETFLXT-ETFLUX*DELT                                00012150
      CHDT=CHDT-CHD1*DELT                                       00012160
      CHST=CHST+CHD2*DELT                                       00012170
      PUMPT=PUMPT-PUMP*DELT                                     00012180
      CFLUXT=CFLUXT+CFLUX*DELT                                  00012190
      TOTL1=STORT+QRET+CFLUXT+CHST+FLXPT                        00012200
      TOTL2=CHDT+PUMPT+ETFLXT+FLXNT                            00012210
      SUMR=QREFLX+CFLUX+CHD2+CHD1+PUMP+ETFLUX+FLUXS+STOR      00012220
      DIFF=TOTL2-TOTL1                                         00012230
      PERCNT=0.0                                               00012240
      IF (TOTL2.EQ.0.) GO TO 230                                00012250
      PERCNT=DIFF/TOTL2*100.                                    00012260
230  RETURN                                                    00012270
C    .....00012280
C    ---PRINT RESULTS---                                        00012290
C    *****00012300
      ENTRY CWRITE                                              00012310
C    *****00012320
      WRITE (6,260) STOR,QREFLX,STORT,CFLUX,QRET,PUMP,CFLUXT,ETFLUX,CHST00012330
1,FLXPT,CHD2,TOTL1,CHD1,FLUX,FLUXS,ETFLXT,CHDT,SUMR,PUMPT,FLXNT,TOT00012340
2L2,DIFF,PERCNT                                              00012350
      WRITE(6,291)                                              00012360
      DO 500 K=1,KO                                             00012370
      ACTNOD=0.                                                 00012380
      TOTABR=0.                                                 00012390
      TOTPCT=0.                                                 00012400
      TTOT=0.                                                  00012410
      RTOT=0.                                                  00012420

```

# CHECK I

RPOS=0.	00012430
RNEG=0.	00012440
POSNOD=0.	00012450
RNEGND=0.	00012460
RMAXD=0.	00012470
RMIND=0.	00012480
RMINT=1000000.	00012490
RMAXT=0.	00012500
RMAXR=0.	00012510
RMINR=0.	00012520
MROW=0	00012530
MCOL=0	00012540
NROW=0	00012550
NCOL=0	00012560
DO 400 I=2,I1	00012570
DO 400 J=2,J1	00012580
IF(T(I,J,K).EQ.0.OR.S(I,J,K).LT.0) GO TO 400	00012590
ACTNOD=ACTNOD+1.	00012600
DDN=STRT(I,J,K)-PHI(I,J,K)	00012610
IF (STRT(I,J,K).EQ.0.0) GO TO 390	00012620
PCT=(ABS(DDN)/STRT(I,J,K))*100.	00012630
TOTPCT=TOTPCT+PCT	00012640
390 TOTABR=TOTABR+ABS(DDN)	00012650
IF(DDN.LT.RMAXD) GO TO 391	00012660
RMAXD=DDN	00012670
MROW=I	00012680
MCOL=J	00012690
391 IF(DDN.GT.RMIND) GO TO 392	00012700
RMIND=DDN	00012710
NROW=I	00012720
NCOL=J	00012730
392 TDUM=T(I,J,K)/(1.5472E-06)	00012740
TTOT=TTOT+TDUM	00012750
IF(TDUM.GE.RMAXT) RMAXT=TDUM	00012760
IF(TDUM.LE.RMINT) RMINT=TDUM	00012770
IF(IQRE.NE.ICHK(7)) GOTO 400	00012780
IF(K.NE.KO) GOTO 400	00012790
RDUM=QRE(I,J,K)/(2.64E-09)	00012800
RTOT=RTOT+RDUM	00012810
IF(RDUM.GE.RMAXR) RMAXR=RDUM	00012820
IF(RDUM.LE.RMINR) RMINR=RDUM	00012830
IF(RDUM.GT.0) RPOS=RPOS+RDUM	00012840
IF(RDUM.GT.0) POSNOD=POSNOD+1.	00012850
IF(RDUM.LT.0) RNEG=RNEG+RDUM	00012860
IF(RDUM.LT.0) RNEGND=RNEGND+1.	00012870
400 CONTINUE	00012880
IF(ACTNOD) 500,500,418	00012890
418 AVABER=TOTABR/ACTNOD	00012900
AVPCT=TOTPCT/ACTNOD	00012910
TAV=TTOT/ACTNOD	00012920
WRITE(6,292) K,AVABER,AVPCT,RMAXD,MROW,MCOL,RMIND,NROW,NCOL,TAV,	00012930
1RMAXT,RMINT,ACTNOD	00012940

# CHECKI

```

      IF(IQRE.NE.ICHK(7)) GOTO 500                                00012950
      IF(K.NE.K0) GOTO 500                                        00012960
      RAV=RTOT/ACTNOD                                           00012970
      IF(POSNOD)420,420,430                                     00012980
420  AVPOSR=0.                                                  00012990
      GO TO 440                                                  00013000
430  AVPOSR=RPOS/POSNOD                                         00013010
440  IF(RNEGND)450,450,460                                       00013020
450  AVNEGR=0.                                                  00013030
      GO TO 470                                                  00013040
460  AVNEGR=RNEG/RNEGND                                         00013050
470  WRITE(6,293)                                               00013060
      WRITE(6,294) K,RAV,AVPOSR,POSNOD,AVNEGR,RNEGND,RMAXR,RMINR 00013070
500  CONTINUE                                                  00013080
      RETURN                                                    00013090
C    ---FORMATS---                                           00013100
C    -----00013110
260  FORMAT ('0',10X,'CUMULATIVE MASS BALANCE:',16X,'L*3',23X,'RATES F00013120
10R THIS TIME STEP:',16X,'L*3/T'/11X,24('-'),43X,25('-')//20X,'SOU00013130
2RCES:',69X,'STORAGE =',F20.4/20X,8('-'),68X,'RECHARGE =',F20.4/27X00013140
3,'STORAGE =',F20.2,35X,'CONSTANT FLUX =',F20.4/26X,'RECHARGE =',F200013150
40.2,41X,'PUMPING =',F20.4/21X,'CONSTANT FLUX =',F20.2,30X,'      00013160
5 ET-RUNOFF =',F20.4/21X,'CONSTANT HEAD =',F20.2,34X,'CONSTANT HEA00013170
6D: '/27X,'LEAKAGE =',F20.2,46X,'IN =',F20.4/21X,'TOTAL SOURCES =',F00013180
720.2,45X,'OUT =',F20.4/96X,'LEAKAGE: '/20X,'DISCHARGES:',45X,'FROM 00013190
8PREVIOUS PUMPING PERIOD =',F20.4/20X,11('-'),68X,'TOTAL =',F20.4/100013200
96X,'      ET-RUNOFF =',F20.2/21X,'CONSTANT HEAD =',F20.2,36X,'S00013210
$UM OF RATES =',F20.4/19X'QUANTITY PUMPED =',F20.2/27X,'LEAKAGE =',00013220
$F20.2/19X,'TOTAL DISCHARGE =',F20.2//17X,'DISCHARGE-SOURCES =',F2000013230
$.2/15X,'PER CENT DIFFERENCE =',F20.2//)                      00013240
270  FORMAT ('OFLOW RATES TO CONSTANT HEAD NODES: '/' ',34('-')//' ',4(500013250
1X,'K',4X,'I',4X,'J',3X,'RATE (IN/YR)')/' ',4(5X,'-',4X,'-',4X,'-'00013260
2,3X,13('-'))//)                                               00013270
280  FORMAT (/(1X,4(I6,2I5,F10.1,6X)))                          00013280
291  FORMAT('0',17X,'AVG',2X,'ABS CHG',5X,'MAX DDN',7X,'MAX RISE',12X,'00013290
1AVG T (GPD/FT)',11X,'MAX T',7X,'MIN T',4X,'ACTIVE NODES')    00013300
292  FORMAT(' ',1X,'LAYER',2X,I4,4X,F4.1,3X,F4.1,'X',3X,F5.1,1X,'(',I2,00013310
1',',I2,')',2X,F5.1,1X,'(',I2,',',I2,')',6X,F11.0,10X,F11.0,4X,F8.000013320
2,2X,F10.0)                                                    00013330
293  FORMAT('0',15X,'AVG RECHG (IN/YR)',7X,'AVG POS RECHG',3X,'NO. POS 00013340
1 NODES',3X,'AVG NEG RECHG',3X,'NO.NEG NODES',3X,'MAX RECHG',7X,'MA00013350
2X DISCHG')                                                    00013360
294  FORMAT(' ',1X,'LAYER',2X,I4,6X,F6.1,16X,F6.1,9X,F7.0,10X,F6.1,10X 00013370
1,F7.0,8X,F6.1,10X,F6.1)                                       00013380
295  FORMAT(11X,I2,6X,I2,3X,E15.5,30X,'HCF FLOW')              00013390
296  FORMAT('OWATER TABLE RISES',F6.1,' FEET ABOVE LSD AT ROW',I5,' COL00013400
1',I5)                                                         00013410
297  FORMAT(20F4.1)                                              00013420
298  FORMAT(5X,'ET RATES FOLLOW')                                00013430
      END                                                        00013440

```

```

C      SUBROUTINE PRNTAI(PHI,STRT,T,S,WELL,DELX,DELY,QRE,TK,ETRAT)      00013450
C      -----00013460
C      PRINT MAPS OF DRAWDOWN, HYDRAULIC HEAD, HEAD DIFFERENCE, RECHARGE,00013470
C      ET-RUNOFF RATE, LEAKAGE RATE, AND PUMPING RATE      00013480
C      -----00013490
C      SPECIFICATIONS:      00013500
C      REAL *8PHI,Z,XLABEL,YLABEL,TITLE,XN1,MESUR      00013510
C      REAL *4K      00013520
C      DIMENSION PHI(IO,JO,KO), STRT(IO,JO,KO), S(IO,JO,KO), WELL(IO,JO,KO00013530
10), DELX(JO), DELY(IO), T(IO,JO,KO),QRE(IQ,JQ,KQ),TK(IK,JK,K5), 00013540
2ETRAT(IO,JO,KO)      00013550
C      COMMON /INTEGR/ IO,JO,KO,I1,J1,K1,I,J,K,NPER,KTH,ITMAX,LENGTH,KP,N00013560
1WEL,NUMT,IFINAL,IT,KT,IHEAD,IDRAW,IFLO,IERR,I2,J2,K2,IMAX,ITMX1,NC00013570
2H,IDK1,IDK2,IWATER,IQRE,IP,JP,IQ,JQ,IK,JK,K5,IPU1,IPU2,ITK,IEQN,KQ00013580
C      COMMON /PR/ XLABEL(3),YLABEL(6),TITLE(6),XN1,MESUR,PRNT(122),BLANK00013590
1(60),DIGIT(122),VF1(6),VF2(6),VF3(7),XSCALE,DINCH,SYM(17),XN(100),00013600
2YN(13),NA(4),N1,N2,N3,YSCALE,FACT1,FACT2,FACT3,FACT4,FACT5,FACT6,F00013610
3ACT7,IWELLO(10)      00013620
C      RETURN      00013630
C      .....00013640
C      ---INITIALIZE VARIABLES FOR PLOT---      00013650
C      *****      00013660
C      ENTRY MAP      00013670
C      *****      00013680
C      YDIM=0.      00013690
C      WIDTH=0.      00013700
C      DO 10 J=2,J1      00013710
10 WIDTH=WIDTH+DELX(J)      00013720
C      DO 20 I=2,I1      00013730
20 YDIM=YDIM+DELY(I)      00013740
30 XSF=DINCH*XSCALE      00013750
YSF=DINCH*YSCALE      00013760
NYD=YDIM/YSF      00013770
IF (NYD*YSF.LE.YDIM-DELY(I1)/2.) NYD=NYD+1      00013780
IF (NYD.LE.12) GO TO 40      00013790
DINCH=YDIM/(12.*YSCALE)      00013800
WRITE (6,330) DINCH      00013810
IF (YSCALE.LT.1.0) WRITE (6,340)      00013820
GO TO 30      00013830
40 NXD=WIDTH/XSF      00013840
IF (NXD*XSF.LE.WIDTH-DELX(J1)/2.) NXD=NXD+1      00013850
N4=NXD*N1+1      00013860
N5=NXD+1      00013870
N6=NYD+1      00013880
N8=N2*NYD+1      00013890
NA(1)=N4/2-1      00013900
NA(2)=N4/2      00013910

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# PRNTAI

NA(3)=N4/2+3	00013920
NC=(N3-N8-10)/2	00013930
ND=NC+N8	00013940
NE=MAX0(N5,N6)	00013950
VF1(3)=DIGIT(ND)	00013960
VF2(3)=DIGIT(ND)	00013970
VF3(3)=DIGIT(NC)	00013980
XLABEL(3)=MESUR	00013990
YLABEL(6)=MESUR	00014000
DO 60 I=1,NE	00014010
NNX=N5-I	00014020
NNY=I-1	00014030
IF (NNY.GE.N6) GO TO 50	00014040
YN(I)=YSF*NNY/YSCALE	00014050
50 IF (NNX.LT.0) GO TO 60	00014060
XN(I)=XSF*NNX/YSCALE	00014070
60 CONTINUE	00014080
RETURN	00014090
.....	00014100
*****	00014110
ENTRY PRNTA(NG,LA)	00014120
*****	00014130
---VARIABLES INITIALIZED EACH TIME A PLOT IS REQUESTED---	00014140
DIST=WIDTH-DELX(J1)/2.	00014150
JJ=J1	00014160
LL=1	00014170
Z=NXD*XSF	00014180
IF (NG.EQ.1.AND.LA.EQ.1) WRITE(6,300)	00014190
IF (NG.EQ.1.AND.LA.EQ.2) WRITE(6,301)	00014200
IF (NG.EQ.2.AND.LA.EQ.1) WRITE(6,302)	00014210
IF (NG.EQ.2.AND.LA.EQ.2) WRITE(6,303)	00014220
IF (NG.EQ.3) WRITE(6,295)	00014230
IF (NG.EQ.4) WRITE(6,297)	00014240
IF (NG.EQ.5) WRITE(6,298)	00014250
IF (NG.EQ.6) WRITE(6,299)	00014260
IF (NG.EQ.7) WRITE(6,311)	00014270
DO 290 I=1,N4	00014280
---LOCATE X AXES---	00014290
IF (I.EQ.1.OR.I.EQ.N4) GO TO 70	00014300
PRNT(1)=SYM(12)	00014310
PRNT(N8)=SYM(12)	00014320
IF ((I-1)/N1*N1.NE.I-1) GO TO 90	00014330
PRNT(1)=SYM(14)	00014340
PRNT(N8)=SYM(14)	00014350
GO TO 90	00014360
---LOCATE Y AXES---	00014370
70 DO 80 J=1,N8	00014380
IF ((J-1)/N2*N2.EQ.J-1) PRNT(J)=SYM(14)	00014390
80 IF ((J-1)/N2*N2.NE.J-1) PRNT(J)=SYM(13)	00014400
---COMPUTE LOCATION OF NODES AND DETERMINE APPROPRIATE SYMBOL---	00014410
90 IF (DIST.LT.0..OR.DIST.LT.Z-XN1*XSF) GO TO 240	00014420
YLEN=DELY(2)/2.	00014430

# PRNTAI

DO 220 L=2,I1	00014440
J=YLEN*N2/YSF+1.5	00014450
IF (T(L,JJ,LA).EQ.0.) GO TO 160	00014460
IF (S(L,JJ,LA).LT.0.) GO TO 210	00014470
INDX3=0	00014480
GO TO (100,110,112,114,116,118,119), NG	00014490
100 K=(STRT(L,JJ,LA)-PHI(L,JJ,LA))*FACT1	00014500
GO TO 120	00014510
110 K=PHI(L,JJ,LA)*FACT2	00014520
GO TO 120	00014530
112 K=PHI(L,JJ,LA+1)-PHI(L,JJ,LA)	00014540
GO TO 120	00014550
114 K=QRE(L,JJ,LA)*FACT4/(2.6424E-09)	00014560
GO TO 120	00014570
116 K=ETRAT(L,JJ,LA)*FACT5	00014580
GO TO 120	00014590
118 K=TK(L,JJ,LA)*(PHI(L,JJ,LA+1)-PHI(L,JJ,LA))*FACT6/(2.6424E-09)	00014600
C --REMOVE C FROM COL 1 OF NEXT CARD TO PUNCH LEAKAGE RATE--	00014610
WRITE(7,350)L,JJ,K	00014620
GO TO 120	00014630
119 K=WELL(L,JJ,LA)*.646317*FACT7*DELX(JJ)*DELY(L)	00014640
120 IF (K) 130,160,140	00014650
130 IF (J-2.GT.0) PRNT(J-2)=SYM(13)	00014660
N=-K+.5	00014670
IF (N.LT.100) GO TO 150	00014680
GO TO 190	00014690
140 N=K+.5	00014700
IF (N.LT.100) GO TO 150	00014710
IF (N.GT.999) GO TO 190	00014720
INDX3=N/100	00014730
IF (J-2.GT.0) PRNT(J-2)=SYM(INDX3)	00014740
N=N-INDX3*100	00014750
150 INDX1=MOD(N,10)	00014760
IF (INDX1.EQ.0) INDX1=10	00014770
INDX2=N/10	00014780
IF (INDX2.GT.0) GO TO 180	00014790
INDX2=10	00014800
IF (INDX3.EQ.0) INDX2=15	00014810
GO TO 180	00014820
160 INDX1=15	00014830
170 INDX2=15	00014840
180 IF (J-1.GT.0) PRNT(J-1)=SYM(INDX2)	00014850
PRNT(J)=SYM(INDX1)	00014860
GO TO 220	00014870
190 DO 200 II=1,3	00014880
JI=J-3+II	00014890
200 IF (JI.GT.0) PRNT(JI)=SYM(11)	00014900
210 IF (S(L,JJ,LA).LT.0.) PRNT(J)=SYM(16)	00014910
220 YLEN=YLEN+(DELY(L)+DELY(L+1))/2.	00014920
230 DIST=DIST-(DELX(JJ)+DELX(JJ-1))/2.	00014930
JJ=JJ-1	00014940
IF (JJ.EQ.0) GO TO 240	00014950

PRNTAI

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      IF (DIST.GT.Z-XN1*XS F) GO TO 230                                00014960
240 CONTINUE                                                            00014970
C  ---PRINT AXES,LABELS, AND SYMBOLS---                                00014980
      IF (I-NA(LL).EQ.0) GO TO 260                                    00014990
      IF ((I-1)/N1*N1-(I-1)) 270,250,270                             00015000
250 WRITE (6,VF1) (BLANK(J),J=1,NC),(PRNT(J),J=1,N8),XN(1+(I-1)/6) 00015010
      GO TO 280                                                        00015020
260 WRITE (6,VF2) (BLANK(J),J=1,NC),(PRNT(J),J=1,N8),XLABEL(LL)    00015030
      LL=LL+1                                                         00015040
      GO TO 280                                                        00015050
270 WRITE (6,VF2) (BLANK(J),J=1,NC),(PRNT(J),J=1,N8)                00015060
C  ---COMPUTE NEW VALUE FOR Z AND INITIALIZE PRNT---                    00015070
280 Z=Z-2.*XN1*XS F                                                  00015080
      DO 290 J=1,N8                                                    00015090
290 PRNT(J)=SYM(15)                                                    00015100
C  ---NUMBER AND LABEL Y AXIS AND PRINT LEGEND---                      00015110
      WRITE (6,VF3) (BLANK(J),J=1,NC),(YN(I),I=1,N6)                00015120
      WRITE (6,320) (YLABEL(I),I=1,6)                                00015130
      IF (NG.EQ.1) WRITE (6,310) FACT1                                00015140
      IF (NG.EQ.2) WRITE (6,310) FACT2                                00015150
      RETURN                                                            00015160
C  ---FORMATS---                                                        00015170
C  -----00015180
295 FORMAT ('1',36X,'HEAD DIFFERENCE *WATER TABLE MINUS POTENTIOMETRIC 00015190
1 SURFACE* FEET',//)                                                  00015200
297 FORMAT ('1',40X,'RATE OF RECHARGE TO SURFICIAL AQUIFER, INCHES PER 00015210
1 YEAR',//)                                                            00015220
298 FORMAT ('1',39X,'ET-RUNOFF FROM SURFICIAL AQUIFER, INCHES PER YEAR 00015230
1',//)                                                                  00015240
299 FORMAT ('1',40X,'RATE OF LEAKAGE TO FLORIDAN AQUIFER, INCHES PER Y 00015250
1EAR',//)                                                              00015260
300 FORMAT ('1',40X,'DRAWDOWN IN FLORIDAN AQUIFER, FEET',//)          00015270
301 FORMAT ('1',40X,'DRAWDOWN IN SURFICIAL AQUIFER, FEET',//)          00015280
302 FORMAT ('1',35X,'ALTITUDE OF POTENTIOMETRIC SURFACE OF THE FLORIDA 00015290
MOD 1N AQUIFER, FEET',//)                                              00015300
303 FORMAT ('1',35X,'ALTITUDE OF WATER TABLE IN THE SURFICIAL AQUIFER, 00015310
1 FEET',//)                                                            00015320
310 FORMAT ('OEXPLANATION'/ ' ',11('-')// ' R = CONSTANT HEAD BOUNDARY'/ 00015330
1' *** = VALUE EXCEEDED 3 FIGURES'/ ' MULTIPLICATION FACTOR =' ,F8.3) 00015340
311 FORMAT('1',40X,'PUMPING RATE FROM FLORIDAN AQUIFER, MGAL/D',//)    00015350
320 FORMAT ('0',39X,6A8)                                                00015360
330 FORMAT ('0',25X,10('*'), ' TO FIT MAP WITHIN 12 INCHES, DINCH REVIS 00015370
1ED TO',G15.7,1X,10('*'))                                              00015380
340 FORMAT ('0',45X,'NOTE: GENERALLY SCALE SHOULD BE > OR = 1.0')      00015390
350 FORMAT(I5,I5,F10.1,40X,'LEAK')                                     00015400
      END                                                              00015410

```

```

BLOCK DATA                                00015420
-----                                00015430
C SPECIFICATIONS:                        00015440
C REAL *XLABEL,YLABEL,TITLE,XN1,MESUR    00015450
COMMON /SARRAY/ ICHK(14),LEVEL1(4),LEVEL2(4),LEVEL3(4),LEVEL4(4),L00015460
1LEVEL5(4),LEVEL6(4),LEVEL7(4)          00015470
COMMON /PR/ XLABEL(3),YLABEL(6),TITLE(6),XN1,MESUR,PRNT(122),BLANK00015480
1(60),DIGIT(122),VF1(6),VF2(6),VF3(7),XSCALE,DINCH,SYM(17),XN(100),00015490
2YN(13),NA(4),N1,N2,N3,YSCALE,FACT1,FACT2,FACT3,FACT4,FACT5,FACT6,F00015500
3ACT7,IWELLO(10)                        00015510
C *****00015520
DATA ICHK/'DRAW','HEAD','MASS','DK1','DK2','WATE','RECH','PUN1','P00015530
1UN2','ITKR','EQN3','TKLR','UNFM','CYCL'/ 00015540
DATA SYM/'1','2','3','4','5','6','7','8','9','0','*','|','-','+',00015550
1 ' ','R','W'/ 00015560
DATA PRNT/122*' '/,N1,N2,N3,XN1/6,10,133,.833333333D-1/,BLANK/60*'00015570
1 ' '/,NA(4)/1000/ 00015580
DATA XLABEL/' X DIS- ','TANCE IN',' MILES '/,YLABEL/'DISTANCE',' 00015590
1FROM OR','IGIN IN ','Y DIRECT','ION, IN ','MILES '/,TITLE/'PLOT 00015600
2OF ','DRAWDOWN',' ','PLOT OF ','HYDRAULI','C HEAD'/ 00015610
DATA DIGIT/'1','2','3','4','5','6','7','8','9','10','11','12','13'00015620
1,'14','15','16','17','18','19','20','21','22','23','24','25','26',00015630
2'27','28','29','30','31','32','33','34','35','36','37','38','39',00015640
340','41','42','43','44','45','46','47','48','49','50','51','52',500015650
43','54','55','56','57','58','59','60','61','62','63','64','65',6600015660
5','67','68','69','70','71','72','73','74','75','76','77','78',7900015670
6','80','81','82','83','84','85','86','87','88','89','90','91','92'00015680
7,'93','94','95','96','97','98','99','100','101','102','103','104'00015690
8,'105','106','107','108','109','110','111','112','113','114','115'00015700
9,'116','117','118','119','120','121','122'/ 00015710
DATA VF1/('(1H ',' ',' ',' ','A1,F','10.2',')'/' 00015720
DATA VF2/('(1H ',' ',' ',' ','A1,1','X,A8',')'/' 00015730
DATA VF3/('(1HO',' ',' ',' ','A1,F','3.1',','12F1','0.2')'/' 00015740
C *****00015750
END 00015760

```



## ATTACHMENT B: DATA-DECK INSTRUCTIONS

The data deck supplies input to a FORTRAN program tailored specifically to the hydrogeologic system conceptualized for the well-field areas near Tampa. Instructions for assembling the data deck have been modified from those presented in Trescott (1975). The modifications pertain mainly to setting up the deck to accommodate the HCF condition in the Floridan aquifer (layer 1), ET-runoff from the water table in the surficial aquifer (layer 2), and the addition of an acceleration parameter BETA. Additionally, the instructions have been modified to produce maps that were not available in the original model. These include maps of head difference between the water table and potentiometric surface, recharge rate to the water table, ET-runoff rate from the water table, leakage rate through the upper confining bed, and the distribution of pumpage. All data-deck modifications are denoted by an asterisk.

## ATTACHMENT B: DATA-DECK INSTRUCTIONS

[modified from Trescott (1975); \* denotes modification]

### Group I: Title, Simulation Options, and Problem Dimensions

This group of cards that are read by the main program contain data required to dimension the model. To specify an option on card 4, punch the characters underlined in the definition. For an option not used, that section of card 4 can be left blank.

Note: Default typing of variables applies for all data input.

<u>CARD</u>	<u>COLUMNS</u>	<u>FORMAT</u>	<u>VARIABLE</u>	<u>DEFINITION</u>
1	1-80	20A4	HEADING	Any title the user wishes to print on one line at the start of output.
2	1-52	13A4	do.	
3	1-10	I10	I0	Number of rows.
	11-20	I10	J0	Number of columns.
	21-30	I10	K0	Number of layers.
	31-40	I10	ITMAX	Maximum number or iterations per time step.
	41-50	I10	NCH	Number of constant head nodes.
4	1-4	A4	IDRAW	<u>DRAW</u> to print drawdown.
	6-9	A4	IHEAD	<u>HEAD</u> to print hydraulic head.
	11-14	A4	IFLOW	<u>MASS</u> to compute a mass balance.
	16-18	A3	IDK1	<u>DK1</u> to read initial head, elapsed time, and mass balance parameters from unit 4 on disk.
	21-23	A3	IDK2	<u>DK2</u> to write computed head, elapsed time, and mass balance parameters on unit 4 (disk).
	26-29	A4	IWATER	<u>WATE</u> if the upper hydrologic unit is unconfined.
	31-34	A4	IQRE	<u>RECH</u> for a constant recharge that may be a function of space.
	36-39	A4	IPU1	<u>PUN1</u> to read initial head, elapsed time, and mass balance parameters from cards.
	41-44	A4	IPU2	<u>PUN2</u> to punch computed head, elapsed time, and mass balance parameters on cards.

<u>CARD</u>	<u>COLUMNS</u>	<u>FORMAT</u>	<u>VARIABLE</u>	<u>DEFINITION</u>
4	46-49	A4	ITK	<u>ITKR</u> to read the value of TK(I,J,K) for simulations in which confining layers are not represented by layers of nodes. $TK(I,J,K) = K_{zz}/b$ .
	51-54	A4	IEQN	<u>EQN3</u> if equation 3 is being solved; otherwise it is assumed that equation 4 is being solved. Leave blank for Q-3-D.

## Group II: Scalar Parameters

The parameters required in every problem are underlined. The other parameters are required as noted; when not required, their location on the card can be left blank. The G format is used to read E, F, and I format data. Minimize mistakes by always right-justifying data in the field. If F format data do not contain significant figures to the right of the decimal point, the decimal point can be omitted.

<u>CARD</u>	<u>COLUMNS</u>	<u>FORMAT</u>	<u>VARIABLE</u>	<u>DEFINITION</u>
1	1-10	G10.0	<u>NPER</u>	Number of pumping periods for the simulation.
	11-20	G10.0	<u>KTH</u>	Number of time steps between print-outs.

Note: To print only the results for the final time step in a pumping period, make KTH greater than the expected number of time steps. The program always prints the results for the final time step.

21-30	G10.0	<u>ERR</u>	Error criterion for closure (L).
-------	-------	------------	----------------------------------

Note: When the head change at all nodes on subsequent iterations is less than this value (for example, 0.01 foot), the program has converged to a solution for the time step.

	31-40	G10.0	<u>LENGTH</u>	Number of iteration parameters.
	*41-50	G10.0	<u>BETA</u>	Acceleration parameter: probable range is 0.5 to 1.5; less than 1 if diverging; greater than 1 if converging too slowly.
2	1-7	G10.0	XSCALE	Factor to convert model length unit to unit used in X direction on maps (for example, to convert from feet to miles, XSCALE = 5,280). <u>For no maps, card 2 is blank.</u>
	*8-14	G10.0	YSCALE	Factor to convert model length unit to unit used in Y direction on maps.
	*15-21	G10.0	DINCH	Number of map units per inch.

<u>CARD</u>	<u>COLUMNS</u>	<u>FORMAT</u>	<u>VARIABLE</u>	<u>DEFINITION</u>
2	*22-24	G3.0	FACT1	Factor to adjust value of drawdown printed. <sup>†</sup>
	*25-28	4I1	LEVEL1	Layers for which drawdown maps are to be printed. List the layers starting in column 41; the first zero entry terminates the printing of drawdown maps.
	*29-31	G3.0	FACT2	Factor to adjust value of head printed.
	*32-35	4I1	LEVEL2	Layers for which head maps are to be printed.
	*36-38	G3.0	FACT3	Factor to adjust value of head difference printed.
	*39-42	4I1	LEVEL3	If map of head difference between water table and potentiometric surface is desired, put a 1 in column 39.
	*43-45	G3.0	FACT 4	Factor to adjust value of recharge rate printed.
	*46-49	4I1	LEVEL4	Layers for which maps of recharge rate are to be printed.
	*50-52	G3.0	FACT5	Factor to adjust value of ET-runoff rate printed.
	*53-56	4I1	LEVEL5	Layers for which maps of ET-runoff rate are to be printed.
	*57-59	G3.0	FACT6	Factor to adjust value of leakage rate printed.
	*60-63	4I1	LEVEL6	If map of leakage rate through the upper confining bed is desired, put a 1 in column 60.
	*64-64	G3.0	FACT7	Factor to adjust value of pumping rate printed.
	*67-70	4I1	LEVEL7	Layers for which maps of pumping rate are to be printed.
	*76-80	A5	MESUR	Name of map length unit.

<sup>†</sup> Value of drawdown or head	FACT1 or FACT2	Printed value
	0.01	1
	0.1	5
52.57	1.0	53
	10.0	526
	100.0	†††

<u>CARD</u>	<u>COLUMNS</u>	<u>FORMAT</u>	<u>VARIABLE</u>	<u>DEFINITION</u>
3	1-20	G20.10	SUM	Parameters in which elapsed time and cumulative volumes for mass balance are stored. For the start of a simulation, insert three blank cards. <u>For continuation</u> of a previous run using cards as input, replace the three blank cards with the first three cards of punched output from the previous run. Using data from disk for input, leave the three blank cards in the data deck.
	21-40	G20.10	SUMP	
	41-60	G20.10	PUMPT	
	61-80	G20.10	CFLTXT	
4	1-20	G20.10	QRET	
	21-40	G20.10	CHST	
	41-60	G20.10	CHDT	
	61-80	G20.10	FLTXT	
5	1-20	G20.10	STORT	
	21-40	G20.10	ETFLXT	
	41-60	G20.10	FLXNT	

### Group III: Array Data

Each of the following data sets (except data set 1) consists of a parameter card and, if the data set contains variable data, a set of data cards for each layer in the model. Each parameter card contains at least five variables.

<u>CARD</u>	<u>COLUMNS</u>	<u>FORMAT</u>	<u>VARIABLE</u>	<u>DEFINITION</u>
Every parameter card	1-10	G10.0	FAC	If IVAR = 0, FAC is the value assigned to every element of the matrix for this layer.
	11-20	G10.0	IVAR	= 0 if no data cards are to be read for this layer. = 1 if data cards for this layer follow.
	21-30	G10.0	IPRN	= 0 if input data for this layer are to be printed. = 1 if input data for the layer are <u>not</u> to be printed.
Transmissivity parameter cards also have these variables	31-40	G10.0	FACT(K,1)	Multiplication factor for transmissivity in x direction.
	41-50	G10.0	FACT(K,2)	Multiplication factor for transmissivity in the y direction.
	51-60	G10.0	FACT(K,3)	Multiplication factor for hydraulic conductivity in the z direction. (Not used when confining bed nodes are eliminated and TK values are read.)

When data cards are included, start each row on a new card. To prepare a set of data cards for an array that is a function of space, the general procedure is to overlay the finite-difference grid on a contoured map of the parameter and record the average value of the parameter for each finite-difference block on coding forms according to the appropriate format. In general, record only significant digits and no decimal points (except for data set 2); use the multiplication factor to convert the data to their appropriate values. For example, if DELX ranges from 1000 to 15000 feet, coded values should range from 1-15; the multiplication factor (FAC) would be 1000.

<u>DATA SET</u>	<u>COLUMNS</u>	<u>FORMAT</u>	<u>VARIABLE</u>	<u>DEFINITION</u>
1	1-80	8F 10.4	PHI(I,J,K)	Head values for continuation of a previous run (L).

Note: For a new simulation, this data set is omitted. Do not include a parameter card with this data set.

2	1-80	8F 10.4	STRT(I,J,K)	Starting head matrix (L).
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Note: Code in HCF potentiometric head in nodes just outside model-grid boundary.

3	1-80	20F 4.0	S(I,J,K)	Storage coefficient (dimensionless).
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Note: This matrix is also used to locate constant head boundaries by coding a negative number at constant head nodes. At these nodes, T must be greater than zero.

4	1-80	20F 4.0	T(I,J,K)	Transmissivity ( $L^2/T$ ).
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Note (1): Zero values are required around the perimeter of the T matrix for each layer for reasons inherent in the computation scheme. This is done automatically by the program.

Note (2): See the previous page for the additional requirements on the parameter cards for this data set.

Note (3): If the upper active layer is unconfined and PERM and BOTTOM are to be read for this layer, insert a parameter card for this layer with only the values for FACT on it.

5	1-80	20F 4.0	TK(I,J,K)	Leakance coefficient $[(L/T)/L]$ .
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Note: This data set is read only if specified in the options. The number of layers of TK values =  $K' - 1$ .

6	1-80	20F 4.0	PERM(I,J)	Hydraulic conductivity (L/T) (see note 1 for data set 4).
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7	1-80	20F 4.0	BOTTOM(I,J)	Altitude of bottom of water-table unit (L).
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Note: Data sets 6 and 7 are required only for simulating unconfined conditions in the upper hydrologic unit.

<u>DATA SET</u>	<u>COLUMNS</u>	<u>FORMAT</u>	<u>VARIABLE</u>	<u>DEFINITION</u>
8	1-80	20F 4.0	QRE(I,J)	Recharge rate (L/T).
9	*1-80	20F 4.0	CSS(I,J,1)	HCF condition leakage factor for layer 1 [(L/T)/L].
10	*1-80	20F 4.0	CSS(I,J,2)	Maximum ET-runoff capture rate divided by maximum ET-runoff capture depth for layer 2 [(L/T)/L].
11	*1-80	20F 4.0	HSS(I,J,1)	HCF condition head factor for layer 1 (L).
12	*1-80	20F 4.0	HSS(I,J,2)	Altitude of the bottom of the ET-runoff capture zone for layer 2 (L).
13	*1-80	20F 4.0	HB(I,J,1)	Blank card.
14	*1-80	20F 4.0	HB(I,J,2)	Altitude of land surface (L).
15	1-80	8G10.0	DELX(J)	Grid spacing in x direction (L).
16	1-80	8G10.0	DELY(I)	Grid spacing in y direction (L).
17	1-80	8G10.0	DELZ(K)	Grid spacing in z direction (L).

Group IV: Parameters That Change with the Pumping Period

The program has two options for the simulation period:

1. To simulate a given number of time steps, set TMAX to a value larger than the expected simulation period. The program will use NUMT, CDLT, and DELT as coded.
2. To simulate a given pumping period, set NUMT larger than the number required for the simulation period (for example, 100). The program will compute the exact DELT (which will be  $\leq$  DELT coded) and NUMT to arrive exactly at TMAX on the last time step.

<u>CARD</u>	<u>COLUMNS</u>	<u>FORMAT</u>	<u>VARIABLE</u>	<u>DEFINITION</u>
1	1-10	G10.0	<u>KP</u>	Number of the pumping period.
	11-20	G10.0	<u>KPM1</u>	Number of the previous pumping period.
<u>Note:</u> KPM1 is currently not used.				
	21-30	G10.0	<u>NWEL</u>	Number of wells for this pumping period.
	31-40	G10.0	<u>TMAX</u>	Number of days in this pumping period.
	41-50	G10.0	<u>NUMT</u>	Number of time steps.
	51-60	G10.0	<u>CDLT</u>	Multiplying factor for DELT.

Note: 1.5 is commonly used.

	61-70	G10.0	<u>DELT</u>	Initial time step in hours.
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If NWEL = 0, the following set of cards is omitted.

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<u>DATA SET</u>	<u>COLUMNS</u>	<u>FORMAT</u>	<u>VARIABLE</u>	<u>DEFINITION</u>
1 (NWEL cards)	1-10	G10.0	K	Layer in which well is located.
	11-20	G10.0	I	Row location of well.
	21-30	G10.0	J	Column location of well.
	31-40	G10.0	WELL(I,J,K)	Pumping rate ( $L^3/T$ ), negative for a pumping well.

Note: Radius is required only for those wells, if any, where computation of drawdown at a real well radius is to be made.



## ATTACHMENT C: SAMPLE INPUT DATA DECK FOR WELL-FIELD PUMPAGE FIELD PROBLEM

The sample input data deck contains 1,097 cards. Each card is keyed to the data-deck instructions (Attachment B) by group number, card number, and variable name.

There are four groups of cards in the data deck:

- Group I. This group contains data that dimensions the model into a 34 x 36 array and provides several job-control options, including the HCF condition.
- Group II. This group contains scalar parameters for mapping computed drawdowns, head, head difference, recharge, ET-runoff, leakage, and pumpage. It also provides tolerances for computational errors.
- Group III. This group contains the data matrices, 17 of which comprise the input parameters to this model. To reduce programming time and the number of layers, a "Leakance coefficient" array replaces transmissivity, storage, and head arrays that would be necessary to represent the confining bed.
- Group IV. This group controls the distribution of pumpage over the model area. The model computes the response of the hydrologic system that will result from imposing pumpage upon the system.

Normally, Groups I, II, and III remain unchanged from the calibrated model. To determine the effects of pumping stresses on the system, Group IV is the only group in which cards are changed.

ATTACHMENT C: SAMPLE INPUT DATA DECK FOR WELL-FIELD PUMPAGE PROBLEM

[illegible]

65.3889	66.1097	66.4003	66.0509	66.3178	66.4870	67.4766	69.0654
70.0857	71.3811	73.9264	74.0000	C.0	0.C	2.5557	4.8134
0.0	0.0	C.0	0.0	20.6859	24.0865	27.7298	31.5630
7.7750	11.2127	14.2871	17.3861	55.1591	60.8870	65.6291	68.8377
35.7838	40.3314	45.1505	50.1103	72.0766	71.9769	72.4435	74.1190
70.5279	71.3339	71.8012	71.9205	C.0	2.5827	4.6102	7.1519
75.1637	76.4040	78.0899	75.8000	22.9037	28.3436	30.0541	33.8251
0.0	0.0	C.0	0.0	58.3872	64.3256	69.0997	71.8125
10.1361	13.2865	16.4124	19.5336	74.8781	75.3546	76.1022	77.9158
37.9872	42.6379	47.4972	52.3718	3.6872	4.8973	6.9596	9.7233
73.3122	74.0085	74.2927	74.5206	25.0167	28.5230	32.0720	35.7332
78.7738	79.6585	80.2615	75.0000	60.0450	66.4617	70.7888	73.3145
0.0	0.0	C.0	1.9441	76.0714	76.9664	78.2486	79.9546
12.1639	15.0950	18.3969	21.6527	4.8743	6.7765	9.0755	11.8496
39.9276	44.7221	49.5566	54.3916	27.3327	30.8145	34.3141	37.8484
74.8631	75.2994	75.1981	75.4517	61.2529	67.4320	71.5824	73.7598
81.1722	82.1909	82.7493	77.5000	76.2044	77.6481	79.6546	81.6916
0.0	0.0	1.6323	3.0819	5.7267	7.8054	10.1524	12.9281
14.3052	17.2527	20.5901	23.9757	29.3484	32.8547	36.3993	39.8185
41.9003	46.4999	51.2278	55.8862	61.9684	67.5879	71.5695	73.4260
74.8790	75.1523	75.0149	75.3288	75.6548	77.8235	80.5319	82.8628
83.1144	84.5128	85.4554	80.3000	C.0	7.8054	10.1524	12.9281
0.0	1.1564	2.4004	4.0420	5.7267	7.8054	10.1524	12.9281
15.7748	18.9597	22.4797	25.9325	29.3484	32.8547	36.3993	39.8185
43.6651	48.0487	52.5745	56.9975	61.9684	67.5879	71.5695	73.4260
73.8929	73.8105	73.5720	74.1947	75.6548	77.8235	80.5319	82.8628
84.6314	86.5430	88.7704	86.5000	6.4049	8.6287	11.3004	14.5605
0.0	1.4301	2.8640	4.5383	31.5300	34.7653	38.0631	41.4985
17.7991	21.2513	24.7413	28.2441	62.2938	67.4630	70.9053	72.5006
45.2933	49.2721	53.5667	57.8802	74.4570	77.3626	80.7836	83.6120
72.1313	71.1739	70.3554	72.0923	7.0757	9.4541	12.5971	16.1907
85.7177	88.0883	90.9251	89.0000	33.7278	36.9147	40.0613	43.3298
0.0	1.7967	3.4496	5.1682	62.5066	67.2659	69.9738	70.8086
19.5794	23.2147	26.8082	30.3697	73.3497	76.8754	80.8070	84.0103
48.0776	51.3359	54.9790	58.3335	7.9769	10.5366	14.1477	17.7363
69.3826	66.7925	67.6972	70.3176	35.5277	38.8042	41.9912	45.0474
86.4018	89.0173	92.0722	90.5000	62.5221	66.7818	68.7703	68.7703
0.0	2.3959	4.2732	6.0999	72.5397	76.3468	80.6537	84.1681
21.1476	24.7928	28.5808	32.1501	8.7149	11.3319	15.1192	18.7702
48.0776	51.3359	54.9790	58.3335	37.0320	40.4923	43.6470	46.5109
66.2470	63.8277	65.3437	68.9543	62.6125	66.3035	67.3421	65.9309
86.7255	89.3011	92.2470	92.0000	71.9595	76.0635	80.7772	84.2553
1.0000	3.3474	5.2280	6.9996	9.5439	12.3367	15.7692	19.3570
22.5025	26.3698	30.1967	33.6645	38.1304	41.6950	44.9027	47.7648
49.3430	52.3888	55.5338	58.8713	62.3509	65.2819	65.4978	63.0638
61.9696	61.3125	63.5638	67.7948	71.2920	75.5911	80.0909	83.2971
86.6817	88.9529	91.2218	90.0000	10.3066	12.8261	15.9067	19.4658
1.5000	4.4723	6.3456	7.9328	38.8583	42.4636	45.7300	48.6122
23.1599	27.2418	31.2307	34.7536	62.1804	64.1555	63.6123	60.9697
50.5078	53.3550	56.1010	58.9833	69.9499	74.4827	78.3553	81.1262
59.3387	59.2531	62.3356	66.8163	10.9924	12.9349	15.8661	19.3932
85.5626	87.6146	89.6818	89.2000	39.1425	42.8549	46.0420	48.9279
2.0000	5.9974	8.0049	9.0919	61.7640	62.7251	61.5754	58.8916
23.3962	27.5948	31.6445	35.3528	10.9924	12.9349	15.8661	19.3932
51.2216	53.9217	56.4323	59.0335	39.1425	42.8549	46.0420	48.9279
57.0294	57.2354	60.8200	65.2545	61.7640	62.7251	61.5754	58.8916
83.1478	84.9136	86.5944	86.0000	10.9924	12.9349	15.8661	19.3932
2.5000	8.0564	9.7001	10.4646	39.1425	42.8549	46.0420	48.9279
23.2870	27.4438	31.5320	35.3614	61.7640	62.7251	61.5754	58.8916
51.5353	54.1277	56.5985	59.1753	10.9924	12.9349	15.8661	19.3932

54.9726	55.2165	58.6779	63.0708	67.4206	71.9373	75.5749	78.0C12
79.6586	81.0709	82.2669	80.5000				
4.0000	9.6760	10.8272	11.2941	11.5929	13.0C15	15.5036	13.7848
22.6332	26.6332	30.6794	34.6157	38.4657	42.2623	45.600C	43.5306
51.0536	53.7034	56.4262	59.2748	61.1561	60.9479	59.1491	56.1376
52.5955	53.3577	56.3623	60.5077	64.6538	68.7562	72.0754	74.2815
75.5736	76.4999	77.1584	73.5000				
5.0000	10.8414	11.6713	11.6755	11.6391	12.7748	14.8718	17.7637
21.1857	25.1638	29.1575	33.2124	37.1181	40.8705	44.4375	47.5082
50.1614	52.7017	55.4981	58.0191	59.2904	58.4716	55.8125	52.5229
50.2308	50.9741	53.7806	57.5772	61.4287	65.1169	68.1755	70.0578
71.0143	71.4960	71.1856	65.0000				
7.0000	11.9294	12.2326	11.7591	11.1475	11.8994	13.6551	16.1925
19.3730	23.1546	27.0973	31.1211	34.9861	38.9C45	42.7333	45.8666
48.5748	50.9663	53.5684	55.5313	56.6419	54.7040	50.6153	47.6638
47.1288	48.5370	50.9995	54.5305	57.8672	61.0943	63.7837	65.4642
66.2429	66.6277	66.5074	63.0C00				
7.5000	12.2216	11.8343	10.6981	9.6796	10.2344	11.7348	14.0C41
17.0000	20.5671	24.4179	28.5870	32.360C	36.3556	40.1933	43.6748
46.7439	48.8514	50.9306	52.3473	52.1317	49.0551	44.3322	43.0387
43.5699	45.3549	47.8579	51.1650	54.1293	56.7614	59.1351	60.7369
61.4994	61.7693	61.6012	57.5000				
8.0000	11.8839	1C.5608	8.9883	7.6679	7.9C90	9.2711	11.3105
14.1752	17.5666	21.3610	25.4442	29.2532	33.1167	36.6788	40.1698
43.5430	45.5091	46.9909	47.3298	45.7842	40.8890	38.8579	38.9184
39.8614	41.5817	44.2157	47.1230	49.8392	52.0501	54.158C	55.8781
56.8567	57.4372	57.7085	55.5000				
0.0	6.0000	8.2909	6.4593	5.2286	5.3144	6.2788	8.1109
10.9565	14.2341	17.8909	21.8743	25.7218	29.2720	32.5358	35.2439
38.2188	39.7565	4C.8019	40.1783	37.9302	34.5776	34.112C	34.5800
35.7287	37.5394	39.9540	42.3083	44.5827	46.8554	48.8105	51.0197
52.6170	53.7311	53.7610	52.1000				
0.0	0.0	5.0000	2.0C00	2.2456	2.4136	2.9835	4.3507
7.5242	10.5532	14.1746	17.9897	21.7928	25.1241	27.7567	29.4954
31.1606	32.0668	32.7306	31.7183	3C.2729	29.1524	29.2718	30.0176
31.1135	32.8903	34.9796	36.8503	39.0538	41.5374	44.1885	46.8193
49.2900	50.9118	5C.6144	47.5000				
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
4.2993	6.2350	9.9602	13.5665	17.4383	20.7176	22.6537	23.5155
23.8846	24.5619	25.4348	25.0404	25.2059	25.0431	25.4198	26.3151
27.5685	29.2432	31.3800	33.4582	35.4355	37.6346	40.9493	44.8397
48.3084	50.5981	50.7613	48.0C00				
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
0.0	1.0000	6.0000	8.0C00	13.000C	15.9794	16.9697	17.5723
18.0222	19.3976	21.2247	22.1963	22.6975	23.1244	23.7186	24.5720
25.8555	27.4882	29.4820	31.8385	34.2409	36.8982	40.0643	44.7213
49.4165	52.7124	53.9960	52.5000				
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
9.5000	13.6765	17.2694	19.9C21	20.9017	21.6626	22.4127	23.2926
24.6871	26.4678	28.4933	30.7339	33.2709	36.2440	40.7141	46.5739
52.0326	56.4692	59.8533	62.0C00				
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
0.0	3.0000	14.1343	17.0848	18.6491	19.6669	20.5698	21.5358
22.9661	24.7715	26.9898	29.8252	32.4673	36.5822	42.5592	48.9162
55.0751	60.4073	65.6766	71.0000				
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
0.0	0.0	6.0000	15.3296	16.1875	17.3397	18.2274	18.9791

[illegible]

11.0772	15.2475	17.3957	18.0419	23.1397	25.0399	32.9313	38.3578
39.6891	42.7015	48.5427	53.5405	61.8924	67.3593	73.8259	75.1159
75.8169	76.1367	76.2636	76.3690	76.5384	76.7622	78.2866	100.9196
127.9570	121.3330	119.0000	0.0	0.0	2.5473	4.3231	14.9181
0.0	0.0	0.0	19.5407	22.6807	27.4757	30.9758	35.9366
8.0777	10.2256	17.2979	57.6099	62.1395	70.4654	74.1055	75.8155
39.7753	45.9502	51.3443	76.8050	77.6162	78.0352	79.5466	93.6315
78.1191	77.7993	76.6883	0.0	6.3950	9.8446	12.8897	21.0934
95.4395	99.9168	114.0000	1.9641	28.5202	32.3926	36.2810	40.1527
0.0	0.0	0.0	26.6471	63.1826	70.9287	75.0006	76.0194
15.5199	17.6650	22.3173	58.7627	76.7679	77.0915	79.0958	85.4853
43.0018	46.9638	53.0455	77.1715	4.2415	7.3800	6.7717	8.6997
77.0741	78.2537	76.6059	0.0	27.6149	33.4723	41.1981	42.8617
89.0822	102.8029	109.0000	6.8868	63.6865	69.9473	74.9842	75.8623
0.0	0.0	1.7704	23.7825	75.0617	78.7748	85.0623	85.0424
9.7349	12.7401	20.4164	59.1065	4.0395	5.5850	6.3136	14.7228
43.2811	48.0885	54.0298	74.7307	31.7621	32.7276	34.8707	38.7257
75.5453	75.5059	75.3678	0.0	65.9689	70.9470	73.5057	75.8597
91.2085	95.2055	101.0000	68.1748	68.2436	72.5505	76.5290	87.7038
0.0	1.0000	0.0	2.3343	0.0	0.0	0.0	0.0
18.0041	23.2143	25.9731	30.9695	2.3328	2.5433	7.8312	17.8221
45.1297	47.2730	54.3871	62.1527	37.5779	38.9450	39.6976	44.4196
74.0684	72.6330	64.5998	0.0	66.9616	71.3125	72.6118	74.0148
92.7591	95.7737	99.0000	66.9442	69.4590	73.2883	75.3831	81.1104
0.0	0.0	1.3951	1.8491	0.0	0.0	0.0	0.0
19.3860	25.6215	28.6480	34.0806	4.4628	5.3679	17.5390	20.3257
46.5809	48.5776	53.3420	61.5032	38.2118	40.7714	44.7309	47.9185
71.2903	61.3368	63.3110	66.9442	66.1700	70.6157	71.4920	72.8586
95.1288	101.0999	97.0000	0.0	70.3185	72.6816	76.8829	90.1942
0.0	0.0	3.7279	5.9750	2.0000	2.0000	17.6980	20.1455
21.0694	24.2613	29.9903	34.7301	39.1502	45.8427	48.3184	48.1452
47.6835	48.8911	55.6016	61.3066	65.6063	70.2334	70.8669	69.9148
68.2101	57.6443	60.0931	68.2215	72.6872	73.9945	92.1479	104.2982
102.6255	101.4106	99.0000	0.0	0.0	0.0	0.0	0.0
0.0	2.0000	4.8646	7.1538	2.0000	2.0000	17.6980	20.1455
24.6160	29.1710	33.7335	36.6269	39.1502	45.8427	48.3184	48.1452
48.1707	52.8491	57.9595	62.4641	65.6063	70.2334	70.8669	69.9148
56.8144	54.5907	58.5827	67.3251	72.6872	73.9945	92.1479	104.2982
107.1154	105.4239	114.0000	0.0	2.0000	11.4947	17.7845	20.3438
0.0	2.0000	4.6162	6.2434	39.3650	47.0454	50.1395	51.3188
23.6950	29.5411	35.3119	38.3391	66.1127	68.7498	70.0333	66.4902
52.8537	56.7417	57.5215	63.1860	75.4799	77.7324	95.0716	105.4321
52.7388	52.9372	60.1495	69.2286	2.0000	11.6051	13.8742	18.1818
110.6195	109.8898	120.0000	0.0	41.1836	47.3091	51.8131	54.0546
0.0	9.0000	13.9875	6.7844	65.2140	67.7809	69.1441	63.4920
23.8306	30.0494	34.9046	39.8255	76.5901	84.7986	92.4993	97.6395
53.3149	57.6443	57.5492	61.7780	2.0000	6.5333	14.0124	20.5363
48.3741	50.5894	59.9044	67.0294	46.1074	53.0624	53.1795	55.6342
105.1340	105.6854	109.0000	0.0	65.0623	66.5781	65.8822	62.6523
0.0	49.0000	58.6524	36.8355	67.7674	79.3900	88.4607	90.3374
25.8432	30.1794	35.2852	39.8864	2.0000	6.5333	14.0124	20.5363
57.0158	58.2528	58.4167	61.7557	46.1074	53.0624	53.1795	55.6342
47.4110	48.5706	55.5706	65.1503	65.0623	66.5781	65.8822	62.6523
93.6200	96.6567	104.0000	0.0	67.7674	79.3900	88.4607	90.3374
0.0	49.0000	43.5350	32.1407	7.8344	7.5904	12.0547	18.9016
22.1606	31.0540	35.0718	39.6477	43.3652	50.7723	53.2273	55.5158
53.8275	58.8237	62.4070	62.8723	65.3318	65.2307	63.3264	58.8755
44.1018	46.3154	52.0600	62.3110	67.1225	74.8634	81.4136	88.4521
89.5225	88.1304	101.0000	0.0	7.3482	10.6599	13.4512	17.9003
0.0	49.0000	49.1606	13.9639	7.3482	10.6599	13.4512	17.9003







[illegible]

[illegible]

[illegible]

[illegible]

[illegible]



[illegible]

[illegible]









1	8	26	-8.22
1	8	27	-5.48
1	8	28	-2.74
1	9	25	-2.74
1	9	26	-5.48
1	10	26	-5.48
1	12	13	-4.96
1	12	14	-2.48
1	13	13	-4.96
1	15	28	-4.65
1	15	29	-13.95
1	15	30	-4.65
1	16	28	-13.95
1	16	27	-4.65
1	16	29	-4.65
1	17	10	-5.64
1	17	11	-1.88
1	18	9	-8.46
1	18	10	-13.16
1	18	11	-9.40
1	18	12	-1.88
1	19	9	-5.64
1	19	10	-8.46
1	19	18	-3.27
1	19	19	-6.54
1	20	18	-9.81
1	20	19	-6.54
1	21	7	-2.90
1	22	7	-1.74
1	21	14	-5.12
1	22	14	-3.84
1	23	12	-6.40
1	23	13	-6.40
1	23	14	-2.56
1	24	12	-5.12
1	23	17	-7.98
1	24	17	-11.97
1	24	18	-7.98
1	27	26	-2.80
1	27	27	-1.40
1	28	26	-2.80
1	28	27	-4.20
1	28	28	-4.20
1	29	26	-1.40
1	29	27	-4.20
1	29	28	-4.20
1	29	29	-2.80
1	26	15	-3.41
1	26	16	-5.11
1	26	17	-5.11

#### ATTACHMENT D: SAMPLE MODEL OUTPUT FOR WELL-FIELD PUMPAGE FIELD PROBLEM

The sample model printout lists only the pumpage data because it is assumed that all other input arrays will be unchanged for predictive runs. Also listed is a mass balance for the system with helpful statistics. The distributions of drawdown, hydraulic head, head difference, recharge, evapotranspiration, leakage, and pumpage comprise a series of useful maps. In addition to the printout, the model punches cards containing drawdown and hydraulic head.

## NORTH TAMPA WELL-FIELD AREAS QUASI 3-DIMENSIONAL MODEL (FL-33200)

PUMP ALL WELL FIELDS AT PERMITTED AVG, RECHARGE AVG

NUMBER OF ROWS = 34  
 NUMBER OF COLUMNS = 36  
 NUMBER OF LAYERS = 2  
 MAXIMUM PERMITTED NUMBER OF ITERATIONS = 100  
 NUMBER OF CONSTANT HEAD NODES = 118

SIMULATION OPTIONS: DRAW HEAD MASS WATE RECH PUN2 ITKR

WORDS OF VECTOR Y USED = 50872

NUMBER OF PUMPING PERIODS = 1  
 TIME STEPS BETWEEN PRINTOUTS = 1

ERROR CRITERIA FOR CLOSURE = .1000000E-01

BETA= 1.00

## ON ALPHAMERIC MAP:

MULTIPLICATION FACTOR FOR X DIMENSION = 5280.000  
 MULTIPLICATION FACTOR FOR Y DIMENSION = 5280.000  
 MAP SCALE IN UNITS OF MILES  
 NUMBER OF MILES PER INCH = 2.000000  
 MULTIPLICATION FACTOR FOR DRAWDOWN = 1.000000  
 MULTIPLICATION FACTOR FOR HEAD = 1.000000  
 MULT FACTOR FOR HEAD DIFFERENC = 1.000000  
 MULTIPLICATION FACTOR FOR RECH = 1.000000  
 MULTIPLICATION FACT FOR ET-RUNOFF = 1.000000  
 MULTIPLICATION FACTOR FOR LEAKAGE = 1.000000  
 MULTIPLICATION FACTOR FOR PUMPAGE = -10.000000

PRINTED FOR LAYERS 1 2 0 0  
 PRINTED FOR LAYERS 1 2 0 0  
 PRINTED FOR LAYERS 1 0 0 0  
 PRINTED FOR LAYERS 2 0 0 0  
 PRINTED FOR LAYERS 2 0 0 0  
 PRINTED FOR LAYERS 1 0 0 0  
 PRINTED FOR LAYERS 1 0 0 0

STORAGE COEFFICIENT = .0 FOR LAYER 1

DIRECTIONAL TRANSMISSIVITY MULTIPLICATION FACTORS FOR LAYER 1

X = 1.000000  
 Y = 1.000000  
 Z = .0

TRANSMISSIVITY = .0 FOR LAYER 2

DIRECTIONAL TRANSMISSIVITY MULTIPLICATION FACTORS FOR LAYER 2

X = 1.000000  
 Y = 1.000000  
 Z = .0

RECHARGE RATE = .0 FOR LAYER 1

ET-RUNOFF/DEPTH = .1020000E-07 FOR LAYER 2

LAND SURFACE = .0 FOR LAYER 1

DELX = 5280.000  
 DELY = 5280.000  
 DELZ = 1.000000

## SOLUTION BY THE STRONGLY IMPLICIT PROCEDURE

PUMPING PERIOD NO. 1: 1.00 DAYS

NUMBER OF TIME STEPS= 1

DELT IN HOURS = 24.000

MULTIPLIER FOR DELT = 1.000

53 WELLS

AQ	ROW	COL	CFS	MGD
1	7	27	-5.48	-3.54
1	7	28	-8.22	-5.31
1	8	25	-2.74	-1.77
1	8	26	-8.22	-5.31
1	8	27	-5.48	-3.54
1	8	28	-2.74	-1.77
1	9	25	-2.74	-1.77
1	9	26	-5.48	-3.54
1	10	26	-5.48	-3.54
1	12	13	-4.96	-3.21
1	12	14	-2.48	-1.60
1	13	13	-4.96	-3.21
1	15	28	-4.65	-3.01
1	15	29	-13.95	-9.02
1	15	30	-4.65	-3.01
1	16	28	-13.95	-9.02
1	16	27	-4.65	-3.01
1	16	29	-4.65	-3.01
1	17	10	-5.64	-3.65
1	17	11	-1.88	-1.22
1	18	9	-8.46	-5.47
1	18	10	-13.16	-8.51
1	18	11	-9.40	-6.08
1	18	12	-1.88	-1.22
1	19	9	-5.64	-3.65
1	19	10	-8.46	-5.47
1	19	18	-3.27	-2.11
1	19	19	-6.54	-4.23
1	20	18	-9.81	-6.34
1	20	19	-6.54	-4.23
1	21	7	-2.90	-1.87
1	22	7	-1.74	-1.12
1	22	14	-5.12	-3.31
1	22	14	-3.84	-2.48
1	23	12	-6.40	-4.14
1	23	13	-6.40	-4.14
1	23	14	-2.56	-1.65
1	24	12	-5.12	-3.31
1	23	17	-7.98	-5.16

1	24	17	-11.97	-7.74
1	24	18	-7.98	-5.16
1	27	26	-2.80	-1.81
1	27	27	-1.40	-0.90
1	28	26	-2.80	-1.81
1	28	27	-4.20	-2.71
1	28	28	-4.20	-2.71
1	29	26	-1.40	-0.90
1	29	27	-4.20	-2.71
1	29	28	-4.20	-2.71
1	29	29	-2.80	-1.81
1	26	15	-3.41	-2.20
1	26	16	-5.11	-3.30
1	26	17	-5.11	-3.30



-----  
TIME STEP NUMBER = 1

SIZE OF TIME STEP IN SECONDS= 86400.00

TOTAL SIMULATION TIME IN SECONDS= 86400.00  
MINUTES= 1440.00  
HOURS= 24.00  
DAYS= 1.00  
YEARS= 0.00

DURATION OF CURRENT PUMPING PERIOD IN DAYS= 1.00  
YEARS= 0.00

CUMULATIVE MASS BALANCE:-----  
L\*\*3  
RATES FOR THIS TIME STEP:-----  
L\*\*3/T  
SOURCES:-----  
STORAGE = 0.0  
RECHARGE = 1506.3989  
CONSTANT FLUX = 0.0  
CONSTANT PUMPING = -289.7986  
ET-RUNOFF = -973.0085  
CONSTANT HEAD: 83.0054  
IN = -12.5743  
OUT = 0.0  
LEAKAGE: -314.0344  
FROM PREVIOUS PUMPING PERIOD = 0.0  
TOTAL = -0.0117  
SUM OF RATES =  
DISCHARGES:-----  
ET-RUNOFF = 84067936.0  
CONSTANT HEAD = 1086423.00  
QUANTITY PUMPED = 25038592.0  
LEAKAGE = 34199408.0  
TOTAL DISCHARGE = 144392352.  
DISCHARGE-SOURCES = 976.00  
PER CENT DIFFERENCE = 0.00

LAYER	1	AVG	ABS CHG	MAX DDN	MAX RISE	AVG T (GPD/FT)	MAX T	MIN T	ACTIVE NODES
LAYER	2	3.6	11.4%	23.2 (15,29)	0.0 ( 0, 0)	624853.	3554809.	193899.	932.
		1.3	3.8%	14.9 ( 8,26)	0.0 ( 0, 0)	1407.	2958.	226.	814.
LAYER	2	AVG RECHG (IN/YR)	AVG POS RECHG	NO. POS	NOES	AVG NEG RECHG	NO.NEG NODES	MAX RECHG	MAX DISCHG
		25.1	25.1	814.		0.0	0.	30.8	0.0

TIME STEP : 1  
-----

ITERATIONS: 38  
-----

**DRAWDOWN IN FLORIDIAN AQUIFER, FEET**

[illegible]

0	0	1	1	1	2	3	3	4	6	9	15	21	18	11	8	7	7	6	4	3	3	2	1	8.00
0	1	1	1	2	2	3	4	5	8	11	17	15	10	8	6	5	4	3	3	2	2	1		
0	1	1	2	2	3	4	6	8	10	9	8	7	6	4	4	3	2	2	1					
1	1	1	2	2	3	4	5	6	6	6	5	4	3	2	2	1	1						5.33	
1	1	1	2	2	3	4	4	4	4	4	4	3	2	2	1	1								
1	1	1	2	2	3	3	3	3	3	3	3	2	2	2	1	1								
1	1	1	1	1	1	1	2	2	2	2	2	2	2	2	1	1							2.67	
1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1								
0	0	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	0.0	
0.0	2.67	5.33	8.00	10.67	13.33	16.00	18.67	21.33	24.00	26.67	29.33	32.00												

DISTANCE FROM ORIGIN IN Y DIRECTION, IN MILES

EXPLANATION  
-----

R = CONSTANT HEAD BOUNDARY  
\*\*\* = VALUE EXCEEDED 3 FIGURES  
MULTIPLICATION FACTOR = 1.000

**34.67**

32.00

**29.33**

**26-67**

24.00

21.33

18-67  
X DIS-

MILES 16.00

### 13.33

10-67

134



ALTITUDE OF POTENTIOMETRIC SURFACE OF THE FLORIDAN AQUIFER, FEET

34.67																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																									</
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3	5	7	9	10	11	12	12	12	10	6	8	13	16	16	15	14	11	9	6	3				
2	4	6	8	9	10	10	10	10	9	10	12	13	13	13	13	11	9	6	3					
2	4	6	7	8	8	8	9	9	10	10	9	10	11	11	9	7	5	2						5.33
2	4	5	6	6	7	7	7	8	9	9	9	10	10	8	6	4	1							
3	4	4	5	5	6	6	6	7	7	8	9	9	9	8	6	4	1							
1	2	3	3	4	4	5	6	7	8	9	10	10	9	8	5									2.67
1	2	2	2	3	4	5	7	8	9	10	11	11	10	7										
1	1	1	1	2	3	4	5	7	8	10	11	11	11											0.0
0.0	2.67	5.33	8.00	10.67	13.33	16.00	18.67	21.33	24.00	26.67	29.33	32.00												

DISTANCE FROM ORIGIN IN Y DIRECTION, IN MILES

# EXPLANATION

R = CONSTANT HEAD BOUNDARY  
 \*\*\* = VALUE EXCEEDED 3 FIGURES  
 MULTIPLICATION FACTOR = 1.000

## 34.67

[illegible]

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HEAD DIFFERENCE \*WATER TABLE MINUS POTENTIOMETRIC SURFACE\* FEET

34.67	174	114	107	71	77	47	40	43	48	41	34	26	15	11	8	9	25	32	24	23	25	16	10	7	8	6-2	3	7	11	11	9	
	181	90	89	77	69	66	68	45	56	44	20	21	12	11	15	15	19	24	22	17	13	12	9	7	9	6-7	-3	5	11	10	10	
32.00	170	74	74	49	31	34	53	38	23	51	17	10	11	11	13	19	23	27	24	15	15	12	7	7	7	1-7	-5	-1	8	11	12	
	144	42	43	23	15	21	24	23	16	23	17	8	7	9	2	10	23	24	18	14	15	14	11	10	6	1-7	-8	-1	-0	3	13	
29.33	141	49	39	20	13	14	11	7	7	4	4	5	11	4	2	1	15	18	16	14	11	13	9	8	4	6-5	-7	-7	-6	-1	8	
	124	24	19	15	14	16	12	6	5	3	3	5	10	9	7	3	2	5	12	9	7	6	9	8	7	6	2	-5	-7	-7	4	6
	120	21	18	17	11	14	9	8	5	4	4	6	10	14	10	6	6	7	9	2	4	3	2	6	9	5	5	-0	-5	-7	12	5
26.67	118	18	16	12	10	9	9	9	6	4	4	7	8	9	11	7	4	5	4	3	3	1	4	7	7	7	5	1	-6	-5	20	22
	118	17	14	10	9	7	9	9	7	5	4	4	6	0	3	-1	-2	-0	1	-2	-3	-3	-2	0	4	8	8	2	-6	10	13	25
24.00	116	15	13	10	10	9	9	9	9	5	5	5	4	5	-2	-3	-5	-6	-6	-6	-7	-0	1	-1	5	6	2	-4	14	13	20	
	115	15	12	10	10	9	9	9	8	5	5	4	3	4	4	4	-4	-6	-8	-7	-8	-4	-6	-4	0	2	1	1	-2	6	13	15
	114	14	12	10	12	6	8	6	7	5	3	3	3	4	4	5	5	4	3	4	3	1	-9	-9	-2	-1	4	1	-2	0	10	11
21.33	8	12	12	10	11	6	5	5	5	6	4	4	4	3	3	4	4	5	6	5	5	4	-1	-6	-3	-3	1	-3	1	12	12	13
	5	11	7	9	10	9	4	3	3	4	4	4	3	4	5	5	5	5	5	5	5	6	6	-3	-5	-3	-2	5	15	13	8	
	7	6	4	5	7	7	5	4	1	4	3	3	3	5	6	6	6	7	6	6	6	5	7	7	7	5	6	12	10	20	21	7
18.67	12	5	6	6	5	6	4	4	5	2	4	4	3	6	5	5	8	10	9	6	7	6	5	7	7	5	9	11	13	16	10	6
X DIS- TANCE IN																																
MILES																																
16.00																																
	7	8	3	4	4	5	3	3	4	2	3	3	3	3	2	4	8	11	11	9	12	9	9	7	6	6	11	8	1	4	1	
	8	8	7	4	3	7	3	2	3	1	2	2	2	0	1	2	6	13	15	12	13	14	14	10	11	10	11	3	2	6		
	-0	8	2	1	3	8	3	3	6	3	1	3	2	2	3	4	4	9	10	14	12	17	17	16	18	15	15	8	10			
	-0	3	2	-1	3	3	2	4	7	6	2	4	5	-0	4	7	7	10	12	14	16	17	17	16	20	13	12	13	16			
13.33	-2	-2	1	2	1	1	2	1	4	5	2	5	3	-0	3	7	10	11	13	15	16	16	18	16	16	16	14	15	18			
	-1	-2	-1	0	-0	1	2	3	1	1	3	6	4	2	6	7	11	12	12	20	19	15	13	14	13	9	10	13	18			
	-1	-2	-1	-3	0	1	2	3	2	3	3	7	2	4	9	8	9	8	15	14	17	11	8	7	8	8	10					
10.67	-2	-2	-3	-1	3	0	2	1	1	1	1	7	2	7	9	10	12	11	11	11	13	16	13	12	9	6	6	9				

2.67	5.33	8.00	10.67	13.33	16.00	18.67	21.33	24.00	26.67	29.33	32.00
- 2 - 2 - 3	5	5	3 - 3	3 - 3	7 9	8 13	13 9	8 10	8 7	4 2	2 2 1
- 3 - 3 - 2	2 - 2	3 - 2	3 - 3	4 5	7 13	10 8	8 4	4 3	2 1	0 1 - 1	
- 2 - 2 - 3	6	9 - 2	3 6	7 7	7 4	6 4	3 2	1 - 1	- 2 - 0		
- 2 - 2 - 2	5 - 2	- 3 - 2	7 7	6 2	3 1	1 2	0 0	- 3 - 1			
- 2 - 2	4	1 - 1	- 5 - 3	- 5	2 2	- 3 - 3	0 0	- 1 - 5	- 3 - 0		
- 3	2 - 0	- 1 - 3	- 2 - 4	- 5 - 5	- 6 - 2	- 3 - 1	- 6 - 6	- 4 - 1			
- 1 - 1	4 - 1	- 2 1	1 - 0	- 1 28	23 4	10 12	20 9				
- 1 - 0	- 2 - 1	0 1	- 1 7	50 34	39 60	56 43	22				
- 1	0 - 1	- 2 - 1	- 2 4	42 41	39 43	58 68					

DISTANCE FROM ORIGIN IN Y DIRECTION, IN MILES

34-67

32.00

**29.33**

26.67

24-00

21.33

18-67  
X DIS-

**MILES 16.00**

**13.33**

10-67



34.67

[illegible]

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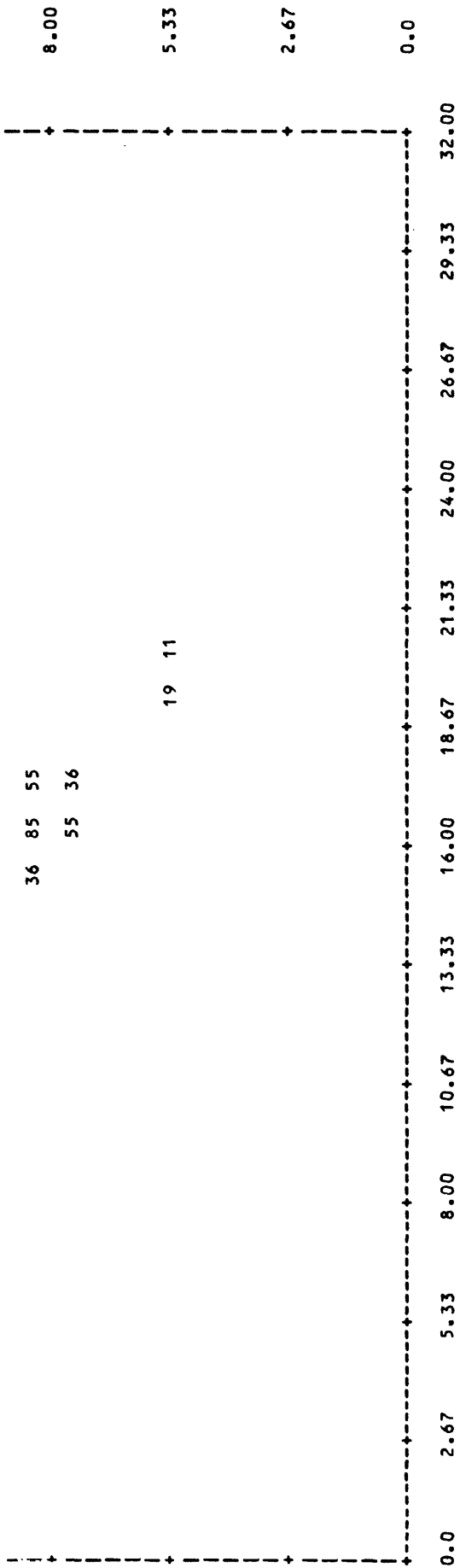
RATE OF LEAKAGE TO FLORIDAN AQUIFER, INCHES PER YEAR

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19 11



	13.83	8.53	4.72	2.38	1.49	1.28	1.40	1.76	2.16	2.51	2.62	2.52	2.33	2.11	1.86	1.58	1.28	0.0
21	0.0	1.19	1.54	2.06	2.80	4.15	6.46	6.66	7.63	8.44	9.00	9.79	10.76	12.77	10.72	10.43	11.07	11.62
	10.14	7.15	4.11	2.15	1.39	1.15	1.16	1.34	1.59	1.78	1.85	1.82	1.75	1.64	1.48	1.28	1.06	0.0
22	0.0	1.24	1.53	1.96	2.58	3.65	5.36	5.51	6.24	7.26	8.69	10.70	12.05	13.56	11.69	11.64	12.42	11.21
	8.86	5.75	3.22	1.89	1.31	1.09	1.08	1.20	1.35	1.44	1.46	1.44	1.40	1.33	1.22	1.08	0.94	0.0
23	0.0	1.20	1.43	1.73	2.20	2.90	3.78	4.39	5.24	6.57	8.88	13.47	14.50	13.39	12.14	13.29	17.07	13.09
	8.76	5.19	2.87	1.70	1.24	1.12	1.14	1.27	1.36	1.38	1.35	1.29	1.22	1.15	1.05	0.93	0.82	0.0
24	0.0	1.12	1.29	1.50	1.83	2.30	2.88	3.50	4.35	5.64	7.77	11.57	11.01	10.95	11.54	13.76	19.47	16.32
	9.14	4.93	2.63	1.61	1.30	1.30	1.42	1.58	1.64	1.59	1.46	1.34	1.18	1.06	0.95	0.83	0.73	0.0
25	0.0	1.03	1.17	1.32	1.52	1.84	2.26	2.78	3.48	4.48	5.86	7.54	8.23	9.05	10.55	12.46	13.86	11.52
	7.34	4.08	2.26	1.51	1.40	1.57	1.88	2.22	2.29	2.10	1.80	1.53	1.25	1.05	0.90	0.77	0.66	0.0
26	0.0	0.96	1.05	1.15	1.25	1.48	1.79	2.18	2.71	3.41	4.31	5.32	6.23	7.54	10.45	13.21	13.93	8.55
	5.07	2.94	1.82	1.42	1.54	1.91	2.58	3.47	3.56	3.04	2.39	1.85	1.40	1.09	0.89	0.72	0.61	0.0
27	0.0	0.0	0.95	1.00	1.05	1.20	1.42	1.70	2.06	2.53	3.11	3.78	4.50	5.41	6.60	7.45	7.07	5.07
	3.21	2.06	1.47	1.41	1.64	2.23	3.45	6.04	5.97	4.58	3.22	2.26	1.60	1.16	0.89	0.69	0.53	0.0
28	0.0	0.0	0.0	0.0	0.89	0.99	1.14	1.33	1.52	1.81	2.18	2.61	3.10	3.63	4.08	4.18	3.70	2.87
	2.05	1.50	1.30	1.36	1.63	2.24	3.50	6.33	7.71	6.79	3.91	2.57	1.75	1.28	0.93	0.70	0.50	0.0
29	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	1.06	1.20	1.42	1.69	1.99	2.34	2.51	2.42	2.09	1.75
	1.42	1.22	1.21	1.32	1.57	2.02	2.80	4.14	5.22	4.70	3.78	2.76	1.96	1.36	0.96	0.69	0.49	0.0
30	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	1.51	1.54	1.48	1.34	1.22
	1.12	1.09	1.15	1.26	1.46	1.78	2.23	2.78	3.20	3.28	2.90	2.37	1.88	1.37	0.95	0.66	0.46	0.0
31	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.82
	0.88	0.98	1.05	1.18	1.34	1.58	1.88	2.20	2.43	2.47	2.30	2.04	1.63	1.18	0.84	0.60	0.42	0.0
32	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	0.74	0.85	0.94	1.05	1.19	1.36	1.57	1.77	1.91	1.98	1.89	1.63	1.26	0.95	0.70	0.51	0.37	0.0
33	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	0.0	0.75	0.80	0.89	1.00	1.14	1.29	1.42	1.51	1.49	1.39	1.16	0.92	0.71	0.53	0.40	0.32	0.0
34	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0



	4.02	2.50	2.17	1.10	0.69	0.59	0.65	1.13	0.63	0.64	0.67	0.64	0.34	0.31	0.27	0.23	0.0	0.0
21	0.0	0.0	0.10	0.13	0.0	1.06	1.64	1.70	2.23	3.22	3.43	2.86	2.75	3.24	2.74	2.66	2.83	3.38
	2.97	3.29	1.90	0.99	0.64	0.53	0.54	0.62	0.46	0.45	0.48	0.46	0.26	0.24	0.22	0.19	0.0	0.0
22	0.0	0.0	0.10	0.12	0.66	0.94	1.37	1.41	1.60	2.12	2.54	2.74	3.09	3.45	2.99	2.97	3.19	3.27
	2.59	2.65	1.49	0.87	0.61	0.50	0.50	0.35	0.35	0.37	0.37	0.37	0.21	0.19	0.18	0.16	0.0	0.0
23	0.0	0.0	0.09	0.44	0.56	0.74	0.97	1.12	1.34	1.68	2.28	3.42	3.68	3.42	3.11	3.40	4.96	3.82
	4.03	2.40	1.33	0.79	0.57	0.51	0.53	0.37	0.35	0.35	0.34	0.33	0.18	0.17	0.15	0.14	0.0	0.0
24	0.0	0.0	0.08	0.38	0.47	0.59	0.74	0.90	1.11	1.44	1.99	2.94	2.82	2.80	2.94	3.52	5.67	4.75
	4.20	2.28	1.21	0.74	0.38	0.34	0.37	0.40	0.42	0.40	0.37	0.20	0.17	0.16	0.14	0.12	0.0	0.0
25	0.0	0.0	0.07	0.08	0.39	0.47	0.58	0.71	0.89	1.14	1.51	1.92	2.11	2.31	2.69	3.19	4.03	3.37
	3.38	1.89	1.05	0.69	0.41	0.40	0.48	0.57	0.59	0.54	0.46	0.22	0.18	0.15	0.13	0.11	0.0	0.0
26	0.0	0.0	0.07	0.07	0.0	0.38	0.46	0.56	0.69	0.87	1.10	1.36	1.60	1.93	2.67	3.36	4.04	2.50
	2.34	1.36	0.84	0.66	0.45	0.49	0.66	0.89	0.91	0.78	0.60	0.27	0.21	0.16	0.13	0.11	0.0	0.0
27	0.0	0.0	0.0	0.0	0.0	0.31	0.36	0.43	0.53	0.65	0.80	0.97	1.15	1.39	1.68	1.92	2.06	1.49
	1.48	0.95	0.68	0.42	0.42	0.57	0.88	1.54	1.52	1.17	0.48	0.33	0.23	0.29	0.23	0.18	0.0	0.0
28	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.39	0.46	0.55	0.67	0.79	0.93	1.04	1.07	0.95	0.73
	0.60	0.44	0.38	0.35	0.42	0.57	0.90	1.61	1.96	1.73	0.99	0.38	0.45	0.52	0.38	0.18	0.0	0.0
29	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.59	0.64	0.62	0.54	0.45
	0.36	0.31	0.31	0.34	0.40	0.52	0.72	1.06	1.33	1.20	0.96	0.71	0.80	0.55	0.39	0.18	0.0	0.0
30	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.31
	0.29	0.28	1.09	0.33	0.59	0.73	0.91	1.14	1.31	1.34	1.18	0.97	0.77	0.35	0.24	0.17	0.0	0.0
31	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	0.23	0.39	0.28	1.07	0.35	0.41	0.48	0.56	0.61	1.01	0.94	0.83	0.42	0.30	0.21	0.15	0.0	0.0
32	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	0.0	0.22	0.86	1.03	1.11	0.35	0.80	1.44	0.57	0.68	0.76	0.42	0.32	0.24	0.18	0.13	0.0	0.0
33	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
34	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0





42.60	50.50	57.46	61.77	62.12	59.69	55.63	55.47	58.66	62.75	67.33	71.96	76.02	79.02	81.29	83.34	85.31	86.00
21	2.50	6.87	8.16	8.19	8.78	9.41	12.73	15.66	19.00	22.53	25.57	28.38	30.08	35.32	38.50	40.46	42.51
	46.46	52.03	57.65	60.19	57.74	53.81	53.88	57.09	61.29	65.57	70.12	73.82	76.36	78.18	79.79	81.21	80.50
22	4.00	8.43	9.30	9.33	9.01	9.35	10.14	13.27	16.22	19.37	21.99	23.91	26.42	28.71	33.91	36.89	42.49
	47.57	53.53	57.93	59.06	57.84	55.05	51.52	52.15	55.02	59.07	63.19	67.32	70.67	72.95	74.35	75.42	73.50
23	5.00	9.64	10.24	9.95	9.44	9.87	11.09	13.38	15.94	18.59	20.28	19.75	22.62	27.48	32.29	34.22	39.61
	46.74	52.83	56.42	56.77	54.57	51.41	49.09	49.71	52.42	56.20	60.08	63.83	66.95	68.91	69.97	70.56	65.00
24	7.00	10.81	10.94	10.26	9.32	9.60	10.77	12.69	15.03	17.52	19.32	19.55	23.98	27.96	31.19	32.11	34.65
	44.43	50.60	54.01	53.10	49.32	46.36	45.71	46.96	49.36	52.94	56.41	59.76	62.60	64.40	65.29	65.79	63.00
25	7.50	11.19	10.67	9.37	8.16	8.40	9.48	11.22	13.52	16.09	18.56	21.05	24.13	27.31	29.65	31.22	37.33
	43.59	48.26	49.87	47.55	42.93	41.47	41.69	43.14	45.56	49.06	52.33	55.24	57.89	59.69	60.60	61.00	57.50
26	8.00	10.92	9.51	7.84	6.42	6.43	7.49	9.13	11.47	14.16	17.05	20.13	23.03	25.58	26.23	26.96	36.96
	41.92	44.39	43.96	39.47	37.32	37.01	37.28	38.11	40.66	44.08	47.45	50.20	52.76	54.79	55.97	56.71	55.50
27	0.0	6.00	7.34	5.46	4.18	4.11	4.86	6.41	8.90	11.71	14.78	18.09	21.22	23.87	25.93	27.79	34.68
	37.59	38.12	36.46	33.17	32.47	32.35	32.27	31.50	33.98	37.73	41.37	44.60	47.21	49.86	51.73	53.04	52.10
28	0.0	0.0	5.00	2.00	1.35	1.43	1.84	3.02	6.00	8.75	12.00	15.38	18.69	21.50	23.68	25.32	29.20
	30.68	30.22	28.97	27.79	27.64	27.78	27.62	26.56	27.27	30.06	35.14	38.97	42.43	45.54	48.36	50.22	47.50
29	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	3.24	5.04	8.54	11.88	15.44	18.37	20.15	21.10	22.81
	24.01	23.82	23.99	23.72	23.85	24.30	24.77	25.10	26.16	28.76	31.66	34.87	38.99	43.48	47.35	49.91	48.00
30	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	1.00	6.00	8.00	13.00	14.47	15.43	16.09	18.17
	20.10	21.10	21.55	21.86	22.25	22.79	23.62	24.71	26.29	28.56	31.34	34.52	38.18	43.35	48.47	52.05	52.50
31	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	8.00	9.50	9.50	12.86
	16.39	18.92	19.85	20.49	21.07	21.71	22.80	24.27	26.06	28.26	30.97	34.20	39.08	45.39	51.19	55.87	62.00
32	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	3.00
	13.39	16.24	17.71	18.62	19.38	20.18	21.40	23.00	25.08	27.85	30.58	34.95	41.30	47.97	54.38	59.90	71.00
33	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	6.00	14.58	15.39	16.45	17.23	17.84	19.15	20.80	22.96	26.07	29.35	35.42	43.24	50.54	57.59	63.20	80.00
34	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	0.0	9.00	10.80	13.50	14.00	13.00	14.50	16.00	18.50	23.50	25.80	34.80	45.50	52.50	61.00	65.50	78.50



53.53	59.28	63.04	66.68	68.46	62.90	47.72	49.46	59.27	66.39	75.92	84.16	92.16	97.33	104.86	105.46	109.00	0.0
21	0.0	49.00	58.56	36.71	2.00	5.47	12.37	18.83	23.62	26.96	31.86	37.03	43.36	49.83	50.44	52.97	54.87
55.45	58.47	63.16	65.58	65.24	62.12	46.87	47.95	55.11	64.70	67.29	78.93	88.20	90.10	93.40	96.47	104.00	0.0
22	0.0	49.00	43.44	32.02	7.17	6.66	10.68	17.49	20.56	28.94	32.53	36.91	40.28	47.33	50.24	52.55	55.56
59.82	60.22	63.84	64.36	62.72	58.37	43.60	45.96	51.71	61.94	66.75	74.50	81.21	88.26	89.34	87.97	101.00	0.0
23	0.0	49.00	49.07	13.52	6.78	9.92	12.49	16.78	19.71	27.03	30.11	35.45	39.78	42.59	48.35	51.12	54.00
55.49	59.30	61.49	61.59	58.99	52.49	45.18	42.36	49.54	57.53	63.54	69.77	79.67	83.41	82.27	82.26	86.00	0.0
24	0.0	54.00	70.88	20.05	8.06	9.86	12.39	15.12	17.97	24.53	28.75	32.25	35.15	40.65	49.33	49.11	46.05
53.26	55.64	61.47	58.92	48.78	37.64	39.45	46.61	47.10	56.65	58.90	69.24	71.83	75.36	72.18	74.73	76.00	0.0
25	0.0	69.00	66.69	28.58	2.32	7.84	9.63	11.78	15.35	20.29	23.31	33.46	31.99	41.02	46.06	47.37	49.19
50.79	55.07	56.86	53.48	37.06	32.64	37.66	44.16	45.78	55.64	58.02	62.80	65.75	69.90	67.87	68.04	68.00	0.0
26	0.0	79.00	52.63	28.09	0.0	1.77	8.59	12.57	16.21	21.10	28.83	30.16	38.48	42.42	47.01	47.55	47.57
47.85	50.91	50.92	36.79	33.95	35.16	37.47	37.52	45.01	51.09	56.47	57.55	57.08	60.72	63.02	65.31	65.00	0.0
27	0.0	0.0	29.00	14.00	0.0	1.14	2.10	4.61	8.95	13.38	16.55	23.95	29.27	32.92	41.52	40.42	44.49
43.69	42.83	41.49	27.84	29.56	31.76	33.96	36.01	42.24	44.70	46.80	50.86	52.87	51.02	52.89	58.88	59.00	0.0
28	0.0	0.0	0.0	0.0	0.0	1.00	1.00	3.00	6.71	10.40	15.68	21.05	27.07	31.52	38.00	37.73	40.16
41.45	38.94	35.46	24.89	29.03	31.30	29.08	32.50	34.92	35.05	39.79	41.39	37.37	38.64	41.57	43.52	48.00	0.0
29	0.0	0.0	0.0	0.0	0.0	0.0	0.0	2.00	6.00	6.00	11.00	21.00	25.00	31.45	34.86	33.64	30.16
32.05	35.10	36.27	22.05	20.70	25.22	26.09	26.92	27.95	29.35	31.58	30.21	32.27	35.43	42.32	46.71	53.00	0.0
30	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	32.00	33.00	32.00	19.80
20.97	34.35	31.36	26.41	23.31	20.89	21.82	20.62	20.75	22.60	26.30	27.61	30.90	42.14	47.83	57.09	61.00	0.0
31	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	19.00
20.07	34.80	40.23	35.95	33.54	21.83	28.89	38.06	36.33	22.79	23.83	26.92	33.21	45.18	59.15	67.03	70.00	0.0
32	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
14.00	25.99	39.09	31.22	31.76	30.64	34.46	35.65	37.89	47.75	42.61	38.90	40.54	51.24	65.08	70.27	76.00	0.0
33	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
0.0	21.00	22.00	24.00	30.00	29.00	34.00	41.00	48.00	48.00	34.00	41.00	51.00	64.00	70.00	73.00	78.00	0.0
34	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

## ATTACHMENT E: MAPS AND THREE-DIMENSIONAL GRAPHICS OF MODEL SIMULATIONS

Model-interrogation runs were made to evaluate well-field interference that may result from pumping all 10 well fields simultaneously. This involved separate simulations of drawdown at each well field for comparison with drawdown due to pumping all 10 well fields. Although not included in the main report, these separate simulations may be useful to water managers and planners. Figures 25-34 map the extent of cones of depression in the water table and Floridan aquifer as simulated by the model under average recharge conditions with pumping fixed at annual average permitted rates.

Three-dimensional graphical representations of the water table and potentiometric surface under predevelopment and pumping conditions were made using SAS/GRAPH (figs. 35-40). Although not useful technically, the plots exhibit depth perspective that cannot be perceived from contour maps. The plots clearly show cones of depression around the well fields and clearly indicate areas where well-field interference should occur. From a management standpoint, the graphical plots certainly simplify the conveyance of technical data to the general public.

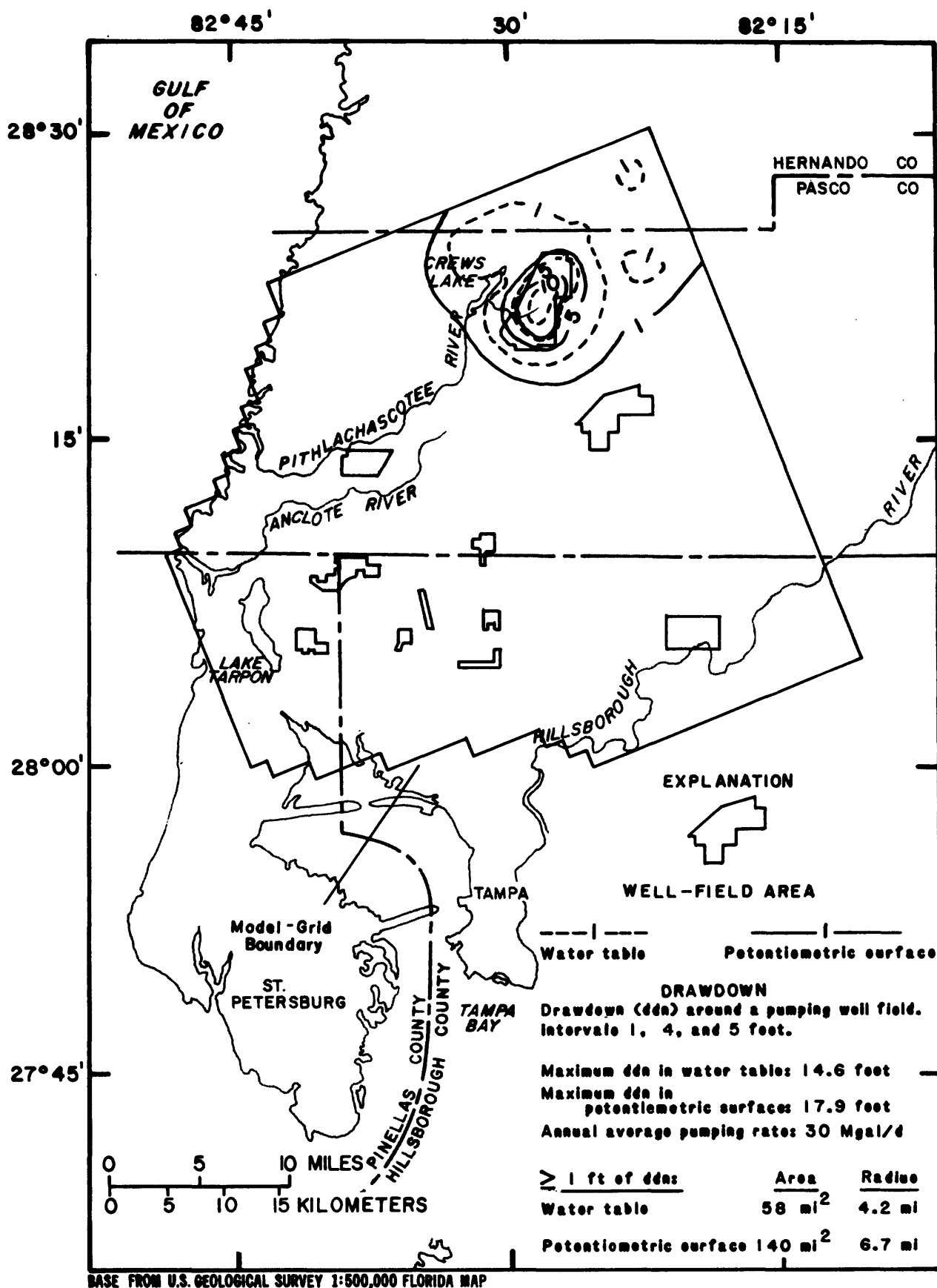


Figure 25.--Model-simulated drawdown at Cross Bar Ranch well field.

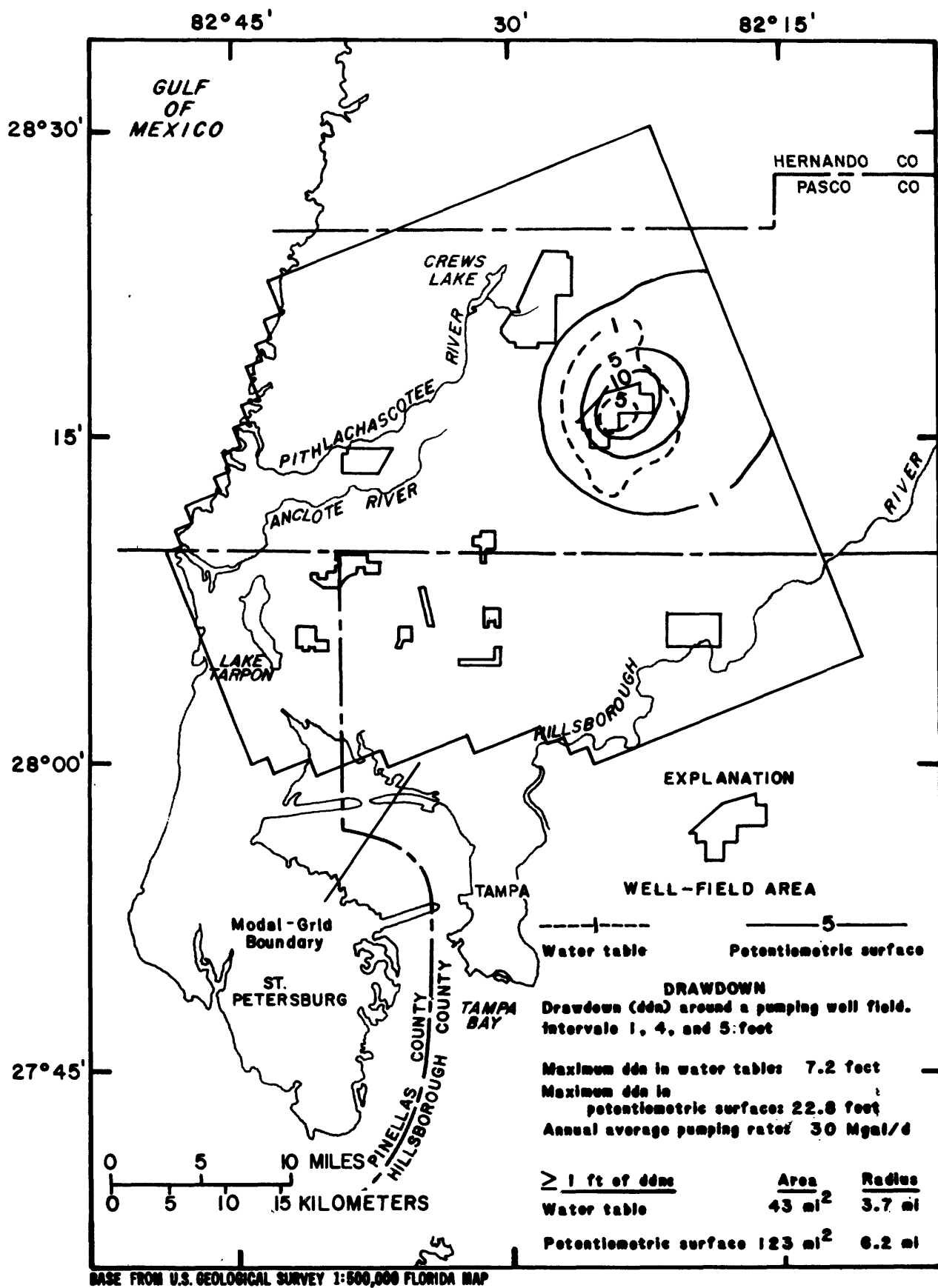


Figure 26.--Model-simulated drawdown at Cypress Creek well field.

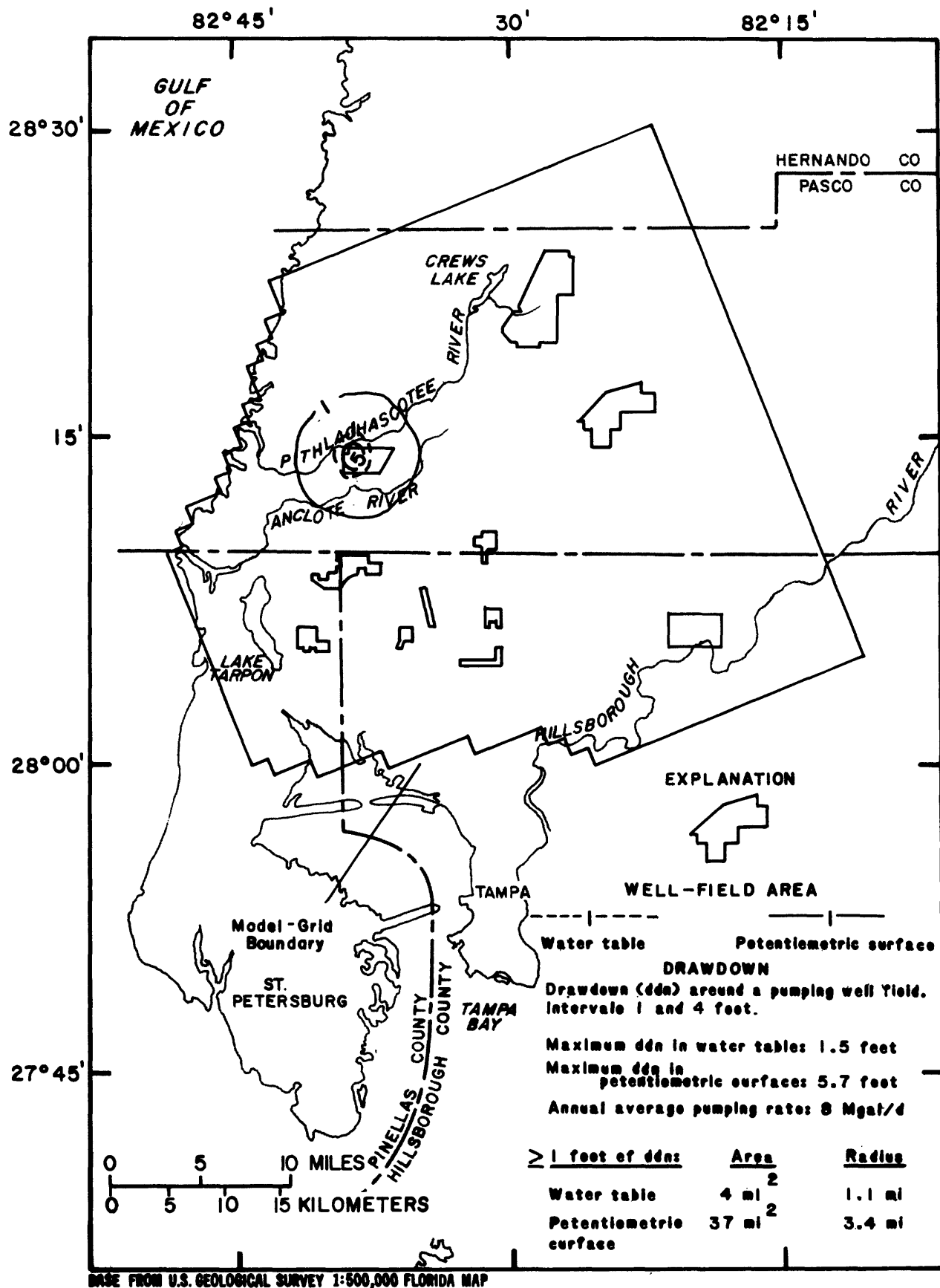


Figure 27.--Model-simulated drawdown at Starkey well field.

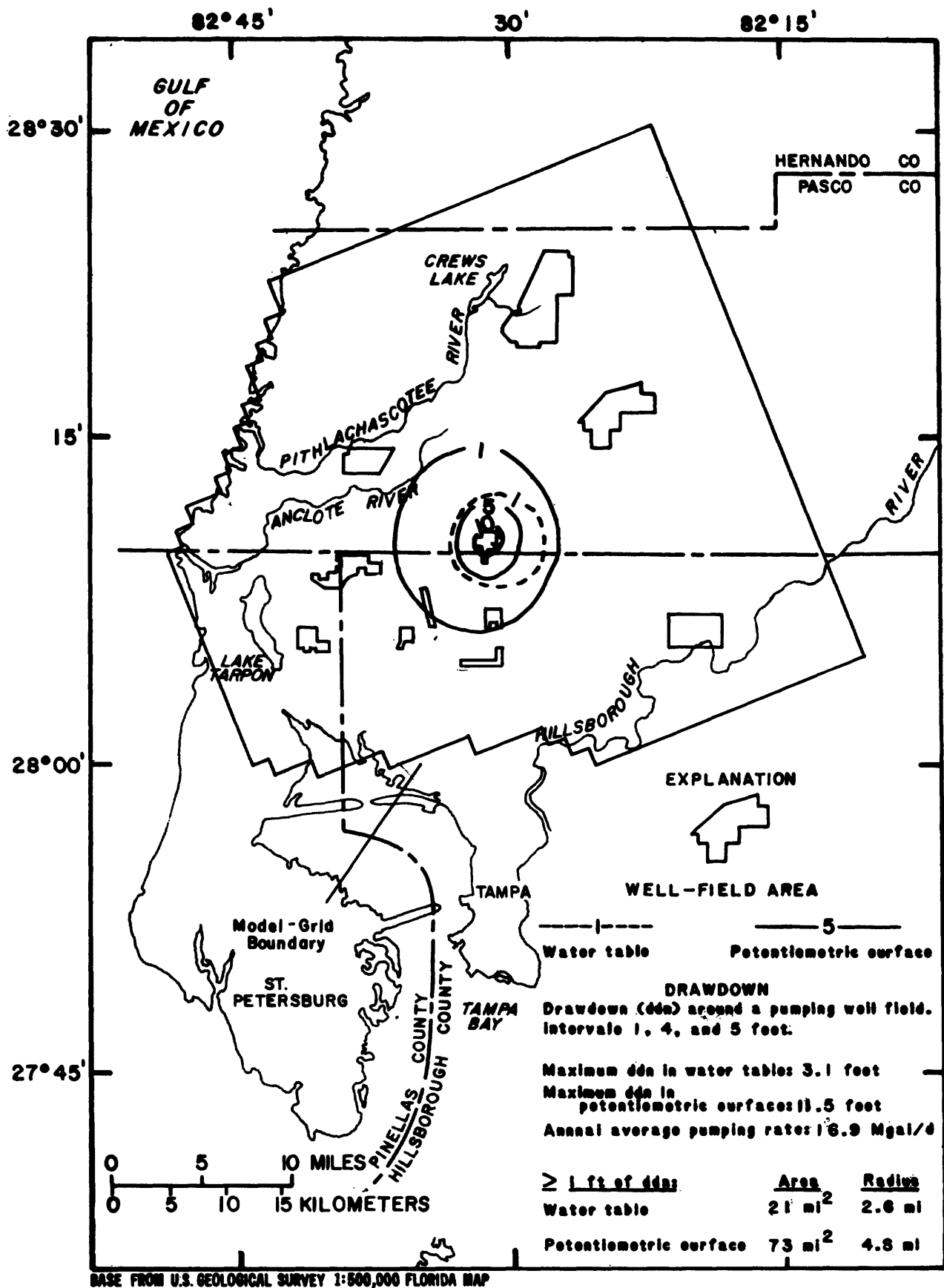


Figure 28.--Model-simulated drawdown at Pasco County well field.



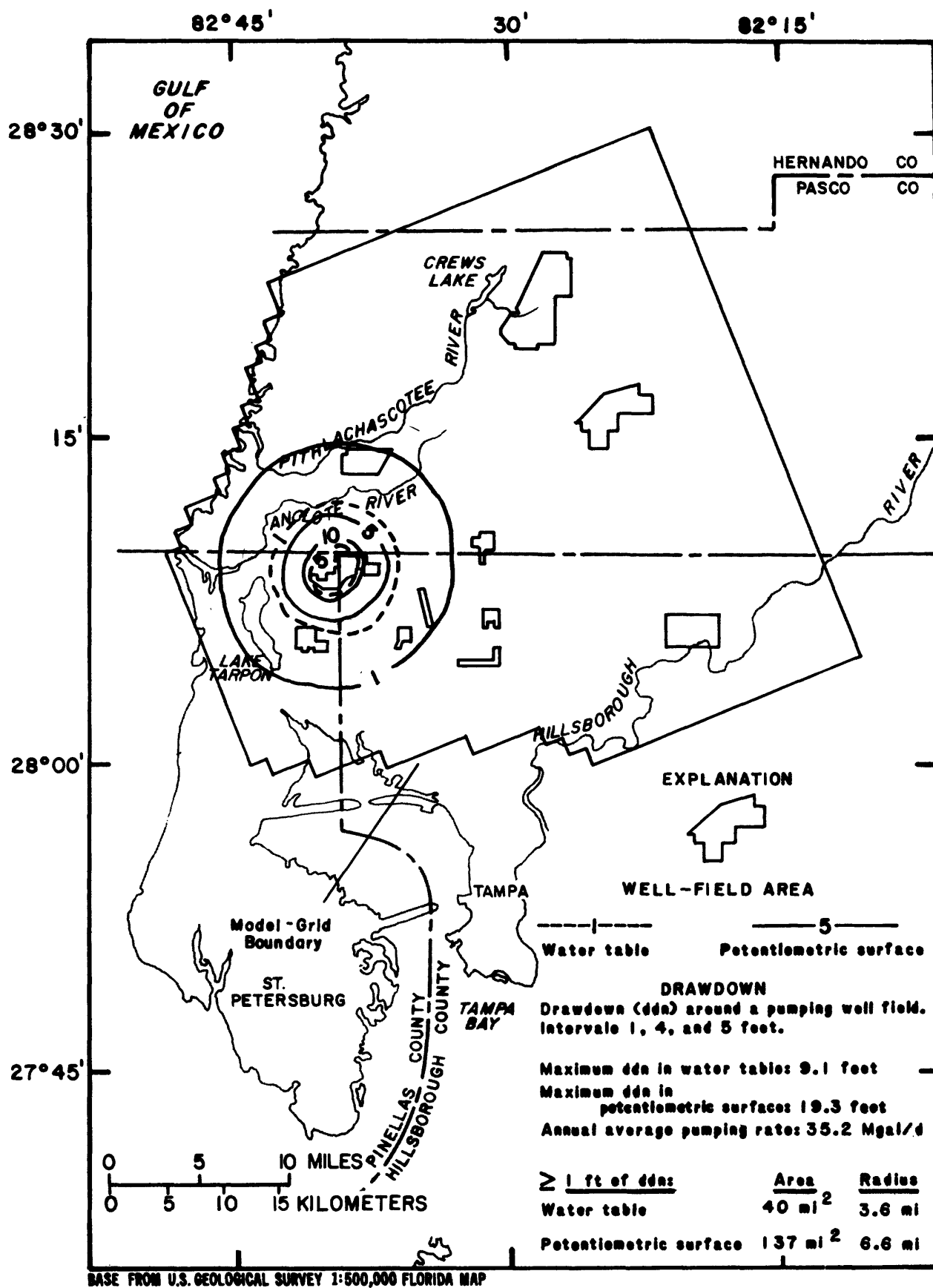


Figure 29.--Model-simulated drawdown at Eldridge-Wilde well field.

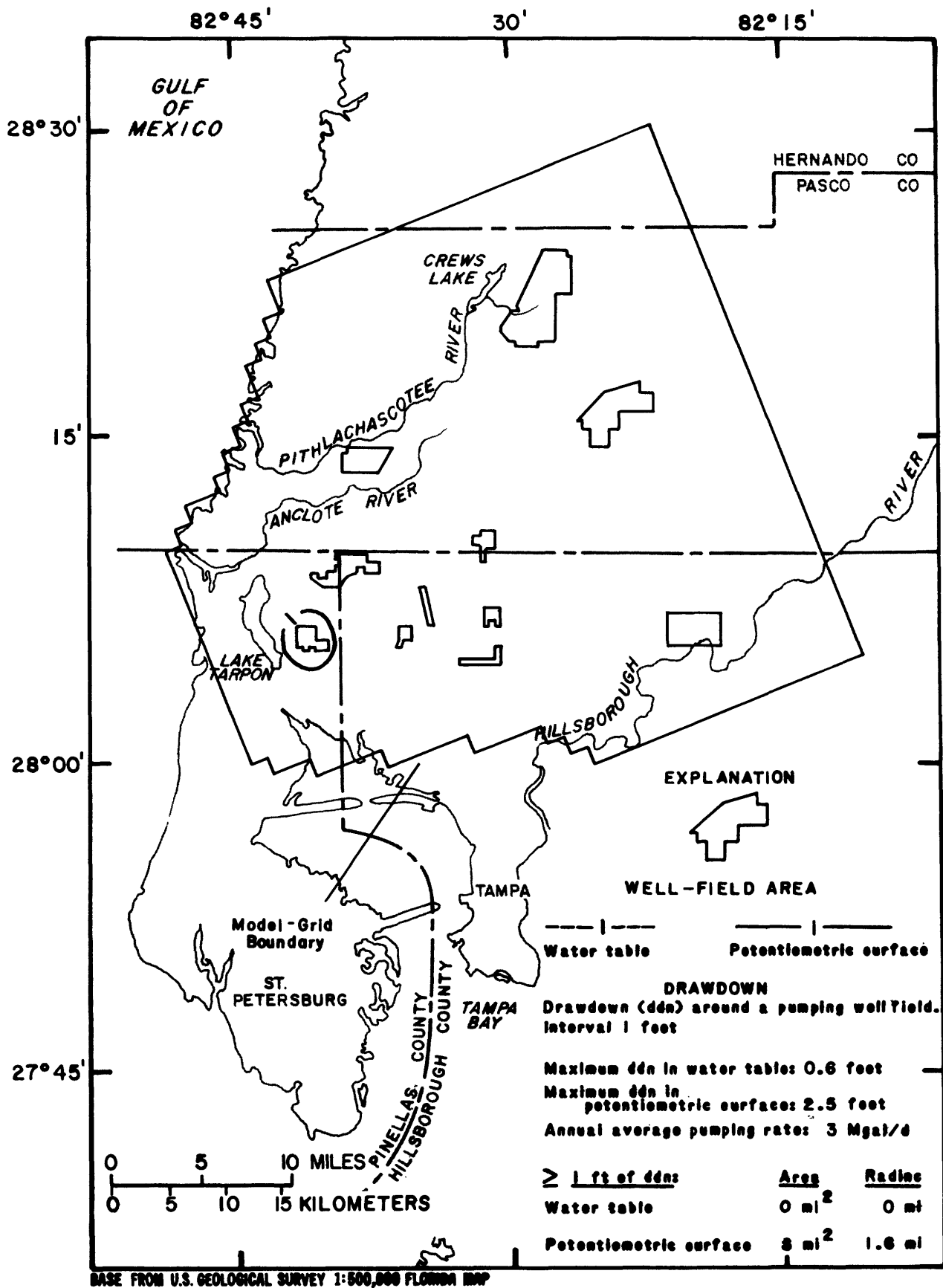


Figure 30.--Model-simulated drawdown at East Lake well field.

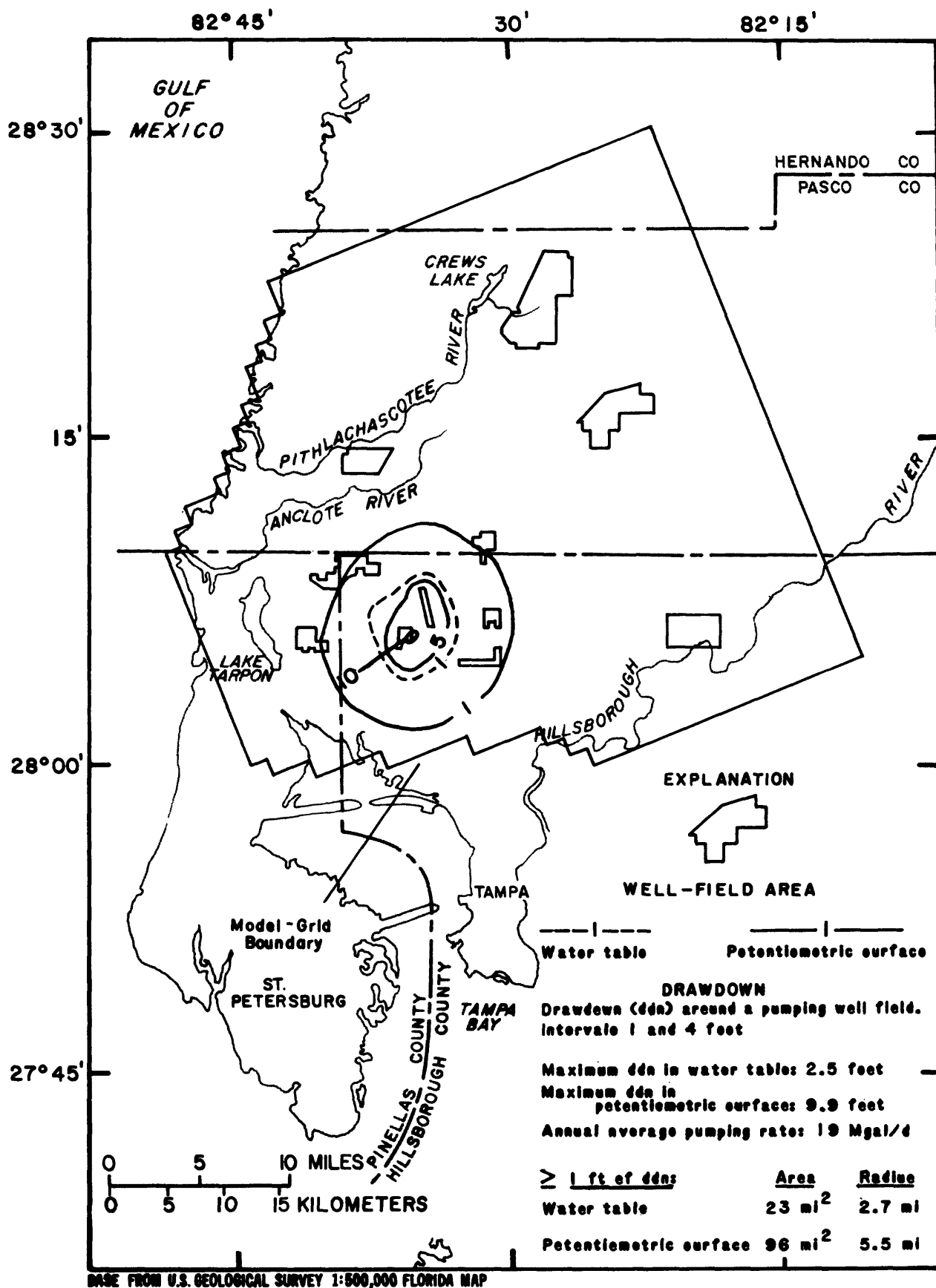


Figure 31.--Model-simulated drawdown at Cosme well field.

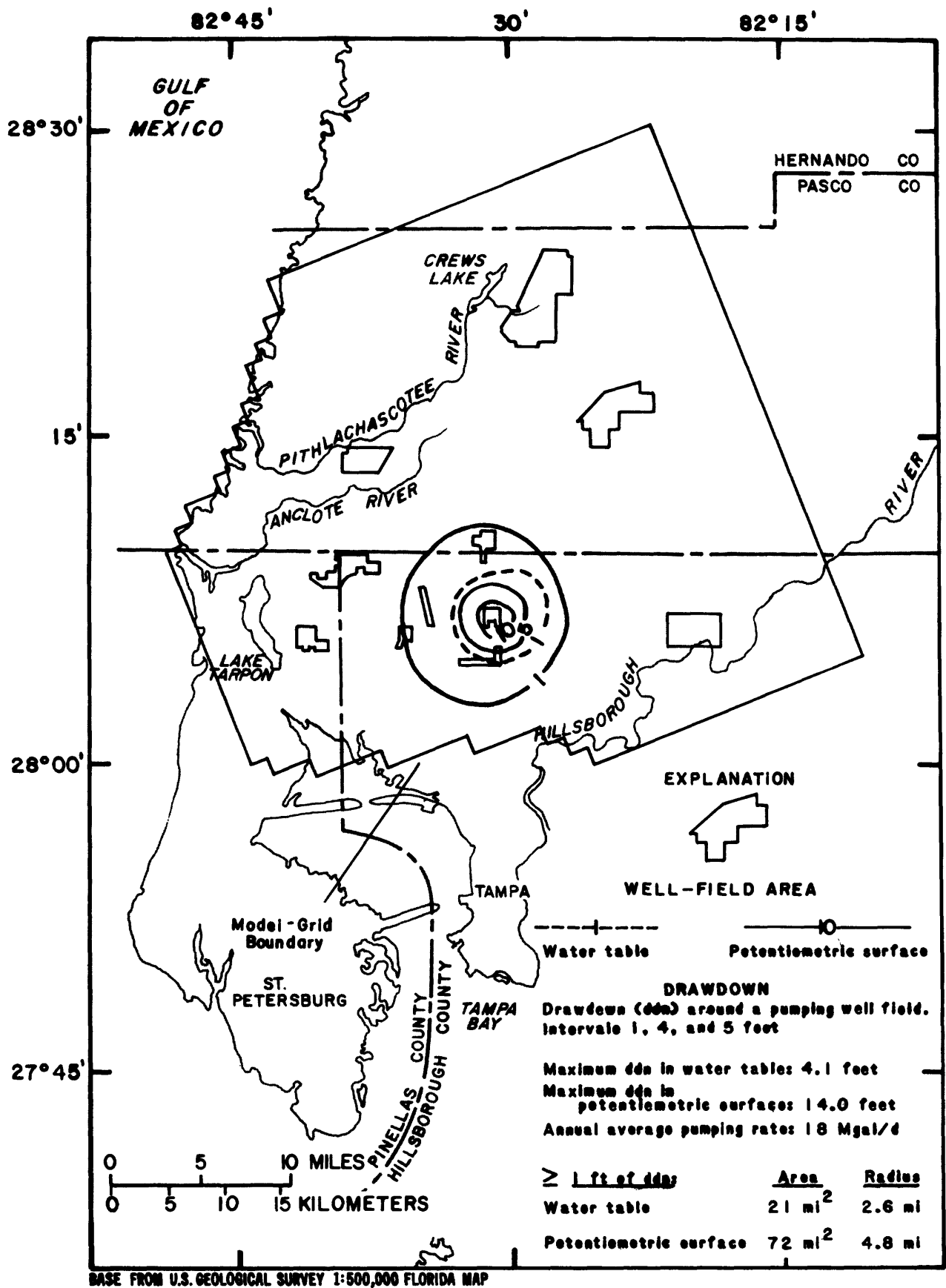


Figure 32.--Model-simulated drawdown\* at Section 21 well field.

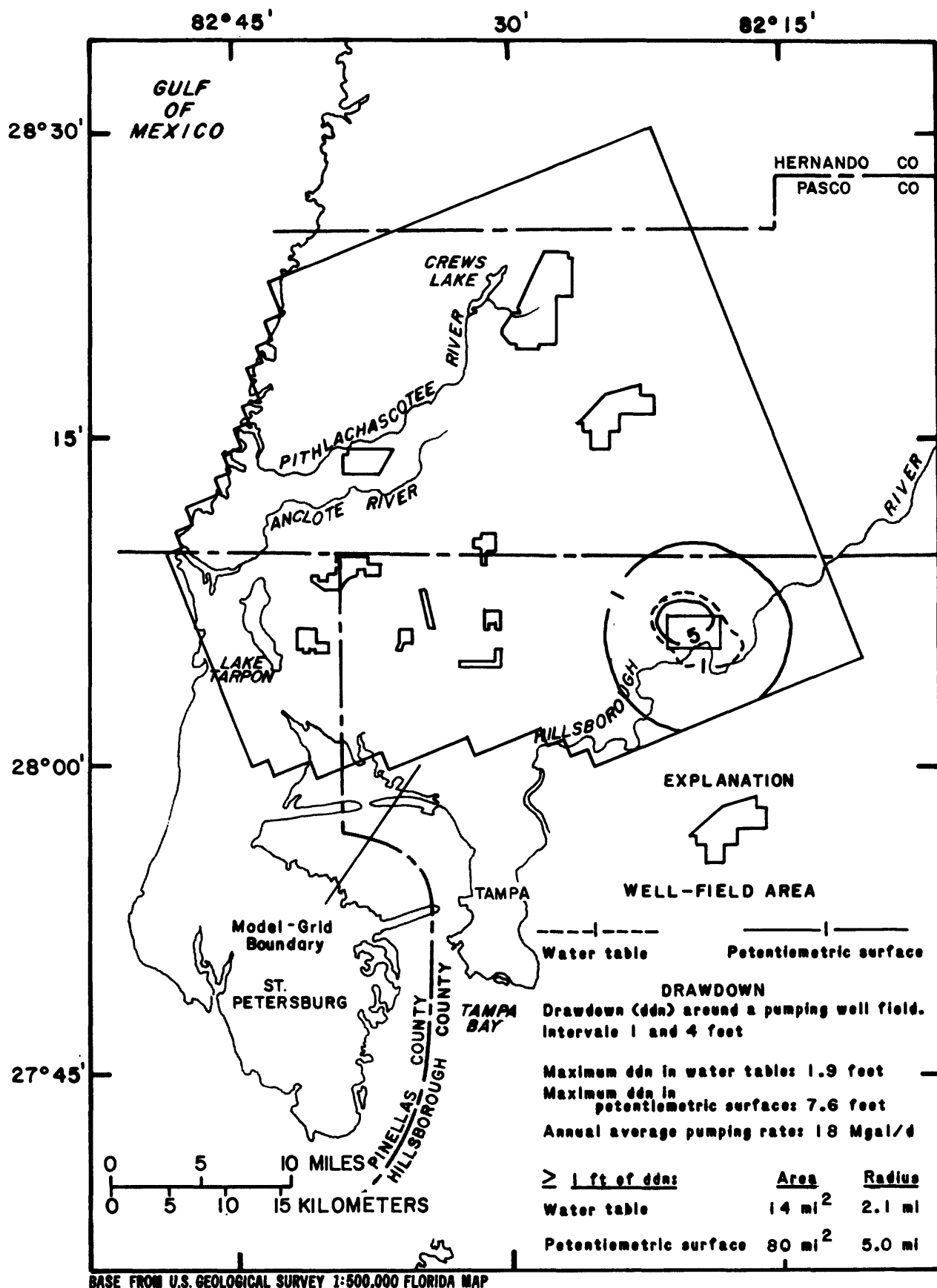


Figure 33.--Model-simulated drawdown at Morris Bridge well field.

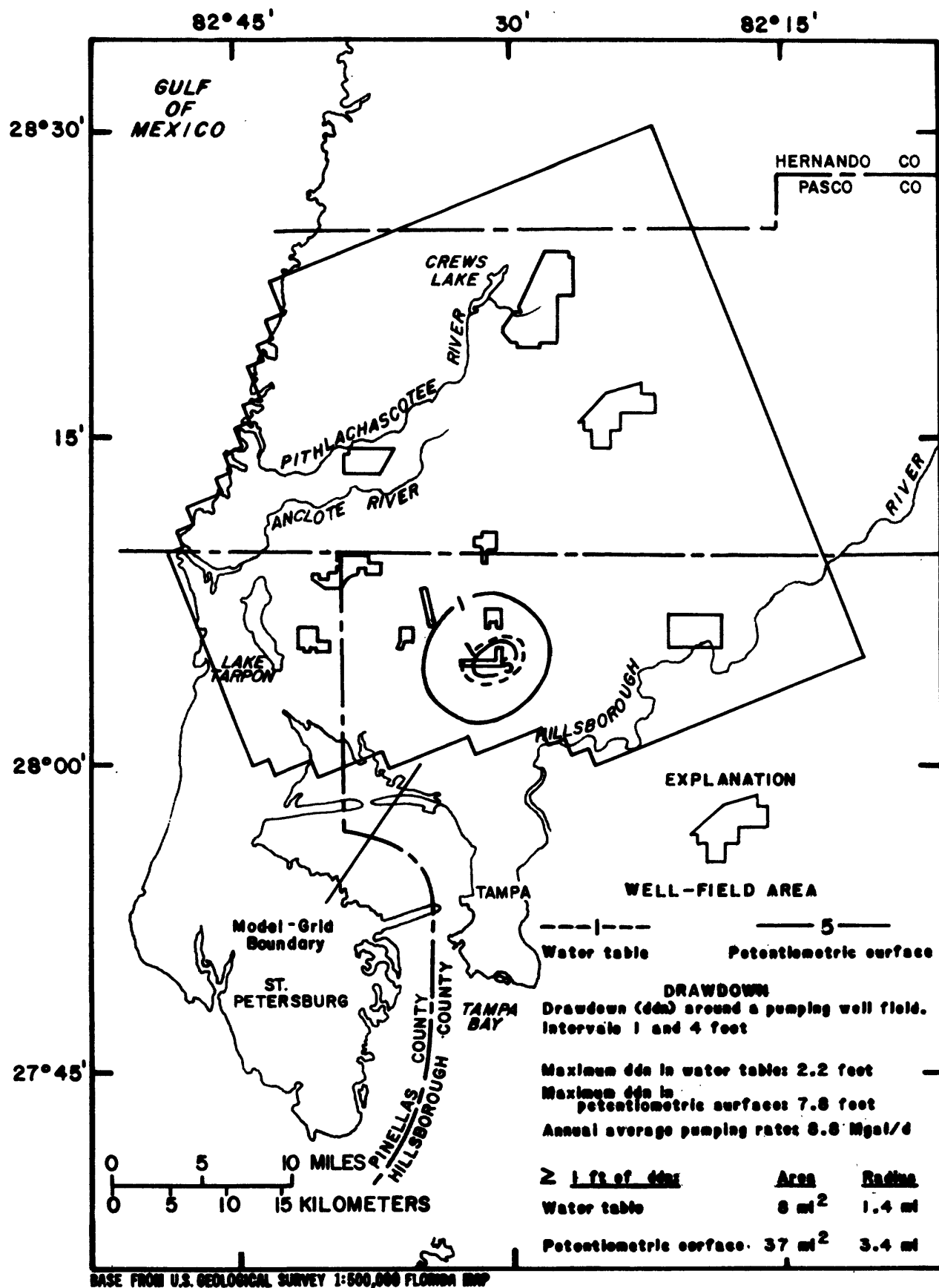


Figure 34.--Model-simulated drawdown at Northwest well field.

# NONPUMPING WATER TABLE

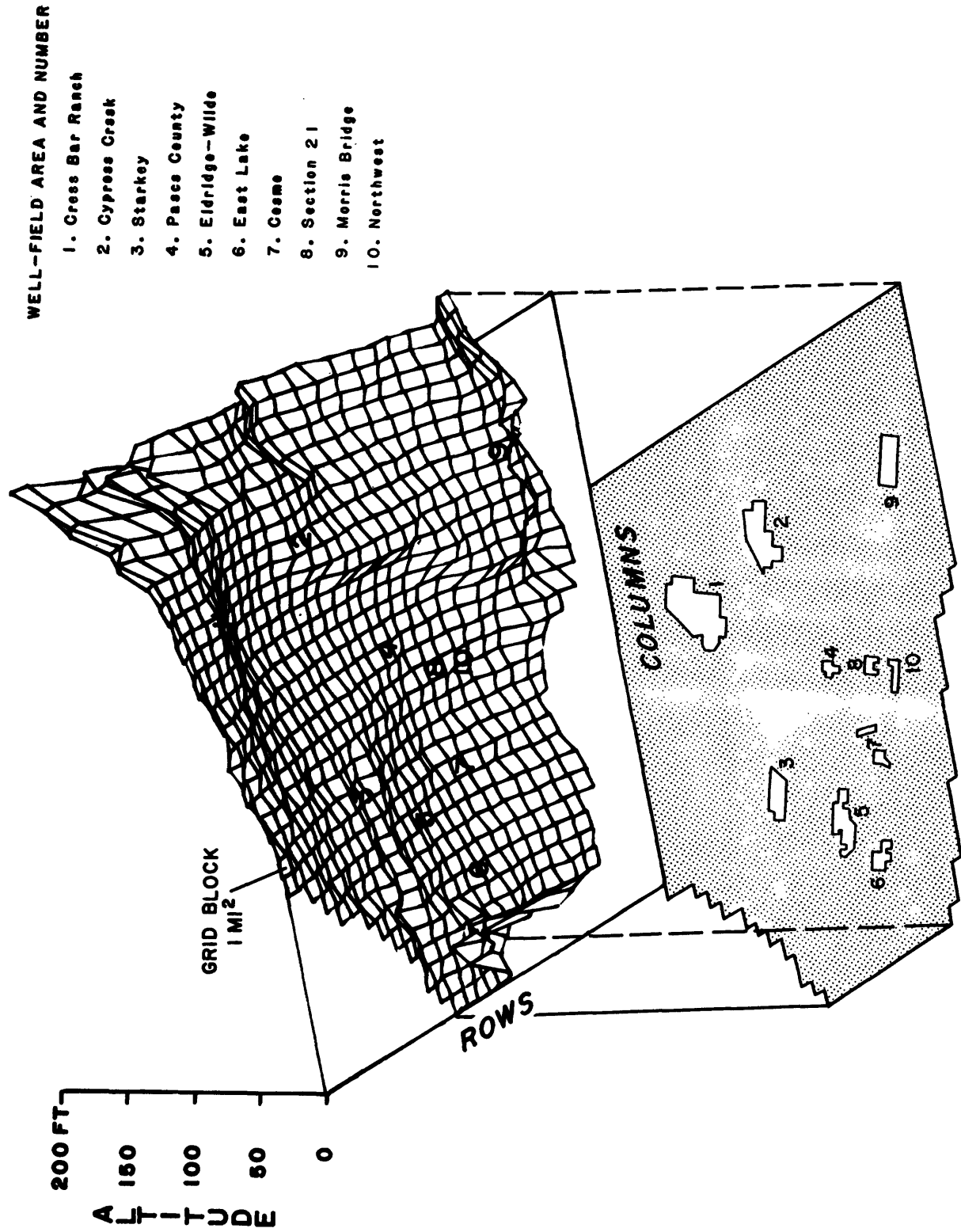


Figure 35.--Water table in the surficial aquifer under nonpumping conditions.

# PUMPING WATER TABLE PUMP TEN WELL FIELDS AT 186.9 MGAL/D, RECHARGE IS AVERAGE

## WELL-FIELD AREA AND NUMBER

1. Gress Bar Ranch
2. Cypress Creek
3. Starkey
4. Pascoe County
5. Eldridge-Wilde
6. East Lake
7. Gesme
8. Section 21
9. Morris Bridge
10. Northwest

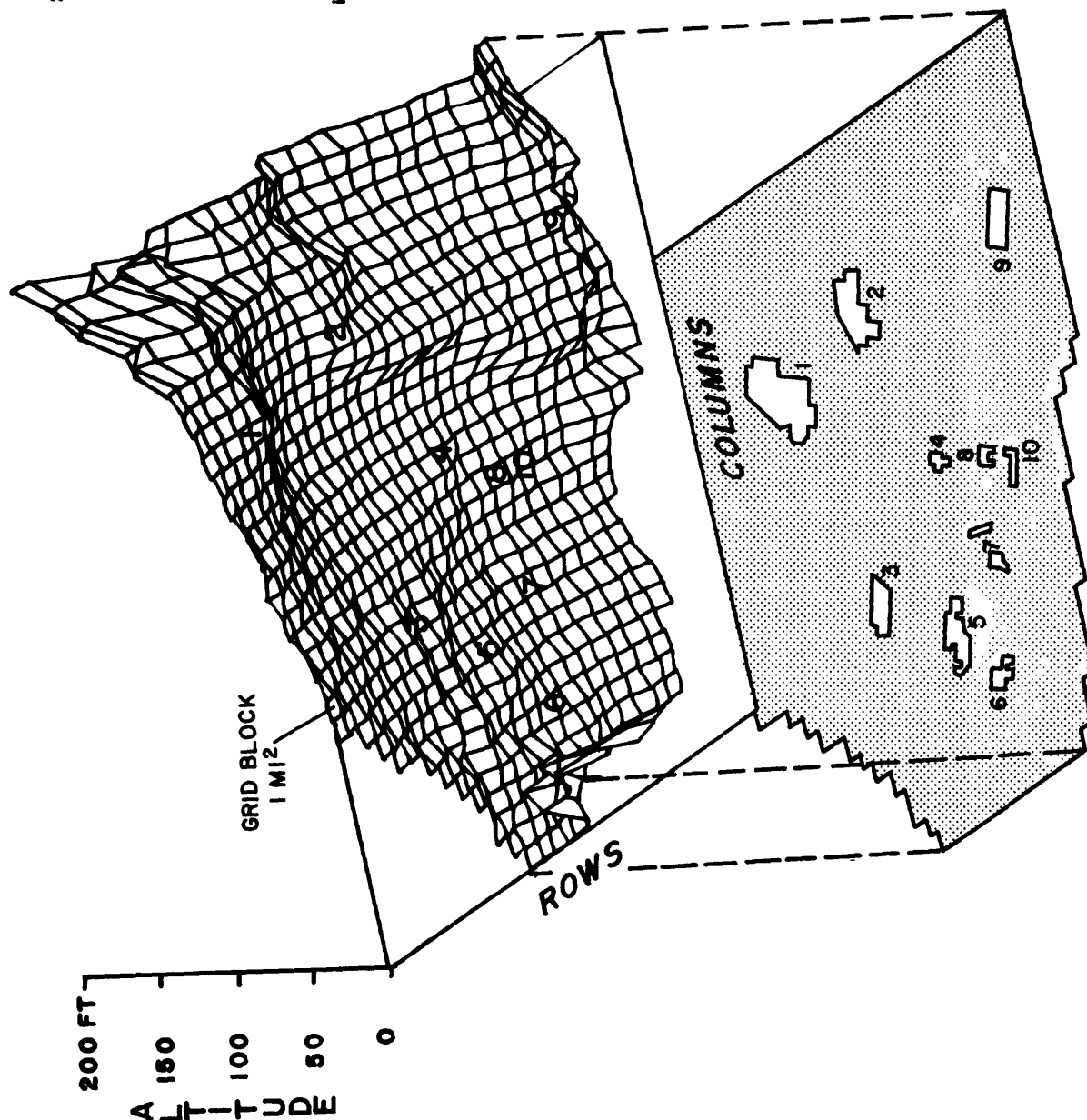


Figure 36.--Water table in the surficial aquifer under pumping conditions.



# NONPUMPING POTENTIOMETRIC SURFACE

## WELL-FIELD AREA AND NUMBER

1. Cross Bar Ranch
2. Cypress Creek
3. Starkey
4. Pace County
5. Eldridge-Wilde
6. East Lake
7. Ceame
8. Section 21
9. Merrie Bridge
10. Northwest

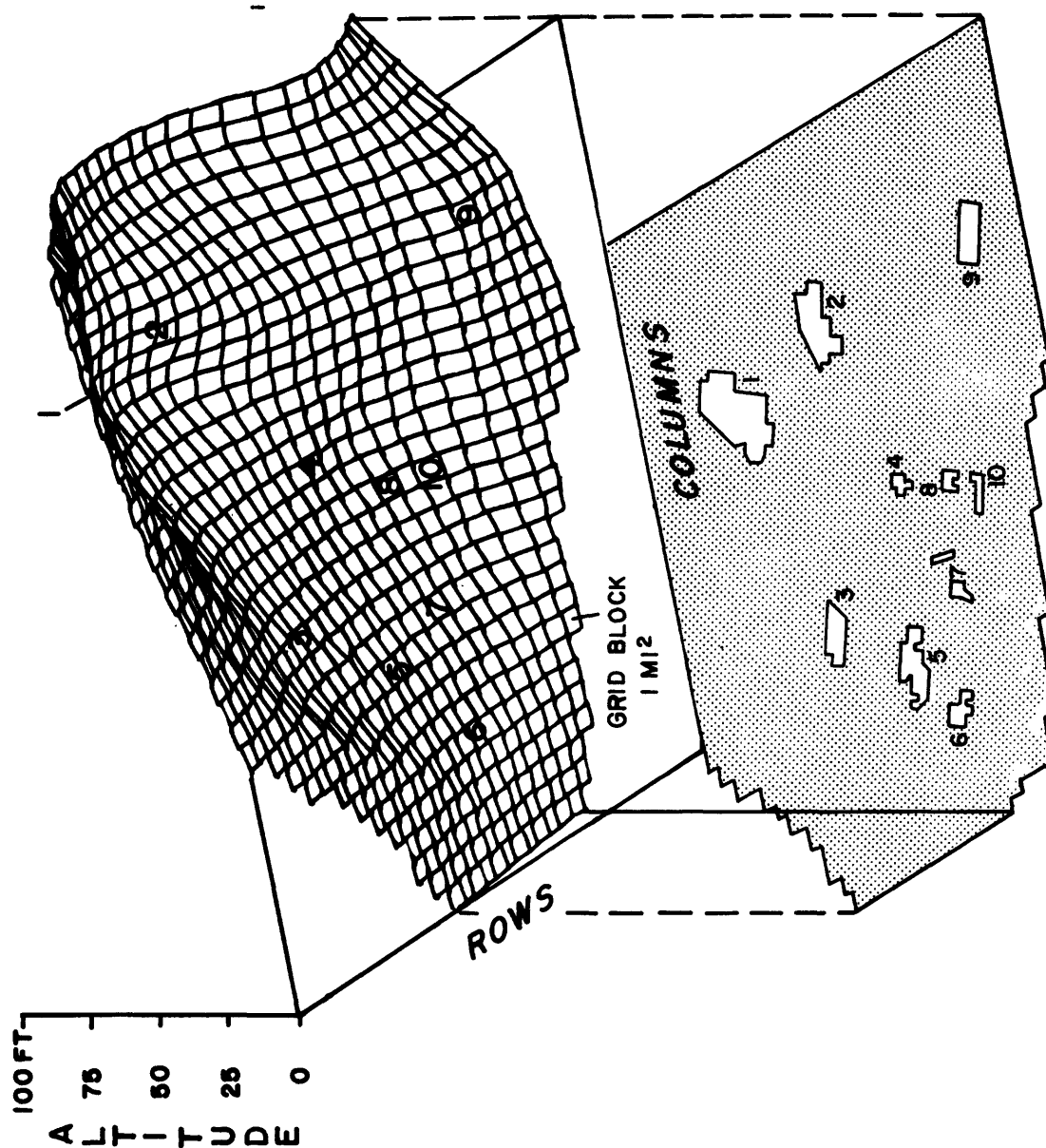


Figure 37.--Potentiometric surface of the Floridan aquifer under nonpumping conditions.

# PUMPING POTENTIOMETRIC SURFACE PUMP TEN WELL FIELDS AT 186.9 MGAL/D, RECHARGE IS AVERAGE

## WELL-FIELD AREA AND NUMBER

1. Cress Bar Ranch
2. Cypress Creek
3. Starkey
4. Pasce County
5. Eldridge-Wilde
6. East Lake
7. Cesme
8. Section 21
9. Morris Bridge
10. Northwest

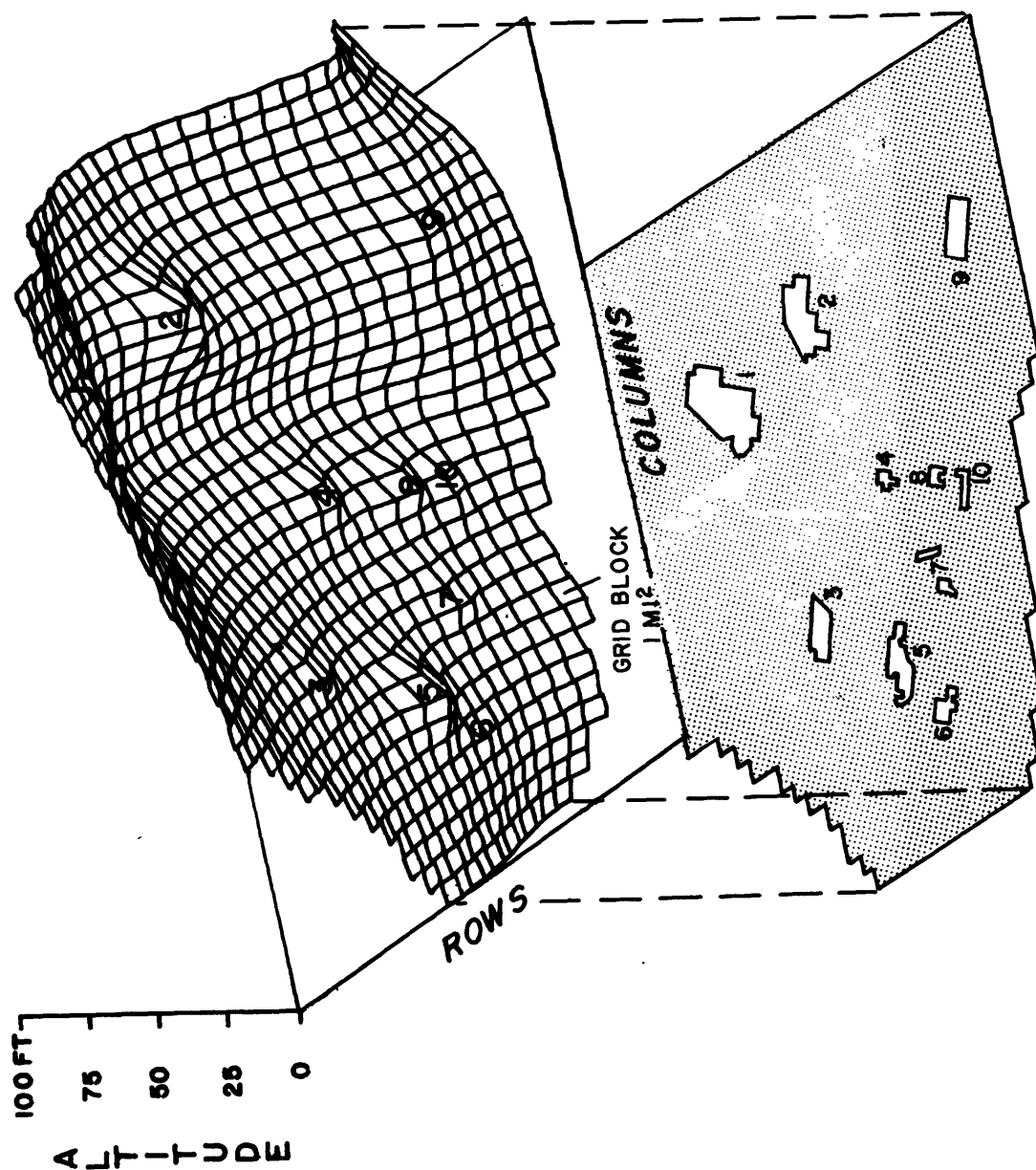


Figure 38.--Potentiometric surface of the Floridan aquifer under pumping conditions.

# DRAWDOWN IN WATER TABLE

PUMP TEN WELLFIELDS AT 186.9 MGAL/D, RECHARGE IS AVERAGE

WELL-FIELD AREA AND NUMBER

1. Cree Bar Ranch
2. Cypress Creek
3. Starkey
4. Pasco County
5. Eldridge-Wilde
6. East Lake
7. Ceeme
8. Section 21
9. Morris Bridge
10. Northwest

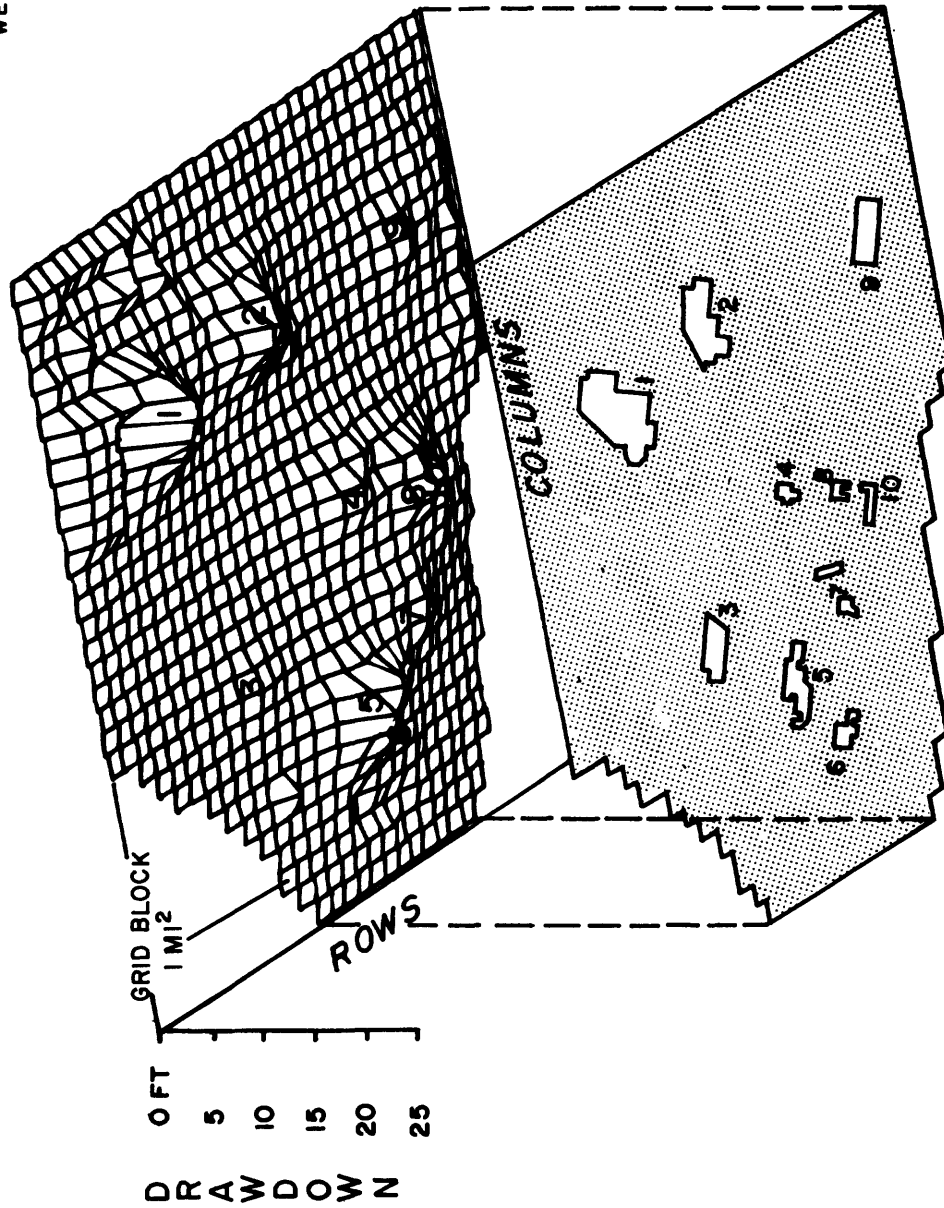


Figure 39.--Drawdown in the water table in the surficial aquifer under pumping conditions.

# DRAWDOWN IN POTENTIOMETRIC SURFACE

## PUMP TEN WELL FIELDS AT 186.9 MGAL/D, RECHARGE IS AVERAGE

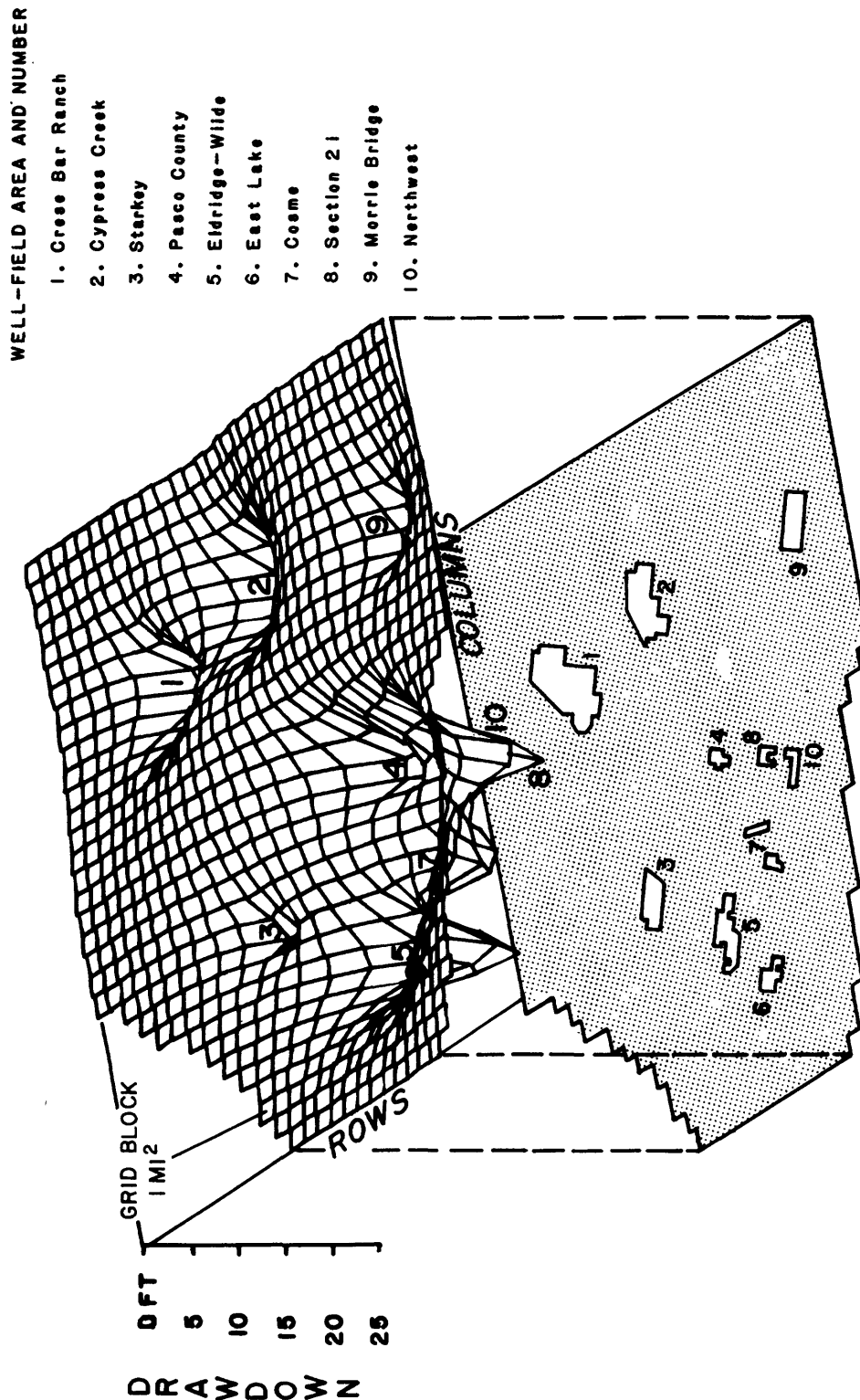


Figure 40.--Drawdown in the potentiometric surface of the Floridan aquifer under pumping conditions.