

THE COMPUTER MODEL SHARP, A QUASI-THREE-DIMENSIONAL FINITE-DIFFERENCE
MODEL TO SIMULATE FRESHWATER AND SALTWATER FLOW IN LAYERED COASTAL
AQUIFER SYSTEMS

By Hedef I. Essaid

U. S. GEOLOGICAL SURVEY

Water-Resources Investigations Report 90-4130

Menlo Park, California

1990



DEPARTMENT OF THE INTERIOR
MANUEL LUJAN, Jr., Secretary
U. S. GEOLOGICAL SURVEY
Dallas L. Peck, Director

For additional information
write to:

Project Chief
U.S. Geological Survey
345 Middlefield Rd., MS 421
Menlo Park, CA 94025

Copies of this report can
be purchased from:

U. S. Geological Survey
Books and Open-File Reports Section
Federal Center, Bldg. 810
Box 25425
Denver, Colorado 80225

CONTENTS

	<u>Page</u>
Abstract	1
Introduction	2
Purpose and Scope	2
Coastal Hydrogeologic Conditions.	2
Saltwater Intrusion Modeling Approaches	5
Disperse Interface Approach.	7
Sharp Interface Approach	8
Discussion of Approaches	9
The Multilayer Freshwater - Saltwater Flow Model SHARP	10
Model Development.	11
Vertical Integration of the Coupled Freshwater - Saltwater Flow Equations.	11
Boundary Terms	13
Leakage Terms.	14
The Integrated Equations	16
Numerical Form of the Freshwater and Saltwater Flow Equations	17
Finite-Difference Approximations of Spatial and Temporal Derivatives.	18
Sources and Sinks.	19
Interface Tip and Toe Tracking	20
Leakage Calculations	25
Discretized Flow Equations	29
Solution of the Coupled Flow Equations	29
The Strongly Implicit Procedure (SIP)	31
Convergence of SIP.	32
Iteration Parameters	32
Relaxation Factor.	33
Model Verification	33
Single Layer Problems	33
Steady-State Interface in a Layered Coastal Aquifer	37
Comparison of Sharp Interface and Disperse Interface Solutions.	39
Description of the Computer Program.	42
MAIN Routine.	42
Subroutine INPUT.	44
Subroutine READ	44
Subroutine IPARAM	44
Subroutine KMEAN.	44
Subroutine SIP.	45
Subroutine TCOEF.	45
Subroutine FACTOR	45
Subroutine RESULTS.	47
Subroutine OUTPUT	47
Subroutine MASBAL	47
Practical Considerations for Model Application	48
Array Dimensions.	48
Input/Output File Units	48
User Specified Input/Output Options	49
Initial Conditions.	49
Boundary Conditions	52
Steady-State Simulations.	53
Notes on Discretization	54

Notes on Parameters RFAC, WFAC, WITER, and NUP.	54
Mass Balance Calculations	55
Leakage Conditions.	56
Example Problems	56
Example 1: Single Layer Model of Southeastern Oahu	57
Example 2: Layered Cross-section of Cape May, New Jersey	78
References	106
Appendix A: Matrix Coefficients	111
Appendix B: The Strongly Implicit Procedure for Three-Dimensional Two-Phase Flow.	114
Attachment A: Data Input Formats.	121
Attachment B: List of Selected Program Variables.	125
Attachment C: Program Listing	130

ILLUSTRATIONS

	<u>Page</u>
Figure 1. Cross-sections showing examples of hydrogeologic conditions in coastal aquifers:	
A. unconfined aquifer with an impermeable bottom,	
B. unconfined island aquifer with a free bottom, and	
C. confined aquifer.	3
2. Idealized cross-section of a layered coastal aquifer system showing paths of freshwater discharge and potential paths for saltwater intrusion:	
A. steady-state system with constant freshwater discharge offshore,	
B. transient system with intruding saltwater and inland interface movement.	4
3. Diagram showing circulation of saltwater from the sea to the transition zone and back to the sea induced by mixing at the interface	5
4. Diagram showing the Ghyben-Herzberg interface model	9
5. Diagram showing the freshwater and saltwater flow domains in a confined aquifer.	11
6. Diagram showing leakage through a confining layer	15
7. Diagram showing the block-centered finite-difference grid	18
8. Diagram showing a well penetrating freshwater and saltwater in an aquifer	20
9. Diagrams showing interface tip projection in the positive x-direction and toe projection in the negative x-direction.	
A. The projected interface at iteration k-1:	
B. The new projected interface position at iteration level k	22
10. Diagrams showing leakage terms.	26
11. Diagram showing the system of coupled freshwater and saltwater flow equations in matrix form	30
12. Graphs showing simulation of a rotating linear interface:	
A. results using SHARP,	
B. results of Mercer and others (1980a).	34
13. Graphs showing simulation of a retreating interface:	
A. results of SHARP,	
B. results of Shamir and Dagan (1971).	36
14. Graphs showing simulation of an intruding interface:	
A. results of SHARP,	
B. results of Shamir and Dagan (1971).	36
15. Diagram of the interface in a coastal aquifer with a thin horizontal semiconfining layer (modified from Mualem and Bear, 1974)	37
16. Diagram showing the geometry of the layered test problem.	38
17. Graphs showing comparison of numerical and analytical solutions:	
A. restricted mixing leakage (method 2),	
B. complete mixing leakage (method 1)	38
18. Modeled cross-section for Cape May simulation (modified from Hill, 1988)	39

19.	Diagrams showing results of steady-state simulations:	
	A. lines of equal chloride concentration from the SUTRA simulation and the sharp interface positions from the SHARP simulations,	
	B. fluid velocity vectors from SUTRA (modified from Hill, 1988)	40
20.	Flow chart for the MAIN program	43
21.	Flow charts for subroutines SIP, TCOEF and FACTOR	46
22.	Southeast Oahu simulation:	
	A. cross-section showing model geometry,	58
	B. map showing steady-state interface elevation.	59
23.	Cross-section showing geometry and boundary conditions for the layered example from Cape May, New Jersey	78
24.	Diagram showing the freshwater leakages from the mass balance for the Cape May example.	79
B1.	Diagram showing the structure of the modified coefficient matrix (A+B) and the sparse lower triangular (L) and upper triangular (U) matrices used in the strongly implicit procedure. . . .	115

TABLES

	<u>Page</u>
Table 1. Summary of numerical saltwater intrusion models.	6
2. Model leakage terms: (A) method 1 - complete mixing, (B) method 2 - restricted mixing	28
3. Parameters used for model verification simulations	35
4. Parameters used in Cape May cross-section simulation	40
5. Default input/output unit numbers.	49
6. Input/output options	51
7. Input file for example 1	60
8. Output file for example 1.	70
9. Input file for example 2	80
10. Output file for example 2.	87

CONVERSION FACTORS

The International System of Units (SI) can be converted to inch-pound units by the following conversion factors:

<u>Multiply metric unit</u>	<u>By</u>	<u>To obtain inch-pound-unit</u>
centimeter (cm)	0.03281	foot
centimeter (cm)	0.3937	inch
gram (gm)	0.002205	pound
meter (m)	3.281	foot
millimeter (mm)	0.0397	inch

LIST OF SYMBOLS

A	finite difference grid block area, L^2
A	septadiagonal block coefficient matrix multiplying unknown heads
B	coefficient submatrix multiplying $i-1, j, k$ heads
B'	thickness of the confining layer, L
B_f	thickness of the freshwater zone, L
B_s	thickness of the saltwater zone, L
D	coefficient submatrix multiplying $i, j-1, k$ heads
D	aquifer thickness, L
D_h	hydrodynamic dispersion coefficient tensor, $L^2 T^{-1}$
E	coefficient submatrix multiplying i, j, k heads
F	coefficient submatrix multiplying $i, j+1, k$ heads
g	gravitational acceleration, LT^{-2}
H	coefficient submatrix multiplying $i+1, j, k$ heads
h_s	depth to interface below sea level, L
i	y-direction discretization index
j	x-direction discretization index
k	layer discretization index
<u>k</u>	permeability tensor, L^2
k'	vertical permeability of confining layer, L^2
K'	vertical hydraulic conductivity of confining layer, LT^{-1}
K_f	freshwater hydraulic conductivity, LT^{-1}
<u>K_f'</u>	= $\underline{k}(x, y) \gamma_f / \mu_f$ vertically averaged freshwater hydraulic conductivity, LT^{-1}
<u>K_s'</u>	= $\underline{k}(x, y) \gamma_s / \mu_s$ vertically averaged saltwater hydraulic conductivity, LT^{-1}
K_{fx}	freshwater hydraulic conductivity in the x-direction, LT^{-1}
K_{sx}	saltwater hydraulic conductivity in the x-direction, LT^{-1}
K_{fy}	freshwater hydraulic conductivity in the y-direction, LT^{-1}
K_{sy}	saltwater hydraulic conductivity in the y-direction, LT^{-1}
L	distance to interface toe from its initial position, L
m	superscript for iteration level index
M	number of finite-difference blocks in system
n	effective porosity
n	superscript for time level index
N	recharge rate, LT^{-1}
p	fluid pressure, $ML^{-1} T^{-2}$

P_f	freshwater fluid pressure, $ML^{-1}T^{-2}$
P_s	saltwater fluid pressure, $ML^{-1}T^{-2}$
P_f	freshwater pumpage from grid block ijk , L^3T^{-1}
P_s	saltwater pumpage from grid block ijk , L^3T^{-1}
P_t	total fluid pumpage from a well, L^3T^{-1}
q_f'	vertically averaged freshwater flux, LT^{-1}
q_{fz}	vertical freshwater specific discharge, LT^{-1}
q_{fl_b}	freshwater leakage across bottom of aquifer, LT^{-1}
q_{fl_t}	freshwater leakage across top of aquifer, LT^{-1}
q_l	vertical leakage, positive upwards, LT^{-1}
q_s	saltwater specific discharge, LT^{-1}
q_s'	vertically averaged saltwater flux, LT^{-1}
q_{sz}	vertical saltwater specific discharge, LT^{-1}
q_{sl_b}	saltwater leakage across bottom of aquifer, LT^{-1}
q_{sl_t}	saltwater leakage across top of aquifer, LT^{-1}
Q	vector of known right-hand-side values
Q'	vector of known right-hand-side values at a node
Q_f	freshwater source/sink term, LT^{-1}
Q_s	saltwater source/sink term, LT^{-1}
Q_{lf}	freshwater leakage term, LT^{-1}
Q_{ls}	saltwater leakage term, LT^{-1}
R	vector of residuals
S	coefficient submatrix multiplying $i, j, k+1$ heads
S_f	freshwater specific storage, L^{-1}
S_s	saltwater specific storage, L^{-1}
t	elapsed time, T
T	transmissivity term, LT^{-1}
Th_f	length of open interval of a well penetrating freshwater, L
Th_t	total open interval of a well, L

x	spatial coordinate, L
y	spatial coordinate, L
Z	coefficient submatrix multiplying i,j,k-1 heads
z_a	elevation of top of confining layer, L
z_b	elevation of bottom of confining layer, L
z	elevation, L
α	parameter = 1 for an unconfined aquifer, = 0 for a confined aquifer
$\bar{\gamma}$	average specific weight, $ML^{-2}T^{-2}$
γ_a	specific weight of fluid above confining layer, $ML^{-2}T^{-2}$
γ_b	specific weight of fluid below confining layer, $ML^{-2}T^{-2}$
γ_f	freshwater specific weight, $ML^{-2}T^{-2}$
γ_s	saltwater specific weight, $ML^{-2}T^{-2}$
δ	= $\gamma_f / (\gamma_s - \gamma_f)$
λ	subscript = f for freshwater, = s for saltwater
μ	dynamic viscosity, $ML^{-1}T^{-1}$
μ_f	freshwater dynamic viscosity, $ML^{-1}T^{-1}$
μ_s	saltwater dynamic viscosity, $ML^{-1}T^{-1}$
Φ	vector of heads
Φ'	vector of heads at a node
Φ_a	hydraulic head above a confining layer, L
Φ_b	hydraulic head below a confining layer, L
Φ_f	freshwater head, L
$\bar{\Phi}_f$	vertically averaged freshwater head, L
Φ_s	saltwater head, L
$\bar{\Phi}_s$	vertically averaged saltwater head, L
ρ	fluid density, ML^{-3}
$\Delta\rho$	density difference = $\rho_s - \rho_f$, ML^{-3}
ζ_0	elevation of base of aquifer, L
ζ_1	elevation of interface, L
ζ_2	elevation of top of aquifer, L
ξ	vector of head changes
ω	weighting factor

THE COMPUTER MODEL SHARP, A QUASI-THREE-DIMENSIONAL FINITE-DIFFERENCE
MODEL TO SIMULATE FRESHWATER AND SALTWATER FLOW IN
LAYERED COASTAL AQUIFER SYSTEMS

by H. I. Essaid

ABSTRACT

This report documents the quasi-three-dimensional, finite-difference model, SHARP, which simulates freshwater and saltwater flow separated by a sharp interface in layered coastal aquifer systems. The model accommodates multiple aquifers separated by confining layers, with spatially variable porous media properties. The uppermost aquifer can be confined, unconfined or semi-confined with areally distributed recharge. Temporal variations in recharge and pumping are accounted for by multiple pumping periods. The boundary conditions which can be simulated in the model are: prescribed flux boundaries, constant freshwater head and/or constant saltwater head boundaries, and leaky head-dependent boundaries.

For each aquifer, the vertically integrated freshwater and saltwater flow equations are solved. These two equations are coupled by the boundary condition at the interface. Leakage between aquifers is calculated by applying Darcy's law. The resulting system of coupled, non-linear partial differential equations is discretized using an implicit finite-difference scheme. The discretized system of equations is solved using the strongly implicit procedure (SIP). The positions of the interface tip and toe, within the discretized finite-difference grid blocks, are tracked using linear extrapolation of the interface elevations calculated at grid points.

This documentation includes an overview of saltwater intrusion modeling approaches and the mathematical formulation of SHARP. The model is verified against experimental and analytical solutions, and sample areal and cross-sectional applications are presented.

INTRODUCTION

Purpose and Scope

Coastal aquifers are an important ground-water resource for urban and agricultural areas bordering seas. In some areas, coastal hydrogeological conditions can simply be represented by an individual unconfined, island, or confined aquifer (fig. 1). More commonly, the hydrogeologic setting in coastal environments is that of a sequence of layers with varying hydraulic properties combining confined and unconfined conditions. A few previously studied examples of such systems are found in the north Atlantic and Israeli coastal plains, and the Llobregat delta in Barcelona, Spain (Collins and Gelhar, 1971; Schmorak, 1967; Custodio, 1981). Protection of water quality in these aquifers during development requires an understanding of the dynamic relation between freshwater and adjacent saltwater.

In this report, the model SHARP, a quasi-three-dimensional, numerical finite-difference model which simulates freshwater and saltwater flow separated by a sharp interface in layered coastal aquifer - confining unit systems is documented. SHARP facilitates regional simulation of coastal ground-water conditions in layered systems, and includes the effects of saltwater dynamics on the freshwater flow system.

The model accommodates multiple aquifers, separated by confining units, with spatially variable porous media properties. The uppermost aquifer of the system may be confined, unconfined, or semi-confined with an areally distributed recharge. Temporal variations in recharge and pumping are accounted for by multiple pumping periods. The boundary conditions which may be simulated in the model are: prescribed flux boundaries, constant freshwater head and/or constant saltwater head boundaries, and leaky head-dependent boundaries in the uppermost aquifer. The program is written in FORTRAN 77.

An overview of coastal hydrogeologic conditions and saltwater intrusion modeling approaches is given in the first part of this report. The development of the equations solved in SHARP is presented in the model development section and is followed by an explanation of the solution technique used. Model verification for single-layer and multilayer cases is presented. The last part of the report describes the computer program, its application, and sample problems. Appendixes A and B give the details of the coefficients of the discretized flow equations and the strongly implicit solution procedure. The data input formats, program variables, and program listing are given in Attachments A through C.

Coastal Hydrogeologic Conditions

To illustrate coastal conditions, an idealized hydrogeologic section through a layered coastal aquifer system extending offshore to a submarine canyon outcrop is shown in figure 2. Under natural, undisturbed conditions, an equilibrium seaward hydraulic gradient exists within each aquifer with freshwater discharging to the sea (fig.2a). In the uppermost, unconfined aquifer the freshwater flows out to sea across the ocean floor. In the lower, confined aquifers the freshwater discharges to the sea by leaking

upward through the overlying layers, and (or) by flowing out the canyon outcrop. Within each layer, saltwater flows in from the sea and a wedge-shaped body of denser saltwater develops beneath the lighter freshwater. Under steady-state conditions a stationary interface is maintained, its shape and position being determined by the freshwater potential and gradient. In the case of one-layer systems, the sea water will essentially be static under steady-state conditions. In a layered system, if there is vertical leakage of freshwater into an overlying saltwater zone, this zone of mixed water will not be static.

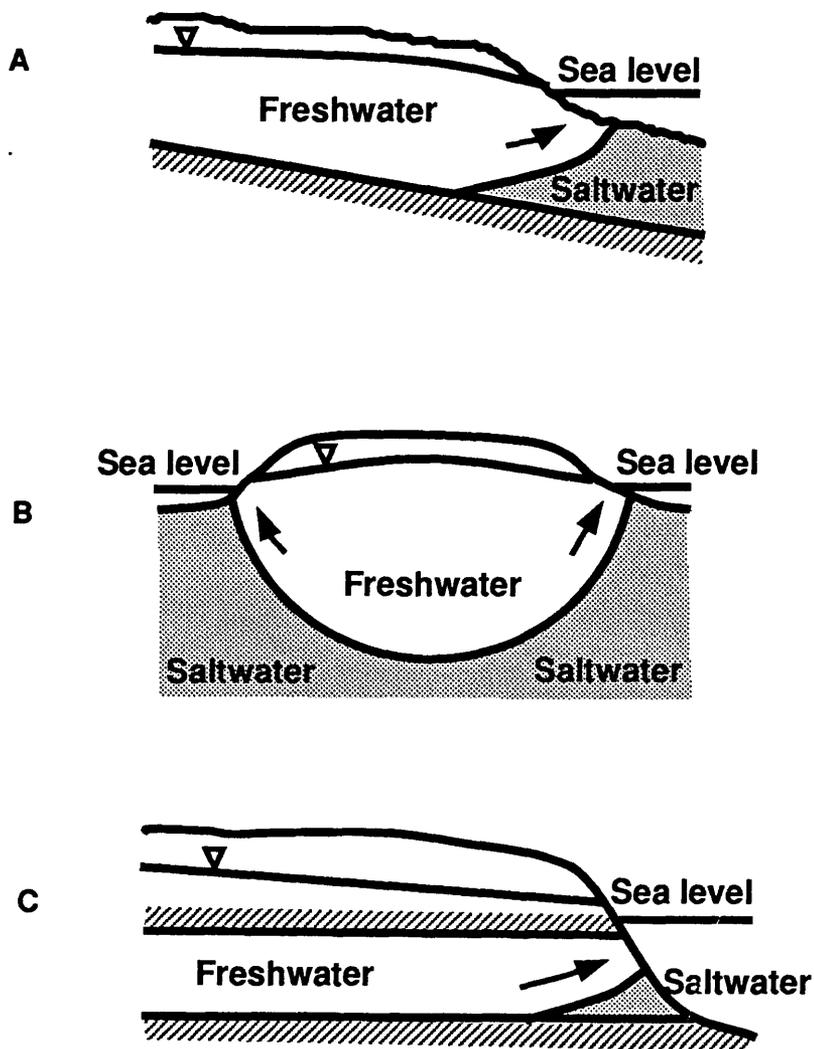


Figure 1.--Examples of hydrogeologic conditions in coastal aquifers:
 A. unconfined aquifer with an impermeable bottom,
 B. unconfined island aquifer with a free bottom, and
 C. confined aquifer.

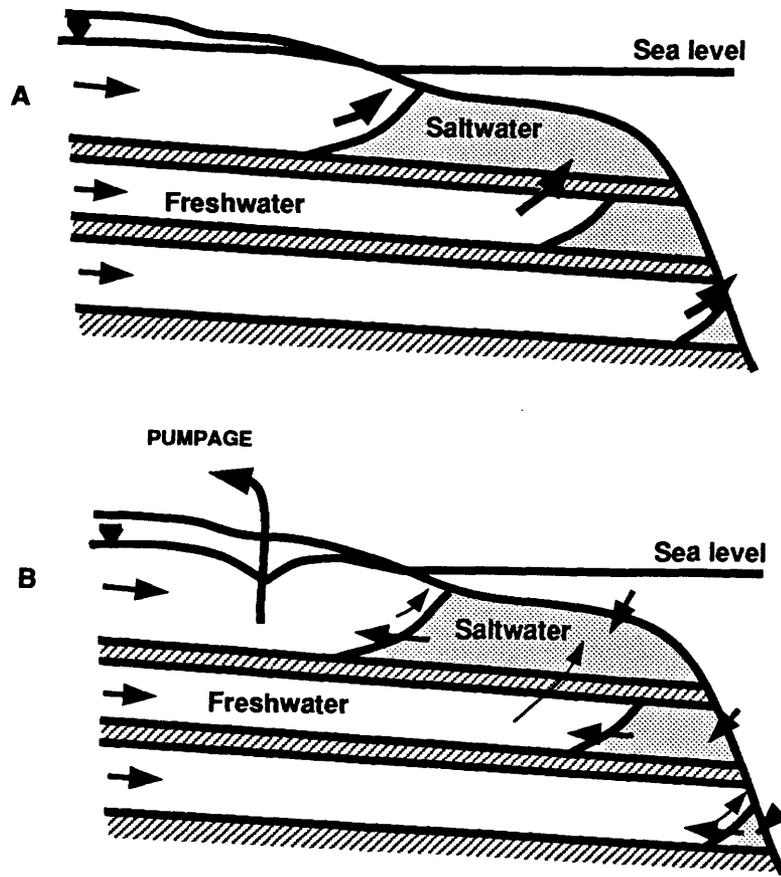


Figure 2.--Idealized cross-section of a layered coastal aquifer system showing paths of freshwater discharge and potential paths for saltwater intrusion:
 A. steady-state system with constant freshwater discharge offshore,
 B. transient system with intruding saltwater and inland interface movement.

In reality, the interface separating fresh and saltwater is a transition zone created by the mixing of waters due to the effects of diffusion and mechanical dispersion. Cooper (1959) and Kohout (1964) have shown that in the zone of mixing, the diluted saltwater is less dense than the original sea water, causing it to rise and move seaward along the interface (fig. 3). This induces a cyclic flow of saltwater from the sea, through the ocean floor, to the zone of mixing, and back to the sea. This cyclic flow occurs even under steady-state conditions.

Inland changes in recharge or discharge modify the flow within the freshwater region, inducing movement of the interface. Reduction in freshwater flow towards the sea causes the interface to move inland and results in the intrusion of saltwater into the aquifer. Conversely, an increase in freshwater flow pushes the interface seaward. The rate of

interface movement and the transient aquifer head response will depend on the boundary conditions and aquifer properties on both sides of the interface. The ease with which saltwater can move into, or out of, an aquifer system affects the rate of interface movement in response to changes in offshore freshwater discharge. In a layered system, saltwater can enter an aquifer by flowing through the aquifer outcrop, and/or leaking across the confining layers and ocean floor (fig. 2b). Therefore, in order to understand coastal systems, it is necessary to examine the dynamics of both the freshwater and saltwater flow domains.

Management of coastal ground-water resources requires an understanding of the physical dynamics of the phenomenon of saltwater intrusion. For this reason, considerable effort has been put into developing numerical models for saltwater intrusion that can represent the physical complexities, as well as the spatial and temporal variations inherent in coastal systems.

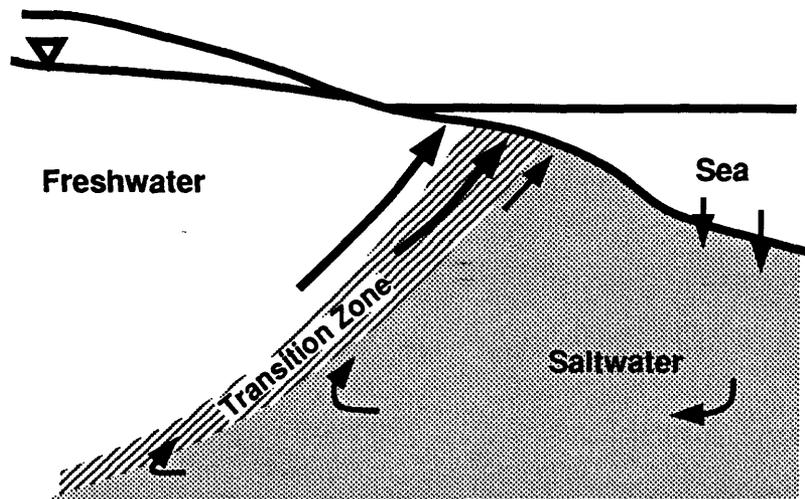


Figure 3.--Circulation of saltwater from the sea to the transition zone and back to the sea induced by mixing at the interface.

Saltwater Intrusion Modeling Approaches

Disperse and sharp interface approaches have been used to analyze saltwater intrusion in coastal aquifers (Reilly and Goodman, 1985). The disperse interface approach explicitly represents the transition zone, where there is mixing of freshwater and saltwater due to the effects of hydrodynamic dispersion (molecular diffusion and mechanical dispersion). The sharp interface approach simplifies the analysis by assuming that freshwater and saltwater do not mix and are separated by an abrupt interface. Both approaches have been used to develop numerical models to study and predict the flow of ground water in coastal aquifers. A summary of the numerical models that have been developed for simulation of saltwater intrusion problems is given in table 1.

Table 1.--Summary of numerical salt water intrusion models (DI-disperse interface, DD-density dependent, NDD-non-density-dependent, SI-sharp interface, F-fresh water only, FS-fresh and salt water, FD-finite difference, FE-finite element, BIM-boundary integral method, A2-D-areal two-dimensional, VCS-vertical cross-section, Q3-D-quasi-three-dimensional, 3-D-three-dimensional, TIME DEP.-time dependence, TR-transient, SS-steady state, DUP. APP.-Dupuit approximation, G-H APP.-Ghyben-Herzberg approximation)

GROUP	AUTHOR	APPROACH	METHOD	GEOMETRY	TIME DEP.	DUP. APP.	G-H APP.	FIELD APPLICATION
I-1	Reddell & Sunada, 1970	DI-NDD	FD	A2-D	TR	YES	NO	
	Bredehoeft & Pinder, 1973	DI-NDD	FD	A2-D	TR	YES	NO	Brunswick, Georgia
	Andrews, 1981	DI-NDD	FE	A2-D	TR	YES	NO	Costa de Hermosillo, Mexico
I-2	Pinder & Cooper, 1970	DI-DD	FD	VCS	TR	NO	NO	
	Volker & Rushton, 1982	DI-DD	FD	VCS	SS	NO	NO	
	Sanford & Konikow, 1985	DI-DD	FD	VCS	TR	NO	NO	
	Segol and others, 1975	DI-DD	FE	VCS	TR	NO	NO	Biscayne aquifer, Florida (Segol & Pinder, 1976)
	Lee & Cheng, 1974	DI-DD	FE	VCS	SS	NO	NO	Biscayne aquifer, Florida
	Frind, 1982a,b	DI-DD	FE	VCS	TR	NO	NO	
	Voss, 1984b	DI-DD	FE	VCS	TR	NO	NO	Southern Oahu, Hawaii (Souza & Voss, 1986)
	INTERA, 1979	DI-DD	FD	3-D	TR	NO	NO	
	Kipp, 1987	DI-DD	FD	3-D	TR	NO	NO	
	Huyakorn and others, 1987	DI-DD	FE	3-D	TR	NO	NO	
II-1	Fetter, 1972	SI-F	FD	A2-D	SS	YES	YES	South Fork, Long Island
	Anderson, 1976	SI-F	FD	VCS	TR	YES	YES	South Fork, Long Island
	Ayers & Vacher, 1983	SI-F	FD	A2-D	TR	YES	YES	Somerset Island, Bermuda
	Guswa & LeBlanc, 1985	SI-F	FD	3-D	SS	NO	YES	Cape Cod, Massachusetts
	Sapik, 1988	SI-F	FD	Q3-D	SS	YES	YES	
	Voss, 1984a	SI-F	FE	A2-D	TR	YES	YES	Southeast Oahu, Hawaii (Eyre, 1985)
	Taigbenu and others, 1984	SI-F	BIM	A2-D	TR	YES	YES	
Volker & Rushton, 1982	SI-F	BIM	VCS	SS	NO	YES		
II-2	Shamir & Dagan, 1971	SI-FS	FD	VCS	TR	YES	NO	
	Bonnet & Sauty, 1975	SI-FS	FD	A2-D	TR	YES	NO	Morrococ Atlantic Coast
	Mercer and others, 1980a,b	SI-FS	FD	A2-D	TR	YES	NO	Maui, Hawaii
	Polo & Ramis, 1983	SI-FS	FD	A2-D	TR	YES	NO	Spain
	Pinder & Page, 1977	SI-FS	FE	A2-D	TR	YES	NO	North Haven, New York
	Wilson & Sa da Costa, 1982	SI-FS	FE	A2-D	TR	YES	NO	Algarve, Portugal (Sa da Costa, 1986)
	Contractor, 1983	SI-FS	FE	A2-D	TR	YES	NO	North Guam
	Liu and others, 1981	SI-FS	BIM	VCS	TR	NO	NO	

Disperse Interface Approach

To represent the physics of a disperse interface separating freshwater and saltwater, the equations governing fluid flow must be solved in conjunction with the solute transport equation for a conservative chemical species (Bear, 1979):

$$\frac{\partial(n\rho)}{\partial t} = -\nabla \cdot \rho \mathbf{q} \quad \text{fluid mass balance,} \quad (1)$$

$$\mathbf{q} = -\frac{\mathbf{k}}{\mu}(\nabla p + \rho g \nabla z) \quad \text{Darcy's law,} \quad (2)$$

$$\frac{\partial(nc)}{\partial t} = \nabla \cdot n \mathbf{D}_h \cdot \nabla c - \nabla \cdot \mathbf{q}c \quad \text{transport equation,} \quad (3)$$

where: \mathbf{k} = permeability tensor (L^2);
 μ = $\mu(c)$ is dynamic viscosity ($ML^{-1}T^{-1}$);
 ρ = $\rho(c)$ is fluid density (ML^{-3});
 p = pressure ($ML^{-1}T^{-2}$);
 z = the vertical dimension (L);
 g = gravitational acceleration (LT^{-2});
 n = porosity;
 c = concentration (ML^{-3});
 \mathbf{D}_h = the hydrodynamic dispersion coefficient tensor (L^2T^{-1});
 \mathbf{q} = the specific discharge or Darcy velocity (LT^{-1});
 t = time (T); and
 $\nabla(\) = \frac{\partial(\)}{\partial x} \mathbf{x} + \frac{\partial(\)}{\partial y} \mathbf{y} + \frac{\partial(\)}{\partial z} \mathbf{z}$, where \mathbf{x} , \mathbf{y} and \mathbf{z} are unit vectors in the x-, y- and z- directions.

Because density is a function of concentration, these equations must be solved simultaneously to simulate coupled density-dependent fluid flow and solute transport. If the transition zone is very disperse and chloride concentrations gradients are low, the effects of variable density may be neglected. This simplification makes it possible to decouple the equations, solving first for the flow field and subsequently for the concentration field. Use of this approach, in conjunction with the Dupuit assumption of horizontal flow, is suitable for areal modeling of aquifer systems with low chloride concentration gradients, or when vertical resolution is not needed (Group I-1, table 1).

In many cases, however, the density gradient is significant and must be accounted for (Group I-2, table 1). Generally, the flow and transport equations are solved simultaneously by iterating between the two equations, increasing computational effort considerably. This has limited most solutions to two-dimensional vertical cross-sections. Three-dimensional,

density-dependent solute transport codes have been developed (INTERA, 1979; Huyakorn and others, 1987; Kipp, 1987; Andersen and others, 1988) but have restricted applicability for regional studies due to computational constraints.

Sharp Interface Approach

When the width of the transition zone is small relative to the thickness of the aquifer it can be assumed, for the purpose of analysis, that the saltwater and freshwater are immiscible fluids separated by a sharp interface. This approach reproduces the general position, shape, and behavior of the interface. These models couple the freshwater and saltwater flow domains through the interfacial boundary condition of continuity of flux and pressure. In three dimensions this boundary condition is highly non-linear (Bear, 1979), making the solution difficult. The problem can be simplified, however, by integrating the flow equations over the vertical and assuming horizontal flow within the aquifer. Sharp interface models generally fall into two categories: those that model coupled freshwater and saltwater flow (two-fluid approach), and those that model freshwater flow only (one-dynamic-fluid approach).

Badon-Ghyben (1889) and Herzberg (1901) related the freshwater head above sea level (Φ_f) to the depth to the interface below sea level (h_s) for a system in static equilibrium; that is, steady horizontal freshwater flow and stationary saltwater (fig. 4). At the interface, the pressure due to the overlying column of freshwater must be equivalent to that due to the column of saltwater; therefore, the following relation must hold:

$$h_s \gamma_s = (h_s + \Phi_f) \gamma_f , \quad (4)$$

or

$$h_s = \delta \Phi_f , \quad (5)$$

where $\delta = \gamma_f / (\gamma_s - \gamma_f)$ and γ_f , γ_s are the fresh and saltwater specific weights, respectively. For the common values of freshwater and saltwater densities (1.0 gm/cm^3 and 1.025 gm/cm^3 , respectively) the value of δ is 40, that is, the depth to the interface below sea level, is forty times the freshwater head. The sharp interface models that simulate flow in the freshwater region only, (Group II-1, table 1) incorporate the Ghyben-Herzberg relation (5) assuming that at each time step saltwater adjusts instantaneously to changes in the freshwater zone, and an equilibrium interface position is achieved.

In the two-fluid approach the freshwater and the saltwater flow equations, which are coupled by the interface boundary condition, are solved simultaneously (Group II-2, table 1). On the basis of the principle of continuity of pressure, the interface elevation can be expressed as a function of the freshwater head and the saltwater head. The movement of the interface is dictated by the freshwater and saltwater flow dynamics.

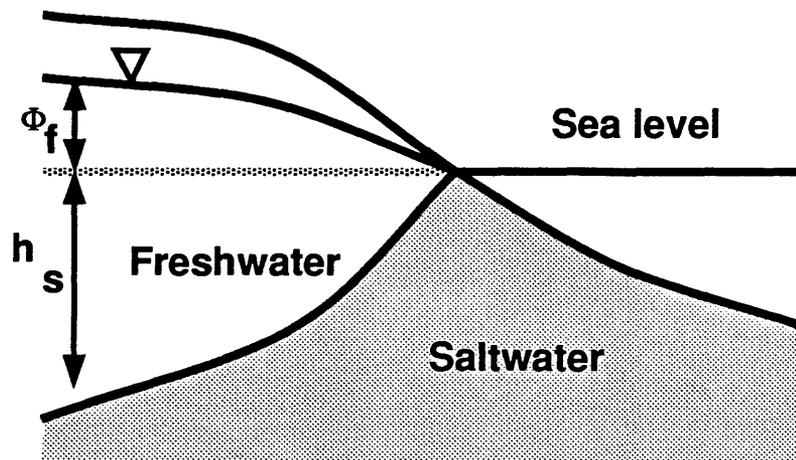


Figure 4.--The Ghyben-Herzberg interface model.

Discussion of Approaches

Each of the above approaches has advantages and limitations, and can be employed successfully only under the appropriate conditions. The dispersive interface is necessary in areas where the transition zone is wide. Density effects can be neglected when chloride concentration gradients are low and the governing equations can be solved areally on a basin-wide scale. However, when the flow is density-dependent, the vertical dimension must be included. Studies using this approach generally have been limited to two-dimensional vertical cross-sections due to computational constraints. There are also numerical instabilities and errors which are encountered when simulating the movement of a narrow concentration front, especially in zones where the transition zone approaches a sharp interface. Frind (1982a) showed that when a velocity-dependent dispersion coefficient is used, instabilities are encountered in areas of stagnant saltwater. Voss and Souza (1986, 1987) indicate that when flow is predominantly horizontal, the vertical discretization must be of the same order of magnitude as the transverse dispersivity in order to avoid introducing numerical dispersion.

The sharp interface approach, in conjunction with the application of the hydraulic approach (integration of the flow equations over the vertical), allows the problem to be reduced by one dimension. Thus, it can be applied areally to large physical systems. This approach does not give information concerning the nature of the transition zone; however, it does represent the overall flow dynamics of the system and will reproduce the general response of the interface to applied stresses. Volker and Rushton (1982) compared steady-state solutions for both the dispersive interface and the sharp interface approaches and showed that as the coefficient of hydrodynamic dispersion decreases, the two solutions approach each other.

Sharp interface models that simulate flow in the freshwater region only by incorporating the Ghyben-Herzberg approximation assume that the saltwater zone adjusts rapidly to the applied stresses. This may be a reasonable assumption in long-term studies if the interface can respond quickly to applied stresses. However, to reproduce the short-term response of a coastal aquifer, it is necessary to include the influence of saltwater flow (Essaid, 1986).

Individually, none of the above approaches can fully characterize the behavior and complexities of coastal aquifer systems. The choice of the approach used to model a particular system will depend on the nature of the system as well as the goals of the modeling effort. The sharp interface approach can represent the overall flow characteristics of the system, but cannot give details concerning the nature of the transition zone. When studying an aquifer system, it is important to first understand its overall behavior before examining smaller scale effects. Therefore, the ideal characterization of such systems may involve a two-step process integrating the sharp interface and dispersed interface modeling approaches.

The Multilayer Freshwater - Saltwater Flow Model SHARP

Saltwater intrusion models have generally been limited to single aquifer problems, although a few have been developed for more complicated geometries (table 1). The two-dimensional, areal, sharp interface, numerical models of Mercer and others (1980a,b) and Voss (1984a) allow for an overlying leaky confining unit. Fetter (1972) and Anderson (1976) represented layers with different hydraulic conductivities by a single layer with an averaged conductivity value. Density-dependent solute transport models that can handle an overlying leaky confining unit have been developed (Frind, 1982a,b; Voss, 1984b; Huyakorn and others, 1987). Mercer and others (1986) used the INTERA (1979) code to build a quasi-three-dimensional solute transport model of Volusia County, Florida by neglecting the density dependence of the problem. Some analytical solutions have dealt with the problem of the position of a steady-state, sharp interface in a layered or stratified aquifer (Collins and others, 1972; Rumer and Shiau, 1968; Mualem, 1973; Collins and Gelhar, 1971 and 1977; Mualem and Bear, 1974). The only sharp interface model of coupled freshwater and saltwater flow presented for a multilayered case is that of Bear and Kapuler (1981), a cross-sectional model for two aquifers separated by a thin impervious layer. Sapik (1988) has developed a multilayered model for freshwater flow only by incorporating the Ghyben-Herzberg approximation into a three-dimensional ground-water flow model.

SHARP is a quasi-three-dimensional, finite-difference model that simulates coupled freshwater and saltwater flow separated by a sharp interface in multilayered coastal systems. It can be used for both areal (regional) and cross-sectional studies.

MODEL DEVELOPMENT

Vertical Integration of the Coupled Freshwater - Saltwater Flow Equations

For each aquifer within a layered coastal system two flow domains must be considered: freshwater and saltwater (fig. 5). The two domains are coupled because they share a common boundary at the interface (Bear, 1979). Within each flow domain the equation of continuity must hold:

$$S_f \frac{\partial \Phi_f}{\partial t} = -\nabla \cdot \mathbf{q}_f \quad \text{freshwater flow domain,} \quad (6a)$$

$$S_s \frac{\partial \Phi_s}{\partial t} = -\nabla \cdot \mathbf{q}_s \quad \text{saltwater flow domain,} \quad (6b)$$

where:

- Φ_f = $z + p_f/\gamma_f$, the freshwater head (L);
- Φ_s = $z + p_s/\gamma_s$, the saltwater head (L);
- z = elevation (L);
- p_f, p_s = the fresh and saltwater fluid pressures ($ML^{-1}T^{-2}$), respectively;
- γ_f, γ_s = the fresh and saltwater specific weights ($ML^{-2}T^{-2}$);
- S_f, S_s = the fresh and saltwater specific storages (L^{-1}); and
- $\mathbf{q}_f, \mathbf{q}_s$ = the fresh and saltwater specific discharges (LT^{-1}).

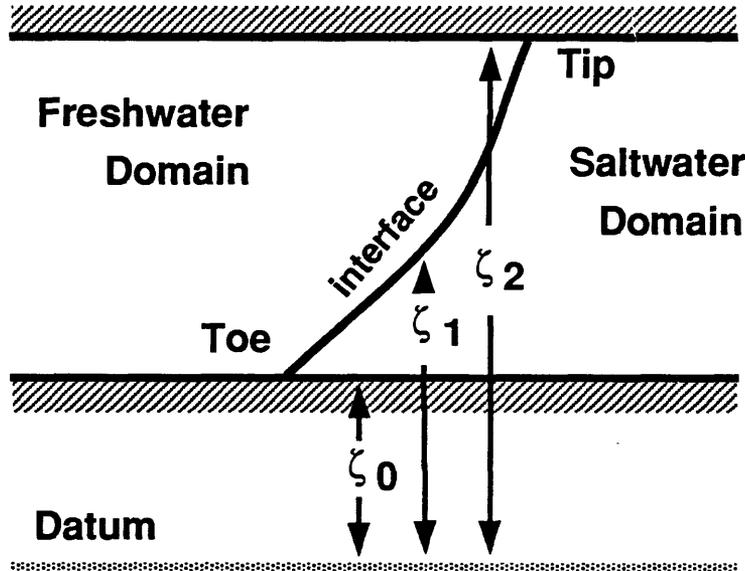


Figure 5.--Freshwater and saltwater flow domains in a confined aquifer

Equations (6a) and (6b) can be integrated over the vertical dimension, within their respective domains. In the vicinity of the interface, the lower boundary of the saltwater domain is the bottom of the aquifer which has an elevation of ζ_0 . The interface is the upper boundary of the saltwater domain and its elevation is given by ζ_1 . The freshwater domain is bounded on the bottom by the interface (ζ_1). In the case of a confined aquifer, the elevation of the top of the aquifer (ζ_2) is the upper boundary. For an unconfined aquifer, the top of the freshwater domain (ζ_2) is given by the water table elevation.

Vertical integration of the freshwater and saltwater flow equations implies the Dupuit approximation of vertical equipotential lines and horizontal flow within the aquifer. This approximation reduces the problem to two spatial dimensions (x and y). For a confined aquifer, integrating the freshwater flow equation from the interface (ζ_1) to the top of the aquifer (ζ_2) and substituting Darcy's Law yields:

$$\int_{\zeta_1}^{\zeta_2} (\nabla \cdot \mathbf{q}_f + S_f \frac{\partial \Phi_f}{\partial t}) dz \approx \nabla' \cdot (-B_f \underline{\mathbf{K}}_f' \cdot \nabla' \bar{\Phi}_f) + S_f B_f \frac{\partial \bar{\Phi}_f}{\partial t} - \mathbf{q}_f' |_{\zeta_2} \cdot \nabla' \zeta_2 + q_{fz} |_{\zeta_2} + \mathbf{q}_f' |_{\zeta_1} \cdot \nabla' \zeta_1 - q_{fz} |_{\zeta_1} = 0, \quad (7a)$$

where: $\bar{\Phi}_f = \frac{1}{B_f} \int_{\zeta_1}^{\zeta_2} \Phi_f dz \approx \Phi_f$, the vertically averaged freshwater head (L);

$\mathbf{q}_f' = \frac{1}{B_f} \int_{\zeta_1}^{\zeta_2} \mathbf{q}_f dz \approx -\underline{\mathbf{K}}_f' \cdot \nabla' \bar{\Phi}_f$, the vertically averaged freshwater flux (LT^{-1});

$\mathbf{q}' = q_x \mathbf{i}_x + q_y \mathbf{i}_y$;

$\nabla'(\) = \frac{\partial(\)}{\partial x} \mathbf{x} + \frac{\partial(\)}{\partial y} \mathbf{y}$;

$B_f = \zeta_2 - \zeta_1$, thickness of the freshwater zone (L);

q_{fz} = the vertical component of freshwater flux (LT^{-1}); and

$\underline{\mathbf{K}}_f' = \underline{\mathbf{K}}_f'(x, y)$ = the vertically averaged freshwater hydraulic conductivity (LT^{-1}).

Similarly, the vertically integrated saltwater equation becomes:

$$\int_{\zeta_0}^{\zeta_1} (\mathbf{V} \cdot \mathbf{q}_s + S_s \frac{\partial \Phi_s}{\partial t}) dz \approx \mathbf{V}' \cdot (-B_s \underline{\mathbf{K}}_s' \cdot \mathbf{V}' \tilde{\Phi}_s) + S_s B_s \frac{\partial \tilde{\Phi}_s}{\partial t} - \mathbf{q}_s' |_{\zeta_1} \cdot \mathbf{V}' \zeta_1 + q_{sz} |_{\zeta_1} + \mathbf{q}_s' |_{\zeta_0} \cdot \mathbf{V}' \zeta_0 - q_{sz} |_{\zeta_0} = 0, \quad (7b)$$

where: $\tilde{\Phi}_s = \frac{1}{B_s} \int_{\zeta_0}^{\zeta_1} \Phi_s dz \approx \Phi_s$, the vertically averaged saltwater head (L);

$\mathbf{q}_s' \approx \frac{1}{B_s} \int_{\zeta_0}^{\zeta_1} \mathbf{q}_s dz \approx -\underline{\mathbf{K}}_s' \cdot \mathbf{V}' \Phi_s$, the vertically averaged saltwater flux (LT^{-1});

$B_s = \zeta_1 - \zeta_0$, the thickness of the saltwater zone (L);

q_{sz} = the vertical component of saltwater flux (LT^{-1}); and

$\underline{\mathbf{K}}_s' = \underline{\mathbf{K}}_s'(x,y)$ = the vertically averaged saltwater hydraulic conductivity (LT^{-1}).

Boundary Terms

The last four terms appearing in equations (7a) and (7b) represent the boundary conditions at the top and bottom of each domain. These terms are given by the boundary conditions at the interface, and at the top and bottom of the aquifer. To satisfy continuity of pressure at the interface, the fluid pressure in the freshwater domain must equal the fluid pressure in the saltwater domain:

$$p_f = (\Phi_f - \zeta_1) \gamma_f = p_s = (\Phi_s - \zeta_1) \gamma_s, \quad (8)$$

where ζ_1 is the interface elevation. Solving for the interface elevation:

$$\zeta_1 = (1+\delta) \Phi_s - \delta \Phi_f, \quad (9)$$

where $\delta = \gamma_f / (\gamma_s - \gamma_f)$.

The geometry of the interface can be described in terms of the elevation (z) and the freshwater and saltwater heads:

$$F \equiv z - \zeta_1 = z - (1+\delta) \Phi_s + \delta \Phi_f = 0. \quad (10)$$

where: q_1 = vertical leakage, positive upwards (LT^{-1});
 k' = vertical permeability of the confining layer (L^2);
 μ = dynamic viscosity ($ML^{-1}T^{-1}$);
 p_a, p_b = fluid pressures above and below the confining layer,
 respectively ($ML^{-1}T^{-2}$);
 ρ = fluid density (ML^{-3});
 Δz = thickness of the confining layer (L); and
 g = gravitational acceleration (LT^{-2}).

Using definitions for hydraulic head ($\Phi = z + p/\gamma$), specific weight ($\gamma = \rho g$), and freshwater hydraulic conductivity of the confining layer ($K' = k' \rho_f g / \mu$), equation (13) can be rewritten as:

$$q_1 = - \frac{K'}{B' \gamma_f} [\gamma_a (\Phi_a - z_a) - \gamma_b (\Phi_b - z_b) + \gamma B'] , \quad (14)$$

where: B' = $z_a - z_b$, thickness of the confining layer (L);
 Φ_a, Φ_b = hydraulic heads above and below the confining layer (L),
 respectively; and
 z_a, z_b = elevations of the top and bottom of the confining layer (L).

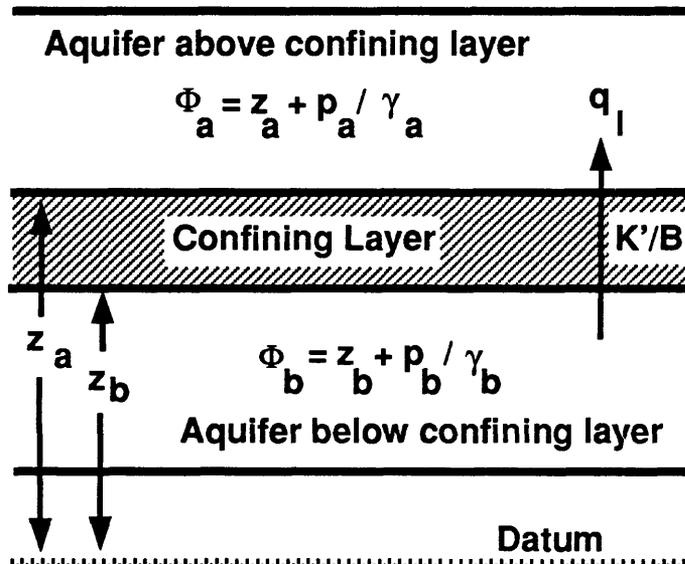


Figure 6.--Leakage through a confining layer.

When freshwater occurs on one side of the confining layer and saltwater occurs on the other, the density distribution within the confining layer depends on the direction of flow. This is unknown until the equation is solved, so, for simplicity, the final term in (14) has been approximated by an average of the specific weights above the confining layer (γ_a) and below (γ_b):

$$\gamma = \bar{\gamma} = (\gamma_a + \gamma_b) / 2 . \quad (15)$$

Rearranging, the general form of the leakage term becomes:

$$q_1 = - \frac{K'}{B'} \left[\frac{\gamma_a}{\gamma_f} \Phi_a - \frac{\gamma_b}{\gamma_f} \Phi_b + \frac{(\gamma_b - \gamma_a)}{\gamma_f} \frac{(z_b + z_a)}{2} \right] , \quad (16)$$

and K'/B' is the leakance of the confining layer (T^{-1}). The first two terms in (16) represent the equivalent freshwater heads above and below the confining layer. The third term incorporates the effect of gravity on the water in the confining layer. For the case of waters with equal density above and below the confining layer, this equation reduces to:

$$q_1 = - \left(\frac{K'}{B'} \right) (\Phi_a - \Phi_b) . \quad (17)$$

The Integrated Equations

Introducing these boundary conditions and accounting for source/sink terms, the vertically integrated equations for freshwater and saltwater flow, respectively, become:

$$\begin{aligned} S_f B_f \frac{\partial \Phi_f}{\partial t} + n \alpha \frac{\partial \Phi_f}{\partial t} + [n \delta \frac{\partial \Phi_f}{\partial t} - n(1+\delta) \frac{\partial \Phi_s}{\partial t}] \\ (1) \quad (2) \quad (3) \\ = \frac{\partial}{\partial x} (B_f K_{fx} \frac{\partial \Phi_f}{\partial x}) + \frac{\partial}{\partial y} (B_f K_{fy} \frac{\partial \Phi_f}{\partial y}) + Q_f + Q_{1f} , \quad (18a) \\ (4) \quad (4) \quad (5) \quad (6) \end{aligned}$$

$$\begin{aligned} S_s B_s \frac{\partial \Phi_s}{\partial t} + [n(1+\delta) \frac{\partial \Phi_s}{\partial t} - n \delta \frac{\partial \Phi_f}{\partial t}] \\ (1) \quad (3) \\ = \frac{\partial}{\partial x} (B_s K_{sx} \frac{\partial \Phi_s}{\partial x}) + \frac{\partial}{\partial y} (B_s K_{sy} \frac{\partial \Phi_s}{\partial y}) + Q_s + Q_{1s} , \quad (18b) \\ (4) \quad (4) \quad (5) \quad (6) \end{aligned}$$

where: K_{fx}, K_{sx} = fresh and saltwater hydraulic conductivities in the x-direction (LT^{-1});
 K_{fy}, K_{sy} = fresh and saltwater hydraulic conductivities in the y-direction (LT^{-1});
 Q_f, Q_s = fresh and saltwater source/sink terms (LT^{-1});
 Q_{1f}, Q_{1s} = fresh and saltwater leakage terms (LT^{-1}) given by equation (14);
 $\alpha = 1$ for an unconfined aquifer, $= 0$ for a confined aquifer;
and all other variables are as defined earlier.

In equations (18a, b) the type (1) terms represent the change in elastic storage within each domain. The type (2) term represents the change in freshwater storage due to drainage at the water table, the type (3) terms represent the change in storage within each domain due to movement of the interface, the type (4) terms represent the divergence of the fluxes in the x and y directions, the type (5) terms (recharge, pumpage) and type (6) terms (leakage) represent the sources and sinks to the aquifer.

Equations (18a) and (18b) represent two coupled, parabolic partial differential equations that must be solved simultaneously for the freshwater head (Φ_f) and the saltwater head (Φ_s). Once these values are obtained the interface elevation (ζ_1) can be calculated from (eq. 9):

$$\zeta_1 = (1+\delta)\Phi_s - \delta\Phi_f .$$

In regions away from the interface, only one type of fluid (freshwater or saltwater) is present in the aquifer and the flow is described by the appropriate single equation without the interface (type (3)) storage terms.

Numerical Form of the Freshwater and Saltwater Flow Equations

The continuous spatial and temporal derivatives of the freshwater and saltwater flow equations are discretized using finite-difference methods. Spatial discretization is achieved by using a block-centered finite-difference grid that allows for variable grid spacing (fig. 7). An implicit scheme that is backward in time has been adopted to ensure stability.

In the development of the finite-difference approximations that follows, the terms in the flow equations have been multiplied by the grid block area. Spatial subscripts are indicated only when differing from i, j, or k, for example freshwater head at block (i,j-1,k) is referred to as $\Phi_{f,j-1}$.

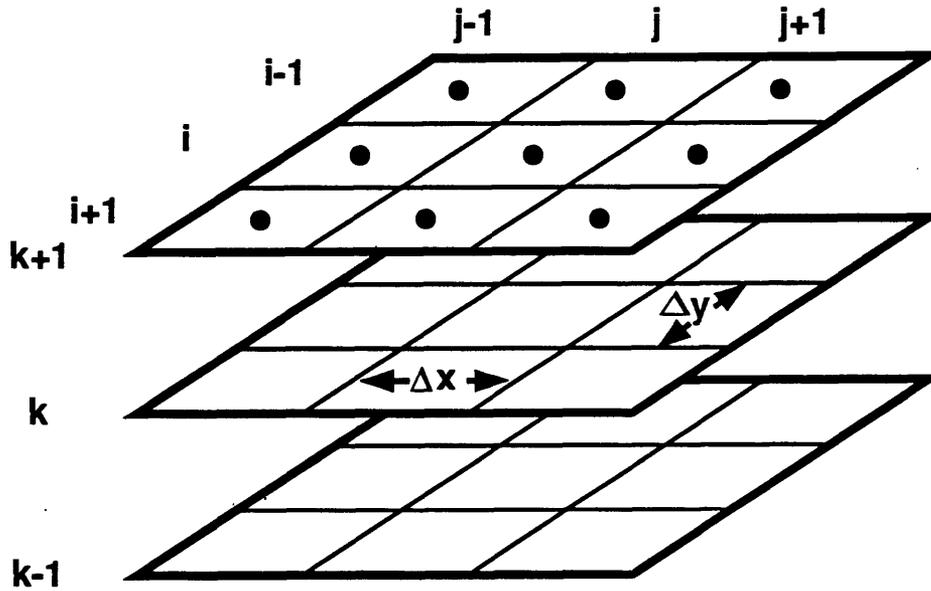


Figure 7.--Block-centered finite-difference grid.

Finite-Difference Approximations of Spatial and Temporal Derivatives

The second-order approximation for the space derivative in the x-direction at grid node (i,j,k) and time level n is given by:

$$A \left[\frac{\partial}{\partial x} (B_{\lambda} K_{\lambda x} \frac{\partial \Phi_{\lambda}}{\partial x}) \right]^n \approx T_{\lambda x, j+1/2}^n \Delta y (\Phi_{\lambda, j+1} - \Phi_{\lambda})^n - T_{\lambda x, j-1/2}^n \Delta y (\Phi_{\lambda} - \Phi_{\lambda, j-1})^n, \quad (19)$$

where $\lambda = 'f'$ for freshwater flow and $'s'$ for saltwater flow, and A is the grid block area ($A = \Delta x \Delta y$). The transmissivity terms at the block boundaries are given by:

$$T_{\lambda x, j+1/2}^n = B_{\lambda, j+1/2}^n \left(\frac{K_{\lambda x}}{\Delta x} \right)_{j+1/2}, \quad (20a)$$

and

$$T_{\lambda x, j-1/2}^n = B_{\lambda, j-1/2}^n \left(\frac{K_{\lambda x}}{\Delta x} \right)_{j-1/2}. \quad (20b)$$

The thicknesses at the block boundaries are linearly interpolated on the basis of adjacent nodal values, and the conductivity terms are estimated using the harmonic mean of the nodal values. The central difference approximations for the space derivative in the y-direction are made in the same manner and the result is:

$$A \left[\frac{\partial}{\partial y} (B_{\lambda} K_{\lambda y} \frac{\partial \Phi_{\lambda}}{\partial y}) \right]^n \approx T_{\lambda y_{i+1/2}}^n \Delta x (\Phi_{\lambda_{i+1}} - \Phi_{\lambda})^n - T_{\lambda y_{i-1/2}}^n \Delta x (\Phi_{\lambda} - \Phi_{\lambda_{i-1}})^n. \quad (21)$$

The time derivatives of the freshwater and saltwater potentials in equations (18a) and (18b) are approximated by using a backward difference:

$$\frac{\partial \Phi_{\lambda}}{\partial t} \approx \frac{\Phi_{\lambda}^n - \Phi_{\lambda}^{n-1}}{\Delta t}. \quad (22)$$

Sources and Sinks

The potential sources (or sinks) of freshwater and saltwater in a block are pumpage (or injection), recharge, and leakage from the overlying or underlying confining layers. The proportion of freshwater and saltwater extracted from a well depends on the position of the interface relative to the elevation of the screened interval of the well (fig. 8). The rate of freshwater extraction at a node is determined by a linear apportionment of the total extraction from the well based on the proportion of the open interval of the well penetrating the freshwater zone:

$$P_f^n \approx \frac{Th_f^n}{Th_t} P_t, \quad (23)$$

where: P_f^n = freshwater pumpage from grid block ijk at time level n
(L^3/T);

Th_f^n = length of the open interval penetrating the freshwater zone
at time level n (L);

Th_t = total open interval of the well (L); and

P_t = total pumpage from the well (L^3/T).

The saltwater pumpage from the well at time level n (P_s^n) is given by:

$$P_s^n \approx P_t - P_f^n. \quad (24)$$

Positive values of P_t represent extraction of water from a well, negative values represent injection of water into a well. Using equations (23) and (24), the source/sink terms in (18a) and (18b) become:

$$A Q_f \approx \alpha N \Delta x \Delta y - \left(\frac{Th_f P_t}{Th_t} \right)^n, \quad (25a)$$

and

$$A Q_s \approx \left(P_t - \frac{Th_f P_t}{Th_t} \right)^n, \quad (25b)$$

where N is the recharge rate (LT^{-1}). Recharge is not allowed at confined grid blocks where $\alpha = 0$.

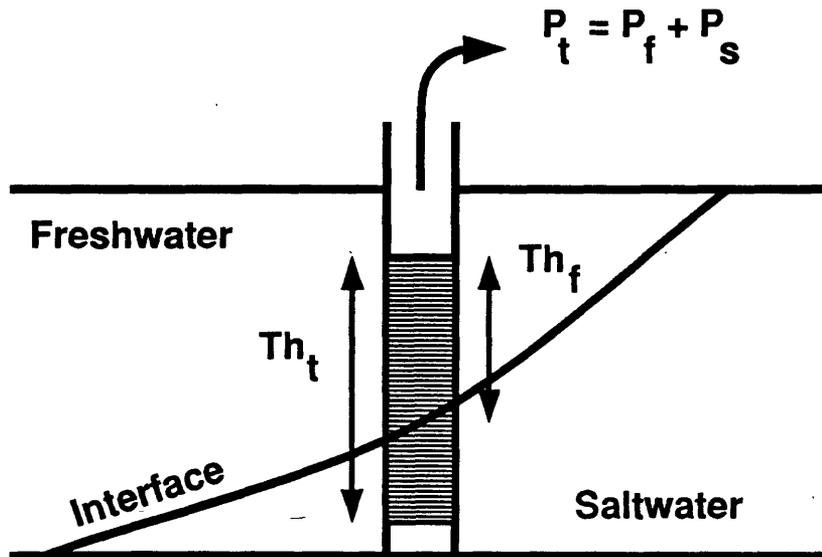


Figure 8.--A well penetrating freshwater and saltwater in an aquifer (Th_t is the total open interval of the well, Th_f is the length of the open interval penetrating freshwater).

Interface Tip and Toe Tracking

To develop a sharp interface model for freshwater and saltwater flow in layered coastal aquifers, representation of the moving interface within a discretized system is necessary. The position of the interface tip (the intersection of the interface with the top of the aquifer) and the interface toe (the intersection of the interface with the bottom of the aquifer) will not always coincide with the block or element boundaries. Shamir and Dagan (1971) overcame this for a vertical cross-section by using a moving grid. At each time step the new position of the toe of the interface was calculated using a linear extrapolation, and rezoning of the grid was

performed in order to align the block boundary with the toe. This same approach was used by Bear and Kapuler (1981) for the case of two aquifers separated by an impermeable layer. Wilson and Sa da Costa (1982) incorporated an indirect toe tracking algorithm into a fixed grid finite-element model. Other sharp interface models have made no attempt to track the interface tip and toe positions. SHARP incorporates a tip and toe tracking algorithm based on a weighted extrapolation of the interface slope at finite-difference blocks containing the tip and toe.

To determine the net freshwater and saltwater leakage into a block, the extent of freshwater and saltwater in contact with the top and bottom of the block must be known. This is achieved by determining the positions of the interface tip and toe within the finite-difference grid for each aquifer (fig. 9). The tip is located by linearly projecting the interface in the x- and y- directions, based on the interface slope, until it intersects the top of the aquifer. Similarly, at the toe the interface is projected until it intersects the bottom of the aquifer. Tip projection is carried out at locations where there is transition from a block containing some freshwater to a block containing no freshwater. Similarly, toe projection is carried out where there is a transition from a block containing some saltwater to a block containing no saltwater.

In the vicinity of the tip and toe, the finite-difference approximation of the interface slope in the x- and y- directions can be obtained by differentiating equation (9) with respect to x and y. The slope is then approximated based on weighted freshwater and saltwater head derivatives as follows:

$$\begin{aligned} \frac{\partial \zeta_1}{\partial x} &= (1+\delta) \frac{\partial \Phi_s}{\partial x} - \delta \frac{\partial \Phi_f}{\partial x} \\ &\approx (1+\delta) \left[(1-\omega) \frac{\partial \Phi_s}{\partial x} \Big|_{j+1/2} + \omega \frac{\partial \Phi_s}{\partial x} \Big|_{j-1/2} \right] - \\ &\quad \delta \left[(1-\omega) \frac{\partial \Phi_f}{\partial x} \Big|_{j+1/2} + \omega \frac{\partial \Phi_f}{\partial x} \Big|_{j-1/2} \right], \end{aligned} \quad (26a)$$

$$\begin{aligned} \frac{\partial \zeta_1}{\partial y} &= (1+\delta) \frac{\partial \Phi_s}{\partial y} - \delta \frac{\partial \Phi_f}{\partial y} \\ &\approx (1+\delta) \left[(1-\omega) \frac{\partial \Phi_s}{\partial y} \Big|_{i+1/2} + \omega \frac{\partial \Phi_s}{\partial y} \Big|_{i-1/2} \right] - \\ &\quad \delta \left[(1-\omega) \frac{\partial \Phi_f}{\partial y} \Big|_{i+1/2} + \omega \frac{\partial \Phi_f}{\partial y} \Big|_{i-1/2} \right], \end{aligned} \quad (26b)$$

where:
$$\frac{\partial \Phi_\lambda}{\partial x} \Big|_{j+1/2} = \frac{\Phi_{\lambda, j+1} - \Phi_\lambda}{0.5(\Delta x_{j+1} + \Delta x)}$$

and

$$\frac{\partial \Phi_{\lambda}}{\partial x} \Big|_{j-1/2} = \frac{\Phi_{\lambda} - \Phi_{\lambda_{j-1}}}{0.5(\Delta x + \Delta x_{j-1})},$$

are the derivatives of freshwater and saltwater heads in front of, and behind the interface tip or toe in the x-direction. Derivatives in the y-direction are calculated in a similar manner.

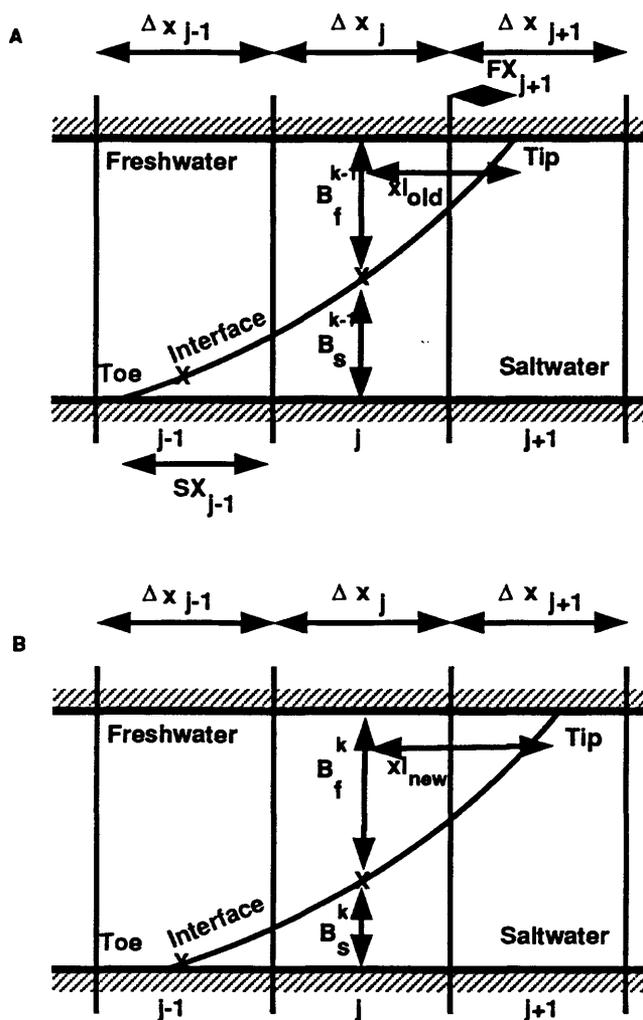


Figure 9.--Interface tip projection in the positive x-direction and toe projection in the negative x-direction (X's represent the calculated interface elevations at center of finite-difference blocks).

- A. The projected interface at iteration k-1, and the FX and SX values used in interface projection calculations for iteration k.
- B. The new projected interface position at iteration level k.

The weighting of factor ω is necessary to prevent abrupt changes in slope as the interface tip or toe crosses from one block to another, because the slope may change rapidly from $j-1/2$ to $j+1/2$. To obtain smooth movement of the interface, ω varies from 0 to 1 as the interface moves. The weight of a derivative increases as the interface tip or toe moves farther into the interval over which it is calculated. For example, for interface tip projection in the positive x-direction (indicated by a + subscript and illustrated in figure 9), the weighting factor is given by:

$$\begin{aligned} \omega_+ &= 1-2(FX_{j+1}) & \text{for } FX_{j+1} \leq 0.5 , \\ \omega_+ &= 0 & \text{for } FX_{j+1} > 0.5 , \end{aligned} \quad (27)$$

where FX_{j+1} is the distance to the interface tip in block $i, j+1, k$ expressed as a ratio of Δx_{j+1} , and the interface slope is obtained from (26a).

The new position of the interface tip is obtained by projecting the interface, based on its slope, until it intersects the top of the aquifer. The slope of the top of the aquifer in the positive x-direction is given by:

$$\left[\frac{\partial Z_{\text{top}}}{\partial x} \right]_+ = \frac{Z_{\text{top } j+1} - Z_{\text{top } j}}{0.5(\Delta x_j + \Delta x_{j+1})} . \quad (28)$$

The new projected distance in the positive x-direction, measured from the center of block ijk to the interface tip, for the current iteration is then calculated using equations (26) and (28) as follows:

$$x1_{\text{new}} = B_f / \left[\left(\frac{\partial \zeta}{\partial x} \right)_+ - \left(\frac{\partial Z_{\text{top}}}{\partial x} \right)_+ \right] , \quad (29)$$

where B_f is the freshwater thickness in block ijk . The final projected distance is a weighted average of the old and new values:

$$x1 = \text{WFAC} * x1_{\text{new}} + (1-\text{WFAC}) * x1_{\text{old}} , \quad (30)$$

where $x1_{\text{old}}$ is the projected interface position from the previous iteration and WFAC is a user specified weighting factor. A WFAC value of 0.5 will give equal weighting to both values, a value greater than 0.5 will give the new interface position a greater weight. Averaging the old and new values smooths out interface tip and toe movement over time. Generally, it is recommended to use a value of 0.5, as values greater than this may lead to oscillations in tip and toe positions, resulting in an unstable solution.

Following interface projection, the block boundary transmissive coefficients are readjusted based on the new interpolated freshwater and saltwater thicknesses at the block boundaries. The new projected interface tip position is then expressed as a ratio of the block dimension Δx (FXN).

For $x_1 \leq 0.5\Delta x_j$:

$$FXN_j = (x_1 + 0.5\Delta x_j)/\Delta x_j ,$$

$$FXN_{j+1} = 0.0 ,$$

and for $x_1 > 0.5\Delta x_j$,

$$FXN_j = 1.0 ,$$

$$FXN_{j+1} = (x_1 - 0.5\Delta x_j)/\Delta x_{j+1} .$$

This procedure is repeated in the negative x-direction with:

$$\omega_- = 2(FX_{j-1}) \quad \text{for } FX_{j-1} \leq 0.5 ,$$

$$\omega_- = 1 \quad \text{for } FX_{j-1} > 0.5 ,$$

to obtain projections of the interface where it extends past a block center in the negative x-direction. The interface projection is then repeated in the positive and negative y-directions to obtain FYN.

The interface toe is projected in a similar manner, but with a different weighting scheme which results in a more stable movement of the interface toe. The equations in the positive x-direction are:

$$\omega = \frac{x_{1_old}}{0.5(\Delta x_j + \Delta x_{j+1})} , \quad (31)$$

where x_{1_old} is taken from the projected interface position from the previous iteration, and the interface slope is calculated using (26b). The slope of the bottom of the aquifer in the positive x-direction is given by:

$$\left[\frac{\partial Z_{bot}}{\partial x} \right]_+ = \frac{Z_{bot_{j+1}} - Z_{bot_j}}{0.5(\Delta x_j + \Delta x_{j+1})} . \quad (32)$$

The new positive x-direction projected distance for this iteration is given by:

$$x_{1_new} = B_s / \left[\left(\frac{\partial Z_{bot}}{\partial x} \right)_+ - \left(\frac{\partial \zeta_1}{\partial x} \right)_+ \right] , \quad (33)$$

where B_s is the thickness of the saltwater zone. Equation (33) is used to calculate SXN, the interface toe projected distance expressed as a fraction of Δx . The entire procedure is repeated in the negative x-direction (as shown in figure 9), and the positive and negative y-directions to obtain SYN.

When the projections of the interface tip and toe in the x- and y-directions are completed, the fractions of the top and bottom of a block in contact with freshwater and saltwater are calculated. At each grid block the fraction of the top of the block in contact with freshwater is (FAREA) and the remaining fraction is in contact with saltwater (1-FAREA). The fraction of the bottom of the block in contact with saltwater is given by

(SAREA), and (1-SAREA) represents the area of freshwater (fig. 10). Hence, if FAREA=0 the block contains only saltwater; if SAREA=0 the block contains only freshwater; if FAREA=SAREA=1 the interface passes through the block and both freshwater and saltwater are present; and if 0<FAREA<1 the interface tip is within the block, or if 0<SAREA<1 the interface toe is within the block.

The block freshwater and saltwater area factors (FAREA and SAREA) are computed by combining the x- and y-direction projections (FXN, FYN, SXN and SYN) in a manner to ensure smooth variation in FAREA and SAREA (between 0 and 1) as the interface moves. This is accomplished as follows. If FXN and FYN are less than or equal to 0.5, then:

$$FAREA = [1 - (FXN * FYN)^{1/2}] (FXN + FYN) , \quad (34)$$

which gives a value of FAREA which varies between 0 and 0.5, otherwise if FXN and FYN are greater than 0.5:

$$FAREA = (FXN * FYN) \left[1 + \frac{1}{(FXN * FYN + 0.75)^7} \right] , \quad (35)$$

and FAREA varies between 0.5 and 1. For the special case of only one projection factor being less than 0.5, equation (34) is used and the resulting FAREA can be greater than 0.5. The exponent seven in the denominator of equation (35) has been chosen to provide a smooth transition from the case of one projection factor being less than 0.5 to both factors being greater than 0.5. Similarly, if SXN or SYN is less than or equal to 0.5, then:

$$SAREA = [1 - (SXN * SYN)^{1/2}] (SXN + SYN) , \quad (36)$$

otherwise:

$$SAREA = (SXN * SYN) \left[1 + \frac{1}{(SXN * SYN + 0.75)^7} \right] . \quad (37)$$

Using these fractional areas, FAREA and SAREA, the leakage across the overlying and underlying aquitards can be calculated.

Leakage Calculations

Once tip and toe positions have been determined, the leakage terms at each block may be calculated. Two methods are available for allocating leakage between model layers. The two methods are identical in the case of single-layer problems. They differ for multilayered problems in the manner which leakage is allocated when freshwater in one aquifer overlies saltwater in another aquifer, or vice versa. Comparisons of simulations using the two different methods are presented in the model verification section.

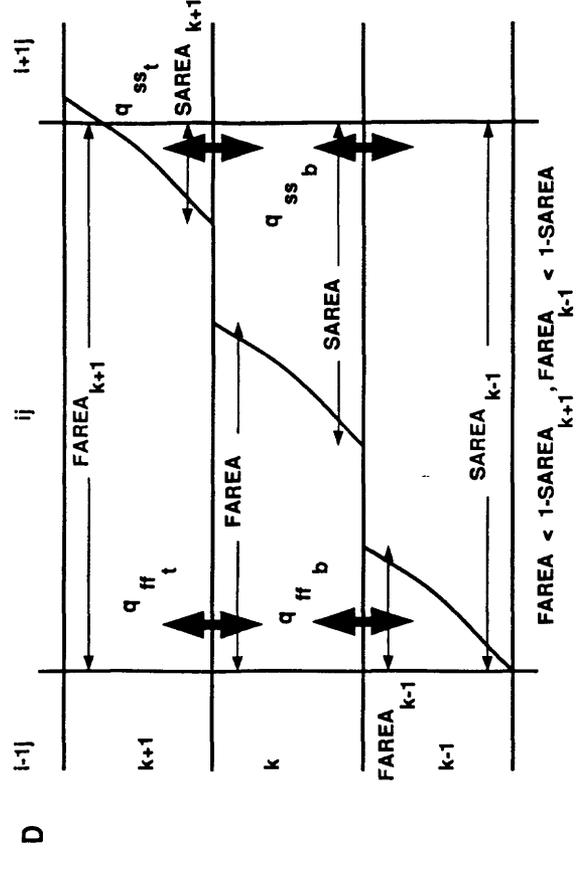
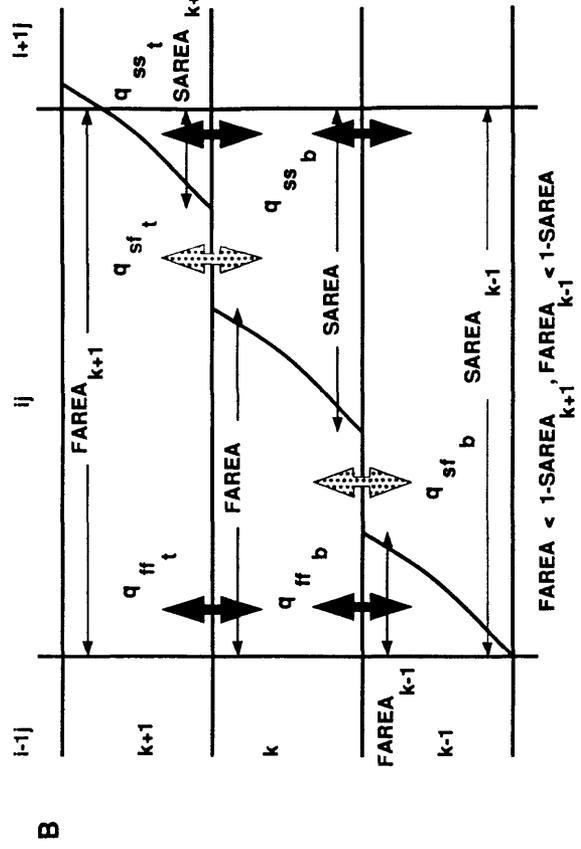
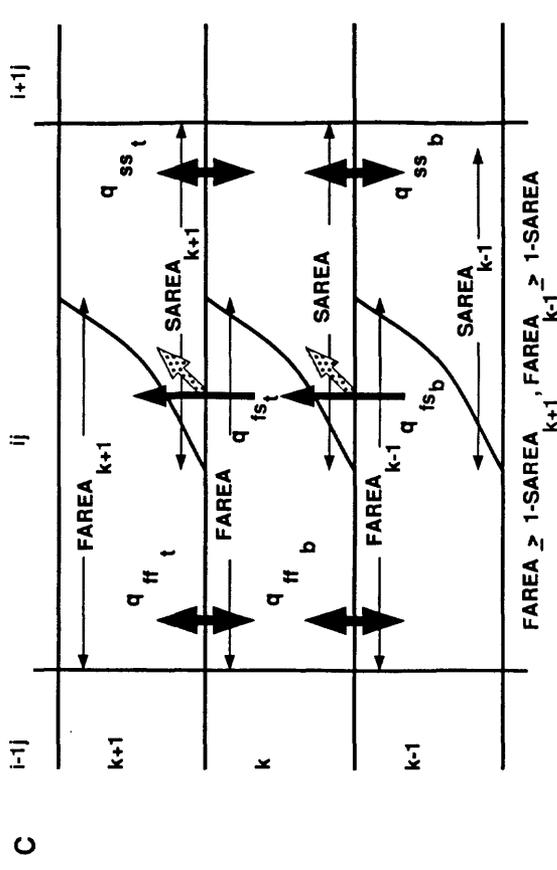
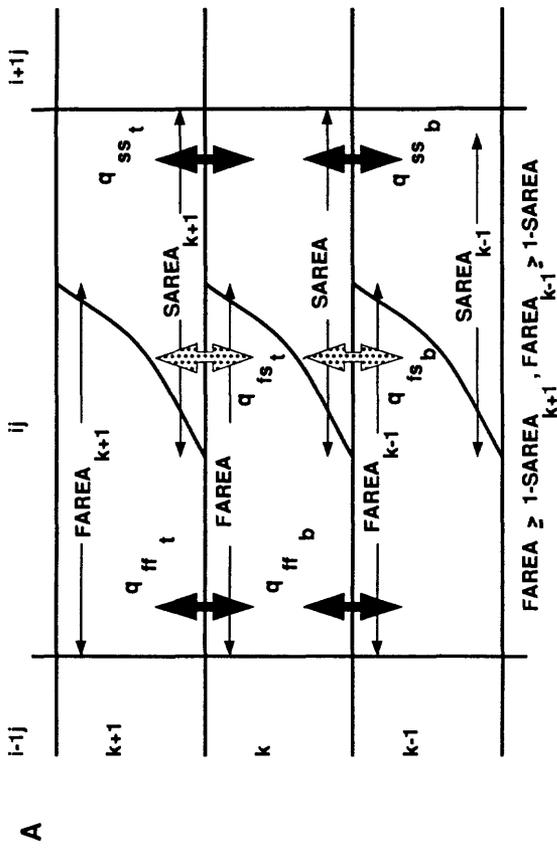


Figure 10.--- Leakage elements (solid arrows represent leakage across confining layers without water type change, shaded arrows represent leakage with water type change - fresh to salt or salt to fresh, arrow heads point in allowed leakage directions):
 A. method 1-complete mixing, $FAREA_{k+1} \geq 1-SAREA_{k+1}$, $FAREA_{k-1} \geq 1-SAREA_{k-1}$, $FAREA_{k+1} < 1-SAREA_{k+1}$, $FAREA_{k-1} < 1-SAREA_{k-1}$.
 C. method 2-restricted mixing, $FAREA_{k+1} \geq 1-SAREA_{k+1}$, $FAREA_{k-1} \geq 1-SAREA_{k-1}$, $FAREA_{k+1} < 1-SAREA_{k+1}$, $FAREA_{k-1} < 1-SAREA_{k-1}$.

In method 1 leakage (complete mixing) it is assumed that when freshwater leaks into saltwater or saltwater leaks into freshwater, the amount of leakage is small relative to the water in place, and the water mixes instantaneously and is incorporated into the flow zone that it leaks into. This means that when freshwater leaks into saltwater it becomes part of the saltwater domain, and vice versa for saltwater leaking into freshwater. This assumption is reasonable for low rates of leakage; however, if there is significant vertical flow in the system from one kind of water into the other it does not always yield good results. For example, if there is considerable flow of freshwater in one layer to saltwater in the layer above, this method does not account for the fact that the saltwater in the overlying layer would eventually be flushed completely by freshwater. Also, if inland pumpage reduces freshwater heads significantly in areas having saltwater in the overlying aquifer, downward saltwater leakage into freshwater can be induced, acting as a source of water.

Method 2 (restricted mixing) limits the mixing of freshwater and saltwater. In this method, saltwater is not allowed to leak into the freshwater zone, and downward leakage of freshwater into saltwater is not allowed. Upward leakage of freshwater is distributed between the overlying freshwater and saltwater zones based on the amount of freshwater in the overlying block as represented by the value of FAREA. If FAREA equals 1 in the overlying block, all freshwater leakage goes into the overlying freshwater zone. If FAREA = 0.5, half goes into the freshwater zone and the other half is incorporated into the saltwater zone. When FAREA = 0.0, all freshwater leakage is incorporated into the overlying saltwater zone. The components of freshwater and saltwater leakage across the top and bottom of a finite-difference block for each method are shown in figure 10. Table 2 summarizes the freshwater and saltwater leakage terms for a block.

Finally, the total net freshwater and saltwater leakage into a block is given by the sum of the leakages across the top and bottom:

$$A Q_{fl} \approx (q_{fl_t} + q_{fl_b}) \Delta x \Delta y, \quad (38a)$$

and

$$A Q_{sl} \approx (q_{sl_t} + q_{sl_b}) \Delta x \Delta y, \quad (38b)$$

Positive leakage represents flow of water into a layer and negative leakage represents flow out of the layer.

Table 2A.--Leakage terms for Method 1 - Complete Mixing

Across Top	FAREA \geq 1-SAREA _{k+1}	FAREA < 1-SAREA _{k+1}
Freshwater Leakage		
$q_{fl_t} =$	$(1-SAREA_{k+1})q_{ff_t} + (FAREA - (1-SAREA_{k+1}))q_{fs_t}$	$(FAREA)q_{ff_t}$
Saltwater Leakage		
$q_{sl_t} =$	$(1-FAREA)q_{ss_t}$	$(1-FAREA-SAREA_{k+1})q_{sf_t} + (SAREA_{k+1})q_{ss_t}$
Across Bottom	FAREA _{k-1} \geq 1-SAREA	FAREA _{k-1} < 1-SAREA
Freshwater Leakage		
$q_{fl_b} =$	$(1-SAREA)q_{ff_b}$	$(FAREA_{k-1})q_{ff_b} + (1-FAREA_{k-1}-SAREA)q_{sf_b}$
Saltwater Leakage		
$q_{sl_b} =$	$(FAREA_{k-1} - (1-SAREA))q_{fs_b} + (1-FAREA_{k-1})q_{ss_b}$	$(SAREA)q_{ss_b}$

Table 2B.--Leakage terms for Method 2 - Restricted Mixing

Across Top	FAREA \geq 1-SAREA _{k+1}	FAREA < 1-SAREA _{k+1}
Freshwater Leakage		
$q_{fl_t} =$	$(1-SAREA_{k+1})q_{ff_t} + \Psi(FAREA - (1-SAREA_{k+1}))q_{fs_t}$	$(FAREA)q_{ff_t}$
Saltwater Leakage		
$q_{sl_t} =$	$(1-FAREA)q_{ss_t}$	$(SAREA_{k+1})q_{ss_t}$
Across Bottom	FAREA _{k-1} \geq 1-SAREA	FAREA _{k-1} < 1-SAREA
Freshwater Leakage		
$q_{fl_b} =$	$(1-SAREA)q_{ff_b} + \Psi[FAREA(FAREA_{k-1} - (1-SAREA))]q_{fs_b}$	$(FAREA_{k-1})q_{ff_b}$
Saltwater Leakage		
$q_{sl_b} =$	$\Psi(1-FAREA)(FAREA_{k-1} - (1-SAREA))q_{fs_b} + (1-FAREA_{k-1})q_{ss_b}$	$(SAREA)q_{ss_b}$

Explanation:

$q_{ff_t} = \left(\frac{K'}{B'}\right) (\phi_{f_{k+1}} - \phi_f)$	$q_{ff_b} = \left(\frac{K'}{B'}\right)_{k-1} (\phi_{f_{k-1}} - \phi_f)$
$q_{fs_t} = \left(\frac{K'}{B'}\right) \left\{ \left(\frac{\gamma_s \phi_s}{\gamma_f} - \phi_f \right) - \frac{z_a + z_b}{2\delta} \right\}$	$q_{fs_b} = \left(\frac{K'}{B'}\right)_{k-1} \left\{ \left(\phi_{f_{k-1}} - \frac{\gamma_s \phi_s}{\gamma_f} \right) + \frac{(z_a + z_b)_{k-1}}{2\delta} \right\}$
$q_{ss_t} = \left(\frac{K'}{B'}\right) (\phi_{s_{k+1}} - \phi_s)$	$q_{ss_b} = \left(\frac{K'}{B'}\right)_{k-1} (\phi_{s_{k-1}} - \phi_s)$
$q_{sf_t} = \left(\frac{K'}{B'}\right) \left\{ \left(\phi_{f_{k+1}} - \frac{\gamma_s \phi_s}{\gamma_f} \right) + \frac{z_a + z_b}{2\delta} \right\}$	$q_{sf_b} = \left(\frac{K'}{B'}\right)_{k-1} \left\{ \left(\frac{\gamma_s \phi_s}{\gamma_f} - \phi_f \right) - \frac{(z_a + z_b)_{k-1}}{2\delta} \right\}$

$\delta = \frac{\gamma_f}{\gamma_s}$, $\Psi = 1$ when direction of leakage is from freshwater to overlying saltwater, $\Psi = 0$ when direction of leakage is from overlying saltwater to freshwater. For all flux terms, a positive value indicates flow into the aquifer layer k, and a negative value indicates flow out of the aquifer layer k.

Discretized Flow Equations

Substituting the finite-difference approximations given by equations (19), (21), (22), (25a, b), (38a, b) into equations (18a) and (18b) and rearranging, the freshwater and saltwater finite-difference equations at each node for time level n are expressed as:

$$\begin{aligned}
 & T_{fx}^n \Delta y (\Phi_{f,j+1} - \Phi_f)^n - T_{fx}^n \Delta y (\Phi_f - \Phi_{f,j-1})^n + T_{fy}^n \Delta x (\Phi_{f,i+1} - \Phi_f)^n - \\
 & T_{fy}^n \Delta x (\Phi_f - \Phi_{f,i-1})^n + \left\{ \frac{S_f B_f^n + n(\alpha + (\text{SAREA})\delta)}{\Delta t} \right\} (\Phi_f^n - \Phi_f^{n-1}) + \\
 & \frac{(\text{SAREA})n(1+\delta)}{\Delta t} (\Phi_s^n - \Phi_s^{n-1}) + (1-\alpha)Q_{f1}^n + \alpha N \Delta x \Delta y - \left\{ \frac{\text{Th}_f P_t}{\text{Th}_t} \right\}^n = 0, \quad (39a)
 \end{aligned}$$

$$\begin{aligned}
 & T_{sx}^n \Delta y (\Phi_{s,j+1} - \Phi_s)^n - T_{sx}^n \Delta y (\Phi_s - \Phi_{s,j-1})^n + T_{sy}^n \Delta x (\Phi_{s,i+1} - \Phi_s)^n - \\
 & T_{sy}^n \Delta x (\Phi_s - \Phi_{s,i-1})^n + \left\{ \frac{S_s B_s^n + (\text{FAREA})n(1+\delta)}{\Delta t} \right\} (\Phi_s^n - \Phi_s^{n-1}) + \\
 & \frac{(\text{FAREA})n\delta}{\Delta t} (\Phi_f^n - \Phi_f^{n-1}) + Q_{1s}^n + \alpha N \Delta x \Delta y - \left\{ P_t - \frac{\text{Th}_f P_t}{\text{Th}_t} \right\}^n = 0. \quad (39b)
 \end{aligned}$$

By introducing FAREA and SAREA into the equations the interface storage terms go to zero when no interface is present in the block.

Rearranging and collecting terms, these two equations can be consolidated into a more compact form with all unknowns on the left hand side and the known values on the right hand side:

$$\mathbf{Z}\Phi'^n_{k-1} + \mathbf{B}\Phi'^n_{i-1} + \mathbf{D}\Phi'^n_{j-1} + \mathbf{E}\Phi'^n + \mathbf{F}\Phi'^n_{j+1} + \mathbf{H}\Phi'^n_{i+1} + \mathbf{S}\Phi'^n_{k+1} = \mathbf{Q}', \quad (40)$$

where \mathbf{Z} , \mathbf{B} , \mathbf{D} , \mathbf{E} , \mathbf{F} , \mathbf{H} and \mathbf{S} are 2×2 coefficient submatrices with the form $\begin{bmatrix} Z^1 & Z^2 \\ Z^3 & Z^4 \end{bmatrix}$; $\Phi' = \begin{bmatrix} \Phi_f \\ \Phi_s \end{bmatrix}$ the unknown head values; and $\mathbf{Q}' = \begin{bmatrix} Q^1 \\ Q^2 \end{bmatrix}$ the known right-hand side values. These matrix coefficients are defined in Appendix A. Equation (40) is non-linear as the values of the coefficients \mathbf{Z} , \mathbf{B} , \mathbf{D} , \mathbf{E} , \mathbf{F} , \mathbf{H} and \mathbf{S} depend on the position of the interface. Their values change with time and are unknown until the solution is obtained.

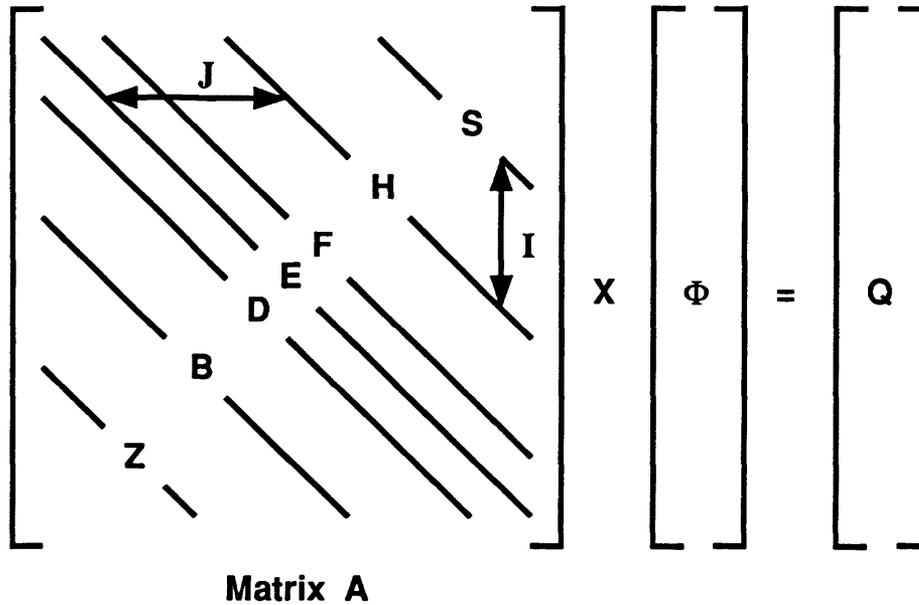


Figure 11.--The system of coupled freshwater and saltwater flow equations in matrix form (I = number of rows, J = number of columns).

The system of M coupled equations representing freshwater and saltwater flow at each node may be expressed in matrix notation as:

$$A\Phi^n = Q, \tag{41}$$

where: M = the number of nodes in the system;
 A = an M by M septadiagonal block matrix of the coefficient submatrices Z, B, D, E, F, H and S;
 Φ^n = a block column vector of the unknown freshwater and saltwater heads at time level n; and
 Q = a block column vector containing the known values.
 The structure of these matrices is shown in figure 11.

SOLUTION OF THE COUPLED FLOW EQUATIONS

To reduce roundoff error and increase solution accuracy during computations, the matrix equation is expressed in residual form by subtracting $A\Phi^{n-1}$ from both sides:

$$A[\Phi^n - \Phi^{n-1}] = Q - A\Phi^{n-1},$$

or

$$A\xi^n = R^{n-1}, \tag{42}$$

where:

$$\begin{aligned} \xi^n &= \Phi^n - \Phi^{n-1} && \text{the head change from } n-1 \text{ to } n; \\ R^{n-1} &= Q - A\Phi^{n-1} && \text{the residual right-hand-side.} \end{aligned}$$

The M by M square coefficient matrix A has a block septadiagonal structure. Each of its rows (with the exception of those representing boundary nodes) contains seven non-zero 2 by 2 coefficient submatrices making up the seven coefficient blocks. Both arrays ξ^n and R^{n-1} are column vectors of length M that also exhibit a block structure. Each entry of ξ^n corresponds to a given grid location and is a 2 by 1 submatrix containing the change in the freshwater head and saltwater head; similarly each entry of R^{n-1} is a 2 by 1 submatrix of freshwater and saltwater residuals.

The Strongly Implicit Procedure (SIP)

Direct solution of equation (42) by Gaussian elimination requires excessive computational effort and storage requirements for large systems. For this reason, an iterative solution method has been implemented. The strongly implicit procedure (SIP) is an iterative solution technique that solves a modified problem obtained by adding an M by M matrix B to coefficient matrix A (Stone, 1968; Weinstein and others, 1969, 1970). The matrix B is chosen so that the resulting matrix (A+B) may be factored into a sparse lower triangular matrix L and a sparse upper triangular matrix U, each of which has four non-zero block elements per row, corresponding to the positions of the non-zero elements of matrix A. After this modification the equation to be solved becomes:

$$(A+B)\xi^m = R^{m-1}, \quad (43)$$

where ξ^m represents the change in heads from iteration level m-1 to m and R^{m-1} represents the residuals for iteration level m-1. In the SIP solution algorithm, only the coefficients of A, and the factored, sparse L and U matrices are stored resulting in considerable savings in storage requirements over direct solution methods.

At each time step equation (43) is solved iteratively with R^0 (the residual for the first iteration) set equal to R^{n-1} , the residual from the previous time step. As iteration proceeds and the solution converges, ξ^m and R^{m-1} go to zero. The expanded version of the SIP algorithm for three-dimensional, two-phase flow is given in Appendix B.

The nonlinearity of equation (40) is incorporated directly within the solution procedure using Picard iteration. The equations are linearized within the iterative solution technique by evaluating the coefficients at the previous iteration level. As mentioned above, the coefficients of matrix A are time-dependent and change with the position of the interface.

The values of these coefficients are updated at the end of each iteration based on the new interface position and then used in the next iteration level calculations.

Convergence of SIP

The rate of convergence of the SIP scheme may be improved by the appropriate choice of values for the iteration parameters and by the introduction of a relaxation factor. Stone (1968), Weinstein and others (1970) and Trescott and others (1976) have shown that SIP converges more rapidly than other iterative techniques when there is considerable anisotropy, heterogeneity, and irregularity of geometry. More recent literature (Meijerink and Van der Vorst, 1977) has indicated that the preconditioned conjugate gradient method may converge more rapidly than SIP; however, this method has not been adapted to this problem.

Iteration Parameters

To study the convergence of the SIP algorithm, Stone (1968) applied Von Neumann error analysis to a simple two-dimensional, homogeneous, isotropic problem of rectangular geometry assuming that the dimensions of the problem were large enough that the influence of the boundary conditions would be negligible. From this analysis he concluded that it was best to use a cycle of iteration parameters within the range of zero to one, because values near one tend to decay the low frequency errors most rapidly, while the values near zero decay the high frequency errors most rapidly. The minimum parameter is always set to zero, but the maximum value is problem dependent. Too large a value will cause divergence, and too small a value will lead to slow convergence. Weinstein and others (1969, 1970) found it best to calculate the maximum SIP iteration parameter (ω_{\max}) using the equation for the minimum ADIP (alternating direction implicit procedure) parameter for a homogeneous one-phase problem with rectangular grid blocks:

$$1 - \omega_{\max} = \min_{\text{over grid}} \left[\frac{\pi^2}{2J^2(1+\rho)}, \frac{\pi^2}{2I^2(1+1/\rho)} \right], \quad (44)$$

where:
$$\rho = \frac{K_{fy} \Delta x^2}{K_{fx} \Delta y^2};$$

J = the number of columns; and
I = the number of rows.

The intermediate parameters are spaced geometrically, based on the value of ω_{\max} :

$$1 - \omega_m = (1 - \omega_{\max})^{m/(M-1)}, \quad (45)$$

where:

$m = 1, 2, 3, \dots, M-1$

$M =$ number of iteration parameters in a cycle.

Stone (1968) recommended using the same parameter on successive normal and reverse iterations; however, Weinstein and others (1970) found this to be of no advantage in solving multiphase two-dimensional problems. Weinstein and others also used the same parameters for all phases and layers and recommended that if a sequence of parameters caused divergence, $(1-\omega_{\max})$ should be multiplied by a factor (WITER) of two to ten, and if convergence was slow, it should be divided by a factor of two to ten. The guidelines presented by Weinstein and others (1970) have been implemented in SHARP. The number of iteration parameters (NITP) is user specified. Generally between four and ten parameters are sufficient. The factor WITER is also user specified. For problems that are highly anisotropic or that have highly variable grid spacing, WITER may need to be considerably larger than ten to achieve convergence (see practical considerations section).

Relaxation Factor

A relaxation factor (RFAC), multiplying the residual terms on the right hand side of equation (43) may be introduced to accelerate the rate of convergence:

$$(A+B)\xi^m = \text{RFAC} * R^{m-1} .$$

A factor between one and two leads to over-relaxation of the solution, increasing the increments of head change for each iteration. For highly non-linear problems, under-relaxation or decreasing the increment of head change per iteration by a factor between zero and one, will generally improve convergence. Under-relaxation may be necessary to prevent overshooting or oscillation in the solution (see practical considerations section).

MODEL VERIFICATION

To verify the numerical solution obtained from the finite-difference model, numerical simulations have been compared with analytical solutions, experimental Hele-Shaw analogs, and other model results.

Single Layer Problems

Model results for one layer problems without leakage have been verified for the motion of a linear interface and a retreating and intruding interface (Essaid, 1984). SHARP results are compared with an analytical solution and model simulations of Mercer and others (1980a) and Shamir and Dagan (1971). These results are summarized below.

Keulegan (1954) presented an analytical solution for the location of the toe of an initially vertical interface rotating towards an equilibrium position in a confined aquifer of uniform thickness:

$$L(t) = \left(\frac{t \Delta \rho K_f D}{n \rho_f} \right)^{1/2}, \quad (46)$$

where $L(t)$ is the distance to the interface toe from its initial position, $\Delta \rho = \rho_s - \rho_f$, and D is the aquifer thickness. To compare SHARP model results with another numerical model and the analytical solution, the parameters of table 3 and the initial conditions of Mercer and others (1980a) were used. The initial position of the interface was set at $L = 20$ m, a position corresponding to a time of 12.28 days of rotation from the vertical position. The results obtained for a simulation period of 20 days and the results of Mercer and others (1980a) are shown in figure 12. Both numerical solutions fit the analytical solution quite well.

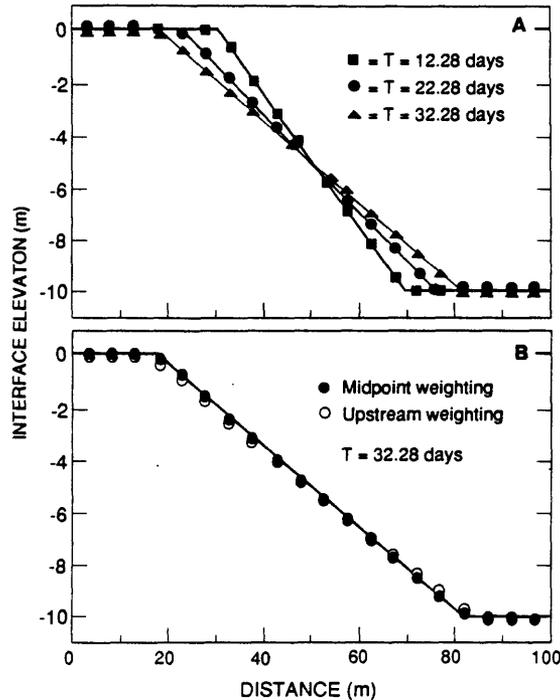


Figure 12.--Simulation of a rotating linear interface (points represent numerical results, lines represent analytical solutions):
 A. results of SHARP,
 B. results of Mercer and others, (1980a).

Table 3.--Parameters used in model verification simulations

Parameter	Rotating Interface	Retreating and Intruding Interface	Layered Aquifer
D (m)	10.	27.	
ρ_f (gm/cm ³)	1.0	1.0	1.0
ρ_s (gm/cm ³)	1.025	1.030	1.025
K_f (m/s)	4.52×10^{-4}	.69	.10
K' (m/s)			8.0×10^{-4}
S_f (m ⁻¹)	1.0×10^{-4}	1.0×10^{-4}	
S_s (m ⁻¹)	1.025×10^{-4}	1.03×10^{-4}	
n	0.3	1.0	0.1
Δx (m)	5.0	0.1	0.05
Δt (s)	86400.0	5.0	

Simulations for a retreating and intruding interface were compared to observed interface behavior in a Hele-Shaw experiment carried out by Bear and Dagan (1964), and the numerical model results of Shamir and Dagan (1971). In both cases the same parameters were used (table 3) and the outflow, or seepage face, was approximated by assigning the boundary node a high leakance value (3.3 s^{-1}) and a zero head in the overlying aquifer. For the retreating interface the seaward freshwater discharge was suddenly increased from a steady-state value of $3.9 \text{ cm}^3/\text{s}$ to $18.8 \text{ cm}^3/\text{s}$ (fig. 13). In the case of the intruding interface, the initial steady-state freshwater flux of $19.1 \text{ cm}^3/\text{s}$ was stopped abruptly and the transient interface allowed to intrude (fig. 14). The numerical results of Shamir and Dagan (1971) and the present model both do not reproduce the experimentally observed interface curvature, and show some lag in interface translation. This can be attributed to the error introduced by the Dupuit assumption of horizontal flow. This approximation deteriorates as vertical flow becomes more pronounced at the outflow face. There is also the difficulty of realistically representing the seepage boundary condition at the outflow face. Shamir and Dagan (1971) introduced the actual experimental values for freshwater head and interface elevation at the outflow face into their solution to improve their results.

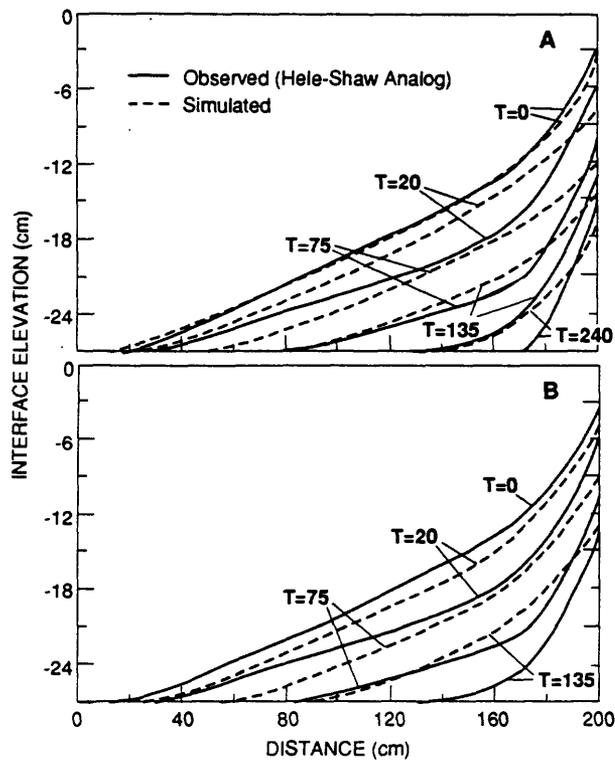


Figure 13.--Simulation of a retreating interface (lines represent experimental results, dashed lines represent numerical results):

- A. results of SHARP,
- B. results of Shamir and Dagan (1971).

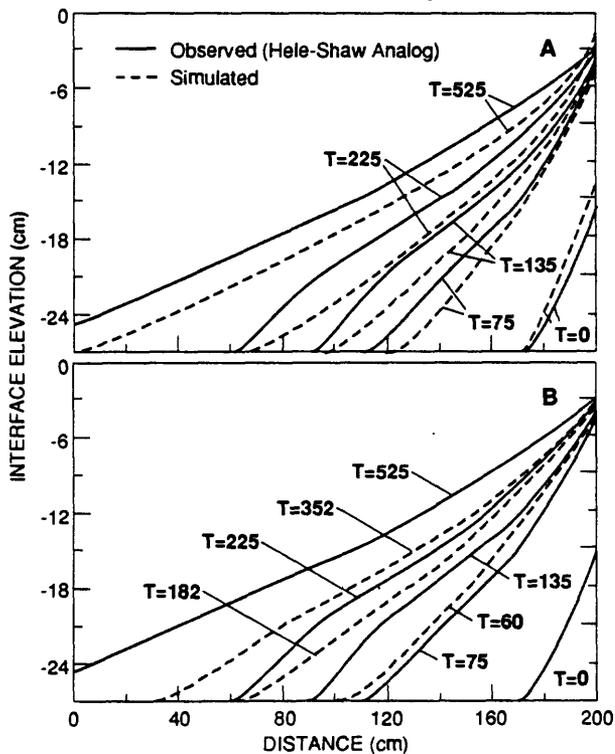


Figure 14.--Simulation of an intruding interface (lines represent experimental results, dashed lines represent numerical results):

- A. results of SHARP,
- B. results of Shamir and Dagan (1971).

Steady-State Interface in a Layered Coastal Aquifer

Incorporation of interface tip and toe tracking and accurate leakage calculations makes comparison of the numerical model with an analytical solution for a layered problem possible. Mualem and Bear (1974) have presented an approximate analytical solution for the steady-state shape of an interface in a coastal aquifer when a thin horizontal semiconfining layer is present (fig. 15). Their solution, which showed good agreement with Hele-Shaw analog experiments they conducted, is based on the Dupuit assumption and a linearization of the flow equations. In addition, Mualem and Bear (1974) made two simplifying assumptions regarding the leakage conditions in the region where saltwater is present above the semiconfining layer: (1) Φ_a was constant above the semiconfining layer in this region, and (2) the freshwater leaking through the semiconfining layer from below was incorporated into the freshwater flow zone above. This is approximately equivalent to the restricted mixing leakage option of SHARP (method 2).

The geometry of the test problem is shown in figure 16, and the parameters used in the simulation are given in table 3. To compare the numerical solution with the analytical solution, method 2 leakage calculations (restricted mixing) were used to approximate Mualem and Bear's assumptions. Figure 17a shows good agreement between the two solutions. The same problem was then simulated using the method 1 leakage conditions (complete mixing), and the results are shown in figure 17b. In this second solution, the freshwater flowing through the semiconfining layer leaks into the overlying saltwater zone. The saltwater zone is no longer static, as was assumed in the analytical solution. The interface below the semipervious layer is slightly deeper while the interface extends further inland above the layer. This result is caused by: (1) the leakage of freshwater into the overlying saltwater, and (2) flow in the saltwater zone, which is actually a mixing zone. In the Hele-Shaw experiments, Mualem and Bear (1974) observed that there was a clear boundary between the freshwater and mixing zone but no clear boundary could be distinguished between the mixing zone and the saltwater zone. In the complete mixing simulation, the position of the interface separates the zone containing freshwater from the zone containing any mixed water.

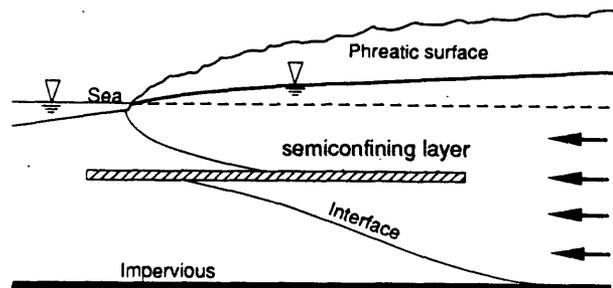


Figure 15.--The interface in a coastal aquifer with a thin horizontal semiconfining layer (modified from Mualem and Bear, 1974).

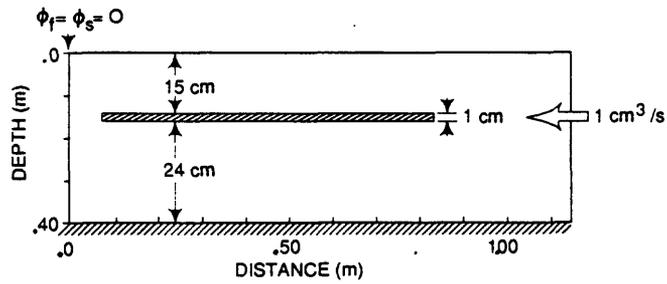


Figure 16.--Geometry of the layered test problem.

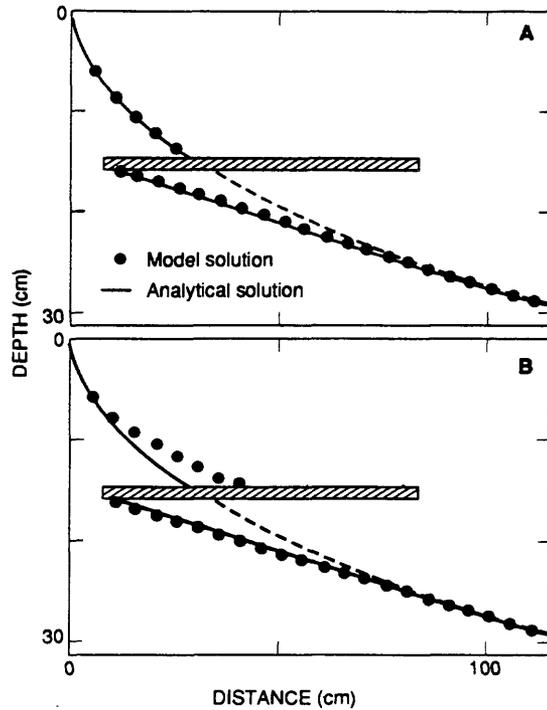


Figure 17.--Comparison of numerical and analytical solutions for the layered case (dashed line represents the position of the interface in the absence of a semiconfining layer):
 A. restricted mixing leakage (method 2),
 B. complete mixing leakage (method 1).

Comparison of Sharp Interface and Disperse Interface Solutions

Hill (1988) presented a comparison of the sharp interface and disperse interface solutions for a generalized cross-section through the ground-water system in the coastal area of Cape May County, New Jersey. The comparison was made using the complete mixing leakage (method 1) calculations in the sharp interface model. The disperse interface simulation was carried out using the density-dependent, convective-dispersive transport model SUTRA, described by Voss (1984b).

The geometry of the simulated cross-section is shown in figure 18. The system consists of an unconfined aquifer overlying two confined aquifers. The boundary conditions imposed on the system were no-flow across the bottom and the landward-vertical boundaries, a constant head of sealevel at the seaward-vertical boundary, and a parabolic distribution of head specified in an overlying layer varying from sea level at the shore to 12 feet above sea level onshore. Offshore, equivalent freshwater heads for the column of seawater were specified in the overlying layer. The upper boundary was a head-dependent boundary condition in SHARP simulations and a constant-head boundary condition in SUTRA simulations. Other parameters used in the simulation are given in table 4, and the details of the convective-dispersive simulation are given by Hill (1988).

The results of Hill's SHARP simulations with complete mixing are shown in figure 19. The sharp interface results of an additional simulation using the restricted mixing leakage method are also shown in figure 19. In general, the sharp interface is closer to land than the lines of equal chloride concentration, expressed as a fraction of seawater, from the SUTRA simulations. This is because the effects of mixing lead to circulation of saltwater at the interface. This can be seen in the plot of velocity vectors from the SUTRA simulations in figure 19b. Cooper (1959), Kohout (1964) and Volker and Rushton (1982) have shown that because the effects of dispersion are neglected, the position of the toe predicted by the sharp interface tends to be farther inland than the actual transition zone. Thus, the sharp interface solution gives a more conservative estimate of saltwater

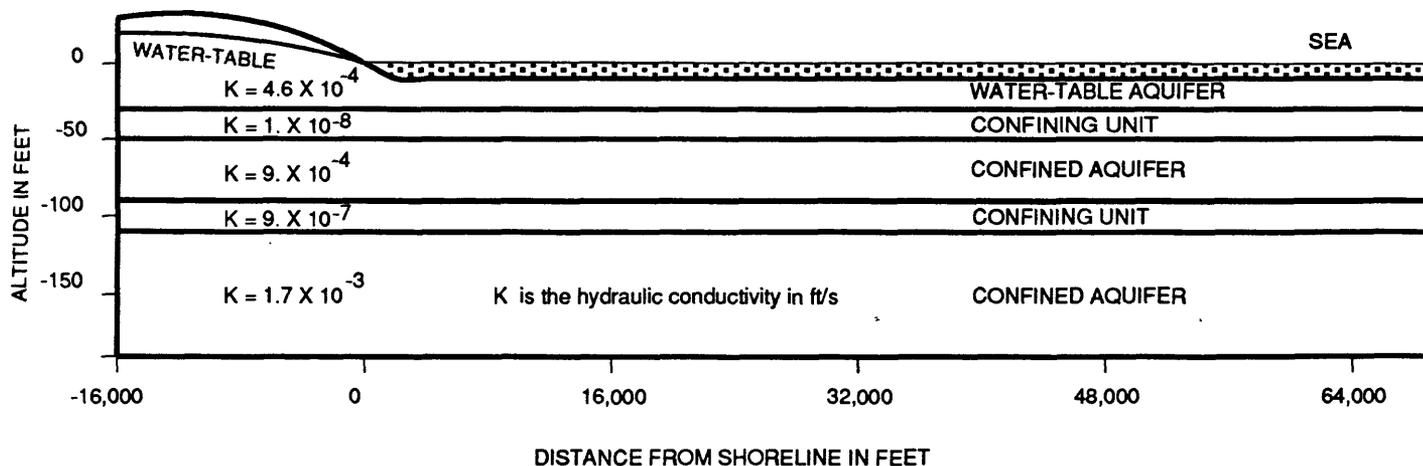


Figure 18.--Modeled cross-section for Cape May simulation (modified from Hill, 1988).

Table 4.--Parameters used in Cape May cross-section simulation

Parameter	Value
Aquifer horizontal to vertical anisotropy	100.
Confining bed horizontal to vertical anisotropy	10.
Aquifer Porosity	0.1
δ	40.
Maximum longitudinal dispersivity (feet)	25.
Minimum longitudinal dispersivity (feet)	2.5
Transverse dispersivity (feet)	2.5

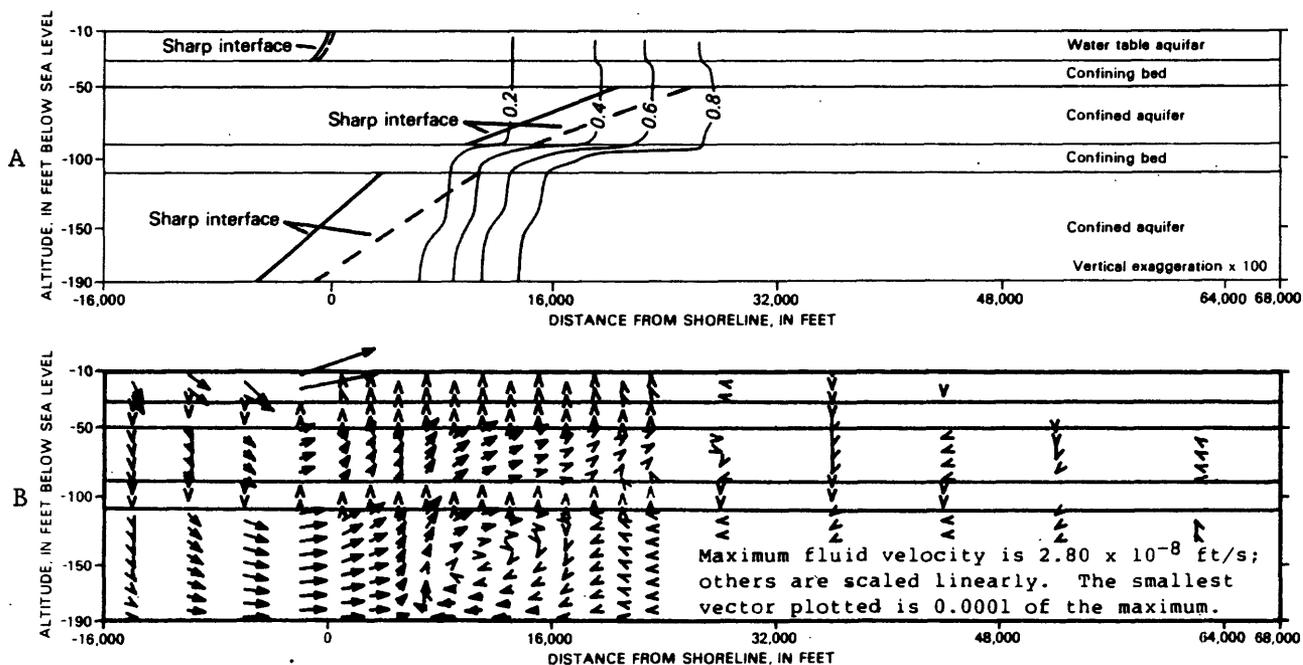


Figure 19.--Results of steady-state simulations:

- lines of equal chloride concentration from the SUTRA simulation and the sharp interface positions from the SHARP simulations (solid lines are interface with method 1 leakage, dashed lines are interface with method 2 leakage),
- fluid velocity vectors from SUTRA (modified from Hill, 1988).

intrusion and interface toe position. Hill (1988) indicated that as the coefficient of hydrodynamic dispersion was reduced, the orientation of the lines of equal concentration approached that of the sharp interface. In the following discussion, the position of the sharp interface is compared to the zone between the 0.4 and 0.6 concentrations lines obtained from the SUTRA simulations. This corresponds to the zone containing approximately half freshwater and half saltwater, and will be referred to as the 0.5 zone.

The sharp interface is considerably landward of the 0.5 zone in the upper unconfined aquifer for both complete and restricted mixing simulations. In this unconfined aquifer, the sharp interface position approximates the boundary of the active freshwater flow zone and the offshore zone of sluggish fluid flow. That is, the interface is the location where most of the freshwater is being discharged from the unconfined layer to the sea. The zone of relatively stagnant freshwater present offshore in the SUTRA simulation is not included in the freshwater domain of either sharp interface simulation. In the SUTRA steady-state simulation, this zone has been flushed of saltwater by upward leakage of freshwater from below. During transient conditions induced by inland pumpage, this zone would not be a useful source of freshwater, because flow directions would quickly be reversed, resulting in saltwater leakage into this zone from the sea.

In the middle aquifer, for both leakage simulations, the sharp interface very roughly passes through the 0.5 zone, but has a different slope (because dispersion is not accounted for). The interface is closer to the shore in the complete mixing simulation because there is some loss of freshwater by downward leakage into the underlying saltwater domain.

The interface in the lowermost aquifer is considerably landward of the 0.5 zone when using the complete mixing leakage conditions. When using the restricted mixing conditions, the tip of the interface is close to the 0.5 zone; however, the toe is still considerably landward. In the complete mixing case, some freshwater from confined aquifer 2 leaks down into the saltwater zone of the bottom aquifer, and is mixed into the saltwater domain. This results in shrinkage of the freshwater zone in the lowermost aquifer. In the restricted mixing calculations, freshwater leakage from above is not allowed into saltwater below, and loss of freshwater to the underlying saltwater domain does not occur. The toe of the interface in this case, however, is still landward of the 0.5 zone as the sharp interface approach does not reproduce the circulation of saltwater in the transition zone. The sharp interface position approximately corresponds to the boundary between the freshwater flow vectors and the zone of saltwater recirculation.

The system illustrated in this example is strongly influenced by vertical flow components, and therefore, the restricted mixing leakage option yields better results.

DESCRIPTION OF THE COMPUTER PROGRAM

The computer program is segmented into a main program and fifteen subroutines which are described below. The flow charts in figures 20 and 21 present the overall structure of the major calculations and operations in the program. The data input formats are given in attachment A, and the main program variables are defined in attachment B. The complete program listing is given in attachment C. In the following description of the program, line numbers referenced are the line numbers in attachment C.

MAIN Routine

The MAIN routine controls the general flow and execution structure of the model. As shown in figure 20, subroutine INPUT is called to read the model input parameters, and other arrays are initialized accordingly. For new model runs (NCONT=0), the interface elevations are used to determine the node type: freshwater only (F), saltwater only (S), or freshwater overlying saltwater (M); and, the interface projection factors (FX, FY, SX and SY) are initialized. For continuation runs (NCONT>0), the interface projections factors are read in by subroutine INPUT. Subroutine IPARAM is then called to calculate the iteration parameters used in the SIP algorithm, and subroutine KMEAN is called to calculate the hydraulic conductivities at grid block boundaries ($i+1/2$, $j+1/2$).

Each pumping period is initiated by calling PUMPER (a secondary entry point in INPUT) to read the time-step and pumping specifications, and recharge for that pumping period. Time-step calculations then commence. Heads from the previous time step are saved and subroutine SIP is called to solve the freshwater and saltwater flow equations.

If the maximum number of time steps allowed has been reached upon completion of a time step, subroutine OUTPUT is called to calculate the mass balance and print time-step results, terminating program execution. If steady-state has been achieved, OUTPUT is called to print results and a new pumping period is initiated. If the last pumping period has been reached, program execution is terminated. If neither of these conditions is met, but the number of time steps between printouts has been reached, OUTPUT is called to print results and a new time step is initiated. Otherwise, subroutine OUTPUT is called to print time-step iteration information and to perform cumulative mass balance calculations, and a new time step is initiated. If the end of a pumping period has been reached, a new pumping period is initiated. If the last pumping period has been completed, OUTPUT is called and calculations are terminated.

The final step of the program, when the simulation has been completed, is to write the simulated heads and interface elevations at observation nodes, for each time step, to a file.

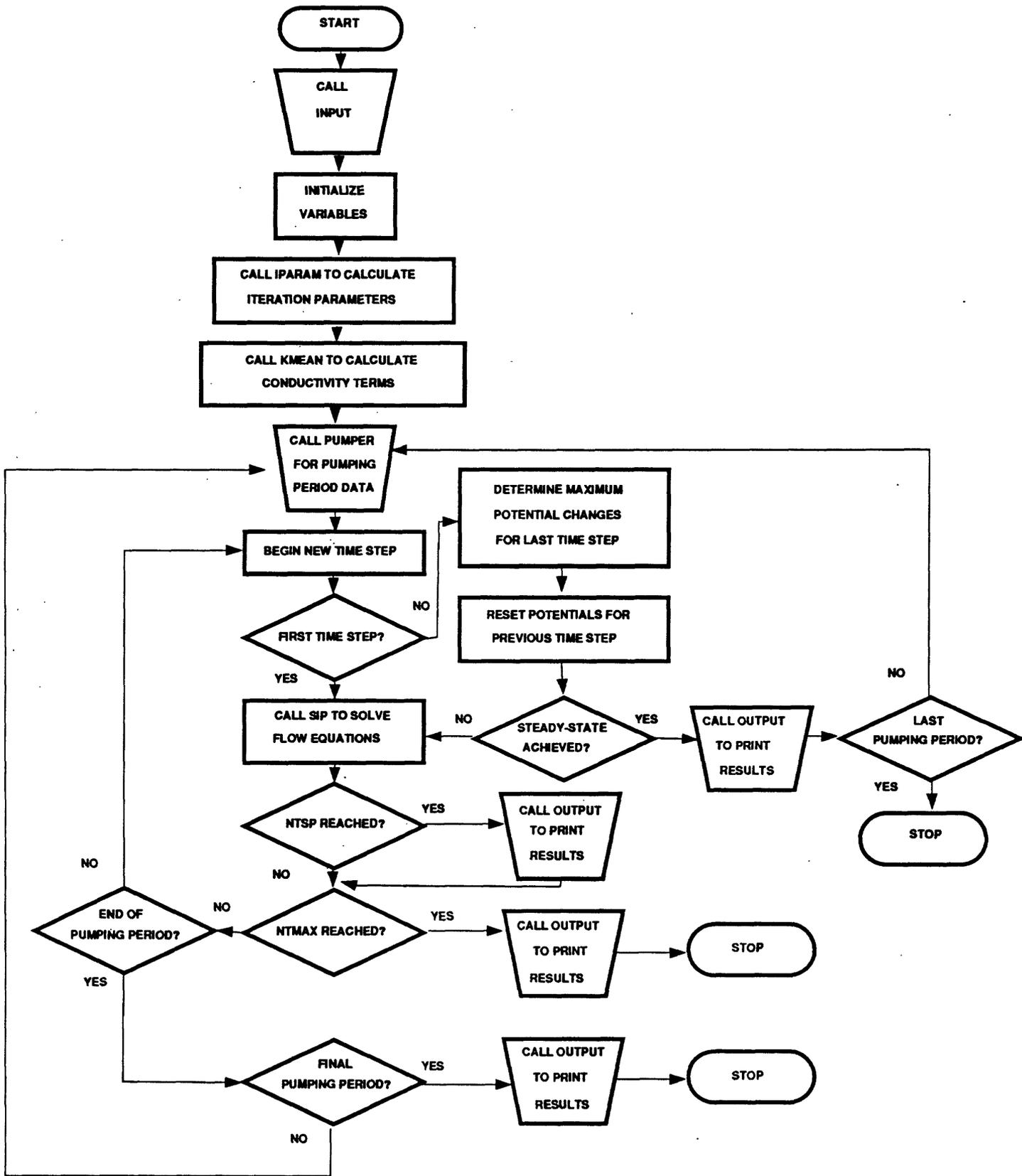


Figure 20.--Flow chart for the MAIN program.

Subroutine INPUT

The input data required for a model run is read in and printed out by subroutine INPUT, which is called from the MAIN routine. Data may be input in any consistent set of units having time units of seconds. The first data group read (lines IN 330 to IN 400) includes the simulation title, grid dimensions, iteration and time-step information, closure criterion, and fluid properties. Following this, locations of observations nodes are read in, and the initialization for the current run is specified (NCONT, TSEC and NINT). At this point, if NOPT=1, user specified input/output options (stored in the array IOP) are read. The data group that is read next (lines IN 900 to IN1370) contains aquifer properties and initial freshwater head and interface elevation values. The secondary entry point PUMPER is accessed at the beginning of each pumping period to read in time-step parameters, well information, and recharge values for that period. A detailed description of data input format is given in Attachment A.

Subroutine READ

Subroutine READ is called from INPUT to read and write two- and three-dimensional arrays. Elements of the array are assigned a fixed value if $M=1$; if $M \neq 1$ the individual element values are read one row at a time, and scaled using a multiplication factor. The first section of the routine is for three-dimensional arrays (RD 40 to RD 380), and the second section, which is accessed through the secondary entry point READ1 is for two-dimensional arrays (RD 410 to RD 690).

Subroutine IPARAM

Subroutine IPARAM is called from the main program to calculate the iteration parameters used in the SIP solution algorithm. The number of iteration parameters is specified in the data input and the calculated sequence is cycled through during SIP iterations. The same set of iteration parameters is used for all layers.

Subroutine KMEAN

Freshwater hydraulic conductivity terms in the x- and y-directions at grid block boundaries are calculated in subroutine KMEAN based on the harmonic means of values at adjacent nodes. Zero values for the x-direction hydraulic conductivity (FKX) must be specified along the exterior of the grid, and also at inactive nodes, to ensure no flow out of the solution domain. At these locations, the y-direction conductivities (FKY) are set to zero during model calculations. The saltwater hydraulic conductivities (SKX and SKY) are calculated from the freshwater conductivities and fluid densities and viscosities.

Subroutine SIP

Subroutine SIP is called from the main routine at each time step to solve the freshwater and saltwater flow equations using the SIP solution algorithm (see Appendix B and fig. 21). Coefficients and variables are initialized in lines SI 450 to SI 590. If the maximum number of iterations has not been reached, calculations for a new iteration commence on line SI 610. If the maximum number of iterations per time step is exceeded, OUTPUT is called to print the results of the last iteration and program execution is terminated. Otherwise, an iteration parameter is selected from the calculated sequence of parameters. Subroutine TCOEF is then called to evaluate the new interface elevations and node types. These are used to calculate the transmissive coefficients, and freshwater and saltwater block area factors from the interface tip and toe projections.

For odd iterations the normal SIP algorithm is applied and for even iterations the reverse algorithm is applied. In both cases subroutine FACTOR is called to evaluate SIP coefficients and to factor the matrix. These results are used to solve for the changes in freshwater and saltwater heads. The maximum changes in freshwater and saltwater head for the iteration are stored, and head values are updated. If convergence has been achieved, values at observation nodes are stored and control is transferred to the MAIN program. Otherwise the iteration procedure is repeated.

Subroutine TCOEF

Subroutine TCOEF is called from SIP at the beginning of each iteration to update transmissive coefficients and projected interface positions (fig. 21). First, new interface elevations are calculated, node types (F, M, S) are determined, and the freshwater and saltwater thicknesses at each grid point are calculated (TC 510 to TC 930). Following this, new transmissive coefficients are calculated using the interpolated freshwater and saltwater thicknesses at block boundaries and mean hydraulic conductivities (TC 950 to TC 1200). The next group of calculations in TCOEF involves the evaluation of the freshwater and saltwater block area factors. The interface tip and toe are projected in the x- and y-directions and the new projected distance factors (FXN, FYN, SXN, and SYN) are used to determine the proportion of the area of the top and bottom of each block that is in contact with freshwater and in contact with saltwater (FAREA and SAREA).

Subroutine FACTOR

Subroutine FACTOR is called from SIP to calculate the residuals (R^{k-1}) and factor the modified matrix (A+B). The matrix (A+B) is decomposed into its corresponding lower triangular (L) and upper triangular (U) matrices. In lines FA 430 to FA2320 the coefficients of matrix A are evaluated. In lines FA2340 to FA2580 the residual terms are calculated. At constant head nodes, all coefficients off the main diagonal and the residual on the right hand side are set to zero, which forces the change in head to be zero.

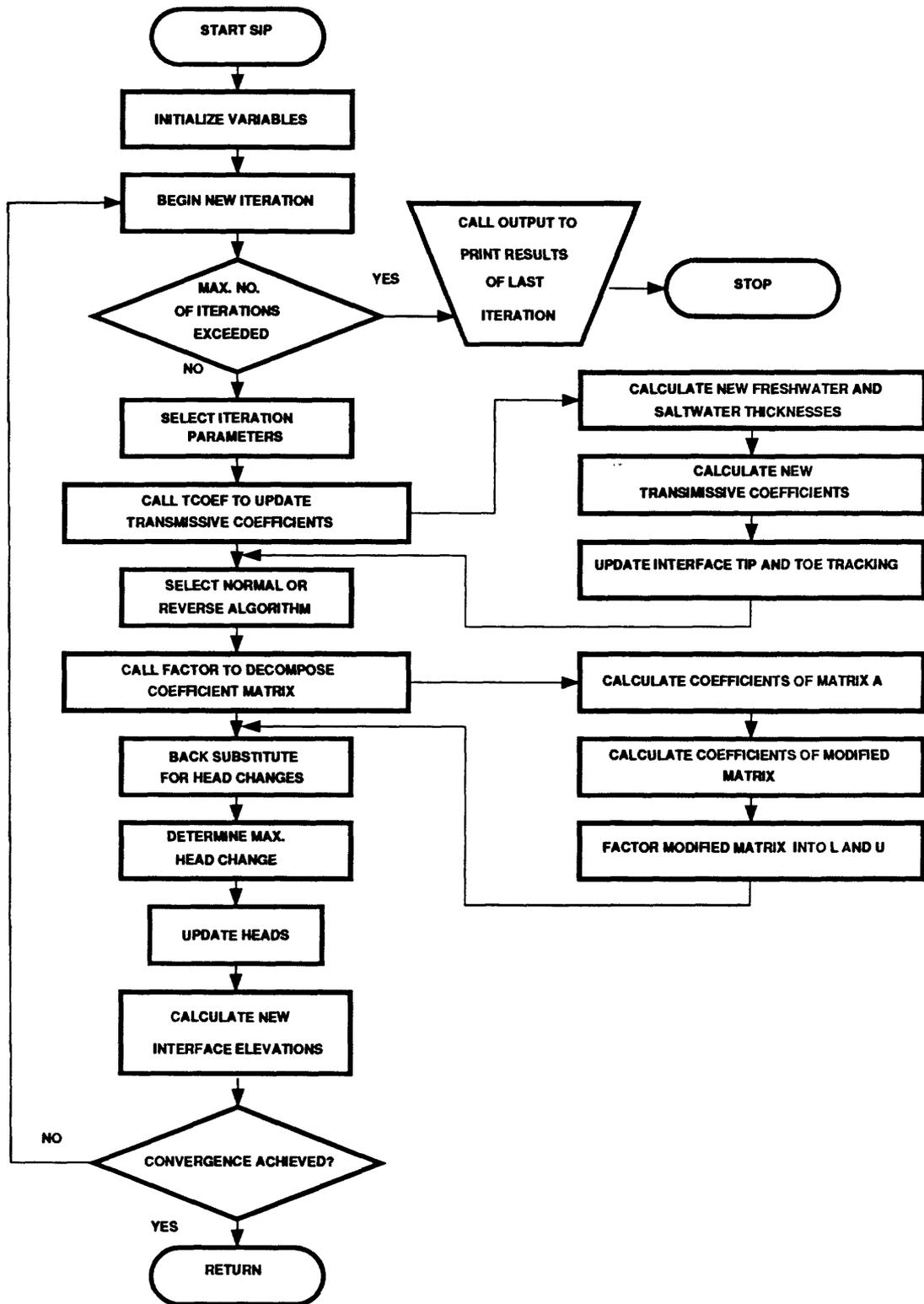


Figure 21.--Flow charts for subroutines SIP, TCOEF and FACTOR.

Modification of matrix **A** and factorization takes place in lines FA2770 to FA3570. Five subroutines are used to simplify the calculations in this process: ASSIGN, MULT, INV, MAT1, and MAT2. Subroutine ASSIGN assigns values to a 2x2 submatrix, subroutine MULT multiplies two submatrices, INV calculates the inverse of a submatrix, and MAT1 and MAT2 carry out submatrix calculations that are used repeatedly in the SIP algorithm.

Subroutine RESULTS

In subroutine RESULTS, the results of the latest time step are written to file in a format that can subsequently be read into the model for a continuation run. The matrices written to file are the freshwater heads, interface elevations and interface tip and toe projection factors (FX, FY, SX, and SY). If IOP(12)=0, the relevant file units are rewound and RESULTS is called from the MAIN program after each time step. In this manner, the results of the latest time-step computation are written to file. If IOP(12)=1, RESULTS is called every time the number of time steps between printouts is reached. In this manner the results for all output time steps are written to file.

Subroutine OUTPUT

The main routine calls OUTPUT at the end of each time step. It will print out the full time-step results to unit IUN(24) when the number of time steps between subsequent printouts is reached (NP), a steady-state solution is achieved, or the final time step is reached. It may also be called from SIP to print out results of the last iteration when a solution does not converge within the maximum number of iterations allowed. OUTPUT prints out time-step and pumping-period information, and the maximum head changes per iteration. Then, for each aquifer (beginning with the lowermost) subroutine MASBAL is called. Mass balance information, freshwater and saltwater heads, interface elevations and a map showing the extent of intrusion will then be printed as specified by the user. An output value of 11111. indicates a null value. For example, if the freshwater head at a block has a value of 11111., there is no freshwater present in that block. For other time steps, OUTPUT is called only to print time-step information and to evaluate the mass balance for the aquifers of the system. Control is then returned to MAIN.

Subroutine MASBAL

Subroutine MASBAL is called from OUTPUT to calculate time-step and cumulative freshwater and saltwater aquifer mass balances. Freshwater and saltwater flows into and out of each layer through constant freshwater head nodes, constant saltwater head nodes, recharge, pumpage and leakage are calculated. For an accurate solution, the difference between the influx and outflux should equal the change in storage. Any deviation represents an error in the solution and is reported as a percent error relative to the total influx and the change in storage. All time-step values are expressed as a volume rate and cumulative values are expressed as total volumes.

PRACTICAL CONSIDERATIONS FOR MODEL APPLICATION

Array Dimensions

The dimensions of the arrays in the program are specified in a PARAMETER statement at the beginning of each subroutine. The values of NR, NC, and NL specified in the PARAMETER statement must be equal to the values of NNR, NNC, and NNL specified in the input. For example, for a problem with 20 rows, 25 columns, and 3 layers the PARAMETER statement in each subroutine must be modified to look like this:

```
PARAMETER(NR=20,NC=25,NL=3).
```

The arrays TIME, FWH, SWH, ZI, FAR, SAR, and IC store the values of time, freshwater head, saltwater head, interface elevation, FAREA, SAREA, and F, M, S code values for each time step at the user specified observation nodes. These arrays are dimensioned to allow for 10 observation nodes and 200 time steps. This may be changed by the user to accommodate problem requirements.

As the model is currently dimensioned, the maximum number of freshwater constant head nodes allowed is 200, as is the maximum number of constant saltwater heads allowed. To increase this, the dimensions of the arrays FFLOW, SFLOW, IFL, JFL, ISL, and JSL must be increased. The maximum number of iterations allowed in the model is 200. If more iterations are needed, the arrays DMAXF and DMAXS must be made larger.

As a measure of required storage, example problem one (see below), which has 28 rows, 8 columns, and 1 layer requires approximately 215 kilobytes of memory.

Input/Output File Units

The file unit numbers used for input and output are specified in the array IUN. The model has default settings for the elements of IUN which are listed in table 5. They are specified in the data statement on line MA 410 of the MAIN program. By default, all input parameters are read from unit 5 and aquifer parameters are specified layer by layer starting from the bottom up. The basic model input parameters and general output are written to the output file on unit 6. The values of the PHIF, ZINT, FX, FY, SX, and SY matrices for each layer are written to file unit 7 in a format that is ready to be used for input. These arrays can then be inserted into the input file to initialize a continuation run. The heads at the user-specified observation blocks are written to file unit 8 and the leakages at each block are written to file unit 9. If the option to open files internally within the program is used, these default file units take the names INPUT, OUTPUT, RESULTS, HEADS, and LEAK, respectively. If the default unit numbers are not used, the data statement on line MA 410 must be modified to reflect the user's input/output file unit number. If the option to open files internally is used, the program must be modified to open the appropriate files (line MA440 to MA500 and MA2550 to MA2610). This allows user flexibility in terms of how input and output is structured. It is possible to set up input so that input files are grouped according to the input

Table 5.--Default input/output file unit numbers.

IUN(i)	Default Input File Unit Number & Name	Data Set
1	5 INPUT	General Information
2	5 INPUT	FKX
3	5 INPUT	FKY
4	5 INPUT	STORF
5	5 INPUT	STORS
6	5 INPUT	POR
7	5 INPUT	THCK
8	5 INPUT	ZBOT
9	5 INPUT	PHIF (initial values)
10	5 INPUT	ZINT (initial values)
11	5 INPUT	FX (initial values)
12	5 INPUT	FY (initial values)
13	5 INPUT	SX (initial values)
14	5 INPUT	SY (initial values)
15	5 INPUT	AQL
16	5 INPUT	HEAD
17	5 INPUT	BATH
18	5 INPUT	IWT
19	5 INPUT	DX
20	5 INPUT	DY
21	5 INPUT	Time Step and Well Information
22	5 INPUT	RECH

IUN(i)	Default Output Unit	Data Set
23	6 OUTPUT	Input Data
24	6 OUTPUT	Time Step Results
25	7 RESULTS	PHIF (last values)
26	7 RESULTS	ZINT (last values)
27	7 RESULTS	FX (last values)
28	7 RESULTS	FY (last values)
29	7 RESULTS	SX (last values)
30	7 RESULTS	SY (last values)
31	8 HEADS	Observation Node Heads
32	9 LEAK	Block Leakages

parameters. For example, conductivities for all layers could be in one input file, heads for all layers could be in another input file, and so on.

User Specified Input/Output Options

Model input can be in any consistent set of units with time in seconds. All elevations must be relative to sea level. The input/output control options for the model are specified in the array IOP. The model has default settings for the elements of IOP, which are set in the data statement on line MA 420 of the MAIN program. The default values, and an explanation of the options are listed in table 6. If NOPT=0, these default values are used. If NOPT=1, the user must specify the values for IOP(2) through IOP(12). IOP(1) controls whether the input/output file units are opened internally within the program using OPEN statements, or if they are opened prior to execution of the program. By default the model opens file units internally (IOP(1)=0). If the user wishes to open files externally (IOP(1)=1), line MA 420 must be replaced with line MA 424.

Initial Conditions

The amount of input information needed to specify initial conditions depends on the type of simulation. Model runs may be initialized in two manners, as a new run (NCONT=0) or as a continuation run (NCONT=1). In all cases, initial saltwater heads are calculated by the model based on initial freshwater head and interface elevation. A continuation run could be a transient simulation initialized using a previously obtained steady-state solution, or a run which is a continuation of a previous transient simulation. In a new run, no information regarding the interface projection factors is needed. In a continuation run, the projection factors obtained from the previous simulation, as well as freshwater heads and interface elevations, must be included in the input.

New runs may be initialized in two different manners. If NINT=0, initial freshwater heads and initial interface elevations must be specified and are used to calculate initial saltwater heads. If NINT=1, only initial freshwater heads are needed and initial interface elevations are calculated as $\zeta_1 = -\delta \Phi_f$ (the Ghyben-Herzberg equilibrium interface). The resulting initial saltwater heads will be zero. It is important that initial saltwater heads be zero, even in blocks not containing saltwater.

For continuation runs, the model output from the previous simulation, which is contained in file units IUN(25) to IUN(30), must be used as the initial values of freshwater head, interface elevation, and interface projection factors (FX, FY, SX, and SY). The new starting time of the simulation in seconds (TSEC) must be specified in the input file.

Problems may arise later with interface projection and movement if initial values are not properly specified. When initializing new runs, it is advisable to use values of head and interface elevation that result in saltwater heads close to zero (even in blocks where saltwater is not present). It is important to keep in mind that the initial values of

Table 6.--Input/output options specified in array IOP (default values are used when NOPT=0, non-default options must be input by the user if NOPT=1)

IOP(i)	Option	Default Value
1	0 = Open files internally 1 = Open file externally	0
2	0 = Do not print input aquifer parameters 1 = Print input aquifer parameters	1
3	0 = Do not print input pumping and recharge values 1 = Print input pumping and recharge values	1
4	0 = Do not print time-step freshwater heads 1 = Print time-step freshwater heads	1
5	0 = Do not print time-step saltwater heads 1 = Print time-step saltwater heads	1
6	0 = Do not print time-step interface elevations 1 = Print time-step saltwater heads	1
7	0 = Do not print time-step FAREA values 1 = Print time-step FAREA values	0
8	0 = Do not print time-step SAREA values 1 = Print time-step SAREA values	0
9	0 = Do not print time-step F-M-S map 1 = Print time-step F-M-S map	1
10	0 = Do not print time-step block leakage values 1 = Print time-step block leakage values	1
11	0 = Do not write iteration information to screen 1 = Write iteration information to screen	0
12	0 = Write latest time-step results to file 1 = Write time-step results to file every NP time-steps	0

freshwater head and interface elevation will determine the initial distribution of freshwater and saltwater in the aquifers. If a problem is initialized with isolated pockets of saltwater in the freshwater zone, or conversely, these pockets will remain as there will be no outlet for that water. To avoid problems, check the initial freshwater and saltwater distribution by running the model for one very small time step (such as one second). Pockets of freshwater or saltwater can be removed by adjusting the initial heads and interface elevations.

Another consideration when specifying initial conditions is the initial position of the interface. If the interface is initialized to a very unstable position (for example a vertical interface), computational instabilities could arise.

Boundary Conditions

The model can simulate freshwater and saltwater constant flux (no-flow or prescribed flux), constant head, and head-dependent leaky boundaries. No-flow boundaries are specified using inactive nodes. Inactive nodes within the grid must be assigned an x-direction freshwater hydraulic conductivity (FKX) of zero. During model calculations, FKY will be set to zero at these blocks. In this manner, the harmonic mean of the conductivities at these boundaries is zero. The outer boundaries of each layer must have a row (or column) of nodes with FKX=0, preventing inflow or outflow from the system. Constant fluxes into or out of a block can be prescribed in two manners. At unconfined blocks in the uppermost aquifer, constant flux can be specified by the recharge matrix (RECH, L/T). At confined blocks in the upper aquifer, recharge is zeroed out. Otherwise, for all layers, the pumpage matrix (PUMP, L³/T) can be used to specify a constant flux. A positive pumpage value represents extraction of water from a block and a negative value represents addition of water to the block.

A constant freshwater head block is specified by assigning a negative specific storage to that location in the matrix STORF. This is a flag indicating that the block should be handled as a constant head block with the head fixed to the initial value specified in PHIF. Likewise, a constant saltwater head block is specified by assigning a negative saltwater specific storage in STORS. In the model, initial saltwater heads are calculated from initial freshwater heads and interface elevations. Therefore, at constant saltwater head nodes, the initial freshwater head and interface elevations must be specified so as to give the desired saltwater head, where:

$$\Phi_s = \frac{\zeta_1 + \delta \Phi_f}{(1 + \delta)} \quad (\text{from equation (9)}).$$

The saltwater head will then remain fixed at its initial value for the duration of the simulation, although freshwater head and interface location at this node may change. Fixing both freshwater and saltwater head at a node also fixes the interface position in that node.

The leaky, head-dependent boundary condition can be used to represent boundaries such as streams, springs, or offshore outcrop areas. The flux at head-dependent boundaries is calculated as follows:

$$\text{flux} = \text{leakance} (\text{fixed head value} - \text{head in aquifer block}).$$

The leakance and fixed head value are specified by the user. The head in the aquifer is calculated in the model solution. Thus, as the head in the aquifer changes, the flux across the boundary changes. The leakance value at the node can be adjusted to obtain the desired interaction between the aquifer and the fixed head node. In the uppermost aquifer, a head-dependent boundary is specified by assigning the fixed value of head to the HEAD matrix, and assigning a vertical leakance to the block in the AQL matrix. At unconfined nodes the flux will be zeroed out, and therefore, to use this type of boundary condition in a block it must be specified as a confined block. If this type of boundary is used offshore, the fixed head value must be the freshwater head equivalent to the overlying column of saltwater:

$$\Phi_f = -Z/\delta ,$$

where:

Z = the bathymetry or elevation of the ocean floor relative to sea level.

In the underlying layers, this same type of boundary condition can be achieved if a leakance is specified and the head in the overlying aquifer block is fixed. This can be done by giving the overlying block a constant head. Another way to handle this, if the overlying block is not part of the active region of the overlying aquifer, is to initialize the freshwater head at the overlying inactive block to be the required fixed value. In this case, the head in the overlying inactive block remains constant throughout the simulation and is used only for leakage calculations at this boundary. At offshore blocks where the bathymetry is less than zero, the water in the overlying inactive block is assumed to be saltwater. If the leakance at a head-dependent boundary block is sufficiently large, the block will act as a constant head block with a head equal to the overlying fixed head value.

Steady-State Simulations

Steady-state simulations can only be obtained by running a transient simulation until it achieves steady state. This is because the interface position must be allowed to move gradually as steady state is approached. Generally, the interface responds quite slowly; however, the approach to steady state can be accelerated in several ways. First, the initial conditions should be set close to the expected steady-state solution. The specific storage of the aquifers should be very small or zero to eliminate elastic storage terms from equations (18a and b). In addition, the movement of the interface can be accelerated by using a very small porosity (as small as .0001). In this manner, the interface storage terms in equations (18a and b) become small, and less water must be moved as the interface changes position. Care must be taken when choosing the porosity, because choosing too small a value will cause the the interface to move too rapidly leading to instability in the solution. In such cases a larger porosity value can be used to get a solution close to steady state, and then a much smaller porosity can be used to continue the simulation. The steady-state solution

is not a function of specific storage or porosity; however, transient solutions are. After obtaining the steady-state solution, the values of specific storage and porosity must be set to their true values for subsequent transient simulations.

Notes on Discretization

Caution should be exercised in choosing both the spatial and temporal discretization. The model is quite sensitive to time-step size. If time steps are too large compared with the rate of interface movement, the solution will become unstable due to too much adjustment of the interface per time step. Also, to obtain accurate interface projections, it is advisable to use smaller blocks in the zone of the interface. Where the interface is present in only one block, the projection of the tip and toe becomes less accurate. If the interface tip or toe oscillates at a block boundary, smaller block sizes or smaller time steps may be needed to stabilize the solution.

Notes on Parameters RFAC, WITER, WFAC, and NUP

The model contains several parameters that are problem dependent and must be adjusted by the user to obtain the most stable solution. For a given case, successive simulations varying these parameters will indicate the best values for the most rapid convergence. The use of these parameters, RFAC, WITER, WFAC, and NUP, is explained below.

An iterative solution technique (SIP) is used in the model. The parameters WITER and RFAC can be adjusted to optimize the convergence of the solution algorithm. WITER is a factor used in the calculation of the iteration parameters (see section on convergence of SIP):

$$1-\omega_m = [\text{WITER} * (1-\omega_{\max})]^{m/(M-1)}. \quad (45)$$

In problems with considerable anisotropy in hydraulic conductivity or block dimensions (for example $\Delta x \gg \Delta y$), the parameters calculated using the above algorithm will tend to be clustered near the upper limit of 1. Generally it is best to use a sequence of iteration parameters with a spread between 0 and 1. If WITER=1 yields clustered iteration parameters, WITER should be increased to a value greater than 1 (for some extreme cases it may be necessary to use a value of one hundred or larger).

RFAC is a relaxation factor used in the iterative solution of the equations (see section on convergence of SIP). As the equations are highly nonlinear (especially for multilayered systems with considerable leakage), problems of overshoot and oscillation may arise. To dampen out these effects, a relaxation factor less than one can be implemented.

Another cause of solution instability is oscillation in the interface tip and toe projections. The parameters WFAC and NUP are used to control this. WFAC is a weighting factor used to calculate the interface

projections based on the projections from the previous and current iterations as given in equation 30:

$$x1 = WFAC*x1_{new} + (1-WFAC)*x1_{old}. \quad (30)$$

Generally, a WFAC of 0.5 gives the smoothest solution.

The above parameters can be adjusted through a trial and error process to achieve the best solution and convergence. However, for some systems the solution will still show small oscillations. The solution will almost converge and then continue to oscillate or hover about the correct solution. This is generally caused by small oscillations in the interface tip and toe positions. The parameter NUP is used to control these oscillations. The interface position is fixed after NUP iterations, and the interface elevations are not updated in following iterations. Care must be taken not to fix the interface too early during the iterative solution. If head changes are still large when the interface is fixed, the solution will not be correct. Generally, this parameter should be made large enough so that it is reached only for time steps in which there is oscillation. For example, a value of NUP equal to 20 is appropriate if most time steps converge in less than 20 iterations. For time steps which take more than 20 iterations, the maximum change in head at iteration 20 should be small (less than .001). If these criteria are not met, NUP should be increased.

Mass Balance Calculations

A mass balance is calculated for each layer in the system. Values are reported as cumulative volumes for the total duration of the simulation and rates for the time step under consideration. The mass balance error is the difference between the change in storage and the net sources minus sinks. To evaluate the significance of the error, it must be compared with the amount of flow through the system, and/or the change in storage in the system. The mass balance error is reported as a percent error relative to the amount of influx into the layer and also the change in storage in the layer. If the change in storage is small (as is the case at steady state) the relative error will tend to be large and is not a good criterion for the mass balance. Similarly, if the influx into the system is small, the error relative to the influx will not be a good criterion. At steady state, both influx and change in storage will be small in the saltwater zone, resulting in apparently large errors even when a good solution is obtained. These considerations must be kept in mind when interpreting the mass balance errors.

Influx of water into a layer by leakage is broken up into components from overlying and underlying freshwater and saltwater. When the restricted mixing option is used, the only source of water to the freshwater zone is from overlying or underlying freshwater. No saltwater can leak into the freshwater. The sources of influx by leakage to the saltwater zone are overlying and underlying saltwater, as well as underlying freshwater. When complete mixing is used, potentially all leakage components could occur (see figure 10).

The global mass balance for all layers can be obtained by summing the inflows and outflows across the boundaries of each layer. Fluxes across constant head boundaries can be taken directly from the mass balance reported for each layer. Since head-dependent boundaries are handled as leakages, fluxes across these boundaries are lumped with the fluxes reported as leakage across the top of each aquifer. The actual values at each head-dependent block can be obtained from the values reported in file unit IUN(32) (LEAK file).

Leakage Conditions

As described earlier, two methods can be used for leakage calculations. The choice of method is important for multilayered problems. In method 1 or complete mixing (NLK=1), it is assumed that the amount of freshwater leaking into saltwater (or conversely) is small and that it mixes instantaneously and becomes part of the saltwater domain. However, in systems with considerable vertical flow and leakage, this assumption may not be valid. For these cases, a second method of handling leakage calculations, restricted mixing, is available (NLK=0). In this method, the mixing of freshwater and saltwater is restricted. Saltwater is not allowed to leak into freshwater, and leakage of freshwater is distributed between the overlying freshwater and saltwater zones based on the amount of freshwater in the overlying block. The decision of which method to use is problem dependent. Method 1 gives the most conservative position of the interface, because the freshwater leaking into saltwater is fully incorporated into the saltwater domain. The interface position predicted by this method is farther inland than that predicted by method 2 (restricted mixing). If the position of the interface seems to be too far inland, and if it displays erratic movement during transient runs, method 2 leakage conditions should be used.

For both leakage methods, the water overlying offshore outcrops is assumed to be saltwater. These outcrops are identified by a bathymetry less than zero for the uppermost layer, and in addition, an overlying inactive block for the deeper layers. When using complete mixing (method 1), leakage across the upper confining layer in the offshore area must be examined closely to see if there is downward leakage of saltwater into a freshwater zone. This would be a potential source of water quality degradation that would not appear as inland movement of the interface. Similarly, when using method 1, leakage between layers must be monitored to see if there is leakage of saltwater into a freshwater zone.

EXAMPLE PROBLEMS

Two example problems are presented below. The first is an areal single layer model of part of the southeastern coast of Oahu, Hawaii. The second is the cross-section layered problem for Cape May, New Jersey, described in the model verification section.

Example 1: Single Layer Model of Southeastern Oahu

The Hawaiian island of Oahu, which is underlain by an extensive freshwater lens, was built by the growth of two volcanoes which eventually joined and overlapped to form one island (Stearns, 1946). The thick sequence of thin bedded basaltic lava flows forms the principal aquifer in which freshwater floats on top of saltwater (Visher and Mink, 1964; Takasaki and Mink, 1982). In its inland area the ground water is unconfined, but toward the coast the ground water is confined by a caprock of reef and marine sedimentary deposits. The seaward flow of freshwater is impeded by the caprock, allowing inland freshwater heads to build up and a thicker freshwater lens to develop. Discharge from the freshwater lens takes place either by springs which occur at the landward edge of the caprock or by upward leakage through the caprock.

In this example a part of the Oahu coast has been simulated as a semiconfined aquifer with a free bottom (see fig 1B). The lava flows are treated as a single model layer which is unconfined inland, but confined near the shore and offshore by the caprock (fig. 22). The modeled area extends about 20 miles offshore to capture the offshore boundary condition. Variable grid spacing is used to extend the model to this distance, as well as to allow for finer discretization near the coast. The area is bounded laterally by relatively impermeable dikes. Variable x-direction grid spacing is used to accurately locate the right-hand-boundary dike. The simulated grid is 28 rows by 8 columns. There is one pumping well at 13,4,1, and recharge is specified at the inland unconfined blocks. The lateral and bottom boundaries are no-flow, and in the areas covered by the caprock, a head-dependent boundary is used with equivalent freshwater heads specified for the offshore areas. This specifies the boundary conditions for both the freshwater and saltwater flow domains and allows leakage of freshwater through the caprock to the sea.

The simulation is initiated as a new run with the model calculating the initial interface elevations from the Ghyben-Herzberg relation. A small storage coefficient and a small porosity (0.0001) are used to accelerate the approach to the steady-state solution. The default input/output options are used, and method 1 leakage is specified. The simulation input and output files are given in tables 7 and 8, respectively. A relaxation factor of 0.8, an interface weighting factor of 0.5, and an iteration parameter weighting factor of 1.0 are used for this simulation. The maximum number of iterations allowed per time step is 60, and the interface position is fixed after 50 (NUP) iterations. During the first two time steps, the solution does not converge within 50 iterations, causing the interface position to be fixed. However, in subsequent time steps the solution converges in less than 50 iterations and no interface fixing takes place.

The change in freshwater and saltwater head from time step 4 to time step 5 is less than the steady-state criterion (0.000001), and the output for the steady-state solution is given. It is important to confirm that a true steady-state solution has been obtained by checking the mass balance and the saltwater heads. At steady state the change in storage, which includes elastic storage and interface storage, should be close to zero. Also, the saltwater heads should be close to zero unless there are zones

where there is freshwater leakage into saltwater. Both of these conditions are satisfied in this simulation.

As discussed in the practical considerations section, the mass balance error is expressed as a percentage relative to total influx and change in storage. The freshwater mass balance indicates that $8.979 \text{ ft}^3/\text{s}$ of freshwater water enters the system as recharge. One ft^3/s of freshwater is pumped out of the aquifer, and $7.979 \text{ ft}^3/\text{s}$ leaves the aquifer by leakage across the top of the aquifer (leakage through the caprock to the sea). The freshwater mass balance error relative to total influx is very small (3.4×10^{-6} percent), indicating an accurate solution. The error relative to change in storage appears to be large because at steady-state the change in storage is very small. There is no saltwater influx to the system, and therefore no saltwater mass balance error relative to influx is reported. As in the case of freshwater, the error relative to change in storage is large. This example illustrates that the user must interpret the mass balance carefully.

Default output options are used in this example resulting in the output of freshwater and saltwater heads, interface elevations, and a map of block codes (F, M, S). Values reported as 11111. indicate absence of the attribute at that block. For example, a freshwater head of 11111. indicates that there is no freshwater in the block.

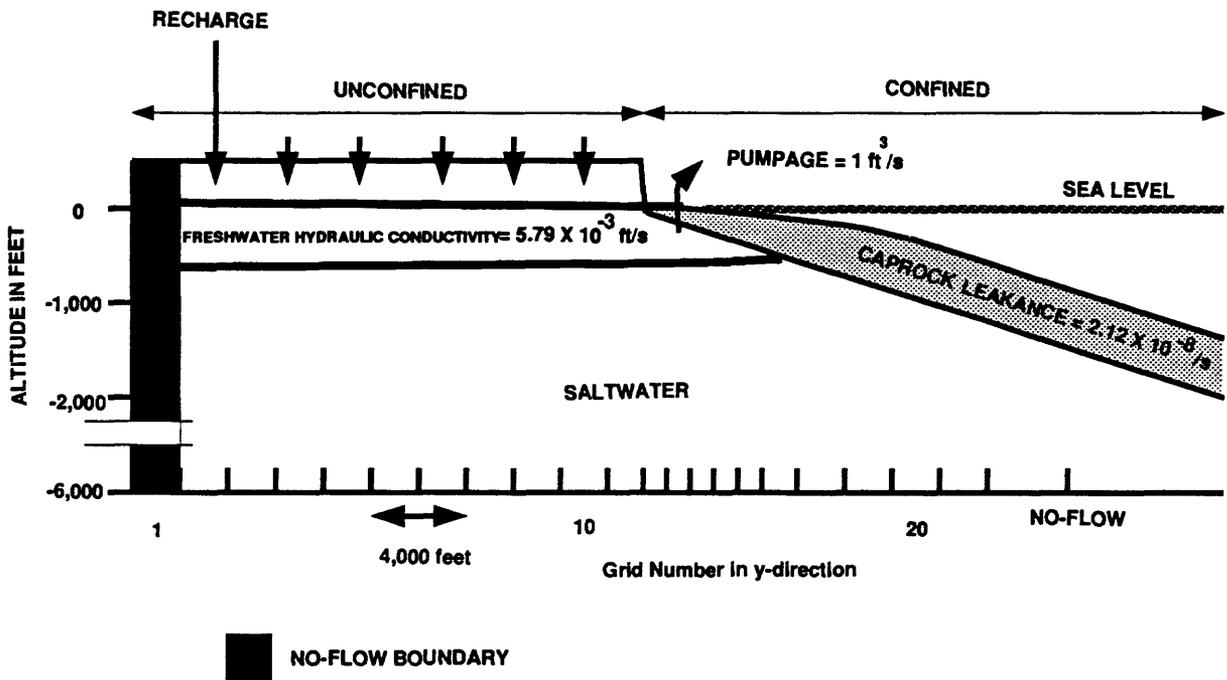


Figure 22.--Southeast Oahu simulation:
A. cross-section showing model geometry.

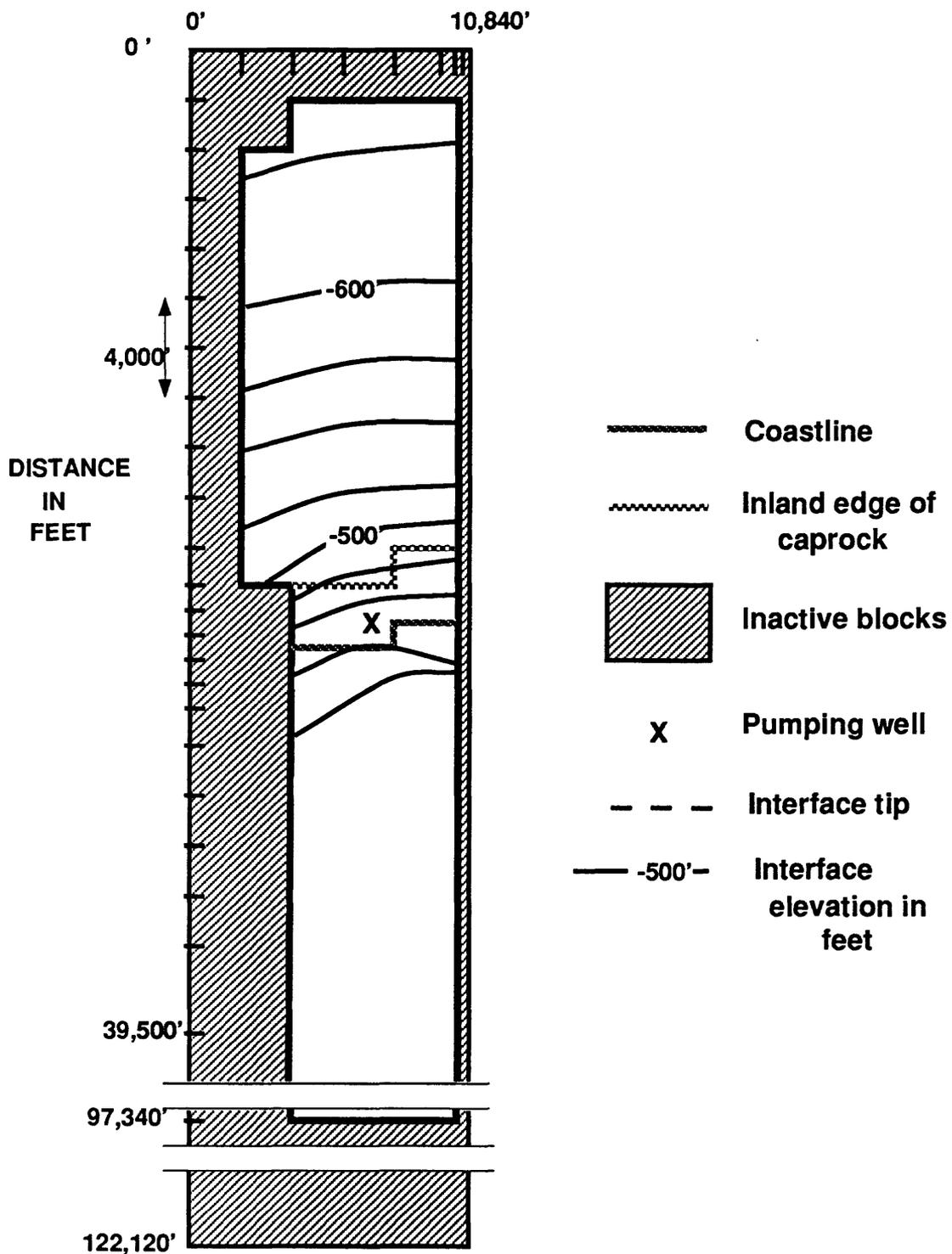


Figure 22.--Southeast Oahu simulation--continued:
 B. map view showing steady-state interface elevation.

Table 7.--Input file for example 1--continued

0.650E+04	0.650E+04	0.600E+04	0.600E+04	0.600E+04	0.600E+04	0.600E+04	0.590E+04
0.650E+04	0.650E+04	0.600E+04	0.600E+04	0.590E+04	0.590E+04	0.590E+04	0.573E+04
0.650E+04	0.650E+04	0.591E+04	0.591E+04	0.581E+04	0.581E+04	0.581E+04	0.590E+04
0.650E+04	0.650E+04	0.582E+04	0.582E+04	0.572E+04	0.572E+04	0.572E+04	0.550E+04
0.650E+04	0.650E+04	0.573E+04	0.573E+04	0.563E+04	0.563E+04	0.563E+04	0.590E+04
0.650E+04	0.650E+04	0.562E+04	0.562E+04	0.552E+04	0.552E+04	0.552E+04	0.540E+04
0.650E+04	0.650E+04	0.546E+04	0.546E+04	0.536E+04	0.536E+04	0.536E+04	0.510E+04
0.650E+04	0.650E+04	0.528E+04	0.528E+04	0.518E+04	0.518E+04	0.518E+04	0.480E+04
0.650E+04	0.650E+04	0.510E+04	0.510E+04	0.500E+04	0.500E+04	0.500E+04	0.465E+04
0.650E+04	0.650E+04	0.492E+04	0.492E+04	0.482E+04	0.482E+04	0.482E+04	0.450E+04
0.650E+04	0.650E+04	0.471E+04	0.471E+04	0.460E+04	0.460E+04	0.460E+04	0.420E+04
0.650E+04	0.650E+04	0.423E+04	0.423E+04	0.412E+04	0.412E+04	0.412E+04	0.350E+04
0.650E+04	0.650E+04	0.350E+04	0.350E+04	0.350E+04	0.350E+04	0.350E+04	0.350E+04
0.650E+04	0.650E+04	0.350E+04	0.350E+04	0.350E+04	0.350E+04	0.350E+04	0.350E+04
0.650E+04	0.650E+04	0.350E+04	0.350E+04	0.350E+04	0.350E+04	0.350E+04	0.350E+04
0.650E+04	0.650E+04	0.350E+04	0.350E+04	0.350E+04	0.350E+04	0.350E+04	0.350E+04
0.650E+04	0.650E+04	0.650E+04	0.650E+04	0.650E+04	0.650E+04	0.650E+04	0.650E+04
1-0.600E+04			ZBOT				
0	1.00		PHIF				
0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000
0.00000	0.00000	14.73666	14.72435	14.69776	14.69058	14.68993	0.00000
0.00000	14.62849	14.64578	14.62728	14.59585	14.58804	14.58745	0.00000
0.00000	14.47214	14.45803	14.42619	14.39189	14.38526	14.38487	0.00000
0.00000	14.18175	14.15061	14.11768	14.08651	14.08082	14.08042	0.00000
0.00000	13.78930	13.75558	13.72692	13.71053	13.70655	13.70611	0.00000
0.00000	13.31096	13.29549	13.26575	13.25028	13.24609	13.24553	0.00000
0.00000	12.79268	12.76784	12.72685	12.69833	12.69146	12.69086	0.00000
0.00000	12.24402	12.17205	12.09534	12.03961	12.02980	12.02910	0.00000
0.00000	11.67991	11.51296	11.36790	11.25449	11.22933	11.22824	0.00000
0.00000	11.23644	10.76127	10.49894	10.13175	10.07038	10.06757	0.00000
0.00000	0.00000	10.13260	9.96250	9.80279	9.76959	9.76798	0.00000
0.00000	0.00000	9.50360	9.42700	9.47479	9.46919	9.46888	0.00000
0.00000	0.00000	9.31360	9.30150	9.47479	9.46919	9.46888	0.00000
0.00000	0.00000	9.12149	9.17606	9.50000	9.50000	9.50000	0.00000
0.00000	0.00000	9.12149	9.17606	9.25000	9.25000	9.25000	0.00000
0.00000	0.00000	9.53750	9.52750	9.00000	9.00000	9.00000	0.00000
0.00000	0.00000	9.00000	9.00000	9.00000	9.00000	9.00000	0.00000
0.00000	0.00000	8.50000	8.50000	8.50000	8.50000	8.50000	0.00000
0.00000	0.00000	8.00000	8.00000	8.00000	8.00000	8.00000	0.00000
0.00000	0.00000	7.50000	7.50000	7.50000	7.50000	7.50000	0.00000
0.00000	0.00000	7.00000	7.00000	7.00000	7.00000	7.00000	0.00000
0.00000	0.00000	6.50000	6.50000	6.50000	6.50000	6.50000	0.00000
0.00000	0.00000	6.00000	6.00000	6.00000	6.00000	6.00000	0.00000
0.00000	0.00000	5.50000	5.50000	5.50000	5.50000	5.50000	0.00000
0.00000	0.00000	5.00000	5.00000	5.00000	5.00000	5.00000	0.00000
0.00000	0.00000	4.50000	4.50000	4.50000	4.50000	4.50000	0.00000
0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000
1	2.12E-08		AQL				

Table 7.--Input file for example 1--continued

0.100	0.160	0.172	0.350	0.300	0.151	0.212	0.212
0.100	0.086	0.151	0.151	0.100	0.100	0.100	0.100
0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100
0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100
0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100
0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100
0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100
0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100
0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100
0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100
0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100
0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100
0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100
0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100
0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100
0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100
0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100
0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100
0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100
0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100
0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100

Table 8.--Output file for example 1

AREAL MODEL OF SE OAHU

NUMBER OF ROWS= 28
NUMBER OF COLUMNS= 8
NUMBER OF AQUIFERS= 1
NUMBER OF TIME STEPS BETWEEN PRINTOUT= 10
MAXIMUM NUMBER OF ITERATIONS= 60
NUMBER OF ITERATION PARAMETERS= 6
NUMBER OF PUMPING PERIODS= 1
INTERFACE FIXED AFTER 50 ITERATIONS
INPUT/OUTPUT OPTIONS' PARAMETER= 0 (0=USE DEFAULTS, 1=READ IN OPTIONS ARRAY)
LEAKAGE OPTION PARAMETER= 0 (0=RESTRICTED MIXING, 1=COMPLETE MIXING)

CONVERGENCE CRITERION= 0.1000E-05
STEADY STATE CRITERION= 0.1000E-05
RELAXATION FACTOR= 0.8000
WEIGHTING FACTOR= 0.5000
ITERATION PARAMETER FACTOR= 1.000
SPECIFIC GRAVITY OF FRESHWATER= 62.41
SPECIFIC GRAVITY OF SALTWATER= 63.97
VISCOSITY OF FRESHWATER= 0.2090E-04
VISCOSITY OF SALTWATER= 0.2090E-04

NEW RUN

INTERFACE ELEVATIONS INITIALIZED TO -DEL*PHIF

LOCATIONS OF 1 OBSERVATION NODES TRACKED:

10 4 1

SPECIFIED INPUT/OUTPUT OPTIONS:

1: 1=PRINT INPUT AQUIFER PARAMETERS, 0=DO NOT PRINT
1: 1=PRINT INPUT PUMPING PERIOD WELL AND RECHARGE DATA, 0=DO NOT PRINT
1: 1=PRINT CALCULATED FRESHWATER HEADS, 0=DO NOT PRINT
1: 1=PRINT CALCULATED SALTWATER HEADS, 0=DO NOT PRINT
1: 1=PRINT CALCULATED INTERFACE ELEVATIONS, 0=DO NOT PRINT
0: 1=PRINT CALCULATED FAREA FACTORS, 0=DO NOT PRINT
0: 1=PRINT CALCULATED SAREA FACTORS, 0=DO NOT PRINT
1: 1=PRINT F-M-S MAP, 0=DO NOT PRINT
1: 1=WRITE BLOCK LEAKAGE VALUES TO FILE, 0=DO NOT WRITE TO FILE
0: 1=PRINT ITERATION INFORMATION TO SCREEN, 0=DO NOT PRINT
0: 0=REWRITE LATEST RESULTS TO FILE AFTER EACH TIME STEP,
1=WRITE RESULTS TO FILE EVERY NP TIME STEPS

Table 8.--Output file for example 1--continued

 AQUIFER PARAMETERS FOR LAYER 1

HYDRAULIC CONDUCTIVITY OF FRESHWATER IN X-DIRECTION

1	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
2	0.0000	0.0000	0.5790E-02	0.5790E-02	0.5790E-02	0.5790E-02	0.5790E-02	0.5790E-02	0.0000
3	0.0000	0.5790E-02	0.0000						
4	0.0000	0.5790E-02	0.0000						
5	0.0000	0.5790E-02	0.0000						
6	0.0000	0.5790E-02	0.0000						
7	0.0000	0.5790E-02	0.0000						
8	0.0000	0.5790E-02	0.0000						
9	0.0000	0.5790E-02	0.0000						
10	0.0000	0.5790E-02	0.0000						
11	0.0000	0.5790E-02	0.0000						
12	0.0000	0.0000	0.5790E-02	0.5790E-02	0.5790E-02	0.5790E-02	0.5790E-02	0.5790E-02	0.0000
13	0.0000	0.0000	0.5790E-02	0.5790E-02	0.5790E-02	0.5790E-02	0.5790E-02	0.5790E-02	0.0000
14	0.0000	0.0000	0.5790E-02	0.5790E-02	0.5790E-02	0.5790E-02	0.5790E-02	0.5790E-02	0.0000
15	0.0000	0.0000	0.5790E-02	0.5790E-02	0.5790E-02	0.5790E-02	0.5790E-02	0.5790E-02	0.0000
16	0.0000	0.0000	0.5790E-02	0.5790E-02	0.5790E-02	0.5790E-02	0.5790E-02	0.5790E-02	0.0000
17	0.0000	0.0000	0.5790E-02	0.5790E-02	0.5790E-02	0.5790E-02	0.5790E-02	0.5790E-02	0.0000
18	0.0000	0.0000	0.5790E-02	0.5790E-02	0.5790E-02	0.5790E-02	0.5790E-02	0.5790E-02	0.0000
19	0.0000	0.0000	0.5790E-02	0.5790E-02	0.5790E-02	0.5790E-02	0.5790E-02	0.5790E-02	0.0000
20	0.0000	0.0000	0.5790E-02	0.5790E-02	0.5790E-02	0.5790E-02	0.5790E-02	0.5790E-02	0.0000
21	0.0000	0.0000	0.5790E-02	0.5790E-02	0.5790E-02	0.5790E-02	0.5790E-02	0.5790E-02	0.0000
	0.0000	0.0000	0.5790E-02	0.5790E-02	0.5790E-02	0.5790E-02	0.5790E-02	0.5790E-02	0.0000
	0.0000	0.0000	0.5790E-02	0.5790E-02	0.5790E-02	0.5790E-02	0.5790E-02	0.5790E-02	0.0000
	0.0000	0.0000	0.5790E-02	0.5790E-02	0.5790E-02	0.5790E-02	0.5790E-02	0.5790E-02	0.0000
26	0.0000	0.0000	0.5790E-02	0.5790E-02	0.5790E-02	0.5790E-02	0.5790E-02	0.5790E-02	0.0000
27	0.0000	0.0000	0.5790E-02	0.5790E-02	0.5790E-02	0.5790E-02	0.5790E-02	0.5790E-02	0.0000
28	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000

HYDRAULIC CONDUCTIVITY OF FRESHWATER IN Y-DIRECTION

0.5790E-02

FRESHWATER SPECIFIC STORAGE

0.1000E-08

SALTWATER SPECIFIC STORAGE

0.1000E-08

EFFECTIVE POROSITY

0.1000E-03

Table 8.--Output file for example 1--continued

AQUIFER THICKNESS

1	6500.	6500.	6500.	6500.	6500.	6500.	6500.	6500.
2	6500.	6500.	6500.	6500.	6500.	6500.	6500.	6500.
3	6500.	6500.	6500.	6500.	6500.	6500.	6500.	6500.
4	6500.	6500.	6500.	6500.	6500.	6500.	6500.	6500.
5	6500.	6500.	6500.	6500.	6500.	6500.	6500.	6500.
6	6500.	6500.	6500.	6500.	6500.	6500.	6500.	6500.
7	6500.	6500.	6500.	6500.	6500.	6500.	6500.	6500.
8	6500.	6500.	6500.	6500.	6500.	6500.	6500.	6500.
9	6500.	6500.	6500.	6500.	6500.	6500.	6500.	6500.
10	6500.	6500.	6500.	6500.	6500.	6500.	6500.	6500.
11	6500.	6500.	6500.	6500.	6000.	6000.	6000.	5900.
12	6500.	6500.	6000.	6000.	6000.	6000.	6000.	5900.
13	6500.	6500.	6000.	6000.	5900.	5900.	5900.	5730.
14	6500.	6500.	5910.	5910.	5810.	5810.	5810.	5900.
15	6500.	6500.	5820.	5820.	5720.	5720.	5720.	5500.
16	6500.	6500.	5730.	5730.	5630.	5630.	5630.	5900.
17	6500.	6500.	5620.	5620.	5520.	5520.	5520.	5400.
18	6500.	6500.	5460.	5460.	5360.	5360.	5360.	5100.
19	6500.	6500.	5280.	5280.	5180.	5180.	5180.	4800.
20	6500.	6500.	5100.	5100.	5000.	5000.	5000.	4650.
21	6500.	6500.	4920.	4920.	4820.	4820.	4820.	4500.
22	6500.	6500.	4710.	4710.	4600.	4600.	4600.	4200.
23	6500.	6500.	4230.	4230.	4120.	4120.	4120.	3500.
24	6500.	6500.	3500.	3500.	3500.	3500.	3500.	3500.
25	6500.	6500.	3500.	3500.	3500.	3500.	3500.	3500.
26	6500.	6500.	3500.	3500.	3500.	3500.	3500.	3500.
27	6500.	6500.	3500.	3500.	3500.	3500.	3500.	3500.
28	6500.	6500.	6500.	6500.	6500.	6500.	6500.	6500.

ELEVATION OF BASE OF AQUIFER

-6000.

INITIAL FRESHWATER HEAD

1	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
2	0.0000	0.0000	14.74	14.72	14.70	14.69	14.69	0.0000
3	0.0000	14.63	14.65	14.63	14.60	14.59	14.59	0.0000
4	0.0000	14.47	14.46	14.43	14.39	14.39	14.38	0.0000
5	0.0000	14.18	14.15	14.12	14.09	14.08	14.08	0.0000
6	0.0000	13.79	13.76	13.73	13.71	13.71	13.71	0.0000
7	0.0000	13.31	13.30	13.27	13.25	13.25	13.25	0.0000
8	0.0000	12.79	12.77	12.73	12.70	12.69	12.69	0.0000
9	0.0000	12.24	12.17	12.10	12.04	12.03	12.03	0.0000
10	0.0000	11.68	11.51	11.37	11.25	11.23	11.23	0.0000
11	0.0000	11.24	10.76	10.50	10.13	10.07	10.07	0.0000
12	0.0000	0.0000	10.13	9.962	9.803	9.770	9.768	0.0000
13	0.0000	0.0000	9.504	9.427	9.475	9.469	9.469	0.0000
14	0.0000	0.0000	9.314	9.302	9.475	9.469	9.469	0.0000
15	0.0000	0.0000	9.121	9.176	9.500	9.500	9.500	0.0000
16	0.0000	0.0000	9.121	9.176	9.250	9.250	9.250	0.0000

Table 8.--Output file for example 1--continued

17	0.0000	0.0000	9.538	9.528	9.000	9.000	9.000	0.0000
18	0.0000	0.0000	9.000	9.000	9.000	9.000	9.000	0.0000
19	0.0000	0.0000	8.500	8.500	8.500	8.500	8.500	0.0000
20	0.0000	0.0000	8.000	8.000	8.000	8.000	8.000	0.0000
21	0.0000	0.0000	7.500	7.500	7.500	7.500	7.500	0.0000
22	0.0000	0.0000	7.000	7.000	7.000	7.000	7.000	0.0000
23	0.0000	0.0000	6.500	6.500	6.500	6.500	6.500	0.0000
24	0.0000	0.0000	6.000	6.000	6.000	6.000	6.000	0.0000
25	0.0000	0.0000	5.500	5.500	5.500	5.500	5.500	0.0000
26	0.0000	0.0000	5.000	5.000	5.000	5.000	5.000	0.0000
27	0.0000	0.0000	4.500	4.500	4.500	4.500	4.500	0.0000
28	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000

INITIAL INTERFACE ELEVATION

1	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
2	0.0000	0.0000	-589.5	-589.0	-587.9	-587.6	-587.6	0.0000
3	0.0000	-585.1	-585.8	-585.1	-583.8	-583.5	-583.5	0.0000
4	0.0000	-578.9	-578.3	-577.0	-575.7	-575.4	-575.4	0.0000
5	0.0000	-567.3	-566.0	-564.7	-563.5	-563.2	-563.2	0.0000
6	0.0000	-551.6	-550.2	-549.1	-548.4	-548.3	-548.2	0.0000
7	0.0000	-532.4	-531.8	-530.6	-530.0	-529.8	-529.8	0.0000
8	0.0000	-511.7	-510.7	-509.1	-507.9	-507.7	-507.6	0.0000
9	0.0000	-489.8	-486.9	-483.8	-481.6	-481.2	-481.2	0.0000
10	0.0000	-467.2	-460.5	-454.7	-450.2	-449.2	-449.1	0.0000
11	0.0000	-449.5	-430.5	-420.0	-405.3	-402.8	-402.7	0.0000
12	0.0000	0.0000	-405.3	-398.5	-392.1	-390.8	-390.7	0.0000
13	0.0000	0.0000	-380.1	-377.1	-379.0	-378.8	-378.8	0.0000
14	0.0000	0.0000	-372.5	-372.1	-379.0	-378.8	-378.8	0.0000
15	0.0000	0.0000	-364.9	-367.0	-380.0	-380.0	-380.0	0.0000
16	0.0000	0.0000	-364.9	-367.0	-370.0	-370.0	-370.0	0.0000
17	0.0000	0.0000	-381.5	-381.1	-360.0	-360.0	-360.0	0.0000
18	0.0000	0.0000	-360.0	-360.0	-360.0	-360.0	-360.0	0.0000
19	0.0000	0.0000	-340.0	-340.0	-340.0	-340.0	-340.0	0.0000
20	0.0000	0.0000	-320.0	-320.0	-320.0	-320.0	-320.0	0.0000
21	0.0000	0.0000	-300.0	-300.0	-300.0	-300.0	-300.0	0.0000
22	0.0000	0.0000	-280.0	-280.0	-280.0	-280.0	-280.0	0.0000
23	0.0000	0.0000	-260.0	-260.0	-260.0	-260.0	-260.0	0.0000
24	0.0000	0.0000	-240.0	-240.0	-240.0	-240.0	-240.0	0.0000
25	0.0000	0.0000	-220.0	-220.0	-220.0	-220.0	-220.0	0.0000
26	0.0000	0.0000	-200.0	-200.0	-200.0	-200.0	-200.0	0.0000
27	0.0000	0.0000	-180.0	-180.0	-180.0	-180.0	-180.0	0.0000
28	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000

INITIAL SALTWATER HEADS

1	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
2	0.0000	0.0000	0.0000	0.0000	-0.1137E-12	-0.1137E-12	0.0000	0.0000
3	0.0000	-0.1137E-12	-0.1137E-12	0.0000	0.1137E-12	0.0000	-0.1137E-12	0.0000
4	0.0000	0.0000	0.0000	-0.1137E-12	0.0000	-0.1137E-12	0.0000	0.0000
5	0.0000	-0.1137E-12	0.1137E-12	0.0000	0.0000	-0.1137E-12	0.0000	0.0000
6	0.0000	-0.1137E-12	-0.1137E-12	0.0000	-0.1137E-12	0.0000	0.0000	0.0000
7	0.0000	-0.1137E-12	-0.1137E-12	-0.1137E-12	0.0000	0.0000	-0.1137E-12	0.0000

Table 8.--Output file for example 1--continued

8	0.0000	-0.1137E-12	-0.1137E-12	-0.1137E-12	0.0000	-0.1137E-12	-0.1137E-12	0.0000
9	0.0000	-0.1137E-12	-0.1137E-12	-0.1137E-12	-0.1137E-12	-0.1137E-12	-0.1137E-12	0.0000
10	0.0000	-0.1137E-12	-0.1137E-12	-0.1137E-12	-0.1137E-12	0.0000	-0.1137E-12	0.0000
11	0.0000	-0.1137E-12	-0.1137E-12	-0.1137E-12	-0.1137E-12	0.0000	-0.1137E-12	0.0000
12	0.0000	0.0000	0.0000	-0.1137E-12	-0.1137E-12	-0.1137E-12	-0.1137E-12	0.0000
13	0.0000	0.0000	-0.1137E-12	-0.1137E-12	-0.1137E-12	0.0000	-0.1137E-12	0.0000
14	0.0000	0.0000	-0.1137E-12	-0.1137E-12	-0.1137E-12	0.0000	-0.1137E-12	0.0000
15	0.0000	0.0000	-0.1137E-12	-0.1137E-12	-0.1137E-12	-0.1137E-12	-0.1137E-12	0.0000
16	0.0000	0.0000	-0.1137E-12	-0.1137E-12	-0.1137E-12	-0.1137E-12	-0.1137E-12	0.0000
17	0.0000	0.0000	-0.1137E-12	-0.1137E-12	-0.1137E-12	-0.1137E-12	-0.1137E-12	0.0000
18	0.0000	0.0000	-0.1137E-12	-0.1137E-12	-0.1137E-12	-0.1137E-12	-0.1137E-12	0.0000
19	0.0000	0.0000	-0.1137E-12	-0.1137E-12	-0.1137E-12	-0.1137E-12	-0.1137E-12	0.0000
20	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
21	0.0000	0.0000	-0.5684E-13	-0.5684E-13	-0.5684E-13	-0.5684E-13	-0.5684E-13	0.0000
22	0.0000	0.0000	-0.5684E-13	-0.5684E-13	-0.5684E-13	-0.5684E-13	-0.5684E-13	0.0000
23	0.0000	0.0000	-0.5684E-13	-0.5684E-13	-0.5684E-13	-0.5684E-13	-0.5684E-13	0.0000
24	0.0000	0.0000	-0.5684E-13	-0.5684E-13	-0.5684E-13	-0.5684E-13	-0.5684E-13	0.0000
25	0.0000	0.0000	-0.5684E-13	-0.5684E-13	-0.5684E-13	-0.5684E-13	-0.5684E-13	0.0000
26	0.0000	0.0000	-0.5684E-13	-0.5684E-13	-0.5684E-13	-0.5684E-13	-0.5684E-13	0.0000
27	0.0000	0.0000	-0.5684E-13	-0.5684E-13	-0.5684E-13	-0.5684E-13	-0.5684E-13	0.0000
28	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000

AQUITARD LEAKANCES (K"/B")

0.2120E-07

HEAD IN OVERLYING AQUIFER

1	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
2	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
3	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
4	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
5	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
6	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
7	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
8	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
9	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
10	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
11	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
12	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
13	0.0000	0.0000	0.0000	0.0000	1.250	1.250	1.250	0.0000
14	0.0000	0.0000	1.250	1.250	3.750	3.750	3.750	0.0000
15	0.0000	0.0000	1.500	1.500	4.000	4.000	4.000	0.0000
16	0.0000	0.0000	1.750	1.750	4.250	4.250	4.250	0.0000
17	0.0000	0.0000	2.000	2.000	4.500	4.500	4.500	0.0000
18	0.0000	0.0000	3.500	3.500	6.000	6.000	6.000	0.0000
19	0.0000	0.0000	5.500	5.500	8.000	8.000	8.000	0.0000
20	0.0000	0.0000	7.500	7.500	10.00	10.00	10.00	0.0000
21	0.0000	0.0000	12.00	12.00	14.50	14.50	14.50	0.0000
22	0.0000	0.0000	17.25	17.25	20.00	20.00	20.00	0.0000
23	0.0000	0.0000	29.25	29.25	32.00	32.00	32.00	0.0000
24	0.0000	0.0000	47.50	47.50	47.50	47.50	47.50	0.0000

Table 8.--Output file for example 1--continued

25	0.0000	0.0000	47.50	47.50	47.50	47.50	47.50	0.0000
26	0.0000	0.0000	47.50	47.50	47.50	47.50	47.50	0.0000
27	0.0000	0.0000	47.50	47.50	47.50	47.50	47.50	0.0000
28	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000

BATHYMETRY

1	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
2	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
3	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
4	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
5	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
6	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
7	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
8	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
9	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
10	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
11	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
12	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
13	0.0000	0.0000	0.0000	0.0000	-50.00	-50.00	-50.00	0.0000
14	0.0000	0.0000	-50.00	-50.00	-150.0	-150.0	-150.0	0.0000
15	0.0000	0.0000	-60.00	-60.00	-160.0	-160.0	-160.0	0.0000
16	0.0000	0.0000	-70.00	-70.00	-170.0	-170.0	-170.0	0.0000
17	0.0000	0.0000	-80.00	-80.00	-180.0	-180.0	-180.0	0.0000
18	0.0000	0.0000	-140.0	-140.0	-240.0	-240.0	-240.0	0.0000
19	0.0000	0.0000	-220.0	-220.0	-320.0	-320.0	-320.0	0.0000
20	0.0000	0.0000	-300.0	-300.0	-400.0	-400.0	-400.0	0.0000
21	0.0000	0.0000	-480.0	-480.0	-580.0	-580.0	-580.0	0.0000
22	0.0000	0.0000	-690.0	-690.0	-800.0	-800.0	-800.0	0.0000
23	0.0000	0.0000	-1170.	-1170.	-1280.	-1280.	-1280.	0.0000
24	0.0000	0.0000	-1900.	-1900.	-1900.	-1900.	-1900.	0.0000
25	0.0000	0.0000	-1900.	-1900.	-1900.	-1900.	-1900.	0.0000
26	0.0000	0.0000	-1900.	-1900.	-1900.	-1900.	-1900.	0.0000
27	0.0000	0.0000	-1900.	-1900.	-1900.	-1900.	-1900.	0.0000
28	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000

CODE FOR UPPER LAYER (1-UNCONFINED NODE,0-CONFINED NODE)

1	1	1	1	1	1	1	1	1
2	1	1	1	1	1	1	1	1
3	1	1	1	1	1	1	1	1
4	1	1	1	1	1	1	1	1
5	1	1	1	1	1	1	1	1
6	1	1	1	1	1	1	1	1
7	1	1	1	1	1	1	1	1
8	1	1	1	1	1	1	1	1
9	1	1	1	1	1	1	1	1
10	1	1	1	1	1	1	1	1
11	1	1	1	1	0	0	0	0
12	0	0	0	0	0	0	0	0
13	0	0	0	0	0	0	0	0
14	0	0	0	0	0	0	0	0

Table 8.--Output file for example 1--continued

15	0	0	0	0	0	0	0	0
16	0	0	0	0	0	0	0	0
17	0	0	0	0	0	0	0	0
18	0	0	0	0	0	0	0	0
19	0	0	0	0	0	0	0	0
20	0	0	0	0	0	0	0	0
21	0	0	0	0	0	0	0	0
22	0	0	0	0	0	0	0	0
23	0	0	0	0	0	0	0	0
24	0	0	0	0	0	0	0	0
25	0	0	0	0	0	0	0	0
26	0	0	0	0	0	0	0	0
27	0	0	0	0	0	0	0	0
28	0	0	0	0	0	0	0	0

DELTA X

2000.	2000.	2000.	2000.	2000.	600.0	120.0	120.0
-------	-------	-------	-------	-------	-------	-------	-------

DELTA Y

2000.	2000.	2000.	2000.	2000.	2000.	2000.	2000.	2000.	2000.
1500.	1000.	1000.	1000.	1000.	1000.	1500.	2000.	2000.	2000.
2000.	3500.	6750.	0.1012E+05	0.1619E+05	0.2478E+05	0.2478E+05	0.2478E+05		

ITERATION PARAMETERS:

FRESHWATER EQUATION:	.0000	.9570	.9981	.9999	1.000	1.000
----------------------	-------	-------	-------	-------	-------	-------

PUMPING PERIOD 1

INITIAL TIME STEP(SEC)= 0.1578E+08
MULTIPLICATION FACTOR FOR DELTA T= 1.000
LENGTH OF PUMPING PERIOD(DAYS)= 0.2045E+05
MAXIMUM NUMBER OF TIME STEPS= 10
NUMBER OF ACTIVE WELLS= 1

PUMPING NODES:

I	J	K	PUMPAGE	SCREEN TOP	SCREEN BOTTOM
13	4	1	1.000	-280.0	-290.0

RECHARGE

1	0.1000E-07								
2	0.1000E-07	0.1000E-07	0.9000E-07	0.9700E-07	0.7500E-07	0.6760E-07	0.3600E-07	0.3600E-07	0.3600E-07
3	0.1000E-07	0.1200E-06	0.1140E-06	0.1000E-06	0.7000E-07	0.6000E-07	0.3800E-07	0.3300E-07	0.3300E-07
4	0.1000E-07	0.1240E-06	0.1140E-06	0.9000E-07	0.6200E-07	0.6000E-07	0.5200E-07	0.2700E-07	0.2700E-07

Table 8.--Output file for example 1--continued

5 0.1000E-07 0.1050E-06 0.6760E-07 0.6000E-07 0.3330E-07 0.3330E-07 0.2030E-07 0.2030E-07
6 0.1000E-07 0.8650E-07 0.3800E-07 0.3650E-07 0.4970E-07 0.4970E-07 0.2670E-07 0.3400E-07
7 0.1000E-07 0.2900E-07 0.5000E-07 0.3500E-07 0.4970E-07 0.4970E-07 0.1840E-07 0.1840E-07
8 0.1000E-07 0.2500E-07 0.4500E-07 0.4000E-07 0.4500E-07 0.4040E-07 0.1630E-07 0.1320E-07
9 0.1000E-07 0.4500E-07 0.2540E-07 0.2600E-07 0.3000E-07 0.4040E-07 0.2120E-07 0.5900E-08
10 0.1000E-07 0.1600E-07 0.1720E-07 0.3500E-07 0.3000E-07 0.1510E-07 0.2120E-07 0.2120E-07
11 0.1000E-07 0.8600E-08 0.1510E-07 0.1510E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07
12 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07
13 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07
14 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07
15 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07
16 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07
17 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07
18 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07
19 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07
20 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07
21 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07
22 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07
23 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07
24 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07
25 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07
26 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07
27 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07
28 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07 0.1000E-07

TIME STEP 1

	SECONDS	HOURS	DAYS	YEARS
TIME STEP SIZE:	0.1578E+08	4383.	182.6	0.5000
PUMPING PERIOD TIME:	0.1578E+08	4383.	182.6	0.5000
TOTAL SIMULATION TIME:	0.1578E+08	4383.	182.6	0.5000

MAXIMUM FRESHWATER HEAD CHANGE: 0.6508 0.2995 0.9971 1.357 0.6734 0.2476 0.6640E-01 0.1621
0.1427 0.1115 0.2869E-01 0.2119E-01 0.6646E-02 0.3227E-01 0.9569E-02 0.5877E-01
0.4315E-01 0.1452E-01 0.2436E-02 0.1173E-01 0.8951E-02 0.1973E-01 0.9869E-02 0.2029E-02
0.4931E-03 0.4165E-02 0.1591E-02 0.7814E-02 0.4815E-02 0.1351E-02 0.2526E-03 0.1524E-02
0.8791E-03 0.2821E-02 0.1591E-02 0.3990E-03 0.8196E-04 0.5699E-03 0.2746E-03 0.1062E-02
0.6253E-03 0.1661E-03 0.3250E-04 0.2110E-03 0.1110E-03 0.3922E-03 0.2264E-03 0.5862E-04
0.1172E-04 0.7849E-04 0.5869E-04 0.5953E-04 0.1510E-04 0.2790E-05 0.1007E-05 0.1095E-04
0.7564E-05 0.6274E-05 0.8986E-06

MAXIMUM SALTWATER HEAD CHANGE: 0.3086E-05 0.2250E-04 0.2236E-02 0.1763E-01 0.4982E-02 0.7243E-04 0.9547E-04 0.3859E-03
0.7973E-03 0.4679E-03 0.5008E-03 0.1623E-04 0.4996E-04 0.9853E-04 0.1150E-03 0.7365E-03
0.5743E-03 0.7263E-05 0.6959E-05 0.4803E-04 0.8676E-04 0.1424E-03 0.1473E-03 0.2305E-05
0.4541E-05 0.1582E-04 0.1266E-04 0.7526E-04 0.6680E-04 0.8891E-06 0.1307E-05 0.6268E-05
0.8386E-05 0.2392E-04 0.2274E-04 0.3065E-06 0.5512E-06 0.2259E-05 0.2485E-05 0.9583E-05
0.8806E-05 0.1180E-06 0.1934E-06 0.8511E-06 0.1032E-05 0.3440E-05 0.3210E-05 0.4312E-07
0.7380E-07 0.3141E-06 0.5071E-06 0.1396E-05 0.2529E-06 0.8121E-08 0.7755E-08 0.7228E-07
0.1193E-06 0.1950E-06 0.9872E-08

Table 8.--Output file for example 1--continued

 TIME STEP 2

	SECONDS	HOURS	DAYS	YEARS
TIME STEP SIZE:	0.1578E+08	4383.	182.6	0.5000
PUMPING PERIOD TIME:	0.3156E+08	8766.	365.2	1.000
TOTAL SIMULATION TIME:	0.3156E+08	8766.	365.2	1.000

MAXIMUM FRESHWATER HEAD CHANGE: 0.7347E-03 0.3701E-02 0.2681E-01 0.6498E-01 0.3909E-01 0.9040E-02 0.3279E-02 0.1499E-01
 0.1152E-01 0.2180E-01 0.1084E-01 0.2252E-02 0.5394E-03 0.4873E-02 0.1736E-02 0.9094E-02
 0.5664E-02 0.1616E-02 0.2952E-03 0.1759E-02 0.1028E-02 0.3231E-02 0.1810E-02 0.4501E-03
 0.9230E-04 0.6538E-03 0.3075E-03 0.1213E-02 0.7154E-03 0.1911E-03 0.3700E-04 0.2406E-03
 0.1264E-03 0.4446E-03 0.2560E-03 0.6623E-04 0.1315E-04 0.8903E-04 0.4441E-04 0.1648E-03
 0.9602E-04 0.2524E-04 0.4950E-05 0.3286E-04 0.1681E-04 0.6078E-04 0.3521E-04 0.9185E-05
 0.1812E-05 0.1214E-04 0.9084E-05 0.9271E-05 0.2377E-05 0.4581E-06

MAXIMUM SALTWATER HEAD CHANGE: 0.5681E-04 0.3210E-03 0.2241E-02 0.7197E-02 0.1777E-02 0.3998E-04 0.3979E-04 0.2041E-03
 0.7214E-03 0.3438E-03 0.1639E-03 0.5878E-05 0.9704E-05 0.1296E-04 0.1995E-04 0.8617E-04
 0.7541E-04 0.1125E-05 0.1517E-05 0.7464E-05 0.1174E-04 0.2616E-04 0.2599E-04 0.3560E-06
 0.6446E-06 0.2576E-05 0.2806E-05 0.1100E-04 0.1009E-04 0.1356E-06 0.2206E-06 0.9689E-06
 0.1176E-05 0.3906E-05 0.3641E-05 0.4902E-07 0.8437E-07 0.3551E-06 0.4065E-06 0.1473E-05
 0.1361E-05 0.1828E-07 0.3064E-07 0.1317E-06 0.1549E-06 0.5387E-06 0.4998E-06 0.6722E-08
 0.1141E-07 0.4855E-07 0.7935E-07 0.2177E-06 0.3987E-07 0.1262E-08

 TIME STEP 3

	SECONDS	HOURS	DAYS	YEARS
TIME STEP SIZE:	0.1578E+08	4383.	182.6	0.5000
PUMPING PERIOD TIME:	0.4734E+08	0.1315E+05	547.9	1.500
TOTAL SIMULATION TIME:	0.4734E+08	0.1315E+05	547.9	1.500

MAXIMUM FRESHWATER HEAD CHANGE: 0.5195E-04 0.1719E-03 0.1276E-02 0.3205E-02 0.1984E-02 0.4638E-03 0.1622E-03 0.7518E-03
 0.5747E-03 0.1111E-02 0.5560E-03 0.1173E-03 0.2773E-04 0.2467E-03 0.8939E-04 0.4605E-03
 0.2861E-03 0.8140E-04 0.1490E-04 0.8910E-04 0.5182E-04 0.1638E-03 0.9185E-04 0.2289E-04
 0.4685E-05 0.3311E-04 0.1561E-04 0.6140E-04 0.3620E-04 0.9662E-05 0.1871E-05 0.1218E-04
 0.6389E-05 0.2251E-04 0.1296E-04 0.3355E-05 0.6660E-06

MAXIMUM SALTWATER HEAD CHANGE: 0.3108E-05 0.1706E-04 0.1093E-03 0.3529E-03 0.8700E-04 0.1989E-05 0.1948E-05 0.9934E-05
 0.3473E-04 0.1612E-04 0.8281E-05 0.2904E-06 0.4821E-06 0.6670E-06 0.1032E-05 0.4341E-05
 0.3820E-05 0.5674E-07 0.7719E-07 0.3771E-06 0.5873E-06 0.1331E-05 0.1318E-05 0.1801E-07
 0.3260E-07 0.1305E-06 0.1424E-06 0.5567E-06 0.5109E-06 0.6863E-08 0.1118E-07 0.4905E-07
 0.5938E-07 0.1979E-06 0.1844E-06 0.2483E-08 0.4270E-08

 TIME STEP 4

	SECONDS	HOURS	DAYS	YEARS
TIME STEP SIZE:	0.1578E+08	4383.	182.6	0.5000
PUMPING PERIOD TIME:	0.6312E+08	0.1753E+05	730.5	2.000
TOTAL SIMULATION TIME:	0.6312E+08	0.1753E+05	730.5	2.000

Table 8.--Output file for example 1--continued

MAXIMUM FRESHWATER HEAD CHANGE: 0.2263E-05 0.7824E-05 0.5932E-04 0.1510E-03 0.9407E-04 0.2193E-04 0.7664E-05 0.3558E-04
 0.2720E-04 0.5265E-04 0.2641E-04 0.5582E-05 0.1317E-05 0.1169E-04 0.4246E-05 0.2181E-04
 0.1354E-04 0.3852E-05 0.7053E-06
 MAXIMUM SALTWATER HEAD CHANGE: 0.1538E-06 0.8427E-06 0.5361E-05 0.1739E-04 0.4233E-05 0.9800E-07 0.9564E-07 0.4838E-06
 0.1691E-05 0.8000E-06 0.3940E-06 0.1402E-07 0.2318E-07 0.3152E-07 0.4986E-07 0.2051E-06
 0.1806E-06 0.2694E-08 0.3662E-08

 STEADY STATE ACHIEVED AT:

 TIME STEP 5

	SECONDS	HOURS	DAYS	YEARS
TIME STEP SIZE:	0.1578E+08	4383.	182.6	0.5000
PUMPING PERIOD TIME:	0.7889E+08	0.2191E+05	913.1	2.500
TOTAL SIMULATION TIME:	0.7889E+08	0.2191E+05	913.1	2.500

MAXIMUM FRESHWATER HEAD CHANGE: 0.4592E-06
 MAXIMUM SALTWATER HEAD CHANGE: 0.7557E-08

AQUIFER 1

MASS BALANCE:

	FRESHWATER		SALTWATER	
	T.S. RATE	CUM. VOLUME	T.S. RATE	CUM. VOLUME
CHANGE IN STORAGE	0.6918E-10	0.9071E+06	-0.6142E-10	-0.8896E+06
SOURCES				
RECHARGE	8.979	0.7084E+09		
CONSTANT HEAD NODES	0.0000	0.0000	0.0000	0.0000
INJECTION	0.0000	0.0000	0.0000	0.0000
LEAKAGE ACROSS TOP	0.0000	0.0000	0.0000	0.0000
LEAKAGE ACROSS BOT	0.0000	0.0000	0.0000	0.0000
TOTAL	8.979	0.7084E+09	0.0000	0.0000
SINKS				
CONSTANT HEAD NODES	0.0000	0.0000	0.0000	0.0000
PUMPAGE	1.000	0.7889E+08	0.0000	0.0000
LEAKAGE ACROSS TOP	7.979	0.6286E+09	0.6164E-05	0.8897E+06
LEAKAGE ACROSS BOT	0.0000	0.0000	0.0000	0.0000
TOTAL	8.979	0.7075E+09	0.6164E-05	0.8897E+06
SOURCES-SINKS	0.3061E-06	0.9069E+06	-0.6164E-05	-0.8897E+06
RELATIVE ERROR (%)				
INFLUX	-0.3408E-05	0.2579E-04	0.0000	0.0000
STORAGE	-0.4424E+06	0.2014E-01	-0.1004E+08	-0.1064E-01

Table 8.--Output file for example 1--continued

FRESHWATER HEADS

1	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
2	11111.	11111.	15.703	15.691	15.666	15.659	15.659	11111.
3	11111.	15.601	15.617	15.600	15.570	15.563	15.563	11111.
4	11111.	15.455	15.441	15.411	15.379	15.373	15.373	11111.
5	11111.	15.183	15.154	15.123	15.094	15.089	15.088	11111.
6	11111.	14.817	14.786	14.759	14.744	14.740	14.740	11111.
7	11111.	14.373	14.358	14.331	14.317	14.313	14.313	11111.
8	11111.	13.895	13.870	13.833	13.808	13.802	13.802	11111.
9	11111.	13.395	13.321	13.251	13.206	13.199	13.198	11111.
10	11111.	12.893	12.711	12.582	12.500	12.484	12.483	11111.
11	11111.	12.580	12.077	11.875	11.742	11.725	11.724	11111.
12	11111.	11111.	11.428	11.288	11.289	11.289	11.289	11111.
13	11111.	11111.	11.011	10.851	10.974	10.988	10.989	11111.
14	11111.	11111.	10.677	10.600	10.699	10.719	10.720	11111.
15	11111.	11111.	10.366	10.333	10.441	10.468	10.469	11111.
16	11111.	11111.	10.069	10.057	10.200	10.246	10.247	11111.
17	11111.	11111.	9.7139	9.7022	11111.	11111.	11111.	11111.
18	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
19	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
20	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
21	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
22	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
23	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
24	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
25	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
26	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
27	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
28	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.

SALTWATER HEADS

1	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
2	11111.	11111.	0.15917E-050.	15936E-050.	15949E-050.	15961E-050.	15962E-05	11111.
3	11111.	0.15852E-050.	15891E-050.	15905E-050.	15918E-050.	15929E-050.	15930E-05	11111.
4	11111.	0.15804E-050.	15823E-050.	15832E-050.	15843E-050.	15854E-050.	15854E-05	11111.
5	11111.	0.15702E-050.	15715E-050.	15720E-050.	15729E-050.	15740E-050.	15740E-05	11111.
6	11111.	0.15560E-050.	15569E-050.	15570E-050.	15576E-050.	15586E-050.	15587E-05	11111.
7	11111.	0.15380E-050.	15385E-050.	15380E-050.	15383E-050.	15392E-050.	15393E-05	11111.
8	11111.	0.15169E-050.	15164E-050.	15150E-050.	15146E-050.	15154E-050.	15155E-05	11111.
9	11111.	0.14936E-050.	14909E-050.	14877E-050.	14863E-050.	14869E-050.	14869E-05	11111.
10	11111.	0.14703E-050.	14614E-050.	14555E-050.	14529E-050.	14532E-050.	14532E-05	11111.
11	11111.	0.14554E-050.	14292E-050.	14215E-050.	14187E-050.	14191E-050.	14191E-05	11111.
12	11111.	11111.	0.13965E-050.	13931E-050.	13913E-050.	13919E-050.	13920E-05	11111.
13	11111.	11111.	0.13706E-050.	13690E-050.	13678E-050.	13685E-050.	13685E-05	11111.
14	11111.	11111.	0.13445E-050.	13438E-050.	13430E-050.	13437E-050.	13438E-05	11111.
15	11111.	11111.	0.13180E-050.	13177E-050.	13172E-050.	13178E-050.	13179E-05	11111.
16	11111.	11111.	0.12911E-050.	12909E-050.	12905E-050.	12909E-050.	12910E-05	11111.
17	11111.	11111.	0.12569E-050.	12567E-050.	12563E-050.	12565E-050.	12565E-05	11111.
18	11111.	11111.	0.12089E-050.	12088E-050.	12086E-050.	12086E-050.	12086E-05	11111.
19	11111.	11111.	0.11551E-050.	11550E-050.	11549E-050.	11549E-050.	11549E-05	11111.
20	11111.	11111.	0.11025E-050.	11024E-050.	11023E-050.	11022E-050.	11022E-05	11111.

Table 8.--Output file for example 1--continued

21	11111.	11111.	0.10511E-050.10510E-050.10509E-050.10509E-050.10509E-05	11111.
22	11111.	11111.	0.98244E-060.98237E-060.98230E-060.98226E-060.98225E-06	11111.
23	11111.	11111.	0.86045E-060.86036E-060.86020E-060.86015E-060.86015E-06	11111.
24	11111.	11111.	0.67495E-060.67486E-060.67471E-060.67467E-060.67466E-06	11111.
25	11111.	11111.	0.44518E-060.44517E-060.44515E-060.44514E-060.44514E-06	11111.
26	11111.	11111.	0.24076E-060.24076E-060.24075E-060.24075E-060.24075E-06	11111.
27	11111.	11111.	0.14716E-060.14716E-060.14716E-060.14716E-060.14716E-06	11111.
28	11111.	11111.	11111. 11111. 11111. 11111. 11111. 11111.	11111.

INTERFACE ELEVATION

1	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
2	11111.	11111.	-628.10	-627.64	-626.64	-626.37	-626.35	11111.
3	11111.	-624.04	-624.69	-624.00	-622.82	-622.52	-622.50	11111.
4	11111.	-618.18	-617.65	-616.46	-615.18	-614.93	-614.91	11111.
5	11111.	-607.32	-606.15	-604.92	-603.76	-603.55	-603.53	11111.
6	11111.	-592.68	-591.42	-590.36	-589.75	-589.60	-589.59	11111.
7	11111.	-574.93	-574.33	-573.23	-572.68	-572.52	-572.50	11111.
8	11111.	-555.82	-554.82	-553.31	-552.32	-552.09	-552.06	11111.
9	11111.	-535.80	-532.85	-530.05	-528.25	-527.96	-527.93	11111.
10	11111.	-515.71	-508.43	-503.28	-499.99	-499.35	-499.32	11111.
11	11111.	-503.21	-483.10	-475.01	-469.67	-468.98	-468.95	11111.
12	11111.	11111.	-457.13	-451.53	-451.56	-451.54	-451.54	11111.
13	11111.	11111.	-440.42	-434.06	-438.94	-439.52	-439.55	11111.
14	11111.	11111.	-427.06	-424.00	-427.97	-428.75	-428.78	11111.
15	11111.	11111.	-414.63	-413.31	-417.65	-418.70	-418.75	11111.
16	11111.	11111.	-402.77	-402.28	11111.	11111.	11111.	11111.
17	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
18	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
19	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
20	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
21	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
22	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
23	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
24	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
25	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
26	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
27	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
28	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.

Table 8.--Output file for example 1--continued

MAP OF EXTENT OF INTRUSION (F-FRESHWATER, M-FRESH AND SALTWATER, S-SALTWATER)

```
1 . . . . .
2 . . M M M M M .
3 . M M M M M M .
4 . M M M M M M .
5 . M M M M M M .
6 . M M M M M M .
7 . M M M M M M .
8 . M M M M M M .
9 . M M M M M M .
10 . M M M M M M .
11 . M M M M M M .
12 . . M M M M M .
13 . . M M M M M .
14 . . M M M M M .
15 . . M M M M M .
16 . . M M S S S .
17 . . S S S S S .
18 . . S S S S S .
19 . . S S S S S .
20 . . S S S S S .
21 . . S S S S S .
22 . . S S S S S .
23 . . S S S S S .
24 . . S S S S S .
25 . . S S S S S .
26 . . S S S S S .
27 . . S S S S S .
28 . . . . .
```

Example 2: Layered Cross-section of Cape May, New Jersey

The geometry of this problem is described in the model verification section and illustrated in figure 23. It is made up of three confined layers. The simulation shown here is for method 2 leakage conditions (restricted mixing). The input is for a continuation run and therefore includes initial interface elevations and projection factors (FX, FY, SX, and SY) for each layer. Default input/output options are used. Grid spacing in the x-direction is 4000 feet except near the coast where it is reduced to 2000 feet to improve interface projection accuracy. The simulation of a cross-section requires three rows. The middle row is the active region of the cross-section slice, and the rows on either side are inactive. These two rows provide the no-flow boundary condition which prevents flow out the sides of the simulated section. Thus, flow in the y-direction is zero, and the value assigned to FKY does not affect the solution. The value assigned to DY specifies the width of the simulated cross-section.

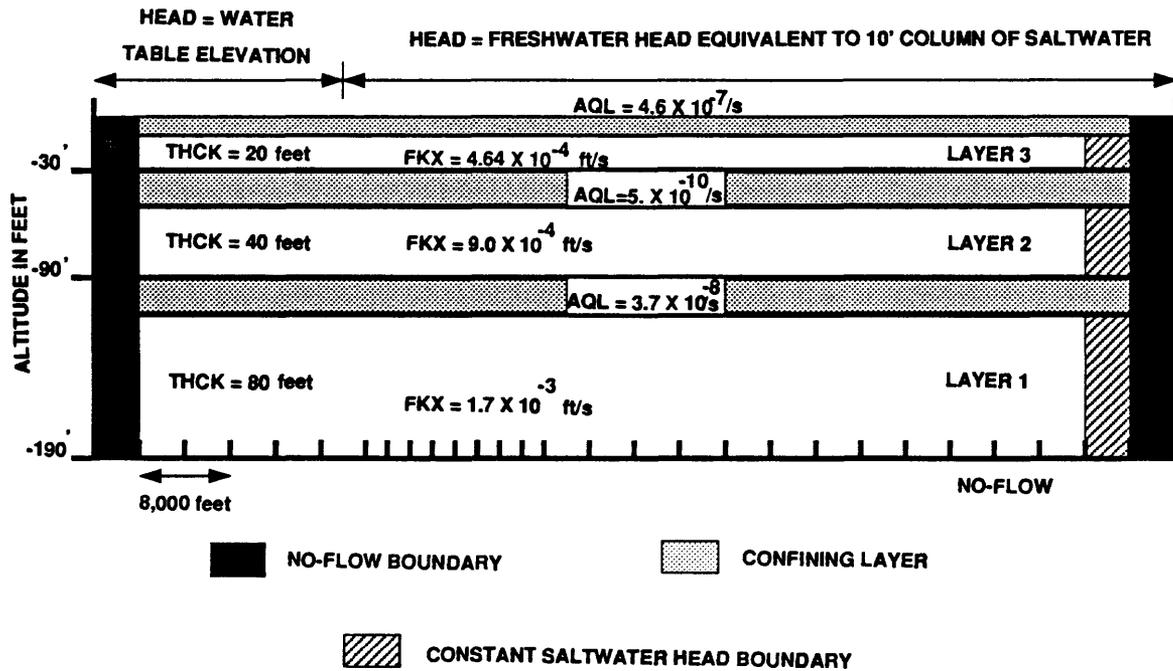


Figure 23.--Cross-section showing the geometry and boundary conditions for the layered example from Cape May, New Jersey. AQL is the confining layer leakance, THCK is the aquifer thickness, and FKX is the aquifer hydraulic conductivity in the x-direction.

A head-dependent boundary is imposed over the top of the upper aquifer. Onshore, the HEAD matrix is assigned the water table elevation, and offshore it is assigned the freshwater head equivalent to a 10 foot deep column of saltwater. A constant saltwater head of 0.0 (sea level) is imposed on the vertical-seaward boundary. The simulation input and output files are given in tables 9 and 10, respectively. In this simulation a relaxation factor (RFAC) of 0.8 was used and an interface projection weighting factor (WFAC) of 0.3 was used. The iteration parameter factor (WITER) is large in this problem (10,000.) because of the strong anisotropy introduced by the dummy FKY value (1.0).

As in example 1, this simulation is a steady-state run. A freshwater and saltwater specific storage of 0.0 is used. At the seaward boundary, the saltwater specific storage is negative to indicate a constant saltwater head boundary. The steady-state criterion (0.0001) is satisfied at time step 13. The simulated interface position is shown in figure 19. The discussion regarding mass balance interpretation given in example 1 is applicable to this problem also. Figure 24 shows the components of freshwater leakage as given in the mass balance. Freshwater enters the system by leaking down into the uppermost aquifer (layer 3). It leaves the system by flowing through the aquifer layers and then leaking out to the sea. Freshwater leaking upward from layer 2 is apportioned to the overlying freshwater and saltwater zones. In this example the global freshwater mass balance is given by the freshwater leakage into the system across the top of layer 3 ($0.3 \times 10^{-2} \text{ ft}^3/\text{s}$). This freshwater is discharged by leakage of freshwater from layer 2 to the overlying saltwater ($0.2 \times 10^{-2} \text{ ft}^3/\text{s}$) and by leakage to the sea across the top of layer 3 ($0.1 \times 10^{-2} \text{ ft}^3/\text{s}$).

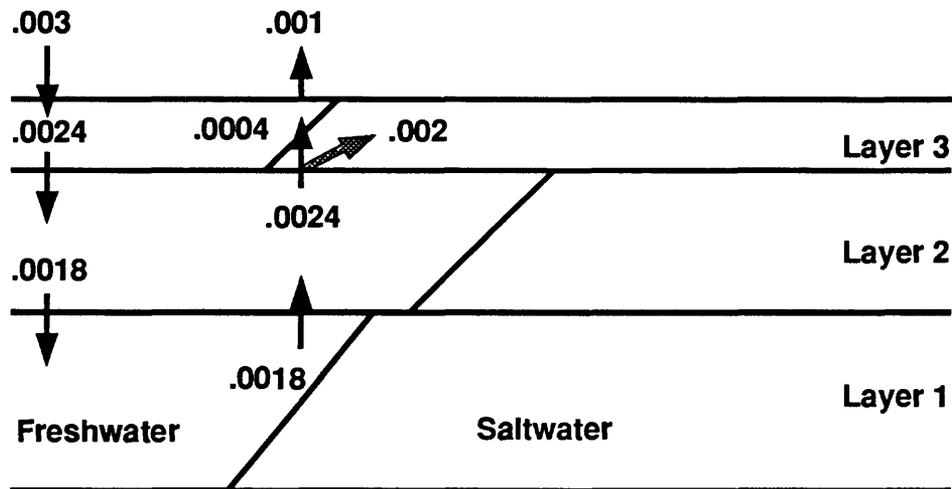


Figure 24.--Diagram showing the freshwater leakages from the mass balance for the Cape May example (leakage is in cubic feet per second, solid arrows represent freshwater leaking into freshwater, shaded arrow represents freshwater leaking into saltwater).

Table 9.--Input file for example 2--continued

0	29.0E-07									
			FKY							
	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	0.0	160	160	160	160	160	160	160	160	160
	160	160	160	160	160	160	160	160	160	160
	160	160	160	160	160	160	160	0.0		
	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
1	1.0			FKY						
1	0.0			STORF						
0	1.0			STORS						
	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	0.0	0.0	0.0	0.0	0.0	0.0	-1.0	0.0		
	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
1	1.0E-04									
1	20.0									
1	-30.0									
0	1.0	PHIF LAYER 3,								
	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
	0.000	11.989	10.387	8.481	5.999	0.011	0.258	0.006	0.005	0.004
	0.004	0.003	0.002	0.002	0.002	0.000	0.000	0.000	0.000	0.000
	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
0	1.0	ZINT LAYER 3,								
	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
	0.000	-479.560	-415.480	-339.240	-239.831	-0.302	-10.196	-0.120	-0.120	-0.080
	-0.080	-0.080	-0.040	-0.040	-0.040	0.000	0.000	0.000	0.000	0.000
	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
0	1.0	FX LAYER 3,								
	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000

Table 9.--Input file for example 2--continued

0	1.0			HEAD (FIXED FRESHWATER HEAD IN OVERLYING LAYER)																
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
0.0	12.0	10.3923	8.4853	6.0000	0.000	.25	.25	.25	.25	.25	.25	.25	.25	.25	.25	.25	.25	.25	.25	
.25	.25	.25	.25	.25	.25	.25	.25	.25	.25	.25	.25	.25	.25	.25	.25	.25	.25	.25	.25	
.25	.25	.25	.25	.25	.25	.25	.25	.25	.25	.25	.25	.25	.25	.25	.25	.25	.25	.25	.25	
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
0	1.0			BATH																
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
0.0	00.0	0.0	0.0	0.0	0.0	0.000	-10.	-10.	-10.	-10.	-10.	-10.	-10.	-10.	-10.	-10.	-10.	-10.	-10.	
-10.	-10.	-10.	-10.	-10.	-10.	-10.	-10.	-10.	-10.	-10.	-10.	-10.	-10.	-10.	-10.	-10.	-10.	-10.	-10.	-10.
-10.	-10.	-10.	-10.	-10.	-10.	-10.	-10.	-10.	-10.	-10.	-10.	-10.	-10.	-10.	-10.	-10.	-10.	-10.	-10.	-10.
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
0	0	0	IWT	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
4000.	4000.	4000.	4000.	4000.	4000.	4000.	2000.	2000.	2000.	2000.	2000.	2000.	2000.	2000.	2000.	2000.	2000.	2000.	2000.	
2000.	2000.	2000.	2000.	2000.	4000.	4000.	4000.	4000.	4000.	4000.	4000.	4000.	4000.	4000.	4000.	4000.	4000.	4000.	4000.	
4000.	4000.	4000.	4000.	4000.	4000.	4000.	4000.	4000.	4000.	4000.	4000.	4000.	4000.	4000.	4000.	4000.	4000.	4000.	4000.	
100.	100.	100.	100.	100.	100.	100.	100.	100.	100.	100.	100.	100.	100.	100.	100.	100.	100.	100.	100.	
3155760.	1.30	3652500.	29	0																
1	0.0																			

Table 10.--Output file for example 2

CAPE MAY - CROSS-SECTIONAL MODEL WITH 3 LAYERS

NUMBER OF ROWS= 3
NUMBER OF COLUMNS= 28
NUMBER OF AQUIFERS= 3
NUMBER OF TIME STEPS BETWEEN PRINTOUT= 50
MAXIMUM NUMBER OF ITERATIONS= 50
NUMBER OF ITERATION PARAMETERS= 6
NUMBER OF PUMPING PERIODS= 1
INTERFACE FIXED AFTER 20 ITERATIONS
INPUT/OUTPUT OPTIONS' PARAMETER= 0 (0=USE DEFAULTS, 1=READ IN OPTIONS ARRAY)
LEAKAGE OPTION PARAMETER= 0 (0=RESTRICTED MIXING, 1=COMPLETE MIXING)

CONVERGENCE CRITERION= 0.1000E-03
STEADY STATE CRITERION= 0.1000E-03
RELAXATION FACTOR= 0.8000
WEIGHTING FACTOR= 0.3000
ITERATION PARAMETER FACTOR= 0.1000E+05
SPECIFIC GRAVITY OF FRESHWATER= 62.41
SPECIFIC GRAVITY OF SALTWATER= 63.97
VISCOSITY OF FRESHWATER= 0.2090E-04
VISCOSITY OF SALTWATER= 0.2090E-04

CONTINUATION RUN

INITIAL TIME = 0.0000000000 SEC

LOCATIONS OF 3 OBSERVATION NODES TRACKED:

2 7 1
2 7 2
2 7 3

SPECIFIED INPUT/OUTPUT OPTIONS:

1: 1=PRINT INPUT AQUIFER PARAMETERS, 0=DO NOT PRINT
1: 1=PRINT INPUT PUMPING PERIOD WELL AND RECHARGE DATA, 0=DO NOT PRINT
1: 1=PRINT CALCULATED FRESHWATER HEADS, 0=DO NOT PRINT
1: 1=PRINT CALCULATED SALTWATER HEADS, 0=DO NOT PRINT
1: 1=PRINT CALCULATED INTERFACE ELEVATIONS, 0=DO NOT PRINT
0: 1=PRINT CALCULATED FAREA FACTORS, 0=DO NOT PRINT
0: 1=PRINT CALCULATED SAREA FACTORS, 0=DO NOT PRINT
1: 1=PRINT F-M-S MAP, 0=DO NOT PRINT
1: 1=WRITE BLOCK LEAKAGE VALUES TO FILE, 0=DO NOT WRITE TO FILE
0: 1=PRINT ITERATION INFORMATION TO SCREEN, 0=DO NOT PRINT
0: 0=REWRITE LATEST RESULTS TO FILE AFTER EACH TIME STEP,
1=WRITE RESULTS TO FILE EVERY NP TIME STEPS

Table 10.--Output file for example 2--continued

 AQUIFER PARAMETERS FOR LAYER 1

HYDRAULIC CONDUCTIVITY OF FRESHWATER IN X-DIRECTION

1	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
2	0.0000	0.1690E-02								
	0.1690E-02									
	0.1690E-02	0.0000	0.0000							
3	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000

HYDRAULIC CONDUCTIVITY OF FRESHWATER IN Y-DIRECTION

1.000

FRESHWATER SPECIFIC STORAGE

0.0000

SALTWATER SPECIFIC STORAGE

1	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
2	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	-1.000	0.0000	0.0000
3	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000

EFFECTIVE POROSITY

0.1000E-03

AQUIFER THICKNESS

80.00

ELEVATION OF BASE OF AQUIFER

-190.0

INITIAL FRESHWATER HEAD

1	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000

Table 10.--Output file for example 2--continued

2	0.0000	5.775	5.634	5.274	4.769	4.197	3.781	3.508	3.234	2.949
	2.621	0.8600E-01	1.625	0.2800E-01	0.0000	0.1000E-02	0.1000E-02	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
3	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
INITIAL INTERFACE ELEVATION										
1	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
2	0.0000	-234.5	-229.0	-214.8	-190.8	-167.9	-151.3	-140.3	-129.4	-118.0
	-104.9	-3.480	-65.04	-1.160	-0.4000E-01	-0.4000E-01	-0.4000E-01	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
3	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
INITIAL SALTWATER HEADS										
1	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
2	0.0000	-0.8585E-01	-0.8780E-01	-0.9366E-01	-0.9512E-03	-0.3659E-03	-0.1073E-02	-0.6098E-03	-0.3171E-03	-0.2927E-03
	-0.9756E-03	-0.9756E-03	-0.9756E-03	-0.9756E-03	-0.9756E-03	-0.6939E-17	-0.6939E-17	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
3	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
FX										
1	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
2	0.0000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
	0.1670	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
3	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
FY										
1	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
2	0.0000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
3	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000

Table 10.--Output file for example 2--continued

SX

1	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
2	0.0000	0.0000	0.0000	0.0000	0.4820	1.000	1.000	1.000	1.000	1.000
	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
	1.000	1.000	1.000	1.000	1.000	1.000	1.000	0.0000		
3	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000

SY

1	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
2	0.0000	0.0000	0.0000	0.0000	0.0000	1.000	1.000	1.000	1.000	1.000
	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
	1.000	1.000	1.000	1.000	1.000	1.000	1.000	0.0000		
3	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000

AQUITARD LEAKANCES (K"/B")

1	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
2	0.0000	0.1865E-07	0.3730E-07							
	0.3730E-07									
	0.3730E-07	0.0000								
3	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000

AQUIFER PARAMETERS FOR LAYER 2

HYDRAULIC CONDUCTIVITY OF FRESHWATER IN X-DIRECTION

1	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
2	0.0000	0.9000E-03								
	0.9000E-03									
	0.9000E-03	0.0000								
3	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000

Table 10.--Output file for example 2--continued

HYDRAULIC CONDUCTIVITY OF FRESHWATER IN Y-DIRECTION

1.000

FRESHWATER SPECIFIC STORAGE

0.0000

SALTWATER SPECIFIC STORAGE

1	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
2	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	-1.000	0.0000	0.0000	0.0000	0.0000
3	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000

EFFECTIVE POROSITY

0.1000E-03

AQUIFER THICKNESS

40.00

ELEVATION OF BASE OF AQUIFER

-90.00

INITIAL FRESHWATER HEAD

1	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
2	0.0000	5.838	5.683	5.307	4.767	4.158	3.746	3.474	3.201	2.912	
	2.577	2.299	2.078	1.878	1.606	1.334	0.3250	0.0000	0.0000	0.0000	
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	
3	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000

INITIAL INTERFACE ELEVATION

1	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
2	0.0000	-237.0	-230.9	-216.1	-195.3	-170.0	-153.4	-142.6	-131.6	-120.3	
	-107.6	-91.99	-83.15	-75.14	-64.28	-53.38	-13.00	0.0000	0.0000	0.0000	
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	

Table 10.--Output file for example 2--continued

SY

1	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000		
2	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
	1.000	1.000	1.000	1.000	1.000	1.000	1.000	0.0000			
3	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000		

AQUITARD LEAKANCES (K"/B")

1	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000		
2	0.0000	0.2500E-09	0.5000E-09								
	0.5000E-09										
	0.5000E-09	0.0000									
3	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000		

AQUIFER PARAMETERS FOR LAYER 3

HYDRAULIC CONDUCTIVITY OF FRESHWATER IN X-DIRECTION

1	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000		
2	0.0000	0.4640E-03									
	0.4640E-03										
	0.4640E-03	0.0000									
3	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000		

HYDRAULIC CONDUCTIVITY OF FRESHWATER IN Y-DIRECTION

1.000

FRESHWATER SPECIFIC STORAGE

0.0000

SALTWATER SPECIFIC STORAGE

1	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000		

Table 10.--Output file for example 2--continued

2	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	-1.000	0.0000		
3	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000		

EFFECTIVE POROSITY

0.1000E-03

AQUIFER THICKNESS

20.00

ELEVATION OF BASE OF AQUIFER

-30.00

INITIAL FRESHWATER HEAD

1	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000		
2	0.0000	11.99	10.39	8.481	5.999	0.1100E-01	0.2580	0.6000E-02	0.5000E-02	0.4000E-02
	0.4000E-02	0.3000E-02	0.2000E-02	0.2000E-02	0.2000E-02	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000		
3	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000		

INITIAL INTERFACE ELEVATION

1	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000		
2	0.0000	-479.6	-415.5	-339.2	-239.8	-0.3020	-10.20	-0.1200	-0.1200	-0.8000E-01
	-0.8000E-01	-0.8000E-01	-0.4000E-01	-0.4000E-01	-0.4000E-01	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000		
3	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000		

INITIAL SALTWATER HEADS

1	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000		
2	0.0000	-0.1137E-12	-0.2274E-12	-0.1137E-12	0.3146E-02	0.3366E-02	0.3024E-02	0.2927E-02	0.1951E-02	0.1951E-02
	0.1951E-02	0.9756E-03	0.9756E-03	0.9756E-03	0.9756E-03	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000		
3	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000		

Table 10.--Output file for example 2--continued

3	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
HEAD IN OVERLYING AQUIFER										
1	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
2	0.0000	12.00	10.39	8.485	6.000	0.0000	0.2500	0.2500	0.2500	0.2500
	0.2500	0.2500	0.2500	0.2500	0.2500	0.2500	0.2500	0.2500	0.2500	0.2500
	0.2500	0.2500	0.2500	0.2500	0.2500	0.2500	0.2500	0.0000		
3	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
BATHYMETRY										
1	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
2	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	-10.00	-10.00	-10.00	-10.00
	-10.00	-10.00	-10.00	-10.00	-10.00	-10.00	-10.00	-10.00	-10.00	-10.00
	-10.00	-10.00	-10.00	-10.00	-10.00	-10.00	-10.00	0.0000		
3	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
CODE FOR UPPER LAYER (1-UNCONFINED NODE, 0-CONFINED NODE)										
1	0	0	0	0	0	0	0	0	0	0
	0	0	0							
2	0	0	0	0	0	0	0	0	0	0
	0	0	0							
3	0	0	0	0	0	0	0	0	0	0
	0	0	0							
DELTA X										
4000.	4000.	4000.	4000.	4000.	4000.	2000.	2000.	2000.	2000.	
2000.	2000.	2000.	2000.	4000.	4000.	4000.	4000.	4000.	4000.	
4000.	4000.	4000.	4000.	4000.	4000.	4000.	4000.			
DELTA Y										
100.0	100.0	100.0								
ITERATION PARAMETERS:										
FRESHWATER EQUATION:	.0000	.8872	.9873	.9986	.9998	1.000				

Table 10.--Output file for example 2--continued

PUMPING PERIOD 1

INITIAL TIME STEP(SEC)= 0.3156E+07
MULTIPLICATION FACTOR FOR DELTA T= 1.300
LENGTH OF PUMPING PERIOD(DAYS)= 0.3652E+07
MAXIMUM NUMBER OF TIME STEPS= 29
NUMBER OF ACTIVE WELLS= 0

RECHARGE

0.0000

TIME STEP 1

	SECONDS	HOURS	DAYS	YEARS
TIME STEP SIZE:	0.3156E+07	876.6	36.53	0.1000
PUMPING PERIOD TIME:	0.3156E+07	876.6	36.53	0.1000
TOTAL SIMULATION TIME:	0.3156E+07	876.6	36.53	0.1000

MAXIMUM FRESHWATER HEAD CHANGE: 0.2175E-01 0.3895E-01 0.3998E-01 0.4530E-01 0.3370E-02 0.1787E-02 0.2179E-02 0.2680E-02
0.2287E-02 0.1434E-02 0.1756E-02 0.4387E-03 0.2180E-03 0.5058E-03 0.1524E-03 0.4178E-03
0.2612E-03 0.8152E-04

MAXIMUM SALTWATER HEAD CHANGE: 0.5923E-02 0.2567E-01 0.2210E-01 0.4110E-01 0.3155E-02 0.1172E-02 0.1057E-02 0.2896E-02
0.1983E-02 0.1525E-02 0.1026E-02 0.4757E-03 0.1226E-03 0.3019E-03 0.1469E-03 0.1504E-03
0.9171E-04 0.4893E-04

TIME STEP 2

	SECONDS	HOURS	DAYS	YEARS
TIME STEP SIZE:	0.4102E+07	1140.	47.48	0.1300
PUMPING PERIOD TIME:	0.7258E+07	2016.	84.01	0.2300
TOTAL SIMULATION TIME:	0.7258E+07	2016.	84.01	0.2300

MAXIMUM FRESHWATER HEAD CHANGE: 0.3241E-02 0.1937E-02 0.6434E-02 0.4095E-02 0.7411E-02 0.8529E-03 0.6667E-03 0.2246E-02
0.1448E-02 0.3731E-02 0.3432E-02 0.2778E-03 0.2860E-03 0.1050E-02 0.5048E-03 0.1243E-02
0.1155E-02 0.1168E-03 0.9512E-04 0.4140E-03 0.1944E-03 0.4521E-03 0.3867E-03 0.3804E-04

MAXIMUM SALTWATER HEAD CHANGE: 0.1731E-01 0.3973E-02 0.4697E-02 0.4275E-02 0.7108E-02 0.2727E-02 0.9611E-03 0.2407E-02
0.1369E-02 0.2987E-02 0.3031E-02 0.6075E-03 0.2829E-03 0.1132E-02 0.4801E-03 0.1253E-02
0.1123E-02 0.1940E-03 0.1144E-03 0.4539E-03 0.1848E-03 0.4583E-03 0.3768E-03 0.6324E-04

TIME STEP 3

	SECONDS	HOURS	DAYS	YEARS
TIME STEP SIZE:	0.5333E+07	1481.	61.73	0.1690
PUMPING PERIOD TIME:	0.1259E+08	3498.	145.7	0.3990
TOTAL SIMULATION TIME:	0.1259E+08	3498.	145.7	0.3990

Table 10.--Output file for example 2--continued

MAXIMUM FRESHWATER HEAD CHANGE: 0.1639E-02 0.2436E-02 0.6955E-02 0.5804E-02 0.8797E-02 0.9678E-03 0.7604E-03 0.2898E-02
 0.1952E-02 0.4601E-02 0.4194E-02 0.4008E-03 0.3404E-03 0.1411E-02 0.7621E-03 0.1713E-02
 0.1606E-02 0.1628E-03 0.1204E-03 0.6062E-03 0.3277E-03 0.6894E-03 0.6027E-03 0.6513E-04
 MAXIMUM SALTWATER HEAD CHANGE: 0.1247E-01 0.4247E-02 0.4963E-02 0.5988E-02 0.8456E-02 0.3048E-02 0.1130E-02 0.3104E-02
 0.1851E-02 0.3827E-02 0.3771E-02 0.7025E-03 0.3678E-03 0.1527E-02 0.7267E-03 0.1726E-02
 0.1562E-02 0.2541E-03 0.1662E-03 0.6643E-03 0.3110E-03 0.6996E-03 0.5874E-03 0.9553E-04

 TIME STEP 4

	SECONDS	HOURS	DAYS	YEARS
TIME STEP SIZE:	0.6933E+07	1926.	80.25	0.2197
PUMPING PERIOD TIME:	0.1952E+08	5424.	226.0	0.6187
TOTAL SIMULATION TIME:	0.1952E+08	5424.	226.0	0.6187

MAXIMUM FRESHWATER HEAD CHANGE: 0.3133E-02 0.2676E-02 0.6400E-02 0.6334E-02 0.8801E-02 0.1420E-02 0.8424E-03 0.3058E-02
 0.2218E-02 0.4813E-02 0.4380E-02 0.4520E-03 0.3473E-03 0.1585E-02 0.9339E-03 0.1960E-02
 0.1854E-02 0.1835E-03 0.1211E-03 0.7348E-03 0.4411E-03 0.8640E-03 0.7698E-03 0.8754E-04
 0.5866E-04
 MAXIMUM SALTWATER HEAD CHANGE: 0.9004E-02 0.4499E-02 0.4400E-02 0.6497E-02 0.8470E-02 0.2975E-02 0.1137E-02 0.3278E-02
 0.2107E-02 0.4095E-02 0.3974E-02 0.7045E-03 0.4002E-03 0.1714E-02 0.8917E-03 0.1974E-02
 0.1804E-02 0.2848E-03 0.1970E-03 0.8034E-03 0.4189E-03 0.8759E-03 0.7505E-03 0.1213E-03
 0.8687E-04

 TIME STEP 5

	SECONDS	HOURS	DAYS	YEARS
TIME STEP SIZE:	0.9013E+07	2504.	104.3	0.2856
PUMPING PERIOD TIME:	0.2854E+08	7927.	330.3	0.9043
TOTAL SIMULATION TIME:	0.2854E+08	7927.	330.3	0.9043

MAXIMUM FRESHWATER HEAD CHANGE: 0.4111E-02 0.2547E-02 0.5406E-02 0.6011E-02 0.7985E-02 0.1648E-02 0.8939E-03 0.2869E-02
 0.2229E-02 0.4547E-02 0.4131E-02 0.4304E-03 0.3210E-03 0.1582E-02 0.9958E-03 0.1986E-02
 0.1892E-02 0.1816E-03 0.1175E-03 0.7837E-03 0.5099E-03 0.9482E-03 0.8593E-03 0.1007E-03
 0.5788E-04
 MAXIMUM SALTWATER HEAD CHANGE: 0.6534E-02 0.4200E-02 0.3526E-02 0.6149E-02 0.7689E-02 0.2672E-02 0.1042E-02 0.3082E-02
 0.2121E-02 0.3903E-02 0.3790E-02 0.6381E-03 0.3919E-03 0.1707E-02 0.9513E-03 0.2000E-02
 0.1841E-02 0.3092E-03 0.2038E-03 0.8545E-03 0.4851E-03 0.9594E-03 0.8377E-03 0.1462E-03
 0.9734E-04

 TIME STEP 6

	SECONDS	HOURS	DAYS	YEARS
TIME STEP SIZE:	0.1172E+08	3255.	135.6	0.3713
PUMPING PERIOD TIME:	0.4025E+08	0.1118E+05	465.9	1.276
TOTAL SIMULATION TIME:	0.4025E+08	0.1118E+05	465.9	1.276

MAXIMUM FRESHWATER HEAD CHANGE: 0.4072E-02 0.2166E-02 0.4261E-02 0.5151E-02 0.6682E-02 0.1566E-02 0.7937E-03 0.2451E-02
 0.2017E-02 0.3924E-02 0.3559E-02 0.3646E-03 0.2716E-03 0.1431E-02 0.9493E-03 0.1821E-02

Table 10.--Output file for example 2--continued

0.1746E-02 0.1636E-03 0.1038E-03 0.7526E-03 0.5202E-03 0.9319E-03 0.8573E-03 0.1025E-03
 0.5047E-04
 MAXIMUM SALTWATER HEAD CHANGE: 0.4672E-02 0.3545E-02 0.2589E-02 0.5266E-02 0.6436E-02 0.2212E-02 0.8785E-03 0.2637E-02
 0.1920E-02 0.3385E-02 0.3289E-02 0.5730E-03 0.3535E-03 0.1537E-02 0.9065E-03 0.1833E-02
 0.1698E-02 0.3020E-03 0.1885E-03 0.8161E-03 0.4958E-03 0.9405E-03 0.8357E-03 0.1542E-03
 0.9659E-04

 TIME STEP 7

	SECONDS	HOURS	DAYS	YEARS
TIME STEP SIZE:	0.1523E+08	4231.	176.3	0.4827
PUMPING PERIOD TIME:	0.5549E+08	0.1541E+05	642.2	1.758
TOTAL SIMULATION TIME:	0.5549E+08	0.1541E+05	642.2	1.758

MAXIMUM FRESHWATER HEAD CHANGE: 0.3333E-02 0.1652E-02 0.3120E-02 0.3989E-02 0.5125E-02 0.1264E-02 0.6068E-03 0.1903E-02
 0.1634E-02 0.3076E-02 0.2782E-02 0.2795E-03 0.2085E-03 0.1166E-02 0.8077E-03 0.1504E-02
 0.1449E-02 0.1388E-03 0.7548E-04 0.6464E-03 0.4707E-03 0.8198E-03 0.7607E-03 0.9173E-04
 0.3731E-04
 MAXIMUM SALTWATER HEAD CHANGE: 0.3210E-02 0.2705E-02 0.1724E-02 0.4079E-02 0.4937E-02 0.1668E-02 0.6751E-03 0.2051E-02
 0.1554E-02 0.2663E-02 0.2586E-02 0.4723E-03 0.2861E-03 0.1248E-02 0.7706E-03 0.1513E-02
 0.1410E-02 0.2636E-03 0.1611E-03 0.6984E-03 0.4523E-03 0.8218E-03 0.7414E-03 0.1428E-03
 0.8701E-04

 TIME STEP 8

	SECONDS	HOURS	DAYS	YEARS
TIME STEP SIZE:	0.1980E+08	5501.	229.2	0.6275
PUMPING PERIOD TIME:	0.7529E+08	0.2091E+05	871.4	2.386
TOTAL SIMULATION TIME:	0.7529E+08	0.2091E+05	871.4	2.386

MAXIMUM FRESHWATER HEAD CHANGE: 0.2280E-02 0.1109E-02 0.2508E-02 0.2905E-02 0.3561E-02 0.8879E-03 0.4470E-03 0.1279E-02
 0.1261E-02 0.2329E-02 0.2140E-02 0.2226E-03 0.1204E-03 0.8290E-03 0.6340E-03 0.1126E-02
 0.1094E-02 0.1034E-03 0.5552E-04 0.4846E-03 0.3589E-03 0.6238E-03 0.5810E-03 0.6836E-04
 0.2481E-04
 MAXIMUM SALTWATER HEAD CHANGE: 0.2054E-02 0.1670E-02 0.1275E-02 0.2968E-02 0.3405E-02 0.1283E-02 0.4881E-03 0.1359E-02
 0.1186E-02 0.1920E-02 0.1895E-02 0.3947E-03 0.2050E-03 0.8846E-03 0.6016E-03 0.1127E-02
 0.1064E-02 0.1995E-03 0.1193E-03 0.5189E-03 0.3440E-03 0.6227E-03 0.5661E-03 0.1119E-03
 0.6733E-04

 TIME STEP 9

	SECONDS	HOURS	DAYS	YEARS
TIME STEP SIZE:	0.2574E+08	7151.	297.9	0.8157
PUMPING PERIOD TIME:	0.1010E+09	0.2806E+05	1169.	3.201
TOTAL SIMULATION TIME:	0.1010E+09	0.2806E+05	1169.	3.201

MAXIMUM FRESHWATER HEAD CHANGE: 0.1314E-02 0.6800E-03 0.1545E-02 0.1797E-02 0.2212E-02 0.5297E-03 0.2645E-03 0.8020E-03
 0.8040E-03 0.1456E-02 0.1336E-02 0.1450E-03 0.7177E-04 0.5323E-03 0.4215E-03 0.7294E-03

Table 10.--Output file for example 2--continued

0.7123E-03 0.6896E-04 0.3462E-04
 MAXIMUM SALTWATER HEAD CHANGE: 0.1199E-02 0.1017E-02 0.6773E-03 0.1836E-02 0.2112E-02 0.7717E-03 0.2980E-03 0.8523E-03
 0.7536E-03 0.1201E-02 0.1180E-02 0.2443E-03 0.1307E-03 0.5669E-03 0.3988E-03 0.7301E-03
 0.6922E-03 0.1315E-03 0.7849E-04

 TIME STEP 10

	SECONDS	HOURS	DAYS	YEARS
TIME STEP SIZE:	0.3347E+08	9296.	387.3	1.060
PUMPING PERIOD TIME:	0.1345E+09	0.3736E+05	1557.	4.262
TOTAL SIMULATION TIME:	0.1345E+09	0.3736E+05	1557.	4.262

MAXIMUM FRESHWATER HEAD CHANGE: 0.5876E-03 0.4042E-03 0.8119E-03 0.1097E-02 0.1274E-02 0.2457E-03 0.1251E-03 0.4793E-03
 0.4940E-03 0.8331E-03 0.7788E-03 0.1021E-03 0.3884E-04
 MAXIMUM SALTWATER HEAD CHANGE: 0.5855E-03 0.5945E-03 0.2743E-03 0.1117E-02 0.1216E-02 0.4100E-03 0.1652E-03 0.5086E-03
 0.4607E-03 0.7195E-03 0.7090E-03 0.1514E-03 0.8184E-04

 TIME STEP 11

	SECONDS	HOURS	DAYS	YEARS
TIME STEP SIZE:	0.4350E+08	0.1208E+05	503.5	1.379
PUMPING PERIOD TIME:	0.1780E+09	0.4944E+05	2060.	5.641
TOTAL SIMULATION TIME:	0.1780E+09	0.4944E+05	2060.	5.641

MAXIMUM FRESHWATER HEAD CHANGE: 0.2581E-03 0.2398E-03 0.4172E-03 0.6943E-03 0.7415E-03 0.1107E-03 0.6151E-04
 MAXIMUM SALTWATER HEAD CHANGE: 0.2642E-03 0.3520E-03 0.1677E-03 0.7053E-03 0.7068E-03 0.2149E-03 0.9177E-04

 TIME STEP 12

	SECONDS	HOURS	DAYS	YEARS
TIME STEP SIZE:	0.5656E+08	0.1571E+05	654.6	1.792
PUMPING PERIOD TIME:	0.2346E+09	0.6516E+05	2715.	7.433
TOTAL SIMULATION TIME:	0.2346E+09	0.6516E+05	2715.	7.433

MAXIMUM FRESHWATER HEAD CHANGE: 0.9342E-04 0.1489E-03 0.2412E-03 0.4586E-03 0.4684E-03 0.5356E-04 0.3564E-04
 MAXIMUM SALTWATER HEAD CHANGE: 0.1118E-03 0.2230E-03 0.1221E-03 0.4657E-03 0.4457E-03 0.1227E-03 0.5523E-04

 STEADY STATE ACHIEVED AT:

 TIME STEP 13

	SECONDS	HOURS	DAYS	YEARS
TIME STEP SIZE:	0.7352E+08	0.2042E+05	851.0	2.330
PUMPING PERIOD TIME:	0.3081E+09	0.8558E+05	3566.	9.763
TOTAL SIMULATION TIME:	0.3081E+09	0.8558E+05	3566.	9.763

Table 10.--Output file for example 2--continued

MAXIMUM FRESHWATER HEAD CHANGE: 0.4119E-04

MAXIMUM SALTWATER HEAD CHANGE: 0.5008E-04

AQUIFER 1

MASS BALANCE:

	FRESHWATER		SALTWATER	
	T.S. RATE	CUM. VOLUME	T.S. RATE	CUM. VOLUME
CHANGE IN STORAGE	0.2796E-08	543.1	-0.2796E-08	-543.1
SOURCES				
RECHARGE	0.0000	0.0000		
CONSTANT HEAD NODES	0.0000	0.0000	0.0000	0.0000
INJECTION	0.0000	0.0000	0.0000	0.0000
FW LEAKAGE ACROSS TOP	0.1800E-02	0.5539E+06	0.0000	0.0000
FW LEAKAGE ACROSS BOT	0.0000	0.0000	0.0000	0.0000
SW LEAKAGE ACROSS TOP	0.0000	0.0000	0.7825E-07	146.8
SW LEAKAGE ACROSS BOT	0.0000	0.0000	0.0000	0.0000
TOTAL	0.1800E-02	0.5539E+06	0.7825E-07	146.8
SINKS				
CONSTANT HEAD NODES	0.0000	0.0000	0.1553E-06	224.3
PUMPAGE	0.0000	0.0000	0.0000	0.0000
LEAKAGE ACROSS TOP	0.1800E-02	0.5534E+06	0.2986E-06	695.5
LEAKAGE ACROSS BOT	0.0000	0.0000	0.0000	0.0000
TOTAL	0.1800E-02	0.5534E+06	0.4540E-06	919.8
SOURCES-SINKS	-0.4922E-07	520.1	-0.3757E-06	-772.9
RELATIVE ERROR (%)				
INFLUX	0.2890E-02	0.4160E-02	476.6	156.5
STORAGE	1860.	4.243	-0.1334E+05	-42.32

FRESHWATER HEADS

1	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
2	11111.	5.8641	5.7254	5.3717	4.8757	4.2943	3.8756	3.6007	3.3266	3.0461
	2.7356	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
3	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.

SALTWATER HEADS

1	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
2	11111.	11111.	11111.	11111.	0.60417E-030	6.0152E-030	5.9940E-030	5.9794E-030	5.9644E-030	5.9490E-030
	0.59332E-030	5.9169E-030	5.8943E-030	5.8224E-030	5.5821E-030	5.1332E-030	4.5684E-030	4.0546E-030	3.5839E-030	3.1360E-030
	0.26944E-030	2.2500E-030	1.8007E-030	1.3488E-030	8.9756E-040	4.4839E-040	0.0000	11111.	11111.	11111.
3	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.

Table 10.--Output file for example 2--continued

3	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.		

SALTWATER HEADS

1	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.		
2	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
	11111.	11111.	0.59451E-030.	58513E-030.	55923E-030.	51368E-030.	45326E-030.	40229E-030.	35568E-030.	31124E-030.
	0.26735E-030.	22316E-030.	17850E-030.	13362E-030.	88852E-040.	44294E-040.	00000	11111.		
3	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.		

INTERFACE ELEVATION

1	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.		
2	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
	11111.	11111.	-86.188	-77.904	-66.647	-55.072	11111.	11111.	11111.	11111.
	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.		
3	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.		

MAP OF EXTENT OF INTRUSION (F-FRESHWATER, M-FRESH AND SALTWATER, S-SALTWATER)

1
2	. F F F F F F F F F F M M M M S S S S S S S S S S S .
3

Table 10.--Output file for example 2--continued

AQUIFER 3

MASS BALANCE:

	FRESHWATER		SALTWATER	
	T.S. RATE	CUM. VOLUME	T.S. RATE	CUM. VOLUME
CHANGE IN STORAGE	0.3316E-14	0.6953	-0.3316E-14	-0.6953
SOURCES				
RECHARGE	0.0000	0.0000		
CONSTANT HEAD NODES	0.0000	0.0000	0.3297E-15	0.1016E-06
INJECTION	0.0000	0.0000	0.0000	0.0000
FW LEAKAGE ACROSS TOP	0.3048E-02	0.9387E+06	0.0000	0.0000
FW LEAKAGE ACROSS BOT	0.3827E-03	0.1179E+06	0.1985E-02	0.6101E+06
SW LEAKAGE ACROSS TOP	0.0000	0.0000	0.0000	0.0000
SW LEAKAGE ACROSS BOT	0.0000	0.0000	0.4927E-06	867.6
TOTAL	0.3431E-02	0.1057E+07	0.1985E-02	0.6110E+06
SINKS				
CONSTANT HEAD NODES	0.0000	0.0000	0.1132E-10	0.1467E-01
PUMPAGE	0.0000	0.0000	0.0000	0.0000
LEAKAGE ACROSS TOP	0.1064E-02	0.3278E+06	0.1985E-02	0.6110E+06
LEAKAGE ACROSS BOT	0.2367E-02	0.7289E+06	0.0000	0.0000
TOTAL	0.3431E-02	0.1057E+07	0.1985E-02	0.6110E+06
SOURCES-SINKS	-0.5195E-09	1.243	0.2415E-07	6.010
RELATIVE ERROR (%)				
INFLUX	0.1514E-04	-0.5183E-04	-0.1216E-02	-0.1097E-02
STORAGE	0.1567E+08	-78.77	0.7282E+09	964.3

FRESHWATER HEADS

1	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
2	11111.	11.989	10.387	8.4813	5.9982	0.11911E-01	11111.	11111.	11111.	11111.
	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
3	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.

SALTWATER HEADS

1	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	
	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	
	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	
2	11111.	11111.	11111.	11111.	11111.	0.30458E-020.30539E-020.27623E-020.24686E-020.21640E-02	0.18216E-020.14916E-020.12437E-020.10207E-020.71777E-030.38088E-030.97665E-060.43643E-060.38486E-060.33615E-06	0.28818E-060.24014E-060.19187E-060.14358E-060.95501E-070.47611E-070.00000	11111.	11111.	11111.
	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	
	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	
3	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	
	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	
	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	

Table 10.--Output file for example 2--continued

INTERFACE ELEVATION

1	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
2	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
3	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.
	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.	11111.

MAP OF EXTENT OF INTRUSION (F-FRESHWATER, M-FRESH AND SALTWATER, S-SALTWATER)

1
2	. F F F F S .
3

REFERENCES

- Andersen, P. F., Mercer, J. W., and White, H. O., Jr., 1988, Numerical modeling of salt-water intrusion at Hallandale, Florida: *Ground Water*, v. 26, no. 5, p. 619-630.
- Anderson, M.P., 1976, Unsteady groundwater flow beneath strip oceanic islands: *Water Resources Research*, v. 12, no. 4, p. 140-644.
- Andrews, R. W., 1981, Salt-water intrusion in the Costa de Hermisillo, Mexico: A numerical analysis of water management proposals: *Ground Water*, v. 19, no. 6, p. 635-647.
- Ayers, J. F., and Vacher, H. L., 1983, A numerical model describing unsteady flow in a fresh water lens: *Water Resources Bulletin*, v. 19, no. 5, p. 785-792.
- Badon-Ghyben, W., 1889, Nota in verband met de voorgenomen putboring nabij Amsterdam (Notes on the probable results of well drilling near Amsterdam): *Tijdschr. Kon. Inst. Ing.*, v. 1888/9, p. 8-22.
- Bear, J., 1979, *Hydraulics of groundwater*: McGraw-Hill, New York, 569 p.
- Bear, J., and Dagan, G., 1964, Moving interface in coastal aquifers: *American Society of Civil Engineers, Journal of the Hydarulics Division*, v. 9, no. HY4, p. 193-215.
- Bear, J., and Kapuler, I., 1981, A numerical solution for the movement of an interface in a layered coastal aquifer: *Journal of Hydrology*, v. 50, p. 273-298.
- Bonnet, M., and Sauty, J., 1975, Un modele simplifie pour la simulation des nappes avec intrusion saline (A simplified model of salt water encroachment in an aquifer): *International Association of Scientific Hydrology*, v. 115, p. 45-56.
- Bredehoeft, J. D., and Pinder, G. F., 1970, Digital analysis of areal flow in multiaquifer groundwater systems: A quasi three-dimensional model: *Water Resources Research*, v. 6, no. 3, p. 883-888.
- Bredehoeft, J. D., and Pinder, G. F., 1973, Mass transport in flowing groundwater: *Water Resources Research*, v. 9, no. 1, p. 194-210.
- Collins, M. and Gelhar, L., 1971, Seawater intrusion in layered aquifers: *Water Resources Research*, v. 7, no. 4, p. 971-979.
- Collins, M. and Gelhar, L., 1977, Comments on 'The shape of the interface in steady flow in a stratified aquifer' by Y. Mualem and J. Bear: *Water Resources Research*, v. 13, no. 2., p. 486-488.
- Collins, M., Gelhar, L., and Wilson, J., 1972, Hele-Shaw model of Long Island aquifer system: *American Society of Civil Engineers, Journal of the Hydraulics Division*, v. 98, no. HY9, p. 1701-1714.

- Contractor, D. N., 1983, Numerical modeling of saltwater intrusion in the Northern Guam lens: Water Resources Bulletin, v. 19, no. 5, p. 745-751.
- Cooper, H. H., Jr., 1959, A hypothesis concerning the dynamic balance of fresh water and salt water in a coastal aquifer: Journal of Geophysical Research, v. 64, no. 4, p. 461-467.
- Custodio, E., 1981, Sea water encroachment in the Llobregat and Besos areas near Barcelona: Intruded and relict groundwater of marine origin: Proceedings of the 7th Salt Water Intrusion Meeting, SWIM-82, Uppsala, Sweden, p. 120-152.
- Essaid, H. I., 1984, A quasi-three dimensional finite difference model for the simulation of fresh water and salt water flow in a coastal aquifer system: M.S. Thesis, Stanford University, 128 p.
- Essaid, H. I., 1986, Fresh water - salt water flow dynamics in coastal aquifer systems: Development and application of a multi-layered sharp interface model, Ph.D. Thesis, Stanford University, 275 p.
- Eyre, P. R., 1985, Simulation of ground-water flow in Southeastern Oahu, Hawaii: Ground Water, v. 23, no. 3, p. 325-330.
- Fetter, C. W., 1972, Position of the saline water interface beneath oceanic islands: Water Resources Research, v. 8, no. 5, p. 1307-1315.
- Freeze, R. A., and Cherry, J. A., 1979, Groundwater: Prentice-Hall, Inc., New Jersey, 604 p.
- Frind, E. O., 1982a, Simulation of long-term transient density-dependent transport in groundwater: Advances in Water Resources, v. 5, p. 73-88.
- Frind, E. O., 1982b, Seawater intrusion in continuous coastal aquifer-aquitard systems: Advances in Water Resources, v. 5, p. 89-97.
- Guswa, J. H., and LeBlanc, D. R., 1985, Digital models of ground-water flow in the Cape Cod aquifer system, Massachusetts: U. S. Geological Survey Water-Supply Paper 2209, 112 p.
- Herzberg, A., 1901, Die wasserversorgung einiger Nordseebaden (The water supply on parts of the North Sea coast in Germany): Z. Gasbeleucht. Wasserversorg., v. 44, p. 815-819; 824-844.
- Hill, M. C., 1988, A comparison of coupled freshwater-saltwater sharp-interface and convective-dispersive models of saltwater intrusion in a layered aquifer system: Proceedings of the VII International Conference on Computational Methods in Water Resources, Elsevier, Amsterdam, p. 211-216.
- Huyakorn, P. S., Andersen, P. F., Mercer, J. W., and White, H. O., 1987, Saltwater intrusion in aquifers: Development and testing of a three-dimensional finite element model: Water Resources Research, v. 23, no. 2, p. 293-312.

- INTERA, 1979, Revisions of the documentation for a model for calculating effects of liquid waste disposal in deep saline aquifers: U. S. Geological Survey Water-Resources Investigations Report 79-96, 73 p.
- Keulegan, H. G., 1954, An example report on model laws for density currents: U. S. National Bureau of Standards, Gaithersburg, Maryland.
- Kipp, K. L., 1987, HST3D: A computer code for simulation of head and solute transport in three-dimensional ground-water flow systems: U. S. Geological Survey Water-Resources Investigations Report 80-26, 517 p.
- Kohout, F., 1964, The flow of fresh water and salt water in the Biscayne aquifer at the Miami area, Florida, in Sea water in coastal aquifers: U. S. Geological Survey Water Supply Paper 1613-c, p. C12-C32.
- Lee, C. and Cheng, R., 1974, On seawater encroachment in coastal aquifers: Water Resources Research, v. 10, no. 5, p. 1039-1043.
- Liu, P., Cheng, A., Liggett, J., and Lee, J., 1981, Boundary integral equation solutions to moving interface between two fluids in porous media: Water Resources Research, v. 17, no. 5, p. 1445-1452.
- Meijerink, J. A. and van der Vorst, H. A., 1977, An iterative solution method for linear systems of which the coefficient matrix is a symmetric M-matrix: Mathematics of Computation, v. 31, no. 137, p. 148-162.
- Mercer, J., Larson, S., and Faust, C., 1980a, Finite-difference model to simulate the areal flow of saltwater and freshwater separated by an interface: U. S. Geological Survey Open-File Report 80-407, 88 p.
- Mercer, J., Larson, S., and Faust, C., 1980b, Simulation of salt-water interface motion: Ground Water, v. 18, no. 4, p. 374-385.
- Mercer, J. W., Lester, B. H., Thomas, S. D., and Bartel, R. L., 1986, Simulation of saltwater intrusion in Volusia County Florida: Water Resources Bulletin, v. 22, no. 6, p. 951-965.
- Mualem, Y., 1973, Interface refraction at the boundary between two porous media: Water Resources Research, v. 9, no. 2, p. 409-414.
- Mualem, Y., and Bear, J., 1974, The shape of the interface in steady flow in a stratified aquifer: Water Resources Research, v. 10, no. 6, p. 1207-1215.
- Pinder, G., and Cooper, H., 1970, A numerical technique for calculating the transient position of the saltwater front: Water Resources Research, v. 6, no. 3, p. 875-882.
- Pinder, G. F., and Page, R. H., 1977, Finite element simulation of salt water intrusion on the South Fork of Long Island: Proceedings of the 1st International Conference on Finite Elements in Water Resources, Pentech Press, London, p. 2.51-2.69.

- Polo, J. F., and Ramis, F. R., 1983, Simulation of salt water - fresh water interface motion: Water Resources Research, v. 19, no. 1, p. 61-68.
- Reddell, D., and Sunada, D., 1970, Numerical simulation of dispersion in groundwater aquifers: Hydrology Paper No. 41, Colorado State Univ., Colorado.
- Reilly, T. E., and Goodman, A. S., 1985, Quantitative analysis of saltwater-freshwater relationships in groundwater systems - A historical perspective: Journal of Hydrology, v. 80, p. 125-160.
- Rumer, R., and Shiau, J., 1968, Salt water interface in a layered coastal aquifer: Water Resources Research, v. 4, no. 6, p. 1235-1247.
- Sa da Costa, A., 1986, Numerical simulation of seawater intrusion with accurate tracking of the seawater toe movement: Proceedings of the 9th Salt Water Intrusion Meeting, Delft University of Technology, Delft, The Netherlands, p. 443-456.
- Sanford, W. E., and Konikow, L. F., 1985, A two-constituent solute-transport model for ground water having variable density: U. S. Geological Survey Water-Resources Investigations Report 85-4279, 88 p.
- Sapik, D. B., 1988, Documentation of a steady-state saltwater-intrusion model for three-dimensional ground-water flow, and user's guide: U. S. Geological Survey Open-File Report 87-526, 174 p.
- Schmorak, S., 1967, Salt water encroachment in the coastal plain of Israel: International Association of Scientific Hydrology, v. 72, p. 305-318.
- Segol, G., and Pinder, G., 1976, Transient simulation of saltwater intrusion in southeastern Florida: Water Resources Research, v. 12, no. 1, p. 65-70.
- Segol, G., Pinder, G., and Gray, W., 1975, A Galerkin-finite element technique for calculating the transient position of the saltwater front: Water Resources Research, v. 11, no. 2, p. 343-347.
- Shamir, U., and Dagan, G., 1971, Motion of the seawater interface in coastal aquifers: A numerical solution: Water Resources Research, v. 7, no. 3, p. 644-657.
- Souza, W. R., and Voss, C. I., 1986, Modeling a regional aquifer containing a narrow transition between freshwater and saltwater using a solute transport simulation: Part II - Analysis of a coastal aquifer system: Proceedings of the 9th Salt Water Intrusion Meeting, Delft University of Technology, Delft, The Netherlands, p. 457-474.
- Stearns, H. T., 1946, Geology of the Hawaiian Islands: U. S. Geological Survey Bulletin 8, Honolulu, Hawaii, 112 p.
- Stone, H., 1968, Iterative solution of implicit approximations of multidimensional partial differential equations: SIAM Journal of Applied Mathematics, v. 5, no. 3, p. 530-559.

- Taigbenu, A. E., Liggett, J. A., and Cheng, A. H-D., 1984, Boundary integral solution to seawater intrusion into coastal aquifers: Water Resources Research, v. 20, no. 8, p. 1150-1158.
- Takasaki, K. J., and Mink, J. F., 1982, Water resources of Southeastern Oahu, Hawaii: U. S. Geological Survey Water-Resources Investigations Report 82-628, 89 p.
- Trescott, P., Pinder, G., and Larson, S., 1976, Finite-difference model for aquifer simulation in two dimensions with results of numerical experiments: Techniques of Water-Resources Investigations of the U. S. Geological Survey, Book 7, Chapter C1, 116 p.
- Visher, F. N., and Mink, J. F., 1964, Ground-water resources in southern Oahu, Hawaii: U. S. Geological Survey Water-Supply Paper 1778, 133 p.
- Volker, R., and Rushton, K., 1982, An assessment of the importance of some parameters for seawater intrusion in aquifers and a comparison of dispersive and sharp-interface modelling approaches: Journal of Hydrology, v. 56, p. 239-250.
- Voss, C. I., 1984a, AQUIFEM-SALT: A finite element model for aquifers containing a seawater interface: U. S. Geological Survey Water-Resources Investigations Report 84-4263, 37 p.
- Voss, C. I., 1984b, SUTRA: A finite-element simulation model for saturated-unsaturated, fluid-density-dependent ground-water flow with energy transport or chemically-reactive single-species solute transport: U. S. Geological Survey Water-Resources Investigations Report 84-4369, 409 p.
- Voss, C. I., and Souza, W. R., 1986, Modeling a regional aquifer containing a narrow transition between freshwater and saltwater using a solute transport simulation: Part I - Theory and Methods: Proceedings of the 9th Salt Water Intrusion Meeting, Delft University of Technology, Delft, The Netherlands, p. 493-514.
- Voss, C. I., and Souza, W. R., 1987, Variable density flow and solute transport simulation of regional aquifers containing a narrow freshwater-saltwater transition zone: Water Resources Research, v. 23, no. 10, p. 1851-1866.
- Weinstein, H., Stone, H., and Kwan, T., 1969, Iterative procedure for solution of systems of parabolic and elliptic equations in three dimensions: Industrial and Engineering Chemistry Fundamentals, v. 8, no. 2, p. 281-287.
- Weinstein, H., Stone, H., and Kwan, T., 1970, Simultaneous solution of multi-phase reservoir flow equations: Society of Petroleum Engineers Journal, v. 10, p. 99-110.
- Wilson, J., and Sa da Costa, A., 1982, Finite element simulation of a saltwater/freshwater interface with indirect toe tracking: Water Resources Research, v. 18, no. 4, p. 1069-1080.

APPENDIX A. MATRIX COEFFICIENTS

The compact finite-difference equation (40) can be expanded into the discretized freshwater and saltwater flow equations, respectively:

$$\begin{aligned}
 Z^1 \Phi_{f_{k-1}}^n + Z^2 \Phi_{s_{k-1}}^n + B^1 \Phi_{f_{i-1}}^n + D^1 \Phi_{f_{j-1}}^n + E^1 \Phi_f^n + E^2 \Phi_s^n + \\
 F^1 \Phi_{f_{j+1}}^n + H^1 \Phi_{f_{i+1}}^n + S^1 \Phi_{f_{k+1}}^n + S^2 \Phi_{s_{k+1}}^n = Q^1 \quad (A.1)
 \end{aligned}$$

$$\begin{aligned}
 Z^3 \Phi_{f_{k-1}}^n + Z^4 \Phi_{s_{k-1}}^n + B^4 \Phi_{s_{i-1}}^n + D^4 \Phi_{s_{j-1}}^n + E^3 \Phi_f^n + E^4 \Phi_s^n + \\
 F^4 \Phi_{s_{j+1}}^n + H^4 \Phi_{s_{i+1}}^n + S^3 \Phi_{f_{k+1}}^n + S^4 \Phi_{s_{k+1}}^n = Q^2 \quad (A.2)
 \end{aligned}$$

where:

$$\begin{aligned}
 B^1 &= T_{fy_{i-1/2}}^n \Delta x & B^4 &= T_{sy_{i-1/2}}^n \Delta x \\
 D^1 &= T_{fx_{j-1/2}}^n \Delta y & D^4 &= T_{sx_{j-1/2}}^n \Delta y \\
 F^1 &= T_{fx_{j+1/2}}^n \Delta y & F^4 &= T_{sx_{j+1/2}}^n \Delta y \\
 H^1 &= T_{fy_{i+1/2}}^n \Delta x & H^4 &= T_{sy_{i+1/2}}^n \Delta x \\
 B^2 &= B^3 = D^2 = D^3 = F^2 = F^3 = H^2 = H^3 = 0
 \end{aligned}$$

$$E^1 = -\left\{ (Z^1 - z^1) + \frac{\gamma_f}{\gamma_s} Z^2 + B^1 + D^1 + F^1 + H^1 + S^1 + \frac{\gamma_f}{\gamma_s} S^2 + \left[\frac{S_f B_f^n + n(\alpha + \text{SAREA}\delta)}{\Delta t} \right] \Delta x \Delta y \right\}$$

$$E^2 = \frac{\text{SAREA } n(1+\delta)}{\Delta t} \Delta x \Delta y - e^5$$

$$E^3 = \frac{\text{FAREA } n\delta}{\Delta t} \Delta x \Delta y$$

$$E^4 = -\left\{ \frac{\gamma_s}{\gamma_f} Z^3 + Z^4 + B^4 + D^4 + F^4 + H^4 + \frac{\gamma_s}{\gamma_f} S^3 + S^4 + \left[\frac{S_s B_s^n + \text{FAREA } n(1+\delta)}{\Delta t} \right] \Delta x \Delta y \right\}$$

$$Q^1 = (E^2 - e^5) \Phi_s^{n-1} \cdot \left[\frac{S_f B_f^n + n(\alpha + \text{SAREA} \delta)}{\Delta t} \right] \Phi_f^{n-1} \Delta x \Delta y - \alpha N \Delta x \Delta y - \left(\frac{\text{Th}_f P_t}{\text{Th}_t} \right)^n - \text{QFL}^1 - \text{QFL}^2$$

$$Q^2 = E^3 \Phi_f^{n-1} \cdot \left[\frac{S_s B_s^n + \text{FAREA} n(1+\delta)}{\Delta t} \right] \Phi_s^{n-1} \Delta x \Delta y - \left(P_t - \frac{\text{Th}_f P_t}{\text{Th}_t} \right)^n - \text{QSL}^1 - \text{QSL}^2$$

and $Z^1, Z^2, Z^3, Z^4, S^1, S^2, S^3, S^4, \text{QFL}$ and QSL are as given below.

$$\begin{aligned} \text{FAREA} \geq 1 - \text{SAREA}_{k+1}: \quad S^1 &= (1 - \text{SAREA}_{k+1}) \left(\frac{K'}{B'} \right) \Delta x \Delta y \\ S^2 &= \omega_1 (\text{FAREA} - (1 - \text{SAREA}_{k+1})) \left(\frac{K'}{B'} \right) \left(\frac{\gamma_s}{\gamma_f} \right) \Delta x \Delta y \\ S^4 &= (1 - \text{FAREA}) \left(\frac{K'}{B'} \right) \Delta x \Delta y \\ \text{QFL}^1 &= -\omega_1 (\text{FAREA} - (1 - \text{SAREA}_{k+1})) \left(\frac{K'}{B'} \right) \frac{z_a + z_b}{2\delta} \Delta x \Delta y \end{aligned}$$

$$\begin{aligned} \text{FAREA} < 1 - \text{SAREA}_{k+1}: \quad S^1 &= \text{FAREA} \left(\frac{K'}{B'} \right) \Delta x \Delta y \\ S^3 &= \omega_2 (1 - \text{FAREA} - \text{SAREA}_{k+1}) \left(\frac{K'}{B'} \right) \Delta x \Delta y \\ S^4 &= \text{SAREA}_{k+1} \left(\frac{K'}{B'} \right) \Delta x \Delta y \\ \text{QSL}^1 &= \omega_1 (1 - \text{FAREA} - \text{SAREA}_{k+1}) \left(\frac{K'}{B'} \right) \frac{z_a + z_b}{2\delta} \Delta x \Delta y \end{aligned}$$

$$\begin{aligned} \text{FAREA}_{k-1} \geq 1 - \text{SAREA}: \quad Z^1 &= (1 - \text{SAREA}) \left(\frac{K'}{B'} \right)_{k-1} \Delta x \Delta y + z^1 \\ Z^3 &= \psi (1 - \omega_3) (\text{FAREA}_{k-1} - (1 - \text{SAREA})) \left(\frac{K'}{B'} \right)_{k-1} \Delta x \Delta y \\ Z^4 &= (1 - \text{FAREA}_{k-1}) \left(\frac{K'}{B'} \right)_{k-1} \Delta x \Delta y \\ \text{QFL}^2 &= z^1 \left(\frac{z_a + z_b}{2\delta} \right)_{k-1} \\ \text{QSL}^2 &= \psi (1 - \omega_3) (\text{FAREA}_{k-1} - (1 - \text{SAREA})) \end{aligned}$$

$$\left(\frac{K'}{B'} \right)_{k-1} \left(\frac{z_a + z_b}{2\delta} \right)_{k-1} \Delta x \Delta y$$

$$\text{FAREA}_{k-1} < 1 - \text{SAREA}: \quad Z^1 = \text{FAREA}_{k-1} \left(\frac{K'}{B'} \right)_{k-1} \Delta x \Delta y$$

$$Z^2 = \omega_4 (1 - \text{FAREA}_{k-1} - \text{SAREA}) \left(\frac{K'}{B'}\right)_{k-1} \frac{\gamma_s}{\gamma_f} \Delta x \Delta y$$

$$Z^4 = \text{SAREA} \left(\frac{K'}{B'}\right)_{k-1} \Delta x \Delta y$$

$$\text{QFL}^2 = \omega_4 (1 - \text{FAREA}_{k-1} - \text{SAREA}) \left(\frac{K'}{B'}\right)_{k-1} \left(\frac{z_a + z_b}{2\delta}\right)_{k+1} \Delta x \Delta y$$

where:

$$z^1 = \psi \omega_3 (\text{FAREA}_{k-1} - (1 - \text{SAREA})) \left(\frac{K'}{B'}\right)_{k-1} \Delta x \Delta y$$

$$e^5 = \frac{\gamma_s}{\gamma_f} z^1$$

and for method 1 leakage (complete mixing):

$$\omega_1 = \omega_2 = \omega_4 = \psi = 1$$

$$\omega_3 = 0$$

for method 2 leakage (restricted mixing):

$$\omega_2 = \omega_4 = 0$$

$$\omega_3 = F_{ar}$$

$$\psi = \omega_1 = 1 \quad \text{for freshwater leaking upwards into saltwater}$$

$$\psi = \omega_1 = 0 \quad \text{for saltwater leaking downwards into freshwater}$$

APPENDIX B. THE STRONGLY IMPLICIT PROCEDURE (SIP) FOR THREE-DIMENSIONAL TWO-PHASE FLOW

The strongly implicit procedure for three-dimensional two-phase flow is developed below. Figure B1 shows the structure of the modified coefficient matrix (**A+B**) and the sparse lower and upper triangular matrices (**LU**) into which this matrix can be factored.

A.1 The SIP algorithm

The system of equations which must be solved is expressed as:

$$\mathbf{A}\xi^n = \mathbf{R}^{n-1} \quad (\text{B.1})$$

where the finite-difference equations at each node are:

$$\mathbf{B}\xi_{i-1jk} + \mathbf{D}\xi_{ij-1k} + \mathbf{E}\xi_{ijk} + \mathbf{F}\xi_{ij+1k} + \mathbf{H}\xi_{i+1jk} + \mathbf{Z}\xi_{ijk-1} + \mathbf{S}\xi_{ijk+1} = \mathbf{R}_{ijk}^{n-1} \quad (\text{B.2})$$

The modified equation solved in the SIP algorithm is:

$$(\mathbf{A}+\mathbf{B})\xi^n = \mathbf{R}^{n-1} \quad (\text{B.3})$$

and at each node (ω is a weighting factor):

$$\begin{aligned} &\mathbf{B}\xi_{i-1jk} + \mathbf{D}\xi_{ij-1k} + \mathbf{E}\xi_{ijk} + \mathbf{F}\xi_{ij+1k} + \mathbf{H}\xi_{i+1jk} + \mathbf{Z}\xi_{ijk-1} + \mathbf{S}\xi_{ijk+1} + \\ &\quad \mathbf{C}[\xi_{i-1j+1k}^{-\omega(-\xi_{ijk} + \xi_{ij+1k} + \xi_{i-1jk})}] + \mathbf{G}[\xi_{i+1j-1k}^{-\omega(-\xi_{ijk} + \xi_{ij-1k} + \xi_{i+1jk})}] + \\ &\quad \mathbf{A}[\xi_{ij+1k-1}^{-\omega(-\xi_{ijk} + \xi_{ij+1k} + \xi_{ijk-1})}] + \mathbf{W}[\xi_{ij-1k+1}^{-\omega(-\xi_{ijk} + \xi_{ij-1k} + \xi_{ijk+1})}] + \\ &\quad \mathbf{T}[\xi_{i+1jk-1}^{-\omega(-\xi_{ijk} + \xi_{ijk-1} + \xi_{i+1jk})}] + \mathbf{U}[\xi_{i-1jk+1}^{-\omega(-\xi_{ijk} + \xi_{ijk+1} + \xi_{i-1jk})}] = \mathbf{R}^{n-1} \end{aligned} \quad (\text{B.4})$$

Rearranging and collecting terms:

$$\begin{aligned} &\mathbf{A}\xi_{ij+1k-1} + \tilde{\mathbf{B}}\xi_{i-1jk} + \mathbf{C}\xi_{i-1j+1k} + \tilde{\mathbf{D}}\xi_{ij-1k} + \tilde{\mathbf{E}}\xi_{ijk} + \tilde{\mathbf{F}}\xi_{ij+1k} + \mathbf{G}\xi_{i+1j-1k} + \tilde{\mathbf{H}}\xi_{i+1jk} + \tilde{\mathbf{Z}}\xi_{ijk-1} + \\ &\quad \tilde{\mathbf{S}}\xi_{ijk+1} + \mathbf{W}\xi_{ij-1k+1} + \mathbf{T}\xi_{i+1jk-1} + \mathbf{U}\xi_{i-1jk+1} = \mathbf{R}^{n-1} \end{aligned} \quad (\text{B.5})$$

where:

$$\tilde{\mathbf{B}} = \mathbf{B} - \omega(\mathbf{C} - \mathbf{U}) \quad (\text{B.6a})$$

$$\tilde{\mathbf{D}} = \mathbf{D} - \omega(\mathbf{G} - \mathbf{W}) \quad (\text{B.6b})$$

$$\tilde{\mathbf{E}} = \mathbf{E} + \omega(\mathbf{C} + \mathbf{G} + \mathbf{A} + \mathbf{W} + \mathbf{T} + \mathbf{U}) \quad (\text{B.6c})$$

$$\tilde{\mathbf{F}} = \mathbf{F} - \omega(\mathbf{C} - \mathbf{A}) \quad (\text{B.6d})$$

$$\tilde{\mathbf{H}} = \mathbf{H} - \omega(\mathbf{G} - \mathbf{T}) \quad (\text{B.6e})$$

$$\tilde{\mathbf{Z}} = \mathbf{Z} - \omega(\mathbf{A} - \mathbf{T}) \quad (\text{B.6f})$$

$$\tilde{\mathbf{S}} = \mathbf{S} - \omega(\mathbf{W} - \mathbf{U}) \quad (\text{B.6g})$$

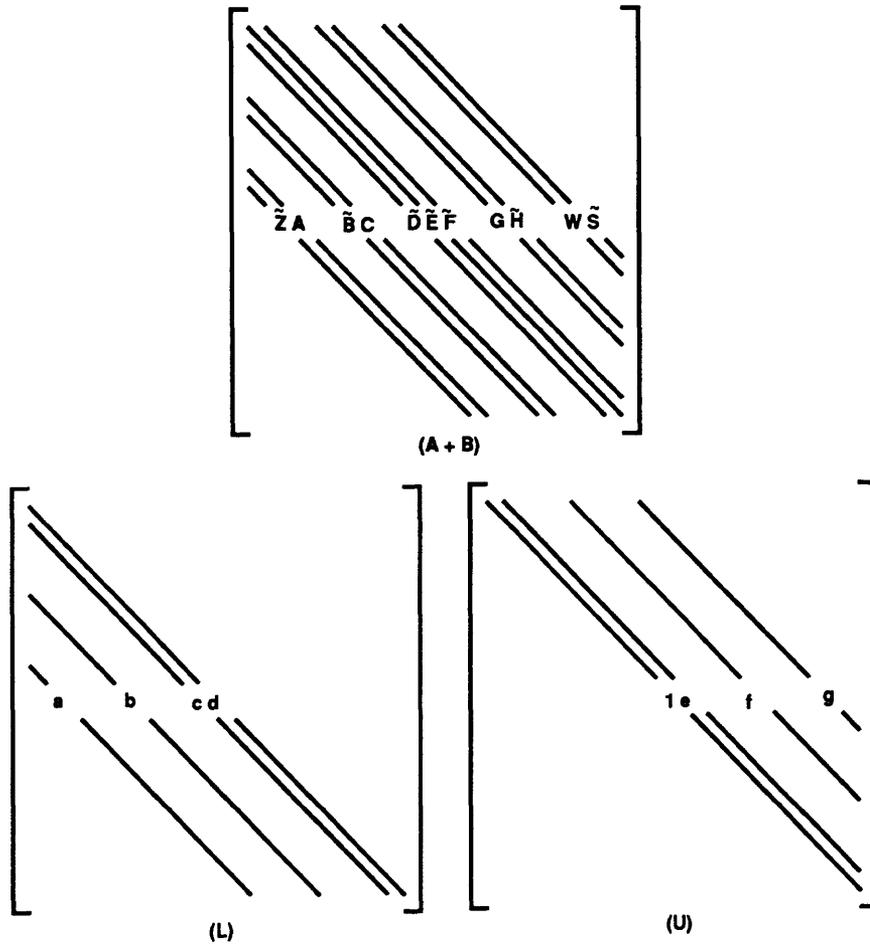


Figure B1.--The structure of the modified coefficient matrix $(A+B)$ and the sparse lower triangular (L) and upper triangular (U) matrices used in the strongly implicit procedure.

By factorization of (A+B):

$$(A+B) = LU$$

the coefficients become:

$$\tilde{Z} = a_{ijk} \quad (B.7a)$$

$$A = a_{ijk} e_{ijk-1} \quad (B.7b)$$

$$T = a_{ijk} f_{ijk-1} \quad (B.7c)$$

$$\tilde{B} = b_{ijk} \quad (B.7d)$$

$$C = b_{ijk} e_{i-1jk} \quad (B.7e)$$

$$\tilde{D} = c_{ijk} \quad (B.7f)$$

$$\tilde{E} = a_{ijk} g_{ijk-1} + b_{ijk} f_{i-1jk} + c_{ijk} e_{ij-1k} + d_{ijk} \quad (B.7g)$$

$$\tilde{F} = d_{ijk} e_{ijk} \quad (B.7h)$$

$$G = c_{ijk} f_{ij-1k} \quad (B.7i)$$

$$\tilde{H} = d_{ijk} f_{ijk} \quad (B.7j)$$

$$U = b_{ijk} g_{i-1jk} \quad (B.7k)$$

$$W = c_{ijk} g_{ij-1k} \quad (B.7l)$$

$$\tilde{S} = d_{ijk} g_{ijk} \quad (B.7m)$$

From (B.6f) and (B.7a):

$$\tilde{Z} = a_{ijk} = Z^{-1}(A-T) \quad (B.8)$$

and substituting (B.7b) and (B.7c):

$$\tilde{Z} = a_{ijk} = Z^{-1}(a_{ijk} e_{ijk-1} - a_{ijk} f_{ijk-1}) \quad (B.9)$$

rearranging:

$$a_{ijk} = Z(1 + \omega(e_{ijk-1} + f_{ijk-1}))^{-1} \quad (B.10)$$

Similarly:

$$b_{ijk} = B(1 + \omega(e_{i-1jk} + f_{i-1jk}))^{-1} \quad (B.11)$$

$$c_{ijk} = D(1 + \omega(f_{ij-1k} + g_{ij-1k}))^{-1} \quad (B.12)$$

From (B.6c) and (B.7g):

$$d_{ijk} = E + \omega(C + G + A + W + T + U) - a_{ijk} g_{ijk-1} - b_{ijk} f_{i-1jk} - c_{ijk} e_{ij-1k} \quad (B.13)$$

From (B.6d) and (B.7h):

$$\mathbf{e}_{ijk} = \mathbf{d}_{ijk}^{-1} (\mathbf{F} - \omega(\mathbf{C} + \mathbf{A})) \quad (\text{B.14})$$

Similarly:

$$\mathbf{f}_{ijk} = \mathbf{d}_{ijk}^{-1} (\mathbf{H} - \omega(\mathbf{G} + \mathbf{T})) \quad (\text{B.15})$$

$$\mathbf{g}_{ijk} = \mathbf{d}_{ijk}^{-1} (\mathbf{S} - \omega(\mathbf{W} + \mathbf{U})) \quad (\text{B.16})$$

Finally the three-dimensional SIP algorithm becomes:

$$\mathbf{a}_{ijk} = \mathbf{Z}(1 + \omega(\mathbf{e}_{ijk-1} + \mathbf{f}_{ijk-1}))^{-1} \quad (\text{B.17})$$

$$\begin{aligned} a^1 &= [Z(1 + \omega(e_{ijk-1}^4 + f_{ijk-1}^4)) - Z^2(\omega(e_{ijk-1}^3 + f_{ijk-1}^3))] \div \det 1 \\ a^2 &= [Z^2(1 + \omega(e_{ijk-1}^1 + f_{ijk-1}^1)) - Z^1(\omega(e_{ijk-1}^2 + f_{ijk-1}^2))] \div \det 1 \\ a^3 &= [Z^3(1 + \omega(e_{ijk-1}^4 + f_{ijk-1}^4)) - Z^4(\omega(e_{ijk-1}^3 + f_{ijk-1}^3))] \div \det 1 \\ a^4 &= [Z^4(1 + \omega(e_{ijk-1}^1 + f_{ijk-1}^1)) - Z^3(\omega(e_{ijk-1}^2 + f_{ijk-1}^2))] \div \det 1 \\ \det 1 &= (1 + \omega(e_{ijk-1}^1 + f_{ijk-1}^1))(1 + \omega(e_{ijk-1}^4 + f_{ijk-1}^4)) - (\omega(e_{ijk-1}^2 + f_{ijk-1}^2))(\omega(e_{ijk-1}^3 + f_{ijk-1}^3)) \end{aligned}$$

$$\mathbf{b}_{ijk} = \mathbf{B}(1 + \omega(\mathbf{e}_{i-1jk} + \mathbf{f}_{i-1jk}))^{-1} \quad (\text{B.18})$$

$$\begin{aligned} b^1 &= [BF(1 + \omega(e_{i-1jk}^4 + g_{i-1jk}^4))] \div \det 2 \\ b^2 &= [-BF(\omega(e_{i-1jk}^2 + g_{i-1jk}^2))] \div \det 2 \\ b^3 &= [-BS(\omega(e_{i-1jk}^3 + g_{i-1jk}^3))] \div \det 2 \\ b^4 &= [BS(1 + \omega(e_{i-1jk}^1 + g_{i-1jk}^1))] \div \det 2 \\ \det 2 &= (1 + \omega(e_{i-1jk}^1 + g_{i-1jk}^1))(1 + \omega(e_{i-1jk}^4 + g_{i-1jk}^4)) - (\omega(e_{i-1jk}^2 + g_{i-1jk}^2))(\omega(e_{i-1jk}^3 + g_{i-1jk}^3)) \end{aligned}$$

$$\mathbf{c}_{ijk} = \mathbf{D}(1 + \omega(\mathbf{f}_{ij-1k} + \mathbf{g}_{ij-1k}))^{-1} \quad (\text{B.19})$$

$$\begin{aligned} c^1 &= [DF(1 + \omega(f_{ij-1k}^4 + g_{ij-1k}^4))] \div \det 3 \\ c^2 &= [-DF(\omega(f_{ij-1k}^2 + g_{ij-1k}^2))] \div \det 3 \\ c^3 &= [-DS(\omega(f_{ij-1k}^3 + g_{ij-1k}^3))] \div \det 3 \\ c^4 &= [DS(1 + \omega(f_{ij-1k}^1 + g_{ij-1k}^1))] \div \det 3 \\ \det 3 &= (1 + \omega(f_{ij-1k}^1 + g_{ij-1k}^1))(1 + \omega(f_{ij-1k}^4 + g_{ij-1k}^4)) - (\omega(f_{ij-1k}^2 + g_{ij-1k}^2))(\omega(f_{ij-1k}^3 + g_{ij-1k}^3)) \end{aligned}$$

$$\mathbf{A} = \mathbf{a}_{ijk} \mathbf{e}_{ijk-1} \quad (\text{B.20})$$

$$A^1 = a_{ijk-1}^1 + a_{ijk-1}^2 + a_{ijk-1}^3$$

$$A^2 = a_{ijk-1}^1 + a_{ijk-1}^2 + a_{ijk-1}^4$$

$$A^3 = a_{ijk-1}^3 + a_{ijk-1}^4 + a_{ijk-1}^3$$

$$A^4 = a_{ijk-1}^3 + a_{ijk-1}^4 + a_{ijk-1}^4$$

$$\mathbf{C} = \mathbf{b}_{ijk} \mathbf{e}_{i-1jk} \quad (\text{B.21})$$

$$C^1 = b_{i-1jk}^1 + b_{i-1jk}^2 + b_{i-1jk}^3$$

$$C^2 = b_{i-1jk}^1 + b_{i-1jk}^2 + b_{i-1jk}^4$$

$$C^3 = b_{i-1jk}^3 + b_{i-1jk}^4 + b_{i-1jk}^3$$

$$C^4 = b_{i-1jk}^3 + b_{i-1jk}^4 + b_{i-1jk}^4$$

$$\mathbf{G} = \mathbf{c}_{ijk} \mathbf{f}_{ij-1k} \quad (\text{B.22})$$

$$G^1 = c_{ij-1k}^1 + c_{ij-1k}^2 + c_{ij-1k}^3$$

$$G^2 = c_{ij-1k}^1 + c_{ij-1k}^2 + c_{ij-1k}^4$$

$$G^3 = c_{ij-1k}^3 + c_{ij-1k}^4 + c_{ij-1k}^3$$

$$G^4 = c_{ij-1k}^3 + c_{ij-1k}^4 + c_{ij-1k}^4$$

$$\mathbf{W} = \mathbf{c}_{ijk} \mathbf{g}_{ij-1k} \quad (\text{B.23})$$

$$W^1 = c_{ij-1k}^1 + c_{ij-1k}^2 + c_{ij-1k}^3$$

$$W^2 = c_{ij-1k}^1 + c_{ij-1k}^2 + c_{ij-1k}^4$$

$$W^3 = c_{ij-1k}^3 + c_{ij-1k}^4 + c_{ij-1k}^3$$

$$W^4 = c_{ij-1k}^3 + c_{ij-1k}^4 + c_{ij-1k}^4$$

$$\mathbf{T} = \mathbf{a}_{ijk} \mathbf{f}_{ijk-1} \quad (\text{B.24})$$

$$T^1 = a_{ijk-1}^1 + a_{ijk-1}^2 + a_{ijk-1}^3$$

$$T^2 = a_{ijk-1}^1 + a_{ijk-1}^2 + a_{ijk-1}^4$$

$$T^3 = a^3 f_{ijk-1}^1 + a^4 f_{ijk-1}^3$$

$$T^4 = a^3 f_{ijk-1}^2 + a^4 f_{ijk-1}^4$$

$$U = b_{ijk} g_{i-1jk} \quad (B.25)$$

$$U^1 = b^1 g_{i-1jk}^1 + b^2 g_{i-1jk}^3$$

$$U^2 = b^1 g_{i-1jk}^2 + b^2 g_{i-1jk}^4$$

$$U^3 = b^3 g_{i-1jk}^1 + b^4 g_{i-1jk}^3$$

$$U^4 = b^3 g_{i-1jk}^2 + b^4 g_{i-1jk}^4$$

$$d_{ijk} = E + \omega(C+G+A+W+T+U) - a_{ijk} g_{ijk-1} - b_{ijk} f_{i-1jk} - c_{ijk} e_{ij-1k} \quad (B.26)$$

$$d^1 = EF + \omega(C^1 + G^1 + A^1 + W^1 + T^1 + U^1) - (a^1 g_{ijk-1}^1 + a^2 g_{ijk-1}^3 + b^1 f_{i-1jk}^1 + b^2 f_{i-1jk}^3 + c^1 e_{ij-1k}^1 + c^2 e_{ij-1k}^3)$$

$$d^2 = EFS + \omega(C^2 + G^2 + A^2 + W^2 + T^2 + U^2) - (a^1 g_{ijk-1}^2 + a^2 g_{ijk-1}^4 + b^1 f_{i-1jk}^2 + b^2 f_{i-1jk}^4 + c^1 e_{ij-1k}^2 + c^2 e_{ij-1k}^4)$$

$$d^3 = ESF + \omega(C^3 + G^3 + A^3 + W^3 + T^3 + U^3) - (a^3 g_{ijk-1}^1 + a^4 g_{ijk-1}^3 + b^3 f_{i-1jk}^1 + b^4 f_{i-1jk}^3 + c^3 e_{ij-1k}^1 + c^4 e_{ij-1k}^3)$$

$$d^4 = ES + \omega(C^4 + G^4 + A^4 + W^4 + T^4 + U^4) - (a^3 g_{ijk-1}^2 + a^4 g_{ijk-1}^4 + b^3 f_{i-1jk}^2 + b^4 f_{i-1jk}^4 + c^3 e_{ij-1k}^2 + c^4 e_{ij-1k}^4)$$

$$e_{ijk} = d_{ijk}^{-1} (F - \omega(C+A)) \quad (B.27)$$

$$e^1 = \{d^4 (F^1 - \omega(C^1 + A^1)) - d^2 (F^3 - \omega(C^3 + A^3))\} \div \det 4$$

$$e^2 = \{d^4 (F^2 - \omega(C^2 + A^2)) - d^2 (F^4 - \omega(C^4 + A^4))\} \div \det 4$$

$$e^3 = \{d^1 (F^3 - \omega(C^3 + A^3)) - d^3 (F^1 - \omega(C^1 + A^1))\} \div \det 4$$

$$e^4 = \{d^1 (F^4 - \omega(C^4 + A^4)) - d^3 (F^2 - \omega(C^2 + A^2))\} \div \det 4$$

$$\det 4 = d^1 d^4 - d^2 d^3$$

$$f_{ijk} = d_{ijk}^{-1} (H - \omega(G+T)) \quad (B.28)$$

$$f^1 = \{d^4 (H^1 - \omega(G^1 + T^1)) - d^2 (H^3 - \omega(G^3 + T^3))\} \div \det 4$$

$$f^2 = \{d^4 (H^2 - \omega(G^2 + T^2)) - d^2 (H^4 - \omega(G^4 + T^4))\} \div \det 4$$

$$f^3 = \{d^1 (H^3 - \omega(G^3 + T^3)) - d^3 (H^1 - \omega(G^1 + T^1))\} \div \det 4$$

$$f^4 = \{d^1 (H^4 - \omega(G^4 + T^4)) - d^3 (H^2 - \omega(G^2 + T^2))\} \div \det 4$$

$$\mathbf{g}_{ijk} = \mathbf{d}_{ijk}^{-1} (\mathbf{S} - \omega(\mathbf{W} + \mathbf{U})) \quad (\text{B.29})$$

$$g^1 = \{d^4 (S^1 - \omega(W^1 + U^1)) - d^2 (S^3 - \omega(W^3 + U^3))\} \div \det 4$$

$$g^2 = \{d^4 (S^2 - \omega(W^2 + U^2)) - d^2 (S^4 - \omega(W^4 + U^4))\} \div \det 4$$

$$g^3 = \{d^1 (S^3 - \omega(W^3 + U^3)) - d^3 (S^1 - \omega(W^1 + U^1))\} \div \det 4$$

$$g^4 = \{d^1 (S^4 - \omega(W^4 + U^4)) - d^3 (S^2 - \omega(W^2 + U^2))\} \div \det 4$$

By forward substitution:

$$\mathbf{v}_{ijk}^n = \mathbf{d}_{ijk}^{-1} (\mathbf{R}^{n-1} - \mathbf{a}_{ijk} \mathbf{v}_{ijk-1}^n - \mathbf{b}_{ijk} \mathbf{v}_{i-1jk}^n - \mathbf{c}_{ijk} \mathbf{v}_{ij-1k}^n) \quad (\text{B.30})$$

$$\mathbf{V}\mathbf{F} = (d^4 \mathbf{X}\mathbf{F} - d^2 \mathbf{X}\mathbf{S}) \div \det 4$$

$$\mathbf{V}\mathbf{S} = (d^1 \mathbf{X}\mathbf{S} - d^3 \mathbf{X}\mathbf{F}) \div \det 4$$

$$\mathbf{X}\mathbf{F} = \mathbf{R}\mathbf{F} - (\mathbf{a}^1 \mathbf{v}_{ijk-1}^n + \mathbf{a}^2 \mathbf{v}_{ijk-1}^n + \mathbf{b}^1 \mathbf{v}_{i-1jk}^n + \mathbf{b}^2 \mathbf{v}_{i-1jk}^n + \mathbf{c}^1 \mathbf{v}_{ij-1k}^n + \mathbf{c}^2 \mathbf{v}_{ij-1k}^n)$$

$$\mathbf{X}\mathbf{S} = \mathbf{R}\mathbf{S} - (\mathbf{a}^3 \mathbf{v}_{ijk-1}^n + \mathbf{a}^4 \mathbf{v}_{ijk-1}^n + \mathbf{b}^3 \mathbf{v}_{i-1jk}^n + \mathbf{b}^4 \mathbf{v}_{i-1jk}^n + \mathbf{c}^3 \mathbf{v}_{ij-1k}^n + \mathbf{c}^4 \mathbf{v}_{ij-1k}^n)$$

By backward substitution:

$$\xi_{ijk}^n = \mathbf{v}_{ijk}^n - (\mathbf{e}_{ijk} \xi_{ij+1k}^n + \mathbf{f}_{ijk} \xi_{i+1jk}^n + \mathbf{g}_{ijk} \xi_{ijk+1}^n) \quad (\text{B.31})$$

$$\xi_f = \mathbf{V}\mathbf{F} - (\mathbf{e}^1 \xi_{ij+1k}^n + \mathbf{e}^2 \xi_{ij+1k}^n + \mathbf{f}^1 \xi_{i+1jk}^n + \mathbf{f}^2 \xi_{i+1jk}^n + \mathbf{g}^1 \xi_{ijk+1}^n + \mathbf{g}^2 \xi_{ijk+1}^n)$$

$$\xi_s = \mathbf{V}\mathbf{S} - (\mathbf{e}^3 \xi_{ij+1k}^n + \mathbf{e}^4 \xi_{ij+1k}^n + \mathbf{f}^3 \xi_{i+1jk}^n + \mathbf{f}^4 \xi_{i+1jk}^n + \mathbf{g}^3 \xi_{ijk+1}^n + \mathbf{g}^4 \xi_{ijk+1}^n)$$

To enhance convergence of SIP, on alternate iterations the directions of forward substitution and backward substitution are reversed. The matrix is swept through in the reverse direction. In the reverse algorithm the S and Z coefficients are switched as well as the B and H coefficients. All k-1 values are switched with k+1 values, and i-1 values are switched with i+1 values. The matrix is swept through in the increasing j, decreasing i, and decreasing k directions.

ATTACHMENT A. DATA INPUT FORMATS

Data Group I: Simulation Parameters

<u>Line</u>	<u>Columns</u>	<u>Format</u>	<u>Variable</u>	<u>Definition</u>
A-1	1-70	A70	TITLE	Title describing simulation
A-2	1-5	I5	NNR	Number of rows
	6-10	I5	NNC	Number of columns
	11-15	I5	NNL	Number of aquifers (layers)
	16-20	I5	NP	Number of time steps between print-outs
	21-25	I5	ITMAX	Maximum number of iterations allowed
	26-30	I5	NITP	Number of iteration parameters
	31-35	I5	NPUMP	Number of pumping periods
	36-40	I5	LW	Number of observation points
	41-45	I5	NUP	Number of iterations in a time step after which interface position is fixed
	46-50	I5	NOPT	Input/output options parameter: NOPT=0 for default options, NOPT=1 for user specified options (must read in values of IOP array below)
	51-55	I5	NLK	Leakage method parameter, if NLK=1 use method 1 (complete mixing), if NLK=0 use method 2 (restricted mixing)
A-3	1-10	G10.0	ERR	Convergence closure criteria
	11-20	G10.0	STST	Steady-state closure criteria
	21-30	G10.0	RFAC	Relaxation factor
	31-40	G10.0	WFAC	Weighting factor used in calcu- lating projected interface position which is given by: $WFAC * \text{new projection} + (1 - WFAC) * \text{old projection}$
	41-50	G10.0	WITER	Factor used in calculating iteration parameters: usually =1, if divergence occurs a value >1 should be used, if convergence is slow a value <1 should be used
	51-60	G10.0	SGF	Fresh water specific gravity
	61-70	G10.0	SGS	Salt water specific gravity
	71-80	G10.0	VIF	Fresh water viscosity
	81-90	G10.0	VIS	Salt water viscosity
A-4	1-5	I5	NCONT	NCONT=0 for new simulations, NCONT=1 for continuation simulations
	6-25	G20.0	TSEC	TSEC=0. for new simulations, TSEC is the starting time in seconds for a continuation

				run
26-30	I5	NINT		Interface initialization parameter, if NINT=0 read in interface elevations, if NINT=1 initialize ZINT to DEL*PHIF (Ghyben-Herzberg interface)

The following three parameters values are specified for LW observation nodes.

A-5 to	1-5	I5	IW(L)	Row index of observation point
LW+4	6-10	I5	JW(L)	Column index of observation point
	11-15	I5	KW(L)	Layer index of observation point (layer 1 is the bottom layer)

The following lines are needed only if NOPT=1, otherwise, the default values for IOP specified in the data statement on line MA 420 of MAIN are used.

A-LW+5	1-5	I5	IOP(2)	Input/output options parameters, see table 5 for explanation of each option. IOP(1) is not an input parameter, but is specified in the data statement on line MA 420.
	6-10	I5	IOP(3)	
	11-15	I5	IOP(4)	
	16-20	I5	IOP(5)	
	21-25	I5	IOP(6)	
	26-30	I5	IOP(7)	
	31-35	I5	IOP(8)	
	36-40	I5	IOP(9)	
	41-45	I5	IOP(10)	
	46-50	I5	IOP(11)	
	51-55	I5	IOP(12)	

Data Group II: Aquifer parameters

Data sets B-1 through B-15 in this input group are preceded by a leading line having the following format and parameters:

1-5	I5	M		Set M=1 if every array element has the same value, set M=0 if the array elements differ in value
6-15	G10.0	DUMMY		If M=1, every element of the array is assigned the value DUMMY and no further input for the data set is necessary. If M=0, the complete array must be read in. In this case all the values are multiplied by the scaling factor, DUMMY.

In all data sets that follow, array elements are read in row order with a new line for each row. Data sets B-1 through B-14 are repeated NL times, beginning with the lowermost aquifer.

Data Set	Columns	Format	Variable	Definition
B-1	1-100	10G10.0	FKX(I,J,K)	Freshwater hydraulic conductivity in x-direction, must be set equal to zero at inactive nodes
B-2	1-100	10G10.0	FKY(I,J,K)	Freshwater hydraulic conductivity in y-direction
B-3	1-100	10G10.0	STORF(I,J,K)	Freshwater specific storage, must be negative at constant freshwater head nodes
B-4	1-100	10G10.0	STORS(I,J,K)	Saltwater specific storage, must be negative at constant saltwater head nodes
B-5	1-100	10G10.0	POR(I,J,K)	Porosity
B-6	1-100	10G10.0	THCK(I,J,K)	Thickness of aquifer, must be greater than maximum saturated thickness for an unconfined aquifer
B-7	1-100	10G10.0	ZBOT(I,J,K)	Elevation of base of aquifer
B-8	1-100	10G10.0	PHIF(I,J,K)	Initial fresh water heads

Data set B-9 is needed only if NINT=0. For NINT=1 ZINT is initialized to -DEL*PHIF (Ghyben-Herzberg interface).

B-9	1-100	10G10.0	ZINT(I,J,K)	Initial interface elevations
-----	-------	---------	-------------	------------------------------

Data sets B-10 through B-13 are not needed for a new simulation (NCONT=0). For a continuation run, input the values obtained from the previous simulation (from file units IUNIT(27) to IUNIT(30)).

B-10	1-100	10G10.0	FX(I,J,K)	Interface tip projection factor in x-direction
B-11	1-100	10G10.0	FY(I,J,K)	Interface tip projection factor in y-direction
B-12	1-100	10G10.0	SX(I,J,K)	Interface toe projection factor in x-direction
B-13	1-100	10G10.0	SY(I,J,K)	Interface toe projection factor in y-direction

This data set must be specified for each layer.

B-14	1-100	10G10.0	AQL(I,J,K)	Leakance (K'/B') of overlying confining layer
------	-------	---------	------------	---

The following data sets, B-15 through B-18 are specified only once.

B-15	1-100	10G10.0	HEAD(I,J)	Fixed fresh water head above top confining layer, used
------	-------	---------	-----------	--

				for leakage calculations at confined nodes in uppermost aquifer
B-16	1-100	10G10.0	BATH(I,J)	Bathymetry, ocean floor elevation
B-17	1-100	25I5	IWT(I,J)	Code for uppermost aquifer (α): =0 at confined nodes, =1 at unconfined nodes
B-18	1-100	10G10.0	DX(J)	Grid block size in x-direction
B-19	1-100	10G10.0	DY(I)	Grid block size in y-direction

Data Group III: Pumping period data

This group of values must be specified NPUMP times, that is, for each pumping period. Line 2 of the group is specified NWEELL times, that is for the number of pumping wells within the pumping period.

Line	Columns	Format	Variable	Definition
C-1	1-10	G10.0	DT	Initial time step size in seconds
	11-20	G10.0	TFAC	Multiplication factor for time step size
	21-30	G10.0	TMAX	Maximum length of pumping period in days
	31-35	I5	NTSP	Maximum number of time steps allowed, simulation is terminated when either TMAX or NTSP is exceeded
	36-40	I5	NWEELL	Number of pumping wells
C-2	1-5	I5	I	Row index of pumping well
	6-10	I5	J	Column index of pumping well
	11-15	I5	K	Layer index of pumping well
	16-25	G10.0	PUMP(I,J,K)	Well pumpage (L^3/T), positive for extraction, negative for injection
	26-35	G10.0	WTOP(I,J,K)	Elevation of top of open interval of well
	36-45	G10.0	WBOT(I,J,K)	Elevation of bottom of open interval of well
C-3	1-5	I5	M	See Group II for explanation
	6-15	G10.0	DUMMY	See Group II for explanation
C-4	1-100	10G10.0	RECH(I,J)	Recharge rate (L/T)

ATTACHMENT B. LIST OF SELECTED PROGRAM VARIABLES

A(4)	Submatrix element of lower triangular matrix L
AA(4)	Submatrix element of modified matrix (A+B)
AL	α , =-1 for an unconfined node, =0 for a confined node
APROP	FAREA(I,J,K)+SAREA(I,J,K)-1.
AQL(NR,NC,NL)	Leakance of confining layer (K'/B')
AQLBOT	Leakance of underlying confining layer times block area
AQLTOP	Leakance of overlying confining layer times block area
ARDT	Block area divided by time step size
AREA	Block area
ARRAY(4)	Submatrix used in SIP factorization
AXL	Projected interface tip/toe distance in x-direction
AYL	Projected interface tip/toe distance in y-direction
B(4)	Submatrix element of lower triangular matrix L
BATH(NR,NC)	Bathymetry
BB(4)	Submatrix element of modified matrix (A+B)
C(4)	Submatrix element of lower triangular matrix L
CC(4)	Submatrix element of modified matrix (A+B)
CFDIF	Net cumulative freshwater influx to layer (volume)
CFIN(5)	Cumulative freshwater influxes to layer (volume)
CFOUT(5)	Cumulative freshwater outflows from layer (volume)
CFRERF	Cumulative freshwater mass balance error relative to influx
CFRERS	Cumulative freshwater mass balance error relative to change in storage
CFSTR	Cumulative freshwater change in storage (volume)
CSDIF	Net cumulative saltwater influx to layer (volume)
CSIN(5)	Cumulative saltwater influxes to layer (volume)
CSOUT(5)	Cumulative saltwater outflows from layer (volume)
CSRERF	Cumulative saltwater mass balance error relative to influx
CSRERS	Cumulative saltwater mass balance error relative to change in storage
CSSTR	Cumulative saltwater change in storage
D(4)	Submatrix element of lower triangular matrix L
DD(4)	Submatrix element of modified matrix (A+B)
DEL	$\gamma_f/(\gamma_s-\gamma_f)$
DFM	Freshwater head derivative at i or j-1/2
DFMAX	Maximum change in freshwater head over time step
DFP	Freshwater head derivative at i or j+1/2
DMAXF(ITMAX)	Maximum changes in freshwater head over iterations
DMAXS(ITMAX)	Maximum changes in saltwater head over iterations
DPHIF(NR,NC,NL)	Change in freshwater head over iteration
DPHIS(NR,NC,NL)	Change in saltwater head over iteration
DSM	Saltwater head derivative at i or j-1/2
DSMAX	Maximum change in saltwater head over time step
DSP	Saltwater head derivative at i or j+1/2
DT	Time step size in seconds
DTDAYS	Time step size in days
DTHRS	Time step size in hours
DTYRS	Time step size in years
DX(NC)	Grid block dimension in x-direction

DY(NR)	Grid block dimension in y-direction
DZBOT	Slope of bottom of aquifer
DZINT	Slope of interface
DZTOP	Slope of top of aquifer
E(NR,NC,NL,4)	Submatrix element of upper triangular matrix U
EE(4)	Submatrix element of modified matrix (A+B)
EI1(4)	Submatrix used in SIP factorization
EIJK(4)	Submatrix used in SIP factorization
EK1(4)	Submatrix used in SIP factorization
ERR	Convergence closure criteria
F(NR,NC,NL,4)	Submatrix element of upper triangular matrix U
FAREA(NR,NC,NL)	Freshwater area factor for top of block
FDIF	Net freshwater influx to layer (rate)
FF(4)	Submatrix element of modified matrix (A+B)
FFLOW(1000)	Freshwater flow in/out of constant head nodes
FIJK(4)	Submatrix used in SIP factorization
FIN(5)	Freshwater influxes to layer (rate)
FJ1(4)	Submatrix used in SIP factorization
FK1(4)	Submatrix used in SIP factorization
FKX(NR,NC,NL)	Freshwater hydraulic conductivity in x-direction
FKY(NR,NC,NL)	Freshwater hydraulic conductivity in y-direction
FLKB(NR,NC)	Freshwater leakage across bottom of aquifer
FLKT(NR,NC)	Freshwater leakage across top of aquifer
FM	Weighting factor for calculating interface slope
FOUT(5)	Freshwater outflows from layer
FP	Weighting factor for calculating interface slope
FR	Code for freshwater nodes
FRERF	Freshwater mass balance error relative to influx
FRERS	Freshwater mass balance error relative to change in storage
FSTOR	Freshwater change in storage (rate)
FWH(LW,NTSP)	Freshwater head at observation points
FX(NR,NC,NL)	Interface tip projection from previous iteration expressed as a ratio of DX
FXN(NR,NC,NL)	Interface tip projection for new iteration expressed as a ratio of DX
FY(NR,NC,NL)	Interface tip projection from previous iteration expressed as a ratio of DY
FYN(NR,NC,NL)	Interface tip projection for new iteration expressed as a ratio of DY
G(NR,NC,NL,4)	Submatrix element of upper triangular matrix U
GG(4)	Submatrix element of modified matrix (A+B)
GI1(4)	Submatrix used in SIP factorization
GIJK(4)	Submatrix used in SIP factorization
GJ1(4)	Submatrix used in SIP factorization
HEAD(NR,NC)	Freshwater head in topmost inactive layer
HH(4)	Submatrix element of modified matrix (A+B)
ICODE(NR,NC,NL)	Code identifying nature of node
IFL(1000)	Row index of constant freshwater head node
IOP(12)	Input/output options parameters (default or user-specified)
IPUMP	Pumping period index
ISL(1000)	Row index of constant saltwater head node
IUN(32)	Input/output unit numbers (default or user-specified)

IW(LW)	Row index of observation point
JFL(1000)	Column index of constant freshwater head node
JSL(1000)	Column index of constant saltwater head node
JW(LW)	Column index of observation point
KW(LW)	Layer index of observation point
LW	Number of observation points
M	Code for mixed nodes
NC	Number of columns
NCONT	=0 for new simulation, =1 for continuation run
NFH	Number of constant freshwater head nodes in layer
NINT	Interface initialization parameter, =0 to read in interface elevations, >0 to initialize interface elevations to -DEL*PHIF
NIT	Iteration index
NITP	Number of iteration parameters
NL	Number of layers
NLK	Leakage method parameter, =0 method 1 (complete mixing, >0 method 2 (restricted mixing)
NOPT	Input/output options parameter, =0 to use default options, >0 to use user specified options
NP	Number of time steps between print outs
NPUMP	Number of pumping periods
NR	Number of rows
NSH	Number of constant saltwater head nodes in layer
NTSP	Maximum number of time steps allowed
NUP	Number of iterations after which interface position is fixed
PDAYS	Elapsed pumping period time in days
PHIF(NR,NC,NL)	Freshwater heads
PHIFP(NR,NC,NL)	Freshwater heads from previous time step
PHIS(NR,NC,NL)	Saltwater heads
PHISP(NR,NC,NL)	Saltwater heads from previous time step
PHRS	Elapsed pumping period time in hours
POR(NR,NC,NL)	Porosity
PUMP(NR,NC,NL)	Total well pumpage (L^3/T)
PUMPF(NR,NC,NL)	Freshwater pumpage (L^3/T)
PUMPS	Saltwater pumpage (L^3/T)
PYRS	Elapsed pumping period time in years
QF	Right hand side of freshwater flow equation
QS	Right hand side of saltwater flow equation
RECH(NR,NC)	Recharge rate (L/T)
RESF	Freshwater equation residual
RESS	Saltwater equation residual
RFAC	Relaxation factor
S	Code for saltwater nodes
SAREA(NR,NC,NL)	Saltwater area factor for bottom of block
SDIF	Net saltwater influx to layer (rate)
SFLOW(1000)	Saltwater flow in/out of constant head nodes
SGF	Freshwater specific gravity
SGR	SGS/SGF
SGS	Saltwater specific gravity
SLKB(NR,NC)	Saltwater leakage across bottom of aquifer

SLKT(NR,NC)	Saltwater leakage across bottom of aquifer
SM	Weighting factor for calculating interface derivative
SOUT(5)	Saltwater outflows from layer (rate)
SP	Weighting factor for calculating interface derivative
SRERF	Saltwater mass balance error relative to influx
SRERS	Saltwater mass balance error relative to change in storage
SS(4)	Submatrix element of modified matrix (A+B)
SSIN(5)	Saltwater influxes to layer (rate)
SSTOR	Saltwater change in storage (rate)
STORF(NR,NC,NL)	Freshwater specific storage
STORS(NR,NC,NL)	Saltwater specific storage
STST	Steady state closure criteria
SWH(LW,NTSP)	Saltwater head at observation points
SX(NR,NC,NL)	Interface toe projection from previous iteration expressed as a ratio of DX
SXN(NR,NC,NL)	Interface toe projection for new iteration expressed as a ratio of DX
SY(NR,NC,NL)	Interface toe projection from previous iteration expressed as a ratio of DY
SYN(NR,NC,NL)	Interface toe projection for new iteration expressed as a ratio of DY
TCFIN	Total cumulative freshwater influx for layer (volume)
TCFOUT	Total cumulative freshwater outflow from layer
TCSIN	Total cumulative saltwater influx for layer (volume)
TCSOUT	Total cumulative saltwater outflow from layer (volume)
TDAYS	Elapsed simulation time in days
TFAC	Time step size multiplication factor
TFIN	Total freshwater influx for layer (rate)
TFOUT	Total freshwater outflow from layer (rate)
TFX(NR,NC-1,NL)	Mean hydraulic conductivity term in x-direction
TFY(NR-1,NC,NL)	Mean hydraulic conductivity term in y-direction
THCK(NR,NC,NL)	Aquifer thickness
THF(NR,NC,NL)	Freshwater thickness
THIC	Thickness of freshwater/saltwater at block boundary
THICK	Saturated thickness of aquifer
THRS	Elapsed simulation time in hours
THS(NR,NC,NL)	Saltwater thickness
TIME(NTSP+1)	Elapsed simulation time for each time step in years?
TITLE	Title describing simulation
TMAX	Length of pumping period in days
TPUMP	Elapsed pumping period time in seconds
TSEC	Elapsed simulation time in seconds
TSOUT	Total saltwater outflow from layer (rate)
TSSIN	Total saltwater influx to layer (rate)
TT(4)	Submatrix element of modified matrix (A+B)
TTFX(NR,NC-1,NL)	Freshwater transmissive coefficient in x-direction
TTFY(NR-1,NC,NL)	Freshwater transmissive coefficient in y-direction
TTSX(NR,NC-1,NL)	Saltwater transmissive coefficient in x-direction
TTSY(NR-1,NC,NL)	Saltwater transmissive coefficient in y-direction
TYRS	Elapsed simulation time in years
UU(4)	Submatrix element of modified matrix (A+B)
VF(NR,NC,NL)	Freshwater element of vector V

VIF	Freshwater viscosity
VIS	Saltwater viscosity
VS(NR,NC,NL)	Saltwater element of vector V
WBOT(NR,NC,NL)	Elevation of bottom of well screen
WFAC	Weighting factor for interface projection
WITER	Iteration parameter factor
WOPTF(NITP)	Iteration parameter sequence
WTOP(NR,NC,NL)	Elevation of top of well screen
WW(4)	Submatrix element of modified matrix (A+B)
XF	Freshwater element of vector X
XLNEW	New x-projected interface distance
XLOLD	X-projected interface distance from previous iteration
XS	Saltwater element of vector X
YLNEW	New y-projected interface distance
YLOLD	Y-projected interface distance from previous iteration
ZBOT(NR,NC,NL)	Elevation of bottom of aquifer
ZI(LW,NTSP)	Interface elevation at observation points
ZINT(NR,NC,NL)	Interface elevation
ZTOP(NR,NC,NL)	Elevation of top of aquifer
ZZ(4)	Submatrix element of modified matrix (A+B)

ATTACHMENT C: PROGRAM LISTING

```

C -----
C                               PROGRAM SHARP
C SIMULATION OF FRESHWATER AND SALTWATER FLOW ASSUMING AN ABRUPT INTERFACE
C           A QUASI-THREE DIMENSIONAL FINITE DIFFERENCE MODEL
C                               VERSION AS OF SEPT. 12, 1990
C SEE WRIR 90-4130, ATTACHMENT A FOR INPUT FORMATS, ATTACHMENT B FOR DEFINITION
C           OF PROGRAM VARIABLES
C -----
C SPECIFICATIONS                                                    MA 10
C   IMPLICIT REAL*8 (A-H,O-Z)                                       MA 20
C   PARAMETER (NR=40,NC=47,NL=5)                                     MA 30
C   CHARACTER ICODE, IC, FR, M, S, TITLE*70                         MA 40
C                                                                    MA 50
C   COMMON /INTPAR/ NNR,NNC,NNL,NTSP,NP,NITP,NIT,NPUMP,IPUMP       MA 60
C   1,ITMAX,NT,IWT(NR,NC),NFLAG,NCONT,NINT,                         MA 70
C   2LW,IW(10),JW(10),KW(10),NUP,NLK                               MA 80
C                                                                    MA 90
C   COMMON /CHAR/ICODE(NR,NC,NL),IC(10,200),FR,M,S                MA 100
C                                                                    MA 110
C   COMMON /VALUES/ DT,TFAC,ERR,RFAC,SGF,SGS,VIF,VIS,DEL,TSEC,TMAX MA 120
C   1,TPUMP,STST,WFAC,WITER                                         MA 130
C                                                                    MA 140
C   COMMON /ARRAYS/ FKX(NR,NC,NL),FKY(NR,NC,NL),                  MA 150
C   1STORF(NR,NC,NL),STORS(NR,NC,NL),POR(NR,NC,NL),THCK(NR,NC,NL), MA 160
C   2ZBOT(NR,NC,NL),ZINT(NR,NC,NL),AQL(NR,NC,NL),RECH(NR,NC),      MA 170
C   3PUMP(NR,NC,NL),PHIF(NR,NC,NL),PHIS(NR,NC,NL),DX(NC),DY(NR),   MA 180
C   4WOPTF(10),TTFX(NR,NC,NL),TTFY(NR,NC,NL),TTSX(NR,NC,NL),      MA 190
C   5TTSY(NR,NC,NL),DMAXF(200),DMAXS(200),THF(NR,NC,NL),          MA 200
C   6THS(NR,NC,NL),HEAD(NR,NC),PHIFP(NR,NC,NL),PHISP(NR,NC,NL),    MA 210
C   7WTOP(NR,NC,NL),WBOT(NR,NC,NL),PUMPF(NR,NC,NL),FAREA(NR,NC,NL), MA 220
C   8SAREA(NR,NC,NL),ZTOP(NR,NC,NL),BATH(NR,NC),                   MA 230
C   9FX(NR,NC,NL),FY(NR,NC,NL),SX(NR,NC,NL),SY(NR,NC,NL)         MA 240
C                                                                    MA 250
C   COMMON /XNEW/FXN(NR,NC,NL),FYN(NR,NC,NL),SXN(NR,NC,NL),       MA 260
C   1SYN(NR,NC,NL)                                                  MA 270
C                                                                    MA 280
C   COMMON /COEF/DPHIF(NR,NC,NL),DPHIS(NR,NC,NL),VF(NR,NC,NL),    MA 290
C   1VS(NR,NC,NL),E(NR,NC,NL,4),F(NR,NC,NL,4),G(NR,NC,NL,4),A(4),  MA 300
C   2B(4),C(4),D(4),AA(4),BB(4),CC(4),DD(4),EE(4),FF(4),GG(4),HH(4), MA 310
C   3SS(4),TT(4),UU(4),WW(4),ZZ(4),BBB(4),ZZZ(4),ARRAY(4),EK1(4), MA 320
C   4EI1(4),FJ1(4),GJ1(4),FK1(4),GI1(4),EIJK(4),FIJK(4),GIJK(4), MA 330
C   5TFX(NR,NC,NL),TFY(NR,NC,NL),TIME(200),FWH(10,200),SWH(10,200), MA 340
C   6ZI(10,200),FAR(10,200),SAR(10,200)                           MA 350
C                                                                    MA 360
C   COMMON /WHEAD/WPHIF(NR,NC),WPHIS(NR,NC),WZINT(NR,NC)         MA 370
C                                                                    MA 380
C   COMMON /IUNITS/IUN(32),IOP(12)                                  MA 390
C                                                                    MA 400
C   DATA IUN/22*5,2*6,6*7,8,9/                                     MA 410
C   DATA IOP/0,1,1,1,1,1,1,0,0,1,1,0,0/                          MA 420
C                                                                    MA 421

```

C	TO OPEN FILES INTERNALLY, REPLACE THE ABOVE DATA STATEMENT WITH THE	MA 422
C	FOLLOWING DATA STATEMENT	MA 423
C	DATA IOP/1,1,1,1,1,1,0,0,1,1,0,0/	MA 424
C		MA 430
	IF(IOP(1).EQ.1)THEN	MA 440
	OPEN(UNIT=5,FILE='INPUT',STATUS='OLD')	MA 450
	OPEN(UNIT=6,FILE='OUTPUT',STATUS='NEW')	MA 460
	OPEN(UNIT=7,FILE='RESULTS',STATUS='NEW')	MA 470
	OPEN(UNIT=8,FILE='HEADS',STATUS='NEW')	MA 480
	OPEN(UNIT=9,FILE='LEAK',STATUS='NEW')	MA 490
	END IF	MA 500
C		MA 510
C	READ AND WRITE MODEL INPUT	MA 520
	CALL INPUT	MA 530
C		MA 540
C	SET VALUES EQUAL TO ZERO AT INACTIVE NODES	MA 550
	DO 10 K=1,NNL	MA 560
	DO 10 I=1,NNR	MA 570
	DO 10 J=1,NNC	MA 580
	TTFX(I,J,K)=0.0	MA 581
	TTFY(I,J,K)=0.0	MA 582
	TTSX(I,J,K)=0.0	MA 583
	TTSY(I,J,K)=0.0	MA 584
	THF(I,J,K)=0.0	MA 585
	THS(I,J,K)=0.0	MA 586
	IF(FKX(I,J,K).NE.0.) GO TO 10	MA 590
	FKY(I,J,K)=0.0	MA 600
	POR(I,J,K)=0.0	MA 610
	THCK(I,J,K)=0.0	MA 620
	ZINT(I,J,K)=0.0	MA 630
	AQL(I,J,K)=0.0	MA 640
	FX(I,J,K)=0.0	MA 641
	FY(I,J,K)=0.0	MA 642
	SX(I,J,K)=0.0	MA 643
	SY(I,J,K)=0.0	MA 644
10	CONTINUE	MA 650
C		MA 660
	FR='F'	MA 670
	M='M'	MA 680
	S='S'	MA 690
	NFLAG=1	MA 700
	DEL=SGF/(SGS-SGF)	MA 710
	C2=SGF/SGS	MA 720
	C3=1./(1.+DEL)	MA 730
C		MA 740
C	CALCULATE INITIAL SALTWATER POTENTIAL AND TYPE OF NODE:	MA 750
C	F-FRESHWATER ONLY, S-SALTWATER ONLY, M-FRESH AND SALTWATER	MA 760
	DO 20 K=1,NNL	MA 770
	DO 20 I=1,NNR	MA 780
	DO 20 J=1,NNC	MA 790
	FAREA(I,J,K)=0.	MA 810
	SAREA(I,J,K)=0.	MA 820
	ICODE(I,J,K)='.'	MA 830
	FXN(I,J,K)=0.	MA 840

FYN(I, J, K)=0.	MA 850
SXN(I, J, K)=0.	MA 860
SYN(I, J, K)=0.	MA 870
ZTOP(I, J, K)=ZBOT(I, J, K)+THCK(I, J, K)	MA 880
IF(FKX(I, J, K).EQ.0.)GO TO 20	MA 890
IF(IWT(I, J).EQ.1.AND.K.EQ.NNL)THEN	MA 900
ZTOP(I, J, K)=PHIF(I, J, K)	MA 901
IF(ZTOP(I, J, K).GT.(ZBOT(I, J, K)+THCK(I, J, K)))WRITE(IUN(23), 170)	MA 902
& I, J, K	MA 903
END IF	MA 904
TOP=ZTOP(I, J, K)	MA 910
THICK=TOP-ZBOT(I, J, K)	MA 920
IF(ZINT(I, J, K).LT.ZBOT(I, J, K))ZINT(I, J, K)=ZBOT(I, J, K)	MA 930
IF(ZINT(I, J, K).GT.TOP)ZINT(I, J, K)=TOP	MA 940
THS(I, J, K)=ZINT(I, J, K)-ZBOT(I, J, K)	MA 950
THF(I, J, K)=ZTOP(I, J, K)-ZINT(I, J, K)	MA 960
IF(NCONT.GT.0)GO TO 20	MA 970
FX(I, J, K)=0.0	MA 971
FY(I, J, K)=0.0	MA 972
SX(I, J, K)=0.0	MA 973
SY(I, J, K)=0.0	MA 974
IF(DABS(ZINT(I, J, K)-ZBOT(I, J, K)).LE.(0.001*THICK))THEN	MA 980
ICODE(I, J, K)=FR	MA 990
FAREA(I, J, K)=1.	MA1000
FX(I, J, K)=1.	MA1010
FXN(I, J, K)=1.	MA1020
FY(I, J, K)=1.	MA1030
FYN(I, J, K)=1.	MA1040
ELSE IF(DABS(ZINT(I, J, K)-TOP).LE.(0.001*THICK))THEN	MA1050
ICODE(I, J, K)=S	MA1060
SAREA(I, J, K)=1.	MA1070
SX(I, J, K)=1.	MA1080
SXN(I, J, K)=1.	MA1090
SY(I, J, K)=1.	MA1100
SYN(I, J, K)=1.	MA1110
ELSE	MA1120
ICODE(I, J, K)=M	MA1130
FAREA(I, J, K)=1.	MA1140
SAREA(I, J, K)=1.	MA1150
FX(I, J, K)=1.	MA1160
FXN(I, J, K)=1.	MA1170
FY(I, J, K)=1.	MA1180
FYN(I, J, K)=1.	MA1190
SX(I, J, K)=1.	MA1200
SXN(I, J, K)=1.	MA1210
SY(I, J, K)=1.	MA1220
SYN(I, J, K)=1.	MA1230
END IF	MA1240
20 CONTINUE	MA1250
C	MA1260
C CALCULATE ITERATION PARAMETERS	MA1270
CALL IPARAM	MA1280
C	MA1290
C CALCULATE HYDRAULIC CONDUCTIVITIES AT GRID BLOCK BOUNDARIES	MA1300

CALL KMEAN	MA1310
C	MA1320
NT=0	MA1330
TIME(1)=TSEC/31557600.	MA1340
DO 25 L=1,LW	MA1350
I=IW(L)	MA1360
J=JW(L)	MA1370
K=KW(L)	MA1380
FWH(L,1)=PHIF(I,J,K)	MA1390
SWH(L,1)=PHIS(I,J,K)	MA1400
ZI(L,1)=ZINT(I,J,K)	MA1410
IC(L,1)=ICODE(I,J,K)	MA1420
FAR(L,1)=FAREA(I,J,K)	MA1430
SAR(L,1)=SAREA(I,J,K)	MA1440
25 CONTINUE	MA1450
C	MA1460
C BEGIN NEW PUMPING PERIOD CALCULATIONS	MA1470
DO 90 IPUMP=1,NPUMP	MA1480
CALL PUMPER(IPUMP)	MA1490
TSEC=TSEC+DT	MA1500
TPUMP=DT	MA1510
C	MA1520
C BEGIN CALCULATIONS FOR NEW TIME STEP	MA1530
30 NT=NT+1	MA1540
DFMAX=0.0	MA1550
DSMAX=0.0	MA1560
C	MA1570
C RESET POTENTIALS FOR PREVIOUS TIME STEP	MA1580
DO 40 K=1,NNL	MA1590
DO 40 I=1,NNR	MA1600
DO 40 J=1,NNC	MA1610
PHIFP(I,J,K)=PHIF(I,J,K)	MA1620
PHISP(I,J,K)=PHIS(I,J,K)	MA1630
40 CONTINUE	MA1640
C	MA1650
C SOLVE FRESHWATER AND SALTWATER FLOW EQUATIONS	MA1660
CALL SIP(*100)	MA1670
C	MA1680
C DETERMINE MAXIMUM FRESHWATER AND SALTWATER POTENTIAL CHANGE OVER	MA1690
C TIME STEP	MA1700
DO 60 K=1,NNL	MA1710
DO 60 I=1,NNR	MA1720
DO 60 J=1,NNC	MA1730
DELF=DABS(PHIFP(I,J,K)-PHIF(I,J,K))	MA1740
DELS=DABS(PHISP(I,J,K)-PHIS(I,J,K))	MA1750
IF(DELF.GT.DFMAX)DFMAX=DELF	MA1760
IF(DELS.GT.DSMAX)DSMAX=DELS	MA1770
60 CONTINUE	MA1780
C	MA1790
C WRITE CURRENT RESULTS TO FILE	MA1800
IF(IOP(12).EQ.0)THEN	MA1810
CALL RESULTS	MA1820
REWIND(IUN(25))	MA1830
REWIND(IUN(26))	MA1840

	REWIND(IUN(27))	MA1850
	REWIND(IUN(28))	MA1860
	REWIND(IUN(29))	MA1870
	REWIND(IUN(30))	MA1880
	END IF	MA1890
C		MA1900
C	STOP IF MAXIMUM NUMBER OF TIME STEPS IS REACHED	MA1910
	IF(NT.GE.NTSP)THEN	MA1920
	CALL OUTPUT(1)	MA1930
	IF(IOP(12).GT.0)CALL RESULTS	MA1940
C		MA1950
C	PRINT RESULTS AND BEGIN NEW PUMPING PERIOD IF STEADY STATE HAS	MA1960
C	BEEN REACHED	MA1970
	ELSE IF(DFMAX.LT.STST.AND.DSMAX.LT.STST)THEN	MA1980
	WRITE(IUN(24),70)	MA1990
	CALL OUTPUT(1)	MA2000
	IF(IOP(12).GT.0)CALL RESULTS	MA2010
	DT=TMAX-TPUMP	MA2020
	TPUMP=TMAX	MA2030
	TSEC=TSEC+DT	MA2040
	GO TO 90	MA2050
C		MA2060
	ELSE	MA2070
C		MA2080
C	PRINT RESULTS IF NUMBER OF TIME STEPS BETWEEN PRINTOUTS IS REACHED	MA2090
	IF(MOD(NT,NP).EQ.0.)THEN	MA2100
	CALL OUTPUT(1)	MA2110
	IF(IOP(12).GT.0)CALL RESULTS	MA2120
	ELSE IF(IPUMP.EQ.NPUMP.AND.TPUMP.GE.TMAX)THEN	MA2130
	CALL OUTPUT(1)	MA2140
	IF(IOP(12).GT.0)CALL RESULTS	MA2150
	GO TO 90	MA2160
	ELSE	MA2170
	CALL OUTPUT(0)	MA2180
	END IF	MA2190
	DT=TFAC*DT	MA2200
C		MA2210
C	BEGIN NEW TIME STEP	MA2220
	IF((TPUMP+DT).LT.TMAX) THEN	MA2230
	TPUMP=TPUMP+DT	MA2240
	TSEC=TSEC+DT	MA2250
	GO TO 30	MA2260
	ELSE IF(TPUMP.NE.TMAX) THEN	MA2270
	DT=TMAX-TPUMP	MA2280
	TPUMP=TMAX	MA2290
	TSEC=TSEC+DT	MA2300
	GO TO 30	MA2310
	END IF	MA2390
	END IF	MA2400
	90 CONTINUE	MA2410
	100 CONTINUE	MA2420
C		MA2430
C	WRITE OBSERVED HEADS AND INTERFACE ELEVATIONS TO FILE	MA2440
	DO 140 L=1,LW	MA2450

I=IW(L)	MA2460
J=JW(L)	MA2470
K=KW(L)	MA2480
WRITE(IUN(31),150)I,J,K	MA2490
WRITE(IUN(31),160)(TIME(N),FWH(L,N),SWH(L,N),ZI(L,N),IC(L,N),	MA2500
&FAR(L,N),SAR(L,N),N=1,NT+1)	MA2510
140 CONTINUE	MA2520
C	MA2530
C CLOSE OUTPUT FILES	MA2540
IF(IOP(1).EQ.1)THEN	MA2550
CLOSE(5,STATUS='KEEP')	MA2560
CLOSE(6,STATUS='KEEP')	MA2570
CLOSE(7,STATUS='KEEP')	MA2580
CLOSE(8,STATUS='KEEP')	MA2590
CLOSE(9,STATUS='KEEP')	MA2600
END IF	MA2610
C	MA2620
70 FORMAT(/1X,25('*'))/ STEADY STATE ACHIEVED AT:/1X,25('*'))	MA2630
80 FORMAT(10F10.3)	MA2640
150 FORMAT(/' NODE(' ,I3,',',I3,',',I3,')':'//' TIME(YR) FW HEAD' ,	MA2650
1' SW HEAD INTERFACE ICODE FAREA SAREA')	MA2660
160 FORMAT(4F12.5,7X,A1,2F12.5)	MA2670
170 FORMAT(/' ***WARNING: WATER TABLE ABOVE LAND SURFACE AT BLOCK:' ,	MA2671
&I3,2X,I3,2X,I3)	MA2672
END	MA2680
C	MA2690
C -----IN 10	
SUBROUTINE INPUT	IN 20
C -----IN 30	
C	IN 40
C A SUBROUTINE TO READ AND WRITE MODEL INPUT PARAMETERS	IN 50
C	IN 60
IMPLICIT REAL*8 (A-H,O-Z)	IN 70
PARAMETER (NR=40,NC=47,NL=5)	IN 80
CHARACTER ICODE,IC,FR,M,S,TITLE*70	IN 90
C	IN 100
COMMON /INTPAR/ NNR,NNC,NNL,NTSP,NP,NITP,NIT,NPUMP,IPUMP	IN 110
1,ITMAX,NT,IWT(NR,NC),NFLAG,NCONT,NINT,	IN 120
2LW,IW(10),JW(10),KW(10),NUP,NLK	IN 130
C	IN 140
COMMON /CHAR/ICODE(NR,NC,NL),IC(10,200),FR,M,S	IN 150
C	IN 160
COMMON /VALUES/ DT,TFAC,ERR,RFAC,SGF,SGS,VIF,VIS,DEL,TSEC,TMAX	IN 170
1,TPUMP,STST,WFAF,WITER	IN 180
C	IN 190
COMMON /ARRAYS/ FKX(NR,NC,NL),FKY(NR,NC,NL),	IN 200
1STORF(NR,NC,NL),STORS(NR,NC,NL),POR(NR,NC,NL),THCK(NR,NC,NL),	IN 210
2ZBOT(NR,NC,NL),ZINT(NR,NC,NL),AQL(NR,NC,NL),RECH(NR,NC),	IN 220
3PUMP(NR,NC,NL),PHIF(NR,NC,NL),PHIS(NR,NC,NL),DX(NC),DY(NR),	IN 230
4WOPTF(10),TTFX(NR,NC,NL),TTFY(NR,NC,NL),TTSX(NR,NC,NL),	IN 240
5TTSY(NR,NC,NL),DMAXF(200),DMAXS(200),THF(NR,NC,NL),	IN 250
6THS(NR,NC,NL),HEAD(NR,NC),PHIFP(NR,NC,NL),PHISP(NR,NC,NL),	IN 260
7WTOP(NR,NC,NL),WBOT(NR,NC,NL),PUMPF(NR,NC,NL),FAREA(NR,NC,NL),	IN 270
8SAREA(NR,NC,NL),ZTOP(NR,NC,NL),BATH(NR,NC),	IN 280

	9FX(NR,NC,NL),FY(NR,NC,NL),SX(NR,NC,NL),SY(NR,NC,NL)	IN 290
C		IN 300
	COMMON /IUNITS/IUN(32),IOP(12)	IN 310
C		IN 320
	READ(IUN(1),10)TITLE	IN 330
	WRITE(IUN(23),20)TITLE	IN 340
	WRITE(IUN(31),20)TITLE	IN 350
	WRITE(IUN(32),20)TITLE	IN 360
	READ(IUN(1),30)NNR,NNC,NNL,NP,ITMAX,NITP,NPUMP,LW,NUP,NOPT,NLK	IN 370
	WRITE(IUN(23),40)NNR,NNC,NNL,NP,ITMAX,NITP,NPUMP,NUP,NOPT,NLK	IN 380
	IF(NNR.GT.NR)WRITE(IUN(23),41)	IN 381
	IF(NNC.GT.NC)WRITE(IUN(23),42)	IN 382
	IF(NNL.GT.NL)WRITE(IUN(23),43)	IN 383
	READ(IUN(1),50)ERR,STST,RFAC,WFAC,WITER,SGF,SGS,VIF,VIS	IN 390
	WRITE(IUN(23),60)ERR,STST,RFAC,WFAC,WITER,SGF,SGS,VIF,VIS	IN 400
C		IN 750
C	CONTINUATION RUN INFORMATION	IN 760
	READ(IUN(1),31)NCONT,TSEC,NINT	IN 770
31	FORMAT(I5,G20.0,I5)	IN 780
	IF(NCONT.GT.0)THEN	IN 790
	WRITE(IUN(23),32)TSEC	IN 800
32	FORMAT(/ 'CONTINUATION RUN'/' INITIAL TIME =',G20.10,'SEC')	IN 810
	NINT=0	IN 820
	ELSE	IN 830
	WRITE(IUN(23),33)	IN 840
33	FORMAT(/' NEW RUN')	IN 850
	IF(NINT.GT.0)WRITE(IUN(23),34)	IN 860
34	FORMAT(' INTERFACE ELEVATIONS INITIALIZED TO -DEL*PHIF')	IN 870
	END IF	IN 880
C		IN 410
	IF(LW.GT.0)THEN	IN 420
	READ(IUN(1),29)(IW(L),JW(L),KW(L),L=1,LW)	IN 430
29	FORMAT(3I5)	IN 440
	WRITE(IUN(23),65)LW	IN 450
	WRITE(IUN(23),29)(IW(L),JW(L),KW(L),L=1,LW)	IN 460
65	FORMAT(/' LOCATIONS OF ',I4,' OBSERVATION NODES TRACKED:')	IN 470
	END IF	IN 480
C		IN 490
C	USER SPECIFIED OPTIONS	IN 500
	IF(NOPT.EQ.1)READ(IUN(1),30)(IOP(I),I=2,12)	IN 510
	WRITE(IUN(23),35)(IOP(I),I=2,12)	IN 520
35	FORMAT(/' SPECIFIED INPUT/OUTPUT OPTIONS: '/	IN 520
	&/I5,' : 1=PRINT INPUT AQUIFER PARAMETERS, 0=DO NOT PRINT' /	IN 540
	&I5,' : 1=PRINT INPUT PUMPING PERIOD WELL AND RECHARGE DATA, '	IN 550
	&' 0=DO NOT PRINT' /I5,' : 1=PRINT CALCULATED FRESHWATER HEADS, '	IN 560
	&' 0=DO NOT PRINT' /I5,' : 1=PRINT CALCULATED SALTWATER HEADS, '	IN 570
	&' 0=DO NOT PRINT' /I5,' : 1=PRINT CALCULATED INTERFACE ELEVATIONS, ',	IN 580
	&' 0=DO NOT PRINT' /I5,' : 1=PRINT CALCULATED FAREA FACTORS, ',	IN 590
	&' 0=DO NOT PRINT' /I5,' : 1=PRINT CALCULATED SAREA FACTORS, ',	IN 600
	&' 0=DO NOT PRINT' /I5,' : 1=PRINT F-M-S MAP, 0=DO NOT PRINT' /I5,	IN 610
	&' : 1=WRITE BLOCK LEAKAGE VALUES TO FILE, 0=DO NOT WRITE TO',	IN 620
	&' FILE' /I5,' : 1=PRINT ITERATION INFORMATION TO SCREEN, 0=DO NOT',	IN 630
	&' PRINT' /I5,' : 0=REWRITE LATEST RESULTS TO FILE AFTER EACH TIME',	IN 640
	&' STEP, '/' 1=WRITE RESULTS TO FILE EVERY NP TIME STEPS')	IN 650

C		IN 660
C	READ AND WRITE AQUIFER PARAMETERS LAYER BY LAYER	IN 900
	DO 70 K=1,NNL	IN 910
	IF(IOP(2).EQ.1)WRITE(IUN(23),80)K	IN 920
	IF(IOP(2).EQ.1)WRITE(IUN(23),90)	IN 930
	CALL READ(FKX,NNR,NNC,K,IUN(2),IUN(23),IOP(2))	IN 940
	IF(IOP(2).EQ.1)WRITE(IUN(23),100)	IN 950
	CALL READ(FKY,NNR,NNC,K,IUN(3),IUN(23),IOP(2))	IN 960
	IF(IOP(2).EQ.1)WRITE(IUN(23),110)	IN 970
	CALL READ(STORF,NNR,NNC,K,IUN(4),IUN(23),IOP(2))	IN 980
	IF(IOP(2).EQ.1)WRITE(IUN(23),120)	IN 990
	CALL READ(STORS,NNR,NNC,K,IUN(5),IUN(23),IOP(2))	IN1000
	IF(IOP(2).EQ.1)WRITE(IUN(23),130)	IN1010
	CALL READ(POR,NNR,NNC,K,IUN(6),IUN(23),IOP(2))	IN1020
	IF(IOP(2).EQ.1)WRITE(IUN(23),140)	IN1030
	CALL READ(THCK,NNR,NNC,K,IUN(7),IUN(23),IOP(2))	IN1040
	IF(IOP(2).EQ.1)WRITE(IUN(23),150)	IN1050
	CALL READ(ZBOT,NNR,NNC,K,IUN(8),IUN(23),IOP(2))	IN1060
	IF(IOP(2).EQ.1)WRITE(IUN(23),160)	IN1070
	CALL READ(PHIF,NNR,NNC,K,IUN(9),IUN(23),IOP(2))	IN1080
	IF(IOP(2).EQ.1)WRITE(IUN(23),170)	IN1090
	IF(NINT.EQ.0)CALL READ(ZINT,NNR,NNC,K,IUN(10),IUN(23),IOP(2))	IN1100
C		IN1110
	DEL=SGF/(SGS-SGF)	IN1120
	C2=SGF/SGS	IN1130
	C3=1./(1.+DEL)	IN1140
	IF(NINT.GT.0)THEN	IN1141
	DO 161 I=1,NNR	IN1142
	DO 161 J=1,NNC	IN1143
161	ZINT(I,J,K)--DEL*PHIF(I,J,K)	IN1144
	DO 162 I=1,NNR	IN1145
162	IF(IOP(2).EQ.1)WRITE(IUN(23),174)I,(ZINT(I,J,K),J=1,NNC)	IN1146
	END IF	IN1147
	DO 171 I=1,NNR	IN1150
	DO 171 J=1,NNC	IN1160
171	PHIS(I,J,K)=C2*PHIF(I,J,K)+C3*ZINT(I,J,K)	IN1170
	IF(IOP(2).EQ.1)WRITE(IUN(23),172)	IN1180
172	FORMAT(/' INITIAL SALTWATER HEADS')	IN1190
	DO 173 I=1,NNR	IN1200
173	IF(IOP(2).EQ.1)WRITE(IUN(23),174)I,(PHIS(I,J,K),J=1,NNC)	IN1210
174	FORMAT(I5,3X,10G11.4/(8X,10G11.4))	IN1220
	IF(NCONT.GT.0)THEN	IN1221
	IF(IOP(2).EQ.1)WRITE(IUN(23),181)	IN1230
181	FORMAT(/' FX')	IN1240
	CALL READ(FX,NNR,NNC,K,IUN(11),IUN(23),IOP(2))	IN1250
	IF(IOP(2).EQ.1)WRITE(IUN(23),182)	IN1260
182	FORMAT(/' FY')	IN1270
	CALL READ(FY,NNR,NNC,K,IUN(12),IUN(23),IOP(2))	IN1280
	IF(IOP(2).EQ.1)WRITE(IUN(23),183)	IN1290
183	FORMAT(/' SX')	IN1300
	CALL READ(SX,NNR,NNC,K,IUN(13),IUN(23),IOP(2))	IN1310
	IF(IOP(2).EQ.1)WRITE(IUN(23),184)	IN1320
184	FORMAT(/' SY')	IN1330
	CALL READ(SY,NNR,NNC,K,IUN(14),IUN(23),IOP(2))	IN1340

	END IF	IN1341
	IF(IOP(2).EQ.1)WRITE(IUN(23),180)	IN1350
	CALL READ(AQL,NNR,NNC,K,IUN(15),IUN(23),IOP(2))	IN1360
70	CONTINUE	IN1370
C		IN1380
	IF(IOP(2).EQ.1)WRITE(IUN(23),190)	IN1390
	CALL READ1(HEAD,NNR,NNC,IUN(16),IUN(23),IOP(2))	IN1400
	IF(IOP(2).EQ.1)WRITE(IUN(23),195)	IN1410
	CALL READ1(BATH,NNR,NNC,IUN(17),IUN(23),IOP(2))	IN1420
195	FORMAT('/ BATHYMETRY')	IN1430
	DO 210 I=1,NNR	IN1440
210	READ(IUN(18),30)(IWT(I,J),J=1,NNC)	IN1450
	IF(IOP(2).EQ.1)THEN	IN1460
	WRITE(IUN(23),220)	IN1470
	DO 230 I=1,NNR	IN1480
230	WRITE(IUN(23),240)I,(IWT(I,J),J=1,NNC)	IN1490
	END IF	IN1500
C		IN1510
	READ(IUN(19),250)(DX(J),J=1,NNC)	IN1520
	IF(IOP(2).EQ.1)WRITE(IUN(23),260)(DX(J),J=1,NNC)	IN1530
	READ(IUN(20),250)(DY(I),I=1,NNR)	IN1540
	IF(IOP(2).EQ.1)WRITE(IUN(23),270)(DY(I),I=1,NNR)	IN1550
C		IN1560
	RETURN	IN1570
C		IN1580
C	-----	IN1590
	ENTRY PUMPER(IP)	IN1600
C	-----	IN1610
C		IN1620
C	READ AND WRITE SPECIFICATIONS FOR NEW PUMPING PERIOD	IN1630
C		IN1640
	DO 280 I=1,NNR	IN1650
	DO 280 J=1,NNC	IN1660
	IF(IWT(I,J).EQ.1.AND.BATH(I,J).LT.0.)WRITE(IUN(23),350)I,J	IN1661
	DO 280 K=1,NNL	IN1670
	WTOP(I,J,K)=0.0	IN1680
	WBOT(I,J,K)=0.0	IN1690
	PUMP(I,J,K)=0.0	IN1700
280	CONTINUE	IN1710
C		IN1720
	READ(IUN(21),290)DT,TFAC,TMAX,NTSP,NWELL	IN1730
	WRITE(IUN(23),300)IP,DT,TFAC,TMAX,NTSP,NWELL	IN1740
	TMAX=TMAX*86400.	IN1750
	IF(DT.GT.TMAX)DT=TMAX	IN1760
C		IN1770
	IF(NWELL.NE.0)THEN	IN1780
	IF(IOP(3).EQ.1)WRITE(IUN(23),310)	IN1790
	DO 320 N=1,NWELL	IN1800
	READ(IUN(21),330)I,J,K,PUMP(I,J,K),WTOP(I,J,K),WBOT(I,J,K)	IN1810
	IF(IOP(3).EQ.1)WRITE(IUN(23),340)I,J,K,PUMP(I,J,K),	IN1820
	&WTOP(I,J,K),WBOT(I,J,K)	IN1830
320	CONTINUE	IN1840
	END IF	IN1850
	IF(IOP(3).EQ.1)WRITE(IUN(23),200)	IN1860

	CALL READ1(RECH,NNR,NNC,IUN(22),IUN(23),IOP(3))	IN1870
C		IN1880
	RETURN	IN1890
C		IN1900
10	FORMAT(A70)	IN1910
20	FORMAT(/1X,79('-')/5X,A70/1X,79('-')/)	IN1920
30	FORMAT(25I5)	IN1930
40	FORMAT(' NUMBER OF ROWS=',I4/' NUMBER OF COLUMNS=',I4/' NUMBER OF	IN1940
	1 AQUIFERS=',I4/' NUMBER OF TIME STEPS BETWEEN PRINTOUT=',I4/	IN1950
	2' MAXIMUM NUMBER OF ITERATIONS=',I4/' NUMBER OF ITERATION '	IN1960
	3' PARAMETERS=',I4/' NUMBER OF PUMPING PERIODS=',I4/	IN1970
	4' INTERFACE FIXED AFTER ',I4,' ITERATIONS'/' INPUT/OUTPUT OPTIONS'	IN1980
	&' PARAMETER=',I4,' (0=USE DEFAULTS, 1=READ IN OPTIONS ARRAY)'/	IN1981
	&' LEAKAGE OPTION PARAMETER=',I4,' (0=RESTRICTED MIXING,'	IN1982
	&' 1=COMPLETE MIXING)')	IN1990
41	FORMAT(/' ***WARNING-NO. OF ROWS GREATER THAN ARRAY DIMENSION')	IN1991
42	FORMAT(/' ***WARNING-NO. OF COLUMNS GREATER THAN ARRAY DIMENSION')	IN1992
43	FORMAT(/' ***WARNING-NO. OF LAYERS GREATER THAN ARRAY DIMENSION')	IN1993
50	FORMAT(10G10.0)	IN2000
60	FORMAT(/' CONVERGENCE CRITERION=',G11.4/' STEADY STATE CRITERION='	IN2010
	1,G11.4/' RELAXATION FACTOR=',G11.4/' WEIGHTING FACTOR=',G11.4/	IN2020
	1' ITERATION PARAMETER FACTOR=',G11.4/	IN2030
	2' SPECIFIC GRAVITY OF FRESHWATER='	IN2040
	2,G11.4/' SPECIFIC GRAVITY OF SALTWATER=',G11.4/	IN2050
	3' VISCOSITY OF FRESHWATER=',G11.4/' VISCOSITY OF SALTWATER='	IN2060
	4,G11.4/)	IN2070
80	FORMAT(/1X,31('-')/' AQUIFER PARAMETERS FOR LAYER',I3/1X,31('-'))	IN2080
90	FORMAT(/' HYDRAULIC CONDUCTIVITY OF FRESHWATER IN X-DIRECTION')	IN2090
100	FORMAT(/' HYDRAULIC CONDUCTIVITY OF FRESHWATER IN Y-DIRECTION')	IN2100
110	FORMAT(/' FRESHWATER SPECIFIC STORAGE')	IN2110
120	FORMAT(/' SALTWATER SPECIFIC STORAGE')	IN2120
130	FORMAT(/' EFFECTIVE POROSITY')	IN2130
140	FORMAT(/' AQUIFER THICKNESS')	IN2140
150	FORMAT(/' ELEVATION OF BASE OF AQUIFER')	IN2150
160	FORMAT(/' INITIAL FRESHWATER HEAD')	IN2160
170	FORMAT(/' INITIAL INTERFACE ELEVATION')	IN2170
180	FORMAT(/' AQUITARD LEAKANCES (K"/B"')	IN2180
190	FORMAT(/' HEAD IN OVERLYING AQUIFER')	IN2190
200	FORMAT(/' RECHARGE')	IN2200
220	FORMAT(/' CODE FOR UPPER LAYER (1-UNCONFINED NODE,'	IN2210
	2' 0-CONFINED NODE)')	IN2220
240	FORMAT(I5,3X,25I5/(8X,25I5))	IN2230
250	FORMAT(10G10.0)	IN2240
260	FORMAT(/' DELTA X'/(1H ,10G11.4))	IN2250
270	FORMAT(/' DELTA Y'/(1H ,10G11.4))	IN2260
290	FORMAT(3G10.0,4I5)	IN2270
300	FORMAT(/1X,79('-')/30X,' PUMPING PERIOD',I3/1X,79('-')//	IN2280
	120X,' INITIAL TIME STEP(SEC)=' ,G11.4/20X,' MULTIPLICATION FACTOR ',	IN2290
	2' FOR DELTA T=' ,G11.4/20X,' LENGTH OF PUMPING PERIOD(DAYS)=' ,G11.4/	IN2300
	320X,' MAXIMUM NUMBER OF TIME STEPS=' ,I3/	IN2310
	420X,' NUMBER OF ACTIVE WELLS=' ,I3)	IN2320
310	FORMAT(/33X,' PUMPING NODES:' /34X,14('-')//19X,' I',4X,' J',	IN2330
	14X,' K',3X,' PUMPAGE',3X,' SCREEN TOP',3X,' SCREEN BOTTOM' /	IN2340
	217X,52('-'))	IN2350

330	FORMAT(3I5,3G10.4)	IN2360
340	FORMAT(15X,3I5,G13.4,G12.4,G14.4)	IN2370
350	FORMAT(/' ***WARNING-UNCONFINED BLOCK WITH BATHYMETRY LESS THAN ',	IN2371
	&'ZERO AT:',I3,',',I3)	IN2372
	END	IN2380
C		IN2390
C	-----RD 10	
	SUBROUTINE READ(DUM,IN,JN,K,IR,IW,IO)	RD 20
C	-----RD 30	
C		RD 40
C	A SUBROUTINE TO READ AND WRITE DATA ARRAYS	RD 50
C		RD 60
	IMPLICIT REAL*8 (A-H,O-Z)	RD 70
	PARAMETER (NR=40,NC=47,NL=5)	RD 80
C		RD 90
	DIMENSION DUM(NR,NC,NL),DUM1(NR,NC)	RD 100
C		RD 130
C	THREE DIMENSIONAL ARRAYS	RD 140
	READ(IR,100)M,DUMMY	RD 150
C		RD 160
	IF(M.EQ.1)THEN	RD 170
	DO 10 I=1,IN	RD 180
	DO 10 J=1,JN	RD 190
	DUM(I,J,K)=DUMMY	RD 200
10	CONTINUE	RD 210
	IF(IO.EQ.1)WRITE(IW,140)DUMMY	RD 220
	ELSE	RD 230
	DO 20 I=1,IN	RD 240
	READ(IR,110)(DUM(I,J,K),J=1,JN)	RD 250
20	CONTINUE	RD 260
	DO 30 I=1,IN	RD 270
	DO 30 J=1,JN	RD 280
	DUM(I,J,K)=DUM(I,J,K)*DUMMY	RD 290
30	CONTINUE	RD 300
	IF(IO.EQ.1)THEN	RD 310
	DO 40 I=1,IN	RD 320
	WRITE(IW,130)I,(DUM(I,J,K),J=1,JN)	RD 330
40	CONTINUE	RD 340
	END IF	RD 350
	END IF	RD 360
C		RD 370
	RETURN	RD 380
C		RD 390
C	-----RD 400	
	ENTRY READ1(DUM1,IN,JN,IR,IW,IO)	RD 410
C	-----RD 420	
C		RD 450
C	TWO DIMENSIONAL ARRAYS	RD 460
	READ(IR,100)M,DUMMY	RD 470
C		RD 480
	IF(M.EQ.1)THEN	RD 490
	DO 60 I=1,IN	RD 500
	DO 60 J=1,JN	RD 510
	DUM1(I,J)=DUMMY	RD 520

60	CONTINUE	RD 530
	IF(IO.EQ.1)WRITE(IW,140)DUMMY	RD 540
	ELSE	RD 550
	DO 70 I=1,IN	RD 560
	READ(IR,110)(DUM1(I,J),J=1,JN)	RD 570
70	CONTINUE	RD 580
	DO 80 I=1,IN	RD 590
	DO 80 J=1,JN	RD 600
	DUM1(I,J)=DUM1(I,J)*DUMMY	RD 610
80	CONTINUE	RD 620
	IF(IO.EQ.1)THEN	RD 630
	DO 90 I=1,IN	RD 640
	WRITE(IW,130)I,(DUM1(I,J),J=1,JN)	RD 650
90	CONTINUE	RD 660
	END IF	RD 670
	END IF	RD 680
C		RD 690
	RETURN	RD 700
C		RD 710
100	FORMAT(I5,G10.0)	RD 720
110	FORMAT(10G10.0)	RD 730
130	FORMAT(I5,3X,10G11.4/(8X,10G11.4))	RD 740
140	FORMAT(/1H ,G11.4)	RD 750
	END	RD 760
C		RD 770
C	-----	IP 10
	SUBROUTINE IPARAM	IP 20
C	-----	IP 30
C		IP 40
C	A SUBROUTINE TO CALCULATE ITERATION PARAMETERS	IP 50
C		IP 60
	IMPLICIT REAL*8 (A-H,O-Z)	IP 70
	PARAMETER (NR=40,NC=47,NL=5)	IP 80
	CHARACTER ICODE,IC,FR,M,S,TITLE*70	IP 90
C		IP 100
	COMMON /INTPAR/ NNR,NNC,NNL,NTSP,NP,NITP,NIT,NPUMP,IPUMP	IP 110
	1,ITMAX,NT,IWT(NR,NC),NFLAG,NCONT,NINT,	IP 120
	2LW,IW(10),JW(10),KW(10),NUP,NLK	IP 130
C		IP 140
	COMMON /CHAR/ICODE(NR,NC,NL),IC(10,200),FR,M,S	IP 150
C		IP 160
	COMMON /VALUES/ DT,TFAC,ERR,RFAC,SGF,SGS,VIF,VIS,DEL,TSEC,TMAX	IP 170
	1,TPUMP,STST,WFAC,WITER	IP 180
C		IP 190
	COMMON /ARRAYS/ FKX(NR,NC,NL),FKY(NR,NC,NL),	IP 200
	1STORF(NR,NC,NL),STORS(NR,NC,NL),POR(NR,NC,NL),THCK(NR,NC,NL),	IP 210
	2ZBOT(NR,NC,NL),ZINT(NR,NC,NL),AQL(NR,NC,NL),RECH(NR,NC),	IP 220
	3PUMP(NR,NC,NL),PHIF(NR,NC,NL),PHIS(NR,NC,NL),DX(NC),DY(NR),	IP 230
	4WOPTF(10),TTFX(NR,NC,NL),TTFY(NR,NC,NL),TTSX(NR,NC,NL),	IP 240
	5TTSY(NR,NC,NL),DMAXF(200),DMAXS(200),THF(NR,NC,NL),	IP 250
	6THS(NR,NC,NL),HEAD(NR,NC),PHIFP(NR,NC,NL),PHISP(NR,NC,NL),	IP 260
	7WTOP(NR,NC,NL),WBOT(NR,NC,NL),PUMPF(NR,NC,NL),FAREA(NR,NC,NL),	IP 270
	8SAREA(NR,NC,NL),ZTOP(NR,NC,NL),BATH(NR,NC),	IP 280
	9FX(NR,NC,NL),FY(NR,NC,NL),SX(NR,NC,NL),SY(NR,NC,NL)	IP 290

C		IP 300
	COMMON /IUNITS/IUN(32),IOP(12)	IP 310
C		IP 320
	SNR=DBLE(NNR*NNR)	IP 330
	SNC=DBLE(NNC*NNC)	IP 340
	X1=DBLE(NITP-1)	IP 350
	WF=0.	IP 360
	WS=1.	IP 370
	C1=(VIF*SGS)/(VIS*SGF)	IP 380
C		IP 390
	DO 10 K=1,NNL	IP 400
	DO 10 I=1,NNR	IP 410
	DO 10 J=1,NNC	IP 420
	IF(FKX(I,J,K).EQ.0.)GO TO 10	IP 430
	IF(FKY(I,J,K).EQ.0.)WRITE(IUN(23),40)	IP 431
	DX2=DX(J)**2	IP 440
	DY2=DY(I)**2	IP 450
	RHOF=(FKY(I,J,K)*DX2)/(FKX(I,J,K)*DY2)	IP 460
	FMAX=DMAX1(SNC*(1.+RHOF),SNR*(1.+1./RHOF))	IP 470
	IF(FMAX.GT.WF)WF=FMAX	IP 480
10	CONTINUE	IP 490
C		IP 500
	WF=3.141592654**2/(2.*WF)	IP 510
	WOPTF(1)=0.DO	IP 520
	DO 20 I=2,NITP	IP 530
	X2=DBLE(I-1)	IP 540
	WOPTF(I)=1.-(WF*WITER)**(X2/X1)	IP 550
20	CONTINUE	IP 560
C		IP 570
	WRITE(IUN(23),30)(WOPTF(I),I=1,NITP)	IP 580
	RETURN	IP 590
C		IP 600
30	FORMAT('/' ITERATION PARAMETERS:'/ ' FRESHWATER EQUATION:'	IP 610
	&,10G9.4/(24X,10G9.4))	IP 620
40	FORMAT('/' ***WARNING-FKY(' ,I3,' ,',I3,' ,',I3,') =0 AT AN ACTIVE ' ,	IP 621
	&' BLOCK')	IP 622
	END	IP 630
C		IP 640
C	-----KM 10	
	SUBROUTINE KMEAN	KM 20
C	-----KM 30	
C		KM 40
C	A SUBROUTINE TO CALCULATE MEAN CONDUCTIVITY TERMS AT BLOCK BOUNDARIES	KM 50
C		KM 60
	IMPLICIT REAL*8 (A-H,O-Z)	KM 70
	PARAMETER (NR=40,NC=47,NL=5)	KM 80
	CHARACTER ICODE,IC,FR,M,S,TITLE*70	KM 90
C		KM 100
	COMMON /INTPAR/ NNR,NNC,NNL,NTSP,NP,NITP,NIT,NPUMP,IPUMP	KM 110
	1,ITMAX,NT,IWT(NR,NC),NFLAG,NCONT,NINT,	KM 120
	2LW,IW(10),JW(10),KW(10),NUP,NLK	KM 130
C		KM 140
	COMMON /CHAR/ICODE(NR,NC,NL),IC(10,200),FR,M,S	KM 150
C		KM 160

	COMMON /VALUES/ DT,TFAC,ERR,RFAC,SGF,SGS,VIF,VIS,DEL,TSEC,TMAX	KM 170
	1, TPUMP, STST, WFAC, WITER	KM 180
C		KM 190
	COMMON /ARRAYS/ FKX(NR,NC,NL),FKY(NR,NC,NL),	KM 200
	1STORF(NR,NC,NL),STORS(NR,NC,NL),POR(NR,NC,NL),THCK(NR,NC,NL),	KM 210
	2ZBOT(NR,NC,NL),ZINT(NR,NC,NL),AQL(NR,NC,NL),RECH(NR,NC),	KM 220
	3PUMP(NR,NC,NL),PHIF(NR,NC,NL),PHIS(NR,NC,NL),DX(NC),DY(NR),	KM 230
	4WOPTF(10),TTFX(NR,NC,NL),TTFY(NR,NC,NL),TTSX(NR,NC,NL),	KM 240
	5TTSY(NR,NC,NL),DMAXF(200),DMAXS(200),THF(NR,NC,NL),	KM 250
	6THS(NR,NC,NL),HEAD(NR,NC),PHIFP(NR,NC,NL),PHISP(NR,NC,NL),	KM 260
	7WTOP(NR,NC,NL),WBOT(NR,NC,NL),PUMPF(NR,NC,NL),FAREA(NR,NC,NL),	KM 270
	8SAREA(NR,NC,NL),ZTOP(NR,NC,NL),BATH(NR,NC),	KM 280
	9FX(NR,NC,NL),FY(NR,NC,NL),SX(NR,NC,NL),SY(NR,NC,NL)	KM 290
C		KM 300
	COMMON /COEF/DPHIF(NR,NC,NL),DPHIS(NR,NC,NL),VF(NR,NC,NL),	KM 310
	1VS(NR,NC,NL),E(NR,NC,NL,4),F(NR,NC,NL,4),G(NR,NC,NL,4),A(4),	KM 320
	2B(4),C(4),D(4),AA(4),BB(4),CC(4),DD(4),EE(4),FF(4),GG(4),HH(4),	KM 330
	3SS(4),TT(4),UU(4),WW(4),ZZ(4),BBB(4),ZZZ(4),ARRAY(4),EK1(4),	KM 340
	4EI1(4),FJ1(4),GJ1(4),FK1(4),GI1(4),EIJK(4),FIJK(4),GIJK(4),	KM 350
	5TFX(NR,NC,NL),TFY(NR,NC,NL),TIME(200),FWH(10,200),SWH(10,200),	KM 360
	6ZI(10,200),FAR(10,200),SAR(10,200)	KM 370
C		KM 380
C	CALCULATE HARMONIC MEANS	KM 390
	DO 30 K=1,NNL	KM 400
C		KM 410
C	X DIRECTION	KM 420
	DO 10 I=1,NNR	KM 430
	DO 10 J=1,NNC-1	KM 440
	J2=J+1	KM 450
	TFX(I,J,K)=0.0	KM 460
	IF(FKX(I,J,K).EQ.0.) GO TO 10	KM 470
	T1=2*FKX(I,J2,K)*FKX(I,J,K)	KM 480
	B1=FKX(I,J2,K)*DX(J)+FKX(I,J,K)*DX(J2)	KM 490
	TFX(I,J,K)=T1/B1	KM 500
10	CONTINUE	KM 510
C		KM 520
C	Y DIRECTION	KM 530
	DO 20 I=1,NNR-1	KM 540
	DO 20 J=1,NNC	KM 550
	I2=I+1	KM 560
	TFY(I,J,K)=0.0	KM 570
	IF(FKX(I,J,K).EQ.0.) GO TO 20	KM 580
	T1=2*FKY(I2,J,K)*FKY(I,J,K)	KM 590
	B1=FKY(I2,J,K)*DY(I)+FKY(I,J,K)*DY(I2)	KM 600
	TFY(I,J,K)=T1/B1	KM 610
20	CONTINUE	KM 620
30	CONTINUE	KM 630
C		KM 640
	RETURN	KM 650
	END	KM 660
C		KM 670
C	-----SI 10	
	SUBROUTINE SIP(*)	SI 20
C	-----SI 30	

C		SI 40
C	A SUBROUTINE TO SOLVE THE FRESHWATER AND SALTWATER FLOW EQUATIONS	SI 50
C	USING THE STRONGLY IMPLICIT PROCEDURE	SI 60
C		SI 70
	IMPLICIT REAL*8 (A-H,O-Z)	SI 80
	PARAMETER (NR=40,NC=47,NL=5)	SI 90
	CHARACTER ICODE,IC,FR,M,S,TITLE*70	SI 100
C		SI 110
	COMMON /INTPAR/ NNR,NNC,NNL,NTSP,NP,NITP,NIT,NPUMP,IPUMP	SI 120
	1,ITMAX,NT,IWT(NR,NC),NFLAG,NCONT,NINT,	SI 130
	2LW,IW(10),JW(10),KW(10),NUP,NLK	SI 140
C		SI 150
	COMMON /CHAR/ICODE(NR,NC,NL),IC(10,200),FR,M,S	SI 160
C		SI 170
	COMMON /VALUES/ DT,TFAC,ERR,RFAC,SGF,SGS,VIF,VIS,DEL,TSEC,TMAX	SI 180
	1,TPUMP,STST,WFAC,WITER	SI 190
C		SI 200
	COMMON /ARRAYS/ FKX(NR,NC,NL),FKY(NR,NC,NL),	SI 210
	1STORF(NR,NC,NL),STORS(NR,NC,NL),POR(NR,NC,NL),THCK(NR,NC,NL),	SI 220
	2ZBOT(NR,NC,NL),ZINT(NR,NC,NL),AQL(NR,NC,NL),RECH(NR,NC),	SI 230
	3PUMP(NR,NC,NL),PHIF(NR,NC,NL),PHIS(NR,NC,NL),DX(NC),DY(NR),	SI 240
	4WOPTF(10),TTFX(NR,NC,NL),TTFY(NR,NC,NL),TTSX(NR,NC,NL),	SI 250
	5TTSY(NR,NC,NL),DMAXF(200),DMAXS(200),THF(NR,NC,NL),	SI 260
	6THS(NR,NC,NL),HEAD(NR,NC),PHIFP(NR,NC,NL),PHISP(NR,NC,NL),	SI 270
	7WTOP(NR,NC,NL),WBOT(NR,NC,NL),PUMPF(NR,NC,NL),FAREA(NR,NC,NL),	SI 280
	8SAREA(NR,NC,NL),ZTOP(NR,NC,NL),BATH(NR,NC),	SI 290
	9FX(NR,NC,NL),FY(NR,NC,NL),SX(NR,NC,NL),SY(NR,NC,NL)	SI 300
C		SI 310
	COMMON /XNEW/FXN(NR,NC,NL),FYN(NR,NC,NL),SXX(NR,NC,NL),	SI 320
	1SYN(NR,NC,NL)	SI 330
C		SI 340
	COMMON /COEF/DPHIF(NR,NC,NL),DPHIS(NR,NC,NL),VF(NR,NC,NL),	SI 350
	1VS(NR,NC,NL),E(NR,NC,NL,4),F(NR,NC,NL,4),G(NR,NC,NL,4),A(4),	SI 360
	2B(4),C(4),D(4),AA(4),BB(4),CC(4),DD(4),EE(4),FF(4),GG(4),HH(4),	SI 370
	3SS(4),TT(4),UU(4),WW(4),ZZ(4),BBB(4),ZZZ(4),ARRAY(4),EK1(4),	SI 380
	4EI1(4),FJ1(4),GJ1(4),FK1(4),GI1(4),EIJK(4),FIJK(4),GIJK(4),	SI 390
	5TFX(NR,NC,NL),TFY(NR,NC,NL),TIME(200),FWH(10,200),SWH(10,200),	SI 400
	6ZI(10,200),FAR(10,200),SAR(10,200)	SI 410
C		SI 420
	COMMON /IUNITS/IUN(32),IOP(12)	SI 430
C		SI 440
C	INITIALIZE VARIABLES	SI 450
5	NIT=0	SI 460
	ITP=0	SI 470
	DO 10 K=1,NNL	SI 480
	DO 10 I=1,NNR	SI 490
	DO 10 J=1,NNC	SI 500
	VF(I,J,K)=0.0	SI 510
	VS(I,J,K)=0.0	SI 520
	DPHIF(I,J,K)=0.0	SI 530
	DPHIS(I,J,K)=0.0	SI 540
	DO 10 L=1,4	SI 550
	E(I,J,K,L)=0.	SI 560
	F(I,J,K,L)=0.	SI 570

G(I,J,K,L)=0.	SI 580
10 CONTINUE	SI 590
C	SI 600
C BEGIN NEW ITERATION	SI 610
100 NIT=NIT+1	SI 620
DFMAX=0.0	SI 630
DSMAX=0.0	SI 640
C	SI 650
C STOP IF MAXIMUM NUMBER OF ITERATIONS IS EXCEEDED	SI 660
IF(NIT.GT.ITMAX)THEN	SI 670
WRITE(IUN(24),20)	SI 680
CALL OUTPUT(1)	SI 690
RETURN 1	SI 700
END IF	SI 710
C	SI 720
C SELECT ITERATION PARAMETER	SI 730
IF(MOD(NIT,NITP))30,30,40	SI 740
30 ITP=0	SI 750
40 ITP=ITP+1	SI 760
W=WOPTF(ITP)	SI 770
C	SI 780
C UPDATE TRANSMISSIVITY COEFFICIENTS AND INTERFACE TRACKING	SI 790
IF(NIT.LE.NUP)CALL TCOEF	SI 800
C	SI 810
C SELECT NORMAL OR REVERSE ALGORITHM	SI 820
IF(MOD(NIT,2))300,300,200	SI 830
C	SI 840
C NORMAL ALGORITHM	SI 850
200 IFLAG=0	SI 860
C	SI 870
C FACTOR COEFFICIENT MATRIX (A+B)	SI 880
DO 50 K=1,NNL	SI 890
DO 50 I=2,NNR-1	SI 900
DO 50 J=2,NNC-1	SI 910
IF(FKX(I,J,K).EQ.0.)GO TO 50	SI 920
AL=0.0	SI 930
IF(K.EQ.NNL.AND.IWT(I,J).EQ.1)AL=1.0	SI 940
CALL FACTOR(I,J,K,IFLAG,AL,W)	SI 950
50 CONTINUE	SI 960
C	SI 970
C BACK SUBSTITUTION FOR CHANGES IN FRESHWATER AND SALTWATER HEADS	SI 980
DO 60 KK=1,NNL	SI 990
K=NNL-KK+1	SI1000
DO 60 II=1,NNR-2	SI1010
I=NNR-II	SI1020
DO 60 JJ=1,NNC-2	SI1030
J=NNC-JJ	SI1040
I2=I+1	SI1050
J2=J+1	SI1060
K2=K+1	SI1070
DFK2=0.	SI1080
DSK2=0.	SI1090
IF(K2.LE.NNL)THEN	SI1100
DFK2=DPHIF(I,J,K2)	SI1110

DSK2=DPHIS(I,J,K2)	SI1120
END IF	SI1130
DPHIF(I,J,K)=VF(I,J,K)-(E(I,J,K,1)*DPHIF(I,J2,K)+E(I,J,K,2)*	SI1140
1DPHIS(I,J2,K)+F(I,J,K,1)*DPHIF(I2,J,K)+F(I,J,K,2)*DPHIS(I2,J,K)	SI1150
2+G(I,J,K,1)*DFK2+G(I,J,K,2)*DSK2)	SI1160
DPHIS(I,J,K)=VS(I,J,K)-(E(I,J,K,3)*DPHIF(I,J2,K)+E(I,J,K,4)*	SI1170
1DPHIS(I,J2,K)+F(I,J,K,3)*DPHIF(I2,J,K)+F(I,J,K,4)*DPHIS(I2,J,K)	SI1180
2+G(I,J,K,3)*DFK2+G(I,J,K,4)*DSK2)	SI1190
60 CONTINUE	SI1200
GO TO 400	SI1210
C	SI1220
C REVERSE ALGORITHM	SI1230
300 IFLAG=1	SI1240
C FACTOR COEFFICIENT MATRIX A	SI1250
DO 70 KK=1,NNL	SI1260
K=NNL-KK+1	SI1270
DO 70 II=1,NNR-2	SI1280
I=NNR-II	SI1290
DO 70 J=2,NNC-1	SI1300
IF(FKX(I,J,K).EQ.0.)GO TO 70	SI1310
AL=0.0	SI1320
IF(K.EQ.NNL.AND.IWT(I,J).EQ.1)AL=1.0	SI1330
CALL FACTOR(I,J,K,IFLAG,AL,W)	SI1340
70 CONTINUE	SI1350
C	SI1360
C BACK SUBSTITUTE FOR FRESHWATER AND SALTWATER HEAD CHANGES	SI1370
DO 80 K=1,NNL	SI1380
DO 80 I=2,NNR-1	SI1390
DO 80 JJ=1,NNC-2	SI1400
J=NNC-JJ	SI1410
I1=I-1	SI1420
J2=J+1	SI1430
K1=K-1	SI1440
DFK1=0.	SI1450
DSK1=0.	SI1460
IF(K1.GE.1)THEN	SI1470
DFK1=DPHIF(I,J,K1)	SI1480
DSK1=DPHIS(I,J,K1)	SI1490
END IF	SI1500
DPHIF(I,J,K)=VF(I,J,K)-(E(I,J,K,1)*DPHIF(I,J2,K)+E(I,J,K,2)*	SI1510
1DPHIS(I,J2,K)+F(I,J,K,1)*DPHIF(I1,J,K)+F(I,J,K,2)*DPHIS(I1,J,K)	SI1520
2+G(I,J,K,1)*DFK1+G(I,J,K,2)*DSK1)	SI1530
DPHIS(I,J,K)=VS(I,J,K)-(E(I,J,K,3)*DPHIF(I,J2,K)+E(I,J,K,4)*	SI1540
1DPHIS(I,J2,K)+F(I,J,K,3)*DPHIF(I1,J,K)+F(I,J,K,4)*DPHIS(I1,J,K)	SI1550
2+G(I,J,K,3)*DFK1+G(I,J,K,4)*DSK1)	SI1560
80 CONTINUE	SI1570
C	SI1580
400 CONTINUE	SI1590
C	SI1600
C DETERMINE LARGEST FRESHWATER AND SALTWATER HEAD CHANGE OVER	SI1610
C ITERATION AND UPDATE VALUES	SI1620
DO 90 K=1,NNL	SI1630
DO 90 I=2,NNR-1	SI1640
DO 90 J=2,NNC-1	SI1650

IF(FKX(I,J,K).EQ.0.)GO TO 90	SI1660
FMULT=1-INT(1.-FAREA(I,J,K))	SI1670
SMULT=1-INT(1.-SAREA(I,J,K))	SI1680
APHIF=DABS(FMULT*DPHIF(I,J,K))	SI1690
APHIS=DABS(SMULT*DPHIS(I,J,K))	SI1700
IF(APHIF.GT.DFMAX)THEN	SI1710
DFMAX=APHIF	SI1720
IF=I	SI1730
JF=J	SI1740
KF=K	SI1750
END IF	SI1760
IF(APHIS.GT.DSMAX)THEN	SI1770
DSMAX=APHIS	SI1780
IS=I	SI1790
JS=J	SI1800
KS=K	SI1810
END IF	SI1820
PHIF(I,J,K)=PHIF(I,J,K)+DPHIF(I,J,K)	SI1830
PHIS(I,J,K)=PHIS(I,J,K)+DPHIS(I,J,K)	SI1840
90 CONTINUE	SI1850
DMAXF(NIT)=DFMAX	SI1860
DMAXS(NIT)=DSMAX	SI1870
C	SI1871
C WRITE ITERATION INFORMATION TO SCREEN	SI1872
IF(IOP(11).GT.0)THEN	SI1873
WRITE(1,'((A),I4,(A),I4,(A),G12.4,3I3,A3,(A),G12.4,3I3,A3))')	SI1874
& 'TS:',NT,' IT:',NIT,' DFMAX:',DPHIF(IF,JF,KF),IF,JF,KF,	SI1875
& ICODE(IF,JF,KF),' DSMAX:',DPHIS(IS,JS,KS),IS,JS,KS,	SI1876
& ICODE(IS,JS,KS)	SI1877
END IF	SI1878
C	SI1880
C BEGIN NEW ITERATION IF CONVERGENCE HAS NOT BEEN ACHIEVED	SI1890
IF(DFMAX.GT.ERR.OR.DSMAX.GT.ERR) GO TO 100	SI1900
C	SI1910
C STORE VALUES AT OBSERVATION NODES	SI1920
N=NT+1	SI1930
TIME(N)=TSEC/31557600.	SI1940
DO 110 L=1,LW	SI1950
I=IW(L)	SI1960
J=JW(L)	SI1970
K=KW(L)	SI1980
FWH(L,N)=PHIF(I,J,K)	SI1990
SWH(L,N)=PHIS(I,J,K)	SI2000
ZI(L,N)=ZINT(I,J,K)	SI2010
IC(L,N)=ICODE(I,J,K)	SI2020
FAR(L,N)=FAREA(I,J,K)	SI2030
SAR(L,N)=SAREA(I,J,K)	SI2040
110 CONTINUE	SI2050
C	SI2060
RETURN	SI2070
C	SI2080
20 FORMAT(/1X,37('*'))/' MAXIMUM NUMBER OF ITERATIONS EXCEEDED'/	SI2090
11X,37('*'))	SI2100
END	SI2110

C		SI2120
C	-----	TC 10
	SUBROUTINE TCOEF	TC 20
C	-----	TC 30
C		TC 40
C	A SUBROUTINE TO CALCULATE FRESHWATER AND SALTWATER TRANSMISSIVITY	TC 50
C	COEFFICIENTS	TC 60
C		TC 70
	IMPLICIT REAL*8 (A-H,O-Z)	TC 80
	PARAMETER (NR=40,NC=47,NL=5)	TC 90
	CHARACTER ICODE,IC,FR,M,S,TITLE*70	TC 100
C		TC 110
	COMMON /INTPAR/ NNR,NNC,NNL,NTSP,NP,NITP,NIT,NPUMP,IPUMP	TC 120
	1,ITMAX,NT,IWT(NR,NC),NFLAG,NCONT,NINT,	TC 130
	2LW,IW(10),JW(10),KW(10),NUP,NLK	TC 140
C		TC 150
	COMMON /CHAR/ICODE(NR,NC,NL),IC(10,200),FR,M,S	TC 160
C		TC 170
	COMMON /VALUES/ DT,TFAC,ERR,RFAC,SGF,SGS,VIF,VIS,DEL,TSEC,TMAX	TC 180
	1,TPUMP,STST,WFAC,WITER	TC 190
C		TC 200
	COMMON /ARRAYS/ FKX(NR,NC,NL),FKY(NR,NC,NL),	TC 210
	1STORF(NR,NC,NL),STORS(NR,NC,NL),POR(NR,NC,NL),THCK(NR,NC,NL),	TC 220
	2ZBOT(NR,NC,NL),ZINT(NR,NC,NL),AQL(NR,NC,NL),RECH(NR,NC),	TC 230
	3PUMP(NR,NC,NL),PHIF(NR,NC,NL),PHIS(NR,NC,NL),DX(NC),DY(NR),	TC 240
	4WOPTF(10),TTFX(NR,NC,NL),TTFY(NR,NC,NL),TTSX(NR,NC,NL),	TC 250
	5TTSY(NR,NC,NL),DMAXF(200),DMAXS(200),THF(NR,NC,NL),	TC 260
	6THS(NR,NC,NL),HEAD(NR,NC),PHIFP(NR,NC,NL),PHISP(NR,NC,NL),	TC 270
	7WTOP(NR,NC,NL),WBOT(NR,NC,NL),PUMPF(NR,NC,NL),FAREA(NR,NC,NL),	TC 280
	8SAREA(NR,NC,NL),ZTOP(NR,NC,NL),BATH(NR,NC),	TC 290
	9FX(NR,NC,NL),FY(NR,NC,NL),SX(NR,NC,NL),SY(NR,NC,NL)	TC 300
C		TC 310
	COMMON /XNEW/FXN(NR,NC,NL),FYN(NR,NC,NL),SXN(NR,NC,NL),	TC 320
	1SYN(NR,NC,NL)	TC 330
C		TC 340
	COMMON /COEF/DPHIF(NR,NC,NL),DPHIS(NR,NC,NL),VF(NR,NC,NL),	TC 350
	1VS(NR,NC,NL),E(NR,NC,NL,4),F(NR,NC,NL,4),G(NR,NC,NL,4),A(4),	TC 360
	2B(4),C(4),D(4),AA(4),BB(4),CC(4),DD(4),EE(4),FF(4),GG(4),HH(4),	TC 370
	3SS(4),TT(4),UU(4),WW(4),ZZ(4),BBB(4),ZZZ(4),ARRAY(4),EK1(4),	TC 380
	4EI1(4),FJ1(4),GJ1(4),FK1(4),GI1(4),EIJK(4),FIJK(4),GIJK(4),	TC 390
	5TFX(NR,NC,NL),TFY(NR,NC,NL),TIME(200),FWH(10,200),SWH(10,200),	TC 400
	6ZI(10,200),FAR(10,200),SAR(10,200)	TC 410
		TC 411
	COMMON /IUNITS/IUN(32),IOP(12)	TC 412
C		TC 420
C	CALCULATE FRESHWATER AND SALTWATER THICKNESSES	TC 430
	C1=(VIF*SGS)/(VIS*SGF)	TC 440
	C2=SGF/SGS	TC 450
	C3=1./(1.+DEL)	TC 460
	DO 10 K=1,NNL	TC 470
	DO 10 I=1,NNR	TC 480
	DO 10 J=1,NNC	TC 490
	IF(FKX(I,J,K).EQ.0.) GO TO 10	TC 500
C		TC 510

C	CALCULATE NEW INTERFACE ELEVATION	TC 520
	ZINT(I,J,K)=(1.+DEL)*PHIS(I,J,K)-DEL*PHIF(I,J,K)	TC 530
	IF(IWT(I,J).EQ.1.AND.K.EQ.NNL)THEN	TC 540
	ZTOP(I,J,K)=PHIF(I,J,K)	TC 541
	IF(ZTOP(I,J,K).GT.(ZBOT(I,J,K)+THCK(I,J,K)))WRITE(IUN(23),170)	TC 542
	& I,J,K	TC 543
	END IF	TC 544
	TOP=ZTOP(I,J,K)	TC 550
	THICK=TOP-ZBOT(I,J,K)	TC 560
	FAREA(I,J,K)=0.	TC 570
	SAREA(I,J,K)=0.	TC 580
	FXN(I,J,K)=0.	TC 590
	FYN(I,J,K)=0.	TC 600
	SXN(I,J,K)=0.	TC 610
	SYN(I,J,K)=0.	TC 620
C		TC 630
C	DETERMINE TYPE OF NODE	TC 640
	IF((ZINT(I,J,K)-ZBOT(I,J,K)).LE.(.01*THICK))THEN	TC 650
	ZINT(I,J,K)=ZBOT(I,J,K)	TC 660
	ICODE(I,J,K)=FR	TC 670
	FAREA(I,J,K)=1.	TC 680
	FXN(I,J,K)=1.	TC 690
	FYN(I,J,K)=1.	TC 700
	ELSE IF((ZTOP(I,J,K)-ZINT(I,J,K)).LE.(.01*THICK))THEN	TC 710
	ZINT(I,J,K)=TOP	TC 720
	ICODE(I,J,K)=S	TC 730
	SAREA(I,J,K)=1.	TC 740
	SXN(I,J,K)=1.	TC 750
	SYN(I,J,K)=1.	TC 760
	ELSE	TC 770
	ICODE(I,J,K)=M	TC 780
	FAREA(I,J,K)=1.	TC 790
	SAREA(I,J,K)=1.	TC 800
	FXN(I,J,K)=1.	TC 810
	FYN(I,J,K)=1.	TC 820
	SXN(I,J,K)=1.	TC 830
	SYN(I,J,K)=1.	TC 840
	END IF	TC 850
C		TC 860
C	CALCULATE FRESHWATER AND SALTWATER THICKNESSES	TC 870
	THS(I,J,K)=ZINT(I,J,K)-ZBOT(I,J,K)	TC 880
	IF(THS(I,J,K).LT.0.0)THS(I,J,K)=0.0	TC 890
	THF(I,J,K)=ZTOP(I,J,K)-ZINT(I,J,K)	TC 900
	IF(THF(I,J,K).LT.0.0)THF(I,J,K)=0.0	TC 910
C		TC 920
10	CONTINUE	TC 930
C		TC 940
C	CALCULATE TRANSMISSIVITY COEFFICIENTS	TC 950
	DO 40 K=1,NNL	TC 960
C		TC 970
C	X DIRECTION	TC 980
	DO 20 I=1,NNR	TC 990
	DO 20 J=1,NNC-1	TC1000
	IF(FKX(I,J,K).EQ.0.)GO TO 20	TC1010

	J2=J+1	TC1020
	FACT=DX(J)/(DX(J2)+DX(J))	TC1030
	THIC=THF(I,J,K)+FACT*(THF(I,J2,K)-THF(I,J,K))	TC1040
	TTFX(I,J,K)=TFX(I,J,K)*THIC	TC1050
	THIC=THS(I,J,K)+FACT*(THS(I,J2,K)-THS(I,J,K))	TC1060
	TTSX(I,J,K)=TFX(I,J,K)*THIC*C1	TC1070
20	CONTINUE	TC1080
C		TC1090
C	Y DIRECTION	TC1100
	DO 30 I=1,NNR-1	TC1110
	DO 30 J=1,NNC	TC1120
	IF(FKX(I,J,K).EQ.0.)GO TO 30	TC1130
	I2=I+1	TC1140
	FACT=DY(I)/(DY(I2)+DY(I))	TC1150
	THIC=THF(I,J,K)+FACT*(THF(I2,J,K)-THF(I,J,K))	TC1160
	TTFY(I,J,K)=TFY(I,J,K)*THIC	TC1170
	THIC=THS(I,J,K)+FACT*(THS(I2,J,K)-THS(I,J,K))	TC1180
	TTSY(I,J,K)=TFY(I,J,K)*THIC*C1	TC1190
30	CONTINUE	TC1200
C		TC1210
C	INTERFACE TIP TRACKING	TC1220
C	X DIRECTION PROJECTION	TC1230
	DO 50 I=2,NNR-1	TC1240
	DO 50 J=2,NNC-1	TC1250
	IF(FKX(I,J,K).EQ.0.)GO TO 50	TC1260
	IF(ICODE(I,J,K).EQ.S)GO TO 50	TC1270
	JP=J+1	TC1290
	JM=J-1	TC1300
	DX2=DX(J)/2	TC1310
C		TC1320
C	HEAD DERIVATIVES IN X DIRECTION	TC1330
	DXM=0.5*(DX(J)+DX(JM))	TC1340
	DXP=0.5*(DX(JP)+DX(J))	TC1350
	DSM=(PHIS(I,J,K)-PHIS(I,JM,K))/DXM	TC1360
	DSP=(PHIS(I,JP,K)-PHIS(I,J,K))/DXP	TC1370
	DFM=(PHIF(I,J,K)-PHIF(I,JM,K))/DXM	TC1380
	DFP=(PHIF(I,JP,K)-PHIF(I,J,K))/DXP	TC1390
C		TC1400
C	PROJECTION IN POSITIVE X DIRECTION	TC1410
	IF(ICODE(I,JP,K).EQ.S)THEN	TC1420
	IF(ICODE(I,JM,K).EQ.' '.OR.ICODE(I,JM,K).EQ.S)THEN	TC1430
	DSM=DSP	TC1440
	DFM=DFP	TC1450
	END IF	TC1460
C		TC1470
C	INTERFACE DERIVATIVE AND SLOPE OF TOP OF AQUIFER	TC1480
	XLOLD=0.	TC1490
	IF(FX(I,J,K).GE.1.0)THEN	TC1500
	XLOLD=(FX(I,J,K)-0.5)*DX(J)+FX(I,JP,K)*DX(JP)	TC1510
	ELSE IF(FX(I,J,K).GT.0.5)THEN	TC1520
	XLOLD=(FX(I,J,K)-0.5)*DX(J)	TC1530
	END IF	TC1540
	FP=2.*FX(I,JP,K)	TC1550
	IF(FP.GT.1.)FP=1.	TC1560

	DZINT=(1.+DEL)*((1.-FP)*DSM+FP*DSP)-DEL*	TC1570
	1((1.-FP)*DFM+FP*DFP)	TC1580
	DZTOP=(ZTOP(I,JP,K)-ZTOP(I,J,K))/DXP	TC1590
C		TC1600
C	NEW PROJECTED DISTANCE AND ADJUSTED TRANSMISSIVITY COEFFICIENTS	TC1610
	DEN=DZINT-DZTOP	TC1620
	IF(DEN.NE.0.0)THEN	TC1630
	XLNEW=THF(I,J,K)/(DZINT-DZTOP)	TC1640
	IF(ICODE(I,J,K).EQ.FR)XLNEW=(ZTOP(I,J,K)-	TC1650
	1((1.+DEL)*PHIS(I,J,K)-DEL*PHIF(I,J,K)))/(DZINT-DZTOP)	TC1660
	ELSE	TC1670
	XLNEW=XLOLD	TC1680
	END IF	TC1690
	AXL=WFACT*DABS(XLNEW)+(1.-WFAC)*XLOLD	TC1700
	IF(AXL.LE.DX2)THEN	TC1710
	FXN(I,J,K)=(AXL+DX2)/DX(J)	TC1720
	TTSX(I,J,K)=TTSX(I,J,K)+TTFX(I,J,K)*C1	TC1730
	TTFX(I,J,K)=0.	TC1740
	IF(K.EQ.NNL.AND.IWT(I,J).EQ.1)FXN(I,J,K)=1.	TC1741
	ELSE	TC1750
	FL=(AXL-DX2)/DX(JP)	TC1760
	IF(FL.GT.1.0)FL=1.0	TC1770
	FXN(I,JP,K)=FL	TC1780
	TTFXN=THF(I,J,K)*(AXL-DX2)/AXL*TFX(I,J,K)	TC1790
	TTSX(I,J,K)=TTSX(I,J,K)+(TTFX(I,J,K)-TTFXN)*C1	TC1800
	TTFX(I,J,K)=TTFXN	TC1810
	END IF	TC1820
	END IF	TC1830
C		TC1840
C	PROJECTION IN THE NEGATIVE X DIRECTION	TC1850
	IF(ICODE(I,JM,K).EQ.S)THEN	TC1860
	IF(ICODE(I,JP,K).EQ.' '.OR.ICODE(I,JP,K).EQ.S)THEN	TC1870
	DSP=DSM	TC1880
	DFP=DFM	TC1890
	END IF	TC1900
C		TC1910
C	INTERFACE DERIVATIVE AND SLOPE OF TOP OF AQUIFER	TC1920
	XLOLD=0.	TC1930
	IF(FX(I,J,K).GE.1.0)THEN	TC1940
	XLOLD=(FX(I,J,K)-0.5)*DX(J)+FX(I,JM,K)*DX(JM)	TC1950
	ELSE IF(FX(I,J,K).GT.0.5)THEN	TC1960
	XLOLD=(FX(I,J,K)-0.5)*DX(J)	TC1970
	END IF	TC1980
	FM=2.*FX(I,JM,K)	TC1990
	IF(FM.GT.1.)FM=1.	TC2000
	DZINT=(1.+DEL)*(FM*DSM+(1.-FM)*DSP)-DEL*	TC2010
	1(FM*DFM+(1.-FM)*DFP)	TC2020
	DZTOP=(ZTOP(I,J,K)-ZTOP(I,JM,K))/DXM	TC2030
C		TC2040
C	NEW PROJECTED DISTANCE AND ADJUSTED TRANSMISSIVITY COEFFICIENTS	TC2050
	DEN=DZINT-DZTOP	TC2060
	IF(DEN.NE.0.0)THEN	TC2070
	XLNEW=THF(I,J,K)/(DZINT-DZTOP)	TC2080
	IF(ICODE(I,J,K).EQ.FR)XLNEW=(ZTOP(I,J,K)-	TC2090

1((1.+DEL)*PHIS(I,J,K)-DEL*PHIF(I,J,K)))/(DZINT-DZTOP)	TC2100
ELSE	TC2110
XLNEW=XLOLD	TC2120
END IF	TC2130
AXL=WFAC*DABS(XLNEW)+(1.-WFAC)*XLOLD	TC2140
IF(AXL.LE.DX2)THEN	TC2150
IF(FXN(I,J,K).LT.1.)THEN	TC2160
FXN(I,J,K)=FXN(I,J,K)+(AXL+DX2)/DX(J)	TC2170
ELSE	TC2180
FXN(I,J,K)=(AXL+DX2)/DX(J)	TC2190
END IF	TC2200
IF(FXN(I,J,K).GT.1.)FXN(I,J,K)=1.0	TC2210
TTSX(I,JM,K)=TTSX(I,JM,K)+TTFX(I,JM,K)*C1	TC2220
TTFX(I,JM,K)=0.	TC2230
IF(K.EQ.NNL.AND.IWT(I,J).EQ.1)FXN(I,J,K)=1.	TC2231
ELSE	TC2240
FL=(AXL-DX2)/DX(JM)	TC2250
IF(FL.GT.1.0)FL=1.0	TC2260
IF(FXN(I,J,K).LT.1.)THEN	TC2270
FXN(I,JM,K)=FXN(I,JM,K)+FL	TC2280
ELSE	TC2290
FXN(I,JM,K)=FL	TC2300
END IF	TC2310
IF(FXN(I,JM,K).GT.1.)FXN(I,JM,K)=1.0	TC2320
TTFXN=THF(I,J,K)*(AXL-DX2)/AXL*TFX(I,JM,K)	TC2330
TTSX(I,JM,K)=TTSX(I,JM,K)+(TTFX(I,JM,K)-TTFXN)*C1	TC2340
TTFX(I,JM,K)=TTFXN	TC2350
END IF	TC2360
END IF	TC2370
C	TC2380
C Y DIRECTION PROJECTION	TC2390
IP=I+1	TC2400
IM=I-1	TC2410
DY2=DY(I)/2.	TC2420
C	TC2430
C HEAD DERIVATIVES IN THE Y DIRECTION	TC2440
DYM=0.5*(DY(I)+DY(IM))	TC2450
DYP=0.5*(DY(IP)+DY(I))	TC2460
DSM=(PHIS(I,J,K)-PHIS(IM,J,K))/DYM	TC2470
DSP=(PHIS(IP,J,K)-PHIS(I,J,K))/DYP	TC2480
DFM=(PHIF(I,J,K)-PHIF(IM,J,K))/DYM	TC2490
DFP=(PHIF(IP,J,K)-PHIF(I,J,K))/DYP	TC2500
C	TC2510
C PROJECTION IN THE POSITIVE Y DIRECTION	TC2520
IF(ICODE(IP,J,K).EQ.S)THEN	TC2530
IF(ICODE(IM,J,K).EQ.' '.OR.ICODE(IM,J,K).EQ.S)THEN	TC2540
DSM=DSP	TC2550
DFM=DFP	TC2560
END IF	TC2570
C	TC2580
C INTERFACE DERIVATIVE AND SLOPE OF TOP OF AQUIFER	TC2590
YLOLD=0.	TC2600
IF(FY(I,J,K).GE.1.0)THEN	TC2610
YLOLD=(FY(I,J,K)-0.5)*DY(I)+FY(IP,J,K)*DY(IP)	TC2620

ELSE IF(FY(I,J,K).GT.0.5)THEN	TC2630
YLOLD=(FY(I,J,K)-0.5)*DY(I)	TC2640
END IF	TC2650
FP=2.*FY(IP,J,K)	TC2660
IF(FP.GT.1.)FP=1.	TC2670
DZINT=(1.+DEL)*((1.-FP)*DSM+FP*DSP)-DEL*	TC2680
1((1.-FP)*DFM+FP*DFP)	TC2690
DZTOP=(ZTOP(IP,J,K)-ZTOP(I,J,K))/DYP	TC2700
C	TC2710
C NEW PROJECTED DISTANCE AND ADJUSTED TRANSMISSIVITY COEFFICIENTS	TC2720
DEN=DZINT-DZTOP	TC2730
IF(DEN.NE.0.0)THEN	TC2740
YLNEW=THF(I,J,K)/(DZINT-DZTOP)	TC2750
IF(ICODE(I,J,K).EQ.FR)YLNEW=(ZTOP(I,J,K)-	TC2760
1((1.+DEL)*PHIS(I,J,K)-DEL*PHIF(I,J,K)))/(DZINT-DZTOP)	TC2770
ELSE	TC2780
YLNEW=YLOLD	TC2790
END IF	TC2800
AYL=WFACT*DABS(YLNEW)+(1.-WFACT)*YLOLD	TC2810
IF(AYL.LE.DY2)THEN	TC2820
FL=(AYL+DY2)/DY(I)	TC2830
FYN(I,J,K)=FL	TC2840
TTSY(I,J,K)=TTSY(I,J,K)+TTFY(I,J,K)*C1	TC2850
TTFY(I,J,K)=0.0	TC2860
IF(K.EQ.NNL.AND.IWT(I,J).EQ.1)FYN(I,J,K)=1.	TC2861
ELSE	TC2870
FL=(AYL-DY2)/DY(IP)	TC2880
IF(FL.GT.1.0)FL=1.0	TC2890
FYN(IP,J,K)=FL	TC2900
TTFYN=THF(I,J,K)*(AYL-DY2)/AYL*TFY(I,J,K)	TC2910
TTSY(I,J,K)=TTSY(I,J,K)+(TTFY(I,J,K)-TTFYN)*C1	TC2920
TTFY(I,J,K)=TTFYN	TC2930
END IF	TC2940
END IF	TC2950
C	TC2960
C PROJECTION IN THE NEGATIVE Y DIRECTION	TC2970
IF(ICODE(IM,J,K).EQ.S)THEN	TC2980
IF(ICODE(IP,J,K).EQ.''.OR.ICODE(IP,J,K).EQ.S)THEN	TC2990
DSP=DSM	TC3000
DFP=DFM	TC3010
END IF	TC3020
C	TC3030
C INTERFACE DERIVATIVE AND SLOPE OF TOP OF AQUIFER	TC3040
YLOLD=0.	TC3050
IF(FY(I,J,K).GE.1.0)THEN	TC3060
YLOLD=(FY(I,J,K)-0.5)*DY(I)+FY(IM,J,K)*DY(IM)	TC3070
ELSE IF(FY(I,J,K).GT.0.5)THEN	TC3080
YLOLD=(FY(I,J,K)-0.5)*DY(I)	TC3090
END IF	TC3100
FM=2.*FY(IM,J,K)	TC3110
IF(FM.GT.1.)FM=1.	TC3120
DZINT=(1.+DEL)*(FM*DSM+(1.-FM)*DSP)-DEL*	TC3130
1(FM*DFM+(1.-FM)*DFP)	TC3140
DZTOP=(ZTOP(I,J,K)-ZTOP(IM,J,K))/DYM	TC3150

C		TC3160
C	NEW PROJECTED DISTANCE AND ADJUSTED TRANSMISSIVITY COEFFICIENTS	TC3170
	DEN=DZINT-DZTOP	TC3180
	IF(DEN.NE.0.0)THEN	TC3190
	YLNEW=THF(I,J,K)/(DZINT-DZTOP)	TC3200
	IF(ICODE(I,J,K).EQ.FR)YLNEW=(ZTOP(I,J,K)-	TC3210
	1((1.+DEL)*PHIS(I,J,K)-DEL*PHIF(I,J,K)))/(DZINT-DZTOP)	TC3220
	ELSE	TC3230
	YLNEW=YLOLD	TC3240
	END IF	TC3250
	AYL=WFACT*DABS(YLNEW)+(1.-WFACT)*YLOLD	TC3260
	IF(AYL.LE.DY2)THEN	TC3270
	FL=(AYL+DY2)/DY(I)	TC3280
	IF(FYN(I,J,K).LT.1.)THEN	TC3290
	FYN(I,J,K)=FYN(I,J,K)+FL	TC3300
	ELSE	TC3310
	FYN(I,J,K)=FL	TC3320
	END IF	TC3330
	IF(FYN(I,J,K).GT.1.)FYN(I,J,K)=1.0	TC3340
	TTSY(IM,J,K)=TTSY(IM,J,K)+TTFY(IM,J,K)*C1	TC3350
	TTFY(IM,J,K)=0.0	TC3360
	IF(K.EQ.NNL.AND.IWT(I,J).EQ.1)FYN(I,J,K)=1.	TC3361
	ELSE	TC3370
	FL=(AYL-DY2)/DY(IM)	TC3380
	IF(FL.GT.1.0)FL=1.0	TC3390
	IF(FYN(IM,J,K).LT.1.)THEN	TC3400
	FYN(IM,J,K)=FYN(IM,J,K)+FL	TC3410
	ELSE	TC3420
	FYN(IM,J,K)=FL	TC3430
	END IF	TC3440
	IF(FYN(IM,J,K).GT.1.)FYN(IM,J,K)=1.0	TC3450
	TTFYN=THF(I,J,K)*(AYL-DY2)/AYL*TTFY(IM,J,K)	TC3460
	TTSY(IM,J,K)=TTSY(IM,J,K)+(TTFY(IM,J,K)-TTFYN)*C1	TC3470
	TTFY(IM,J,K)=TTFYN	TC3480
	END IF	TC3490
	END IF	TC3500
50	CONTINUE	TC3510
C		TC3520
C	INTERFACE TOE TRACKING	TC3530
C	X DIRECTION PROJECTION	TC3540
	DO 80 I=2,NNR-1	TC3550
	DO 80 J=2,NNC-1	TC3560
	IF(FKX(I,J,K).EQ.0.)GO TO 80	TC3570
	IF(ICODE(I,J,K).EQ.FR)GO TO 80	TC3580
	JP=J+1	TC3590
	JM=J-1	TC3600
	DX2=DX(J)/2.	TC3610
C		TC3620
C	HEAD DERIVATIVES IN X DIRECTION	TC3630
	DXM=0.5*(DX(J)+DX(JM))	TC3640
	DXP=0.5*(DX(JP)+DX(J))	TC3650
	DSM=(PHIS(I,J,K)-PHIS(I,JM,K))/DXM	TC3660
	DSP=(PHIS(I,JP,K)-PHIS(I,J,K))/DXP	TC3670
	DFM=(PHIF(I,J,K)-PHIF(I,JM,K))/DXM	TC3680

	DFP=(PHIF(I,JP,K)-PHIF(I,J,K))/DXP	TC3690
C		TC3700
C	PROJECTION IN POSITIVE X DIRECTION	TC3710
	IF(ICODE(I,JP,K).EQ.FR)THEN	TC3720
	IF(ICODE(I,JM,K).EQ.' '.OR.ICODE(I,JM,K).EQ.FR)THEN	TC3730
	DSM=DSP	TC3740
	DFM=DFP	TC3750
	END IF	TC3760
C		TC3770
C	INTERFACE DERIVATIVE AND SLOPE OF BOTTOM OF AQUIFER	TC3780
	XLOLD=0.	TC3790
	IF(SX(I,J,K).GE.1.0)THEN	TC3800
	XLOLD=(SX(I,J,K)-0.5)*DX(J)+SX(I,JP,K)*DX(JP)	TC3810
	ELSE IF(SX(I,J,K).GT.0.5)THEN	TC3820
	XLOLD=(SX(I,J,K)-0.5)*DX(J)	TC3830
	END IF	TC3840
	SP=XLOLD/DXP	TC3850
	IF(SP.GT.1.)SP=1.	TC3860
	DZINT=(1.+DEL)*((1.-SP)*DSM+SP*DSP)-DEL*	TC3870
	1((1.-SP)*DFM+SP*DFP)	TC3880
	DZBOT=(ZBOT(I,JP,K)-ZBOT(I,J,K))/DXP	TC3890
C		TC3900
C	NEW PROJECTED DISTANCE AND ADJUSTED TRANSMISSIVITY COEFFICIENTS	TC3910
	DEN=DZBOT-DZINT	TC3920
	IF(DEN.NE.0.0)THEN	TC3930
	XLNEW=THS(I,J,K)/(DZBOT-DZINT)	TC3940
	IF(ICODE(I,J,K).EQ.S)XLNEW=((1.+DEL)*PHIS(I,J,K)-	TC3950
	1DEL*PHIF(I,J,K))-ZBOT(I,J,K))/(DZBOT-DZINT)	TC3960
	ELSE	TC3970
	XLNEW=XLOLD	TC3980
	END IF	TC3990
	AXL=WFAC*DABS(XLNEW)+(1.-WFAC)*XLOLD	TC4000
	IF(AXL.LE.DX2)THEN	TC4010
	SXN(I,J,K)=(AXL+DX2)/DX(J)	TC4020
	IF(ICODE(I,J,K).NE.S)THEN	TC4030
	TTFX(I,J,K)=TTFX(I,J,K)+TTSX(I,J,K)/C1	TC4040
	TTSX(I,J,K)=0.	TC4050
	ENDIF	TC4060
	ELSE	TC4070
	SL=(AXL-DX2)/DX(JP)	TC4080
	IF(SL.GT.1.0)SL=1.0	TC4090
	SXN(I,JP,K)=SL	TC4100
	IF(ICODE(I,J,K).NE.S)THEN	TC4110
	TTSXN=THS(I,J,K)*(AXL-DX2)/AXL*TFX(I,J,K)*C1	TC4120
	TTFX(I,J,K)=TTFX(I,J,K)+(TTSX(I,J,K)-TTSXN)/C1	TC4130
	TTSX(I,J,K)=TTSXN	TC4140
	ENDIF	TC4150
	END IF	TC4160
	END IF	TC4170
C		TC4180
C	PROJECTION IN NEGATIVE X DIRECTION	TC4190
	IF(ICODE(I,JM,K).EQ.FR)THEN	TC4200
	IF(ICODE(I,JP,K).EQ.' '.OR.ICODE(I,JP,K).EQ.FR)THEN	TC4210
	DSP=DSM	TC4220

	DFP=DFM	TC4230
	END IF	TC4240
C		TC4250
C	INTERFACE DERIVATIVE AND SLOPE OF BOTTOM OF AQUIFER	TC4260
	XLOLD=0.	TC4270
	IF(SX(I,J,K).GE.1.0)THEN	TC4280
	XLOLD=(SX(I,J,K)-0.5)*DX(J)+SX(I,JM,K)*DX(JM)	TC4290
	ELSE IF(SX(I,J,K).GT.0.5)THEN	TC4300
	XLOLD=(SX(I,J,K)-0.5)*DX(J)	TC4310
	END IF	TC4320
	SM=XLOLD/DXM	TC4330
	IF(SM.GT.1.)SM=1.	TC4350
	DZINT=(1.+DEL)*(SM*DSM+(1.-SM)*DSP)-DEL*	TC4360
	1(SM*DFM+(1.-SM)*DFP)	TC4370
	DZBOT=(ZBOT(I,J,K)-ZBOT(I,JM,K))/DXM	TC4380
C		TC4390
C	NEW PROJECTED DISTANCE AND ADJUSTED TRANSMISSIVITY COEFFICIENTS	TC4400
	DEN=DZBOT-DZINT	TC4410
	IF(DEN.NE.0.0)THEN	TC4420
	XLNEW=THS(I,J,K)/(DZBOT-DZINT)	TC4430
	IF(ICODE(I,J,K).EQ.S)XLNEW=(((1.+DEL)*PHIS(I,J,K)-	TC4440
	1DEL*PHIF(I,J,K))-ZBOT(I,J,K))/(DZBOT-DZINT)	TC4450
	ELSE	TC4460
	XLNEW=XLOLD	TC4470
	END IF	TC4480
	AXL=WFAC*DABS(XLNEW)+(1.-WFAC)*XLOLD	TC4490
	IF(AXL.LE.DX2)THEN	TC4500
	IF(SXN(I,J,K).LT.1.)THEN	TC4510
	SXN(I,J,K)=SXN(I,J,K)+(AXL+DX2)/DX(J)	TC4520
	ELSE	TC4530
	SXN(I,J,K)=(AXL+DX2)/DX(J)	TC4540
	END IF	TC4550
	IF(SXN(I,J,K).GT.1.)SXN(I,J,K)=1.	TC4560
	IF(ICODE(I,J,K).NE.S) THEN	TC4570
	TTFX(I,JM,K)=TTFX(I,JM,K)+TTSX(I,JM,K)/C1	TC4580
	TTSX(I,JM,K)=0.	TC4590
	ENDIF	TC4600
	ELSE	TC4610
	SL=(AXL-DX2)/DX(JM)	TC4620
	IF(SL.GT.1.0)SL=1.0	TC4630
	IF(SXN(I,JM,K).LT.1.)THEN	TC4640
	SXN(I,JM,K)=SXN(I,JM,K)+SL	TC4650
	ELSE	TC4660
	SXN(I,JM,K)=SL	TC4670
	END IF	TC4680
	IF(SXN(I,JM,K).GT.1.)SXN(I,JM,K)=1.0	TC4690
	IF(ICODE(I,J,K).EQ.S) THEN	TC4700
	FACT=DX(JM)/(DX(J)+DX(JM))	TC4710
	THIC=THCK(I,JM,K)+FACT*(THCK(I,J,K)-THCK(I,JM,K))	TC4720
	ELSE	TC4730
	TTSXN=THS(I,J,K)*(AXL-DX2)/AXL*TFX(I,JM,K)*C1	TC4740
	TTFX(I,JM,K)=TTFX(I,JM,K)+(TTSX(I,JM,K)-TTSXN)/C1	TC4750
	TTSX(I,JM,K)=TTSXN	TC4760
	END IF	TC4770

END IF	TC4780
END IF	TC4790
C	TC4800
C Y DIRECTION PROJECTION	TC4810
IP=I+1	TC4820
IM=I-1	TC4830
DY2=DY(I)/2.	TC4840
C	TC4850
C HEAD DERIVATIVES IN Y DIRECTION	TC4860
DYM=0.5*(DY(I)+DY(IM))	TC4870
DYP=0.5*(DY(IP)+DY(I))	TC4880
DSM=(PHIS(I,J,K)-PHIS(IM,J,K))/DYM	TC4890
DSP=(PHIS(IP,J,K)-PHIS(I,J,K))/DYP	TC4900
DFM=(PHIF(I,J,K)-PHIF(IM,J,K))/DYM	TC4910
DFP=(PHIF(IP,J,K)-PHIF(I,J,K))/DYP	TC4920
C	TC4930
C PROJECTION IN POSITIVE Y DIRECTION	TC4940
IF(ICODE(IP,J,K).EQ.FR)THEN	TC4950
IF(ICODE(IM,J,K).EQ.' '.OR.ICODE(IM,J,K).EQ.FR)THEN	TC4960
DSM=DSP	TC4970
DFM=DFP	TC4980
END IF	TC4990
C	TC5000
C INTERFACE DERIVATIVE AND SLOPE OF BASE OF AQUIFER	TC5010
YLOLD=0.	TC5020
IF(SY(I,J,K).GE.1.0)THEN	TC5030
YLOLD=(SY(I,J,K)-0.5)*DY(I)+SY(IP,J,K)*DY(IP)	TC5040
ELSE IF(SX(I,J,K).GT.0.5)THEN	TC5050
YLOLD=(SY(I,J,K)-0.5)*DY(I)	TC5060
END IF	TC5070
SP=YLOLD/DYP	TC5080
IF(SP.GT.1.)SP=1.	TC5090
DZINT=(1.+DEL)*((1.-SP)*DSM+SP*DSP)-DEL*	TC5100
1((1.-SP)*DFM+SP*DFP)	TC5110
DZBOT=(ZBOT(IP,J,K)-ZBOT(I,J,K))/DYP	TC5120
C	TC5130
C NEW PROJECTED DISTANCE AND ADJUSTED TRANSMISSIVITY COEFFICIENTS	TC5140
DEN=DZBOT-DZINT	TC5150
IF(DEN.NE.0.0)THEN	TC5160
YLNEW=THS(I,J,K)/(DZBOT-DZINT)	TC5170
IF(ICODE(I,J,K).EQ.S)YLNEW=((1.+DEL)*PHIS(I,J,K)-	TC5180
1DEL*PHIF(I,J,K))-ZBOT(I,J,K))/(DZBOT-DZINT)	TC5190
ELSE	TC5200
YLNEW=YLOLD	TC5210
END IF	TC5220
AYL=WFAC*DABS(YLNEW)+(1.-WFAC)*YLOLD	TC5230
IF(AYL.LE.DY2)THEN	TC5240
SL=(AYL+DY2)/DY(I)	TC5250
SYN(I,J,K)=SL	TC5260
IF(ICODE(I,J,K).NE.S) THEN	TC5270
TTFY(I,J,K)=TTFY(I,J,K)+TTSY(I,J,K)/C1	TC5280
TTSY(I,J,K)=0.0	TC5290
ENDIF	TC5300
ELSE	TC5310

SL=(AYL-DY2)/DY(IP)	TC5320
IF(SL.GT.1.0)SL=1.0	TC5330
SYN(IP,J,K)=SL	TC5340
IF (ICODE(I,J,K).NE.S) THEN	TC5350
TTSYN=THS(I,J,K)*(AYL-DY2)/AYL*TFY(I,J,K)*C1	TC5360
TTFY(I,J,K)=TTFY(I,J,K)+(TTSY(I,J,K)-TTSYN)/C1	TC5370
TTSY(I,J,K)=TTSYN	TC5380
ENDIF	TC5390
END IF	TC5400
END IF	TC5410
C	TC5420
C PROJECTION IN NEGATIVE Y DIRECTION	TC5430
IF(ICODE(IM,J,K).EQ.FR)THEN	TC5440
IF(ICODE(IP,J,K).EQ.' '.OR.ICODE(IP,J,K).EQ.FR)THEN	TC5450
DSP=DSM	TC5460
DFP=DFM	TC5470
ENDIF	TC5480
C	TC5490
C INTERFACE DERIVATIVE AND SLOPE OF BASE OF AQUIFER	TC5500
YLOLD=0.	TC5510
IF(SY(I,J,K).GE.1.0)THEN	TC5520
YLOLD=(SY(I,J,K)-0.5)*DY(I)+SY(IM,J,K)*DY(IM)	TC5530
ELSE IF(SY(I,J,K).GT.0.5)THEN	TC5540
YLOLD=(SY(I,J,K)-0.5)*DY(I)	TC5550
ENDIF	TC5560
SM=YLOLD/DYM	TC5570
IF(SM.GT.1.)SM=1.	TC5580
DZINT=(1.+DEL)*(SM*DSM+(1.-SM)*DSP)-DEL*	TC5590
1(SM*DFM+(1.-SM)*DFP)	TC5600
DZBOT=(ZBOT(I,J,K)-ZBOT(IM,J,K))/DYM	TC5610
C	TC5620
C NEW PROJECTED DISTANCE AND ADJUSTED TRANSMISSIVITY COEFFICIENTS	TC5630
DEN=DZBOT-DZINT	TC5640
IF(DEN.NE.0.0)THEN	TC5650
YLNEW=THS(I,J,K)/(DZBOT-DZINT)	TC5660
IF(ICODE(I,J,K).EQ.S)YLNEW=((1.+DEL)*PHIS(I,J,K)-	TC5670
1DEL*PHIF(I,J,K))-ZBOT(I,J,K))/(DZBOT-DZINT)	TC5680
ELSE	TC5690
YLNEW=YLOLD	TC5700
ENDIF	TC5710
AYL=WFAC*DABS(YLNEW)+(1.-WFAC)*YLOLD	TC5720
IF(AYL.LE.DY2)THEN	TC5730
SL=(AYL+DY2)/DY(I)	TC5740
IF(SYN(I,J,K).LT.1.)THEN	TC5750
SYN(I,J,K)=SYN(I,J,K)+SL	TC5780
ELSE	TC5790
SYN(I,J,K)=SL	TC5800
ENDIF	TC5810
IF(SYN(I,J,K).GT.1.)SYN(I,J,K)=1.	TC5820
IF (ICODE(I,J,K).NE.S) THEN	TC5830
TTFY(IM,J,K)=TTFY(IM,J,K)+TTSY(IM,J,K)/C1	TC5840
TTSY(IM,J,K)=0.0	TC5850
ENDIF	TC5860
ELSE	TC5870

SL=(AYL-DY2)/DY(IM)	TC5880
IF(SL.GT.1.0)SL=1.0	TC5890
IF(SYN(IM,J,K).LT.1.)THEN	TC5900
SYN(IM,J,K)=SYN(IM,J,K)+SL	TC5910
ELSE	TC5920
SYN(IM,J,K)=SL	TC5930
END IF	TC5940
IF(SYN(IM,J,K).GT.1.)SYN(IM,J,K)=1.0	TC5950
IF(ICODE(I,J,K).NE.S) THEN	TC5960
TTSYN=THS(I,J,K)*(AYL-DY2)/AYL*TFY(IM,J,K)*C1	TC5970
TTFY(IM,J,K)=TTFY(IM,J,K)+(TTSY(IM,J,K)-TTSYN)/C1	TC5980
TTSY(IM,J,K)=TTSYN	TC5990
ENDIF	TC6000
END IF	TC6010
END IF	TC6020
80 CONTINUE	TC6030
C	TC6040
C CALCULATE NEW FRESHWATER AND SALTWATER AREA FACTORS	TC6050
DO 100 I=2,NNR-1	TC6060
DO 100 J=2,NNC-1	TC6070
IF(FKX(I,J,K).EQ.0.)GO TO 100	TC6080
IF(FXN(I,J,K).EQ.0.0.OR.FXN(I,J,K).EQ.1.0)THEN	TC6090
FAREA(I,J,K)=FYN(I,J,K)	TC6100
ELSE IF(FYN(I,J,K).EQ.0.0.OR.FYN(I,J,K).EQ.1.0)THEN	TC6110
FAREA(I,J,K)=FXN(I,J,K)	TC6120
ELSE IF(FXN(I,J,K).LE.0.5)THEN	TC6130
FAREA(I,J,K)=(1.-DSQRT(FXN(I,J,K)*FYN(I,J,K)))*	TC6140
1(FXN(I,J,K)+FYN(I,J,K))	TC6150
ELSE IF(FYN(I,J,K).LE.0.5)THEN	TC6160
FAREA(I,J,K)=(1.-DSQRT(FXN(I,J,K)*FYN(I,J,K)))*	TC6170
1(FXN(I,J,K)+FYN(I,J,K))	TC6180
ELSE	TC6190
FL=FXN(I,J,K)*FYN(I,J,K)	TC6200
FAREA(I,J,K)=FL*(1.+(1./((FL+.75)**7)))	TC6210
IF(FAREA(I,J,K).GT.1.0)FAREA(I,J,K)=1.0	TC6220
ENDIF	TC6230
IF(SXN(I,J,K).EQ.0.0.OR.SXN(I,J,K).EQ.1.0)THEN	TC6240
SAREA(I,J,K)=SYN(I,J,K)	TC6250
ELSE IF(SYN(I,J,K).EQ.0.0.OR.SYN(I,J,K).EQ.1.0)THEN	TC6260
SAREA(I,J,K)=SXN(I,J,K)	TC6270
ELSE IF(SXN(I,J,K).LE.0.5)THEN	TC6280
SAREA(I,J,K)=(1.-DSQRT(SXN(I,J,K)*SYN(I,J,K)))*	TC6290
1(SXN(I,J,K)+SYN(I,J,K))	TC6300
ELSE IF(SYN(I,J,K).LE.0.5)THEN	TC6310
SAREA(I,J,K)=(1.-DSQRT(SXN(I,J,K)*SYN(I,J,K)))*	TC6320
1(SXN(I,J,K)+SYN(I,J,K))	TC6330
ELSE	TC6340
SL=SXN(I,J,K)*SYN(I,J,K)	TC6350
SAREA(I,J,K)=SL*(1.+(1./((SL+.75)**7)))	TC6360
IF(SAREA(I,J,K).GT.1.0)SAREA(I,J,K)=1.0	TC6370
ENDIF	TC6380
FX(I,J,K)=FXN(I,J,K)	TC6390
FY(I,J,K)=FYN(I,J,K)	TC6400
SX(I,J,K)=SXN(I,J,K)	TC6410

	SY(I,J,K)=SYN(I,J,K)	TC6420
100	CONTINUE	TC6430
C		TC6440
40	CONTINUE	TC6450
C		TC6460
170	FORMAT(/' ***WARNING: WATER TABLE ABOVE LAND SURFACE AT BLOCK:',	TC6461
	&I3,2X,I3,2X,I3)	TC6462
C		TC6463
	RETURN	TC6470
	END	TC6480
C		TC6490
C	-----FA 10	
	SUBROUTINE FACTOR(I,J,K,IFLAG,AL,W)	FA 20
C	-----FA 30	
C		FA 40
C	A SUBROUTINE TO CALCULATE COEFFICIENTS OF MATRIX (A) AND FACTOR	FA 50
C	MODIFIED COEFFICIENT MATRIX (A+B)	FA 60
C		FA 70
	IMPLICIT REAL*8 (A-H,O-Z)	FA 80
	PARAMETER (NR=40,NC=47,NL=5)	FA 90
	CHARACTER ICODE,IC,FR,M,S,TITLE*70	FA 100
C		FA 110
	COMMON /INTPAR/ NNR,NNC,NNL,NTSP,NP,NITP,NIT,NPUMP,IPUMP	FA 120
	1,ITMAX,NT,IWT(NR,NC),NFLAG,NCONT,NINT,	FA 130
	2LW,IW(10),JW(10),KW(10),NUP,NLK	FA 140
C		FA 150
	COMMON /CHAR/ICODE(NR,NC,NL),IC(10,200),FR,M,S	FA 160
C		FA 170
	COMMON /VALUES/ DT,TFAC,ERR,RFAC,SGF,SGS,VIF,VIS,DEL,TSEC,TMAX	FA 180
	1,TPUMP,STST,WFAC,WITER	FA 190
C		FA 200
	COMMON /ARRAYS/ FKX(NR,NC,NL),FKY(NR,NC,NL),	FA 210
	1STORF(NR,NC,NL),STORS(NR,NC,NL),POR(NR,NC,NL),THCK(NR,NC,NL),	FA 220
	2ZBOT(NR,NC,NL),ZINT(NR,NC,NL),AQL(NR,NC,NL),RECH(NR,NC),	FA 230
	3PUMP(NR,NC,NL),PHIF(NR,NC,NL),PHIS(NR,NC,NL),DX(NC),DY(NR),	FA 240
	4WOPTF(10),TTFX(NR,NC,NL),TTFY(NR,NC,NL),TTSX(NR,NC,NL),	FA 250
	5TTSY(NR,NC,NL),DMAXF(200),DMAXS(200),THF(NR,NC,NL),	FA 260
	6THS(NR,NC,NL),HEAD(NR,NC),PHIFP(NR,NC,NL),PHISP(NR,NC,NL),	FA 270
	7WTOP(NR,NC,NL),WBOT(NR,NC,NL),PUMPF(NR,NC,NL),FAREA(NR,NC,NL),	FA 280
	8SAREA(NR,NC,NL),ZTOP(NR,NC,NL),BATH(NR,NC),	FA 290
	9FX(NR,NC,NL),FY(NR,NC,NL),SX(NR,NC,NL),SY(NR,NC,NL)	FA 300
C		FA 310
	COMMON /COEF/DPHIF(NR,NC,NL),DPHIS(NR,NC,NL),VF(NR,NC,NL),	FA 320
	1VS(NR,NC,NL),E(NR,NC,NL,4),F(NR,NC,NL,4),G(NR,NC,NL,4),A(4),	FA 330
	2B(4),C(4),D(4),AA(4),BB(4),CC(4),DD(4),EE(4),FF(4),GG(4),HH(4),	FA 340
	3SS(4),TT(4),UU(4),WW(4),ZZ(4),BBB(4),ZZZ(4),ARRAY(4),EK1(4),	FA 350
	4EI1(4),FJ1(4),GJ1(4),FK1(4),GI1(4),EIJK(4),FIJK(4),GIJK(4),	FA 360
	5TFX(NR,NC,NL),TFY(NR,NC,NL),TIME(200),FWH(10,200),SWH(10,200),	FA 370
	6ZI(10,200),FAR(10,200),SAR(10,200)	FA 380
C		FA 390
	COMMON /IUNITS/IUN(32),IOP(12)	FA 400
C		FA 410
	DIMENSION G1(4)	FA 420
C		FA 430

	I1=I-1	FA 440
	I2=I+1	FA 450
	J1=J-1	FA 460
	J2=J+1	FA 470
	K1=K-1	FA 480
	K2=K+1	FA 490
	AREA=DX(J)*DY(I)	FA 500
C		FA 510
	DO 5 L=1,4	FA 520
	BB(L)=0.	FA 530
	DD(L)=0.	FA 540
	EE(L)=0.	FA 550
	FF(L)=0.	FA 560
	HH(L)=0.	FA 570
	SS(L)=0.	FA 580
	ZZ(L)=0.	FA 590
	EK1(L)=0.	FA 600
	FK1(L)=0.	FA 610
	A(L)=0.0	FA 611
	5 CONTINUE	FA 620
C		FA 630
C	ASSIGN VALUES OF TRANSMISSIVITY COEFFICIENTS	FA 640
	BB(1)=TTFY(I1,J,K)*DX(J)	FA 650
	DD(1)=TTFX(I,J1,K)*DY(I)	FA 660
	FF(1)=TTFX(I,J,K)*DY(I)	FA 670
	HH(1)=TTFY(I,J,K)*DX(J)	FA 680
	BB(4)=TTSY(I1,J,K)*DX(J)	FA 690
	DD(4)=TTSX(I,J1,K)*DY(I)	FA 700
	FF(4)=TTSX(I,J,K)*DY(I)	FA 710
	HH(4)=TTSY(I,J,K)*DX(J)	FA 720
C		FA 730
C	CALCULATE LEAKAGE COEFFICIENTS	FA 740
	AQLTOP=(1.-AL)*AQL(I,J,K)*AREA	FA 750
	AQLBOT=0.0	FA 760
	SGR=SGS/SGF	FA 770
	IF(K.NE.1)AQLBOT=AQL(I,J,K1)*AREA	FA 780
	QF2=0.	FA 790
	QS2=0.	FA 800
	QF1=0.	FA 810
	QS1=0.	FA 820
	QFL=0.	FA 830
	QSL=0.	FA 840
	ZZ1=0.	FA 850
	E5=0.	FA 860
	WT1=1.0	FA 870
	WT2=1.0	FA 880
	WT3=0.0	FA 890
	RT3=1.0	FA 900
	WT4=1.0	FA 910
	WT5=1.0	FA 911
	IF(NLK.EQ.0)THEN	FA 920
	WT2=0.0	FA 960
	WT3=FAREA(I,J,K)	FA 970
	WT4=0.0	FA1010

IF(K.NE.NNL)THEN	FA 921
DIRP=(SGR*PHIS(I,J,K+1)-PHIF(I,J,K))-(ZBOT(I,J,K2)+	FA 930
&ZTOP(I,J,K))/(2.*DEL)	FA 940
IF(DIRP.GT.0.0)WT1=0.0	FA 950
IF(ICODE(I,J,K2).EQ.' ')THEN	FA 955
WT1=1.0	FA 960
WT2=1.0	FA 970
IF(PHIF(I,J,K2).GT.PHIF(I,J,K).AND.BATH(I,J).LT.0.)WT5=0.0	FA 980
END IF	FA 990
END IF	FA1000
IF(K.NE.1)THEN	FA1010
DIRM=(PHIF(I,J,K1)-SGR*PHIS(I,J,K))+(ZBOT(I,J,K)+	FA1020
&ZTOP(I,J,K1))/(2.*DEL)	FA1030
IF(DIRM.LT.0.0)RT3=0.0	FA1040
END IF	FA1050
END IF	FA1060
IF(K.EQ.NNL)THEN	FA1070
PHTOPF=(1.-AL)*HEAD(I,J)	FA1080
PHTOPS=(1.-AL)*(HEAD(I,J)/SGR+BATH(I,J)/(DEL+1.))	FA1090
QF1=FAREA(I,J,K)*AQLTOP*PHTOPF	FA1100
QS1=(1.-FAREA(I,J,K))*AQLTOP*PHTOPS	FA1110
QF2=FAREA(I,J,K)*AQLTOP	FA1120
QS2=(1.-FAREA(I,J,K))*AQLTOP	FA1130
IF(NLK.EQ.0)THEN	FA1131
IF(HEAD(I,J).GT.PHIF(I,J,K).AND.BATH(I,J).LT.0.)THEN	FA1132
QF1=0.0	FA1133
QF2=0.0	FA1134
END IF	FA1135
END IF	FA1136
C	FA1140
ELSE	FA1150
IF(FAREA(I,J,K).GE.(1.-SAREA(I,J,K2)))THEN	FA1160
SS(1)=(1.-SAREA(I,J,K2))*AQLTOP	FA1170
SS(2)=(FAREA(I,J,K)-(1.-SAREA(I,J,K2)))*AQLTOP*SGR*WT1	FA1180
SS(4)=(1.-FAREA(I,J,K))*AQLTOP	FA1190
QFL=-SS(2)/SGR*(ZBOT(I,J,K2)+ZTOP(I,J,K))/(2.*DEL)	FA1200
ELSE	FA1210
SS(1)=FAREA(I,J,K)*AQLTOP*WT5	FA1220
SS(3)=((1.-FAREA(I,J,K))-SAREA(I,J,K2))*AQLTOP*	FA1230
&WT2	FA1240
SS(4)=SAREA(I,J,K2)*AQLTOP	FA1250
QSL=SS(3)*(ZBOT(I,J,K2)+ZTOP(I,J,K))/(2.*DEL)	FA1260
END IF	FA1270
END IF	FA1280
IF(K.NE.1)THEN	FA1290
IF(FAREA(I,J,K1).GT.(1.-SAREA(I,J,K)))THEN	FA1300
ZZ1=RT3*WT3*(FAREA(I,J,K1)-(1.-SAREA(I,J,K)))*AQLBOT	FA1310
E5=ZZ1*SGR	FA1320
ZZ(1)=(1.-SAREA(I,J,K))*AQLBOT+ZZ1	FA1330
ZZ(3)=(FAREA(I,J,K1)-(1.-SAREA(I,J,K)))*AQLBOT*(1.-	FA1340
&WT3)*RT3	FA1350
ZZ(4)=(1.-FAREA(I,J,K1))*AQLBOT	FA1360
QFL=QFL+ZZ1*(ZBOT(I,J,K)+ZTOP(I,J,K1))/(2.*DEL)	FA1370
QSL=QSL+ZZ(3)*(ZBOT(I,J,K)+ZTOP(I,J,K1))/(2.*DEL)	FA1380

	ELSE	FA1390
	ZZ(1)=FAREA(I, J, K1)*AQLBOT	FA1400
	ZZ(2)=WT4*((1. - FAREA(I, J, K1)) - SAREA(I, J, K))*AQLBOT*SGR	FA1410
	ZZ(4)=SAREA(I, J, K)*AQLBOT	FA1420
	QFL=QFL-ZZ(2)/SGR*(ZBOT(I, J, K)+ZTOP(I, J, K1))/(2.*DEL)	FA1430
	END IF	FA1440
	END IF	FA1450
C		FA1460
	E2=SS(1)+SS(2)/SGR+(ZZ(1) - ZZ1)+ZZ(2)/SGR+QF2	FA1470
	E4=SS(3)*SGR+SS(4)+ZZ(3)*SGR+ZZ(4)+QS2	FA1480
C		FA1490
C	CALCULATE REMAINING COEFFICIENTS	FA1500
	APROP=FAREA(I, J, K)+SAREA(I, J, K) - 1.	FA1510
	ARDT=AREA/DT	FA1520
	IF(APROP.GT.0.)THEN	FA1530
	E1=(POR(I, J, K)*DEL*APROP+(POR(I, J, K)*AL+STORF(I, J, K)*	FA1540
	1THF(I, J, K))*FAREA(I, J, K))*ARDT	FA1550
	EE(2)=POR(I, J, K)*(1.+DEL)*APROP*ARDT	FA1560
	EE(3)=POR(I, J, K)*DEL*APROP*ARDT	FA1570
	E3=(POR(I, J, K)*(1.+DEL)*APROP+STORS(I, J, K)*THS(I, J, K)*	FA1580
	1SAREA(I, J, K))*ARDT	FA1590
	ELSE	FA1600
	E1=SAREA(I, J, K)+(STORF(I, J, K)*THF(I, J, K)+AL*POR(I, J, K))*	FA1610
	1FAREA(I, J, K))*ARDT	FA1620
	EE(2)=(1. - FAREA(I, J, K))*SGR	FA1630
	EE(3)=(1. - SAREA(I, J, K))/SGR	FA1640
	E3=FAREA(I, J, K)+STORS(I, J, K)*THS(I, J, K)*SAREA(I, J, K)*ARDT	FA1650
	END IF	FA1660
C		FA1670
	EE(1)=- (BB(1)+DD(1)+FF(1)+HH(1)+E1+E2)	FA1680
	EE(2)=EE(2) - E5	FA1690
	EE(4)=- (BB(4)+DD(4)+FF(4)+HH(4)+E3+E4)	FA1700
C		FA1710
C	CALCULATE FRESHWATER AND SALTWATER PUMPAGES	FA1720
	PUMPF(I, J, K)=0.0	FA1720
	PUMPS=0.0	FA1730
	IF(PUMP(I, J, K).EQ.0.) GO TO 10	FA1740
	TOP=WTOP(I, J, K)	FA1750
	BOT=WBOT(I, J, K)	FA1760
	IF(WTOP(I, J, K).GT.ZTOP(I, J, K))TOP=ZTOP(I, J, K)	FA1770
	IF(TOP.LE.WBOT(I, J, K))THEN	FA1780
	PUMP(I, J, K)=0.	FA1790
	WRITE(IUN(24), 20)I, J, K	FA1800
	GO TO 10	FA1810
	END IF	FA1820
C		FA1830
	IF(TOP.GT.ZINT(I, J, K))THEN	FA1840
	IF(BOT.LT.ZINT(I, J, K))THEN	FA1850
	THICF=TOP-ZINT(I, J, K)	FA1860
	THICS=ZINT(I, J, K) - BOT	FA1870
	ELSE	FA1880
	THICF=TOP - BOT	FA1890
	THICS=0.0	FA1900
	END IF	FA1910

ELSE	FA1920
THICF=0.0	FA1930
THICS=TOP-BOT	FA1940
END IF	FA1950
C	FA1960
THICK=THICF+THICS	FA1970
PUMPF(I,J,K)=THICF*PUMP(I,J,K)/THICK	FA1980
PUMPS=PUMP(I,J,K)-PUMPF(I,J,K)	FA1990
C	FA2000
C ZERO COEFFICIENTS AT CONSTANT HEAD NODES	FA2010
10 IF(STORF(I,J,K).LT.0.)THEN	FA2020
BB(1)=0.	FA2030
DD(1)=0.	FA2040
FF(1)=0.	FA2050
HH(1)=0.	FA2060
SS(1)=0.	FA2070
SS(2)=0.	FA2080
ZZ(1)=0.	FA2090
ZZ(2)=0.	FA2100
QF1=0.	FA2110
QF2=0.	FA2120
EE(1)=1.0E+06	FA2130
EE(2)=0.	FA2140
QFL=0.	FA2150
E5=0.	FA2160
END IF	FA2170
IF(STORS(I,J,K).LT.0.)THEN	FA2180
BB(4)=0.	FA2190
DD(4)=0.	FA2200
FF(4)=0.	FA2210
HH(4)=0.	FA2220
SS(3)=0.	FA2230
SS(4)=0.	FA2240
ZZ(3)=0.	FA2250
ZZ(4)=0.	FA2260
QS1=0.	FA2270
QS2=0.	FA2280
EE(3)=0.	FA2290
EE(4)=1.0E+06	FA2300
QSL=0.	FA2310
END IF	FA2320
C	FA2330
C CALCULATE FRESHWATER AND SALTWATER RESIDUALS	FA2340
QF=(EE(2)+E5)*PHISP(I,J,K)-E1*PHIFP(I,J,K)	FA2350
1-AL*RECH(I,J)*AREA+PUMPF(I,J,K)-QF1-QFL	FA2360
Z1=0.	FA2370
S1=0.	FA2380
IF(K.NE.1)Z1=ZZ(1)*PHIF(I,J,K1)+ZZ(2)*PHIS(I,J,K1)	FA2390
IF(K.NE.NNL)S1=SS(1)*PHIF(I,J,K2)+SS(2)*PHIS(I,J,K2)	FA2400
RESF=QF-(BB(1)*PHIF(I1,J,K)+DD(1)*PHIF(I,J1,K)+EE(1)*PHIF(I,J,K)	FA2410
1+EE(2)*PHIS(I,J,K)+FF(1)*PHIF(I,J2,K)+HH(1)*PHIF(I2,J,K)+Z1+S1)	FA2420
IF(FAREA(I,J,K).LE.0.)RESF=0.	FA2430
QS=EE(3)*PHIFP(I,J,K)-E3*PHISP(I,J,K)	FA2440
1+PUMPS-QS1-QSL	FA2450

	Z2=0.	FA2460
	S2=0.	FA2470
	IF(K.NE.1)Z2=ZZ(3)*PHIF(I,J,K1)+ZZ(4)*PHIS(I,J,K1)	FA2480
	IF(K.NE.NNL)S2=SS(3)*PHIF(I,J,K2)+SS(4)*PHIS(I,J,K2)	FA2490
	RESS=QS-(BB(4)*PHIS(I1,J,K)+DD(4)*PHIS(I,J1,K)+EE(3)*PHIF(I,J,K)	FA2500
	1+EE(4)*PHIS(I,J,K)+FF(4)*PHIS(I,J2,K)+HH(4)*PHIS(I2,J,K)+Z2+S2)	FA2510
	IF(SAREA(I,J,K).LE.0.)RESS=0.	FA2520
	IF(STORF(I,J,K).LT.0.)THEN	FA2530
	RESF=0.	FA2540
	END IF	FA2550
	IF(STORS(I,J,K).LT.0.)THEN	FA2560
	RESS=0.	FA2570
	END IF	FA2580
C		FA2590
C	PICK COEFFICIENTS FOR NORMAL OR REVERSE FACTORIZATION	FA2600
	IF(IFLAG.EQ.0)THEN	FA2610
	II=I1	FA2620
	KK=K1	FA2630
	ELSE	FA2640
	DO 7 L=1,4	FA2650
	BBB(L)=HH(L)	FA2660
	HH(L)=BB(L)	FA2670
	BB(L)=BBB(L)	FA2680
	ZZZ(L)=SS(L)	FA2690
	SS(L)=ZZ(L)	FA2700
	ZZ(L)=ZZZ(L)	FA2710
7	CONTINUE	FA2720
	II=I2	FA2730
	KK=K2	FA2740
	ENDIF	FA2750
C		FA2760
C	CALCULATION OF MODIFIED COEFFICIENT MATRIX (A+B) AND FACTORIZATION	FA2770
C		FA2780
	IF(KK.GE.1.AND.KK.LE.NNL)THEN	FA2790
	CALL ASSIGN(E,EK1,I,J,KK)	FA2800
	CALL ASSIGN(F,FK1,I,J,KK)	FA2810
	CALL MAT1(EK1,FK1,ARRAY,W)	FA2820
	CALL INV(ARRAY,ARRAY)	FA2830
	CALL MULT(ZZ,ARRAY,A)	FA2840
	END IF	FA2850
C		FA2860
	CALL ASSIGN(E,EI1,II,J,K)	FA2870
	CALL ASSIGN(G,GI1,II,J,K)	FA2880
	CALL MAT1(EI1,GI1,ARRAY,W)	FA2890
	CALL INV(ARRAY,ARRAY)	FA2900
	CALL MULT(BB,ARRAY,B)	FA2910
C		FA2920
	CALL ASSIGN(F,FJ1,I,J1,K)	FA2930
	CALL ASSIGN(G,GJ1,I,J1,K)	FA2940
	CALL MAT1(FJ1,GJ1,ARRAY,W)	FA2950
	CALL INV(ARRAY,ARRAY)	FA2960
	CALL MULT(DD,ARRAY,C)	FA2970
C		FA2980
	CALL MULT(A,EK1,AA)	FA2990

	CALL MULT(B, EI1, CC)	FA3000
	CALL MULT(C, FJ1, GG)	FA3010
	CALL MULT(C, GJ1, WW)	FA3020
	CALL MULT(A, FK1, TT)	FA3030
	CALL MULT(B, GI1, UU)	FA3040
C	DO 100 L=1,4	FA3050
	D(L)=EE(L)+W*(CC(L)+GG(L)+AA(L)+WW(L)+TT(L)+UU(L))	FA3060
100	CONTINUE	FA3070
C	IF(KK.LT.1.OR.KK.GT.NNL)THEN	FA3080
	DO 150 L=1,4	FA3090
	G1(L)=0.	FA3100
	A(L)=0.	FA3110
150	CONTINUE	FA3120
	VF1=0.	FA3130
	VS1=0.	FA3140
	ELSE	FA3150
	DO 160 L=1,4	FA3160
	G1(L)=G(I,J, KK, L)	FA3170
160	CONTINUE	FA3180
	VF1=VF(I,J, KK)	FA3190
	VS1=VS(I,J, KK)	FA3200
	END IF	FA3210
C		FA3220
	D(1)=D(1) - (A(1)*G1(1)+A(2)*G1(3)+B(1)*F(II, J, K, 1)	FA3230
	1+B(2)*F(II, J, K, 3)+C(1)*E(I, J1, K, 1)+C(2)*E(I, J1, K, 3))	FA3240
	D(2)=D(2) - (A(1)*G1(2)+A(2)*G1(4)+B(1)*F(II, J, K, 2)	FA3250
	1+B(2)*F(II, J, K, 4)+C(1)*E(I, J1, K, 2)+C(2)*E(I, J1, K, 4))	FA3260
	D(3)=D(3) - (A(3)*G1(1)+A(4)*G1(3)+B(3)*F(II, J, K, 1)	FA3270
	1+B(4)*F(II, J, K, 3)+C(3)*E(I, J1, K, 1)+C(4)*E(I, J1, K, 3))	FA3280
	D(4)=D(4) - (A(3)*G1(2)+A(4)*G1(4)+B(3)*F(II, J, K, 2)	FA3290
	1+B(4)*F(II, J, K, 4)+C(3)*E(I, J1, K, 2)+C(4)*E(I, J1, K, 4))	FA3300
C		FA3310
	CALL MAT2(FF, CC, AA, ARRAY, W)	FA3320
	CALL INV(D, D)	FA3330
	CALL MULT(D, ARRAY, EIJK)	FA3340
C		FA3350
	CALL MAT2(HH, GG, TT, ARRAY, W)	FA3360
	CALL MULT(D, ARRAY, FIJK)	FA3370
C		FA3380
	CALL MAT2(SS, WW, UU, ARRAY, W)	FA3390
	CALL MULT(D, ARRAY, GIJK)	FA3400
C		FA3410
	DO 200 L=1,4	FA3420
	E(I, J, K, L)=EIJK(L)	FA3430
	F(I, J, K, L)=FIJK(L)	FA3440
	G(I, J, K, L)=GIJK(L)	FA3450
200	CONTINUE	FA3460
C		FA3470
C	FORWARD SUBSTITUTION FOR VECTOR (V)	FA3480
	XF=RFAC*RESF - (A(1)*VF1+A(2)*VS1+B(1)*VF(II, J, K)	FA3490
	1+B(2)*VS(II, J, K)+C(1)*VF(I, J1, K)+C(2)*VS(I, J1, K))	FA3500
	XS=RFAC*RESS - (A(3)*VF1+A(4)*VS1+B(3)*VF(II, J, K)	FA3510
		FA3520
		FA3530

	1+B(4)*VS(II,J,K)+C(3)*VF(I,J1,K)+C(4)*VS(I,J1,K))	FA3540
C		FA3550
	VF(I,J,K)=D(1)*XF+D(2)*XS	FA3560
	VS(I,J,K)=D(3)*XF+D(4)*XS	FA3570
C		FA3580
	RETURN	FA3590
C		FA3600
20	FORMAT(' WELL(' ,I2,' ' ,I2,' ' ,I2,' ') GOES DRY')	FA3610
	END	FA3620
C		FA3630
C	-----	MU 10
	SUBROUTINE MULT(A,B,C)	MU 20
C	-----	MU 30
C		MU 40
C	A SUBROUTINE TO MULTIPLY MATRIX A TIMES MATRIX B	MU 50
	IMPLICIT REAL*8 (A-H,O-Z)	MU 60
	DIMENSION A(4),B(4),C(4)	MU 70
C		MU 80
	C(1)=A(1)*B(1)+A(2)*B(3)	MU 90
	C(2)=A(1)*B(2)+A(2)*B(4)	MU 100
	C(3)=A(3)*B(1)+A(4)*B(3)	MU 110
	C(4)=A(3)*B(2)+A(4)*B(4)	MU 120
C		MU 130
	RETURN	MU 140
	END	MU 150
C		MU 160
C	-----	IV 10
	SUBROUTINE INV(A,AINV)	IV 20
C	-----	IV 30
C		IV 40
C	A SUBROUTINE TO TAKE THE INVERSE OF MATRIX A	IV 50
	IMPLICIT REAL*8 (A-H,O-Z)	IV 60
	DIMENSION A(4),AINV(4)	IV 70
C		IV 80
	DET=A(1)*A(4)-A(2)*A(3)	IV 90
	A1=A(1)	IV 100
	AINV(1)=A(4)/DET	IV 110
	AINV(2)=-A(2)/DET	IV 120
	AINV(3)=-A(3)/DET	IV 130
	AINV(4)=A1/DET	IV 140
C		IV 150
	RETURN	IV 160
	END	IV 170
C		IV 180
C	-----	MT 10
	SUBROUTINE MAT1(A,B,C,W)	MT 20
C	-----	MT 30
C		MT 40
C	A SUBROUTINE TO DO MATRIX CALCULATIONS USED IN FACTORIZATION	MT 50
	IMPLICIT REAL*8 (A-H,O-Z)	MT 60
	DIMENSION A(4),B(4),C(4)	MT 70
C		MT 80
	DO 10 I=1,4,3	MT 90
	C(I)=1.+W*(A(I)+B(I))	MT 100

10	CONTINUE	MT 110
	DO 20 I=2,3	MT 120
	C(I)=W*(A(I)+B(I))	MT 130
20	CONTINUE	MT 140
C		MT 150
	RETURN	MT 160
	END	MT 170
C		MT 180
C	-----	MR 10
	SUBROUTINE MAT2(A,B,C,D,W)	MR 20
C	-----	MR 30
C		MR 40
C	A SUBROUTINE TO DO MATRIX CALCULATIONS USED IN FACTORIZATION	MR 50
	IMPLICIT REAL*8 (A-H,O-Z)	MR 60
	DIMENSION A(4),B(4),C(4),D(4)	MR 70
C		MR 80
	DO 10 I=1,4	MR 90
	D(I)=A(I)-W*(B(I)+C(I))	MR 100
10	CONTINUE	MR 110
C		MR 120
	RETURN	MR 130
	END	MR 140
C		MR 150
C	-----	AS 10
	SUBROUTINE ASSIGN(A,B,I,J,K)	AS 20
C	-----	AS 30
C		AS 40
C	A SUBROUTINE TO ASSIGN MATRIX ELEMENTS	AS 50
	IMPLICIT REAL*8 (A-H,O-Z)	AS 60
	PARAMETER (NR=40,NC=47,NL=5)	AS 70
	DIMENSION A(NR,NC,NL,4),B(4)	AS 80
C		AS 90
	DO 10 L=1,4	AS 100
	B(L)=A(I,J,K,L)	AS 110
10	CONTINUE	AS 120
C		AS 130
	RETURN	AS 140
	END	AS 150
C		AS 160
C	-----	RE 10
	SUBROUTINE RESULTS	RE 20
C	-----	RE 30
C		RE 40
C	A SUBROUTINE TO WRITE TIME STEP RESULTS TO FILE	RE 50
C		RE 60
	IMPLICIT REAL*8 (A-H,O-Z)	RE 70
	PARAMETER (NR=40,NC=47,NL=5)	RE 80
	CHARACTER ICODE,IC,FR,M,S,TITLE*70	RE 90
C		RE 100
	COMMON /INTPAR/ NNR,NNC,NNL,NTSP,NP,NITP,NIT,NPUMP,IPUMP	RE 110
	1,ITMAX,NT,IWT(NR,NC),NFLAG,NCONT,NINT,	RE 120
	2LW,IW(10),JW(10),KW(10),NUP,NLK	RE 130
C		RE 140
	COMMON /CHAR/ICODE(NR,NC,NL),IC(10,200),FR,M,S	RE 150

C		RE 160
	COMMON /VALUES/ DT,TFAC,ERR,RFAC,SGF,SGS,VIF,VIS,DEL,TSEC,TMAX	RE 170
	1, TPUMP, STST, WFAC, WITER	RE 180
C		RE 190
	COMMON /ARRAYS/ FKX(NR,NC,NL),FKY(NR,NC,NL),	RE 200
	1STORF(NR,NC,NL),STORS(NR,NC,NL),POR(NR,NC,NL),THCK(NR,NC,NL),	RE 210
	2ZBOT(NR,NC,NL),ZINT(NR,NC,NL),AQL(NR,NC,NL),RECH(NR,NC),	RE 220
	3PUMP(NR,NC,NL),PHIF(NR,NC,NL),PHIS(NR,NC,NL),DX(NC),DY(NR),	RE 230
	4WOPTF(10),TTFX(NR,NC,NL),TTFY(NR,NC,NL),TTSX(NR,NC,NL),	RE 240
	5TTSY(NR,NC,NL),DMAXF(200),DMAXS(200),THF(NR,NC,NL),	RE 250
	6THS(NR,NC,NL),HEAD(NR,NC),PHIFP(NR,NC,NL),PHISP(NR,NC,NL),	RE 260
	7WTOP(NR,NC,NL),WBOT(NR,NC,NL),PUMPF(NR,NC,NL),FAREA(NR,NC,NL),	RE 270
	8SAREA(NR,NC,NL),ZTOP(NR,NC,NL),BATH(NR,NC),	RE 280
	9FX(NR,NC,NL),FY(NR,NC,NL),SX(NR,NC,NL),SY(NR,NC,NL)	RE 290
C		RE 300
	COMMON /IUNITS/IUN(32),IOP(12)	RE 310
C		RE 320
	DO 130 K=1,NNL	RE 330
	WRITE(IUN(25),81)K,TSEC	RE 340
81	FORMAT(' 0 1.0 PHIF LAYER',I3,', T (SEC)=' ,G20.10)	RE 350
	DO 110 I=1,NNR	RE 360
110	WRITE(IUN(25),80)(PHIF(I,J,K),J=1,NNC)	RE 370
	DO 115 I=1,NNR	RE 380
	DO 115 J=1,NNC	RE 390
	ZINT(I,J,K)=(1.+DEL)*PHIS(I,J,K)-DEL*PHIF(I,J,K)	RE 400
115	CONTINUE	RE 410
	WRITE(IUN(26),82)K,TSEC	RE 420
82	FORMAT(' 0 1.0 ZINT LAYER',I3,', T (SEC)=' ,G20.10)	RE 430
	DO 120 I=1,NNR	RE 440
120	WRITE(IUN(26),80)(ZINT(I,J,K),J=1,NNC)	RE 450
	WRITE(IUN(27),83)K,TSEC	RE 460
83	FORMAT(' 0 1.0 FX LAYER',I3,', T (SEC)=' ,G20.10)	RE 470
	DO 121 I=1,NNR	RE 480
121	WRITE(IUN(27),80)(FX(I,J,K),J=1,NNC)	RE 490
	WRITE(IUN(28),84)K,TSEC	RE 500
84	FORMAT(' 0 1.0 FY LAYER',I3,', T (SEC)=' ,G20.10)	RE 510
	DO 122 I=1,NNR	RE 520
122	WRITE(IUN(28),80)(FY(I,J,K),J=1,NNC)	RE 530
	WRITE(IUN(29),85)K,TSEC	RE 540
85	FORMAT(' 0 1.0 SX LAYER',I3,', T (SEC)=' ,G20.10)	RE 550
	DO 123 I=1,NNR	RE 560
123	WRITE(IUN(29),80)(SX(I,J,K),J=1,NNC)	RE 570
	WRITE(IUN(30),86)K,TSEC	RE 580
86	FORMAT(' 0 1.0 SY LAYER',I3,', T (SEC)=' ,G20.10)	RE 590
	DO 124 I=1,NNR	RE 600
124	WRITE(IUN(30),80)(SY(I,J,K),J=1,NNC)	RE 610
130	CONTINUE	RE 620
C		RE 630
	80 FORMAT(10F10.3)	RE 640
C		RE 650
	RETURN	RE 660
	END	RE 670
C		RE 680
C	-----	OP 10

	SUBROUTINE OUTPUT(KWRITE)	OP 20
C	-----	OP 30
C		OP 40
C	A SUBROUTINE TO PRINT MODEL SOLUTION	OP 50
	IMPLICIT REAL*8 (A-H,O-Z)	OP 60
	PARAMETER (NR=40,NC=47,NL=5)	OP 70
	CHARACTER ICODE,IC,FR,M,S,TITLE*70	OP 80
C		OP 90
	COMMON /INTPAR/ NNR,NNC,NNL,NTSP,NP,NITP,NIT,NPUMP,IPUMP	OP 100
	1,ITMAX,NT,IWT(NR,NC),NFLAG,NCONT,NINT,	OP 110
	2LW,IW(10),JW(10),KW(10),NUP,NLK	OP 120
C		OP 130
	COMMON /CHAR/ICODE(NR,NC,NL),IC(10,200),FR,M,S	OP 140
C		OP 150
	COMMON /VALUES/ DT,TFAC,ERR,RFAC,SGF,SGS,VIF,VIS,DEL,TSEC,TMAX	OP 160
	1,TPUMP,STST,WFAC,WITER	OP 170
C		OP 180
	COMMON /ARRAYS/ FKX(NR,NC,NL),FKY(NR,NC,NL),	OP 190
	1STORF(NR,NC,NL),STORS(NR,NC,NL),POR(NR,NC,NL),THCK(NR,NC,NL),	OP 200
	2ZBOT(NR,NC,NL),ZINT(NR,NC,NL),AQL(NR,NC,NL),RECH(NR,NC),	OP 210
	3PUMP(NR,NC,NL),PHIF(NR,NC,NL),PHIS(NR,NC,NL),DX(NC),DY(NR),	OP 220
	4WOPTF(10),TTFX(NR,NC,NL),TTFY(NR,NC,NL),TTSX(NR,NC,NL),	OP 230
	5TTSY(NR,NC,NL),DMAXF(200),DMAXS(200),THF(NR,NC,NL),	OP 240
	6THS(NR,NC,NL),HEAD(NR,NC),PHIFP(NR,NC,NL),PHISP(NR,NC,NL),	OP 250
	7WTOP(NR,NC,NL),WBOT(NR,NC,NL),PUMPF(NR,NC,NL),FAREA(NR,NC,NL),	OP 260
	8SAREA(NR,NC,NL),ZTOP(NR,NC,NL),BATH(NR,NC),	OP 270
	9FX(NR,NC,NL),FY(NR,NC,NL),SX(NR,NC,NL),SY(NR,NC,NL)	OP 280
C		OP 290
	COMMON /BAL/FIN(7),FOUT(5),SSIN(7),SOUT(5),CFIN(7,NL),CFOUT(5,NL),	OP 300
	1CSIN(7,NL),CSOUT(5,NL),FFLOW(200),SFLOW(200),IFL(200),	OP 310
	2JFL(200),ISL(200),JSL(200),FLKT(NR,NC),SLKT(NR,NC),FLKB(NR,NC),	OP 320
	3SLKB(NR,NC),FSTOR,CFSTR(10),SSTOR,CSSTR(10),TFIN,TCFIN,TSSIN,	OP 330
	4TCSIN,TFOUT,TCFOUT,TSOUT,TCSOUT,FSLKT(NR,NC),SFLKT(NR,NC),	OP 340
	5FDIF,CFDIF,SDIF,CSDIF,FRERF,FRERS,FSLKB(NR,NC),SFLKB(NR,NC),	OP 350
	6CFRERF,CFRERS,SRERF,SRERS,CSRERF,CSRERS	OP 360
C		OP 370
	COMMON /WHEAD/WPHIF(NR,NC),WPHIS(NR,NC),WZINT(NR,NC)	OP 380
C		OP 390
	COMMON /IUNITS/IUN(32),IOP(12)	OP 400
C		OP 410
C	PRINT TIME STEP INFORMATION	OP 420
	DTHRS=DT/3600.	OP 430
	DTDAYS=DTHRS/24.	OP 440
	DTYRS=DTDAYS/365.25	OP 450
	THRS=TSEC/3600.	OP 460
	TDAYS=THRS/24.	OP 470
	TYRS=TDAYS/365.25	OP 480
	PHRS=TPUMP/3600.	OP 490
	PDAYS=PHRS/24.	OP 500
	PYRS=PDAYS/365.25	OP 510
	WRITE(IUN(24),10)NT,DT,DTHRS,DTDAYS,DTYRS,TPUMP,PHRS,PDAYS,PYRS,	OP 520
	1TSEC,THRS,TDAYS,TYRS	OP 530
	WRITE(IUN(24),20)(DMAXF(I),I=1,NIT)	OP 540
	WRITE(IUN(24),30)(DMAXS(I),I=1,NIT)	OP 550

C		OP 560
C	IF NUMBER OF TIME STEPS BETWEEN PRINTOUTS HAS NOT BEEN REACHED,	OP 570
C	CALCULATE MASS BALANCE AND BEGIN NEW TIME STEP	OP 580
	IF(KWRITE.EQ.0)THEN	OP 590
	DO 199 K=1,NNL	OP 600
199	CALL MASBAL(NFH,NSH,K)	OP 610
	RETURN	OP 620
	ELSE	OP 630
	DO 200 K=1,NNL	OP 640
	WRITE(IUN(24),40)K	OP 650
	WRITE(IUN(32),45)NT,K	OP 660
C		OP 670
C	CALCULATE AND PRINT AQUIFER MASS BALANCE	OP 680
	CALL MASBAL(NFH,NSH,K)	OP 690
	WRITE(IUN(24),50)	OP 700
	WRITE(IUN(24),60)FSTOR,CFSTR(K),SSTOR,CSSTR(K),FIN(1),CFIN(1,K)	OP 710
	WRITE(IUN(24),70)(FIN(I),CFIN(I,K),SSIN(I),CSIN(I,K),I=2,7),	OP 720
	1TFIN,TCFIN,TSSIN,TCSIN	OP 730
	WRITE(IUN(24),80)(FOUT(I),CFOUT(I,K),SOUT(I),CSOUT(I,K),I=2,5),	OP 740
	1TFOUT,TCFOUT,TSOUT,TCSOUT,FDIF,CFDIF,SDIF,GSDIF,FRERF,	OP 750
	1CFRERF,SRERF,CSRERF,FRERS,CFRERS,SRERS,CSRERS	OP 760
C		OP 770
	DO 99 I=1,NNR	OP 780
	DO 99 J=1,NNC	OP 790
	WPHIF(I,J)=PHIF(I,J,K)	OP 800
	WPHIS(I,J)=PHIS(I,J,K)	OP 810
	WZINT(I,J)=11111.	OP 820
	IF(FAREA(I,J,K).LE.0.)THEN	OP 830
	WPHIF(I,J)=11111.	OP 840
	END IF	OP 850
	IF(SAREA(I,J,K).LE.0.)THEN	OP 860
	WPHIS(I,J)=11111.	OP 870
	END IF	OP 880
	IF(ICODE(I,J,K).EQ.M)THEN	OP 890
	WZINT(I,J)=ZINT(I,J,K)	OP 900
	END IF	OP 910
99	CONTINUE	OP 920
C		OP 930
C	PRINT AQUIFER POTENTIALS AND INTERFACE ELEVATIONS	OP 940
	IF(IOP(4).EQ.1)THEN	OP 950
	WRITE(IUN(24),90)	OP 960
	DO 100 I=1,NNR	OP 970
100	WRITE(IUN(24),110)I,(WPHIF(I,J),J=1,NNC)	OP 980
	END IF	OP 990
	IF(IOP(5).EQ.1)THEN	OP1000
	WRITE(IUN(24),120)	OP1010
	DO 130 I=1,NNR	OP1020
130	WRITE(IUN(24),110)I,(WPHIS(I,J),J=1,NNC)	OP1030
	END IF	OP1040
	IF(IOP(6).EQ.1)THEN	OP1050
	WRITE(IUN(24),140)	OP1060
	DO 150 I=1,NNR	OP1070
150	WRITE(IUN(24),110)I,(WZINT(I,J),J=1,NNC)	OP1080
	END IF	OP1090

	IF(IOP(7).EQ.1)THEN	OP1100
	WRITE(IUN(24),220)	OP1110
	DO 190 I=1,NNR	OP1120
190	WRITE(IUN(24),240)I,(FAREA(I,J,K),J=1,NNC)	OP1130
	END IF	OP1140
	IF(IOP(8).EQ.1)THEN	OP1150
	WRITE(IUN(24),230)	OP1160
	DO 195 I=1,NNR	OP1170
195	WRITE(IUN(24),240)I,(SAREA(I,J,K),J=1,NNC)	OP1180
	END IF	OP1190
	IF(IOP(9).EQ.1)THEN	OP1200
	WRITE(IUN(24),160)	OP1210
	DO 170 I=1,NNR	OP1220
170	WRITE(IUN(24),180)I,(ICODE(I,J,K),J=1,NNC)	OP1230
	END IF	OP1240
	IF(IOP(10).EQ.1)THEN	OP1250
	WRITE(IUN(32),250)	OP1260
	DO 196 I=1,NNR	OP1270
196	WRITE(IUN(32),111)I,(FLKT(I,J),J=1,NNC)	OP1280
	WRITE(IUN(32),251)	OP1281
	DO 296 I=1,NNR	OP1282
296	WRITE(IUN(32),111)I,(FSLKT(I,J),J=1,NNC)	OP1283
	WRITE(IUN(32),260)	OP1290
	DO 197 I=1,NNR	OP1300
197	WRITE(IUN(32),111)I,(FLKB(I,J),J=1,NNC)	OP1310
	WRITE(IUN(32),261)	OP1311
	DO 297 I=1,NNR	OP1312
297	WRITE(IUN(32),111)I,(FSLKB(I,J),J=1,NNC)	OP1313
	WRITE(IUN(32),270)	OP1320
	DO 198 I=1,NNR	OP1330
198	WRITE(IUN(32),111)I,(SLKT(I,J),J=1,NNC)	OP1340
	WRITE(IUN(32),271)	OP1341
	DO 298 I=1,NNR	OP1342
298	WRITE(IUN(32),111)I,(SFLKT(I,J),J=1,NNC)	OP1343
	WRITE(IUN(32),280)	OP1350
	DO 201 I=1,NNR	OP1360
201	WRITE(IUN(32),111)I,(SLKB(I,J),J=1,NNC)	OP1370
	WRITE(IUN(32),281)	OP1371
	DO 301 I=1,NNR	OP1372
301	WRITE(IUN(32),111)I,(SFLKB(I,J),J=1,NNC)	OP1373
	IF(NFH.GT.0)THEN	OP1380
	WRITE(IUN(32),290)	OP1390
	WRITE(IUN(32),300)(IFL(I),JFL(I),FFLOW(I),I=1,NFH)	OP1400
	END IF	OP1410
	IF(NSH.GT.0)THEN	OP1420
	WRITE(IUN(32),310)	OP1430
	WRITE(IUN(32),300)(ISL(I),JSL(I),SFLOW(I),I=1,NSH)	OP1440
	END IF	OP1450
	END IF	OP1460
200	CONTINUE	OP1470
	END IF	OP1480
C		OP1490
	RETURN	OP1500
C		OP1510

10	FORMAT(/1X,13('-'))/' TIME STEP',I4/1X,13('-')/26X,'SECONDS',	OP1520
	16X,'HOURS',6X,'DAYS',7X,'YEARS'/' TIME STEP SIZE:',7X,4G11.4/	OP1530
	2' PUMPING PERIOD TIME:',2X,4G11.4/' TOTAL SIMULATION TIME:',	OP1540
	44G11.4/)	OP1550
20	FORMAT(' MAXIMUM FRESHWATER HEAD CHANGE:',8G11.4	OP1560
	1/(33X,8G11.4))	OP1570
30	FORMAT(' MAXIMUM SALTWATER HEAD CHANGE: ',8G11.4	OP1580
	1/(33X,8G11.4))	OP1590
40	FORMAT(/' AQUIFER',I4/1X,11('-'))	OP1600
45	FORMAT(/1X,13('-'))/' TIME STEP',I4/1X,13('-')//	OP1610
	1' AQUIFER',I4/1X,11('-'))	OP1620
50	FORMAT(5X,' MASS BALANCE:')	OP1630
60	FORMAT(38X,' FRESHWATER ',21X,' SALTWATER'	OP1640
	1/35X,' T.S. RATE',3X,' CUM. VOLUME',9X,' T.S. RATE',3X,	OP1650
	2' CUM. VOLUME'/9X,' CHANGE IN STORAGE',7X,2G11.4,10X,2G11.4/	OP1660
	410X,' SOURCES'/10X,' RECHARGE',14X,2G11.4)	OP1670
70	FORMAT(10X,' CONSTANT HEAD NODES',3X,2G11.4,10X,2G11.4/	OP1680
	110X,' INJECTION',13X,2G11.4,10X,2G11.4/	OP1690
	210X,' FW LEAKAGE ACROSS TOP',1X,2G11.4,10X,2G11.4/	OP1700
	210X,' FW LEAKAGE ACROSS BOT',1X,2G11.4,10X,2G11.4/	OP1710
	210X,' SW LEAKAGE ACROSS TOP',1X,2G11.4,10X,2G11.4/	OP1711
	210X,' SW LEAKAGE ACROSS BOT',1X,2G11.4,10X,2G11.4/	OP1712
	310X,' TOTAL',17X,2G11.4,10X,2G11.4)	OP1720
80	FORMAT(10X,' SINKS'/10X,' CONSTANT HEAD NODES',3X,2G11.4,10X,	OP1730
	12G11.4/10X,' PUMPAGE',15X,2G11.4,10X,2G11.4/10X,' LEAKAGE',	OP1740
	1' ACROSS TOP',4X,2G11.4,10X,2G11.4/10X,' LEAKAGE ACROSS BOT',	OP1750
	24X,2G11.4,10X,2G11.4/10X,' TOTAL',17X,2G11.4,10X,2G11.4/	OP1760
	310X,' SOURCES-SINKS',11X,2G11.4,10X,2G11.4/	OP1770
	48X,' RELATIVE ERROR (%)'/	OP1780
	512X,' INFLUX',16X,2G11.4,10X,2G11.4/12X,' STORAGE',15X,2G11.4,10X,	OP1790
	62G11.4)	OP1800
90	FORMAT(/' FRESHWATER HEADS')	OP1810
110	FORMAT(I5,3X,10G11.5/(8X,10G11.5))	OP1820
111	FORMAT(I5,3X,10G11.4/(8X,10G11.4))	OP1830
120	FORMAT(/' SALTWATER HEADS')	OP1840
140	FORMAT(/' INTERFACE ELEVATION')	OP1850
160	FORMAT(/' MAP OF EXTENT OF INTRUSION',	OP1860
	1' (F-FRESHWATER, M-FRESH AND SALTWATER, S-SALTWATER)'/)	OP1870
180	FORMAT(I5,3X,50A2/(8X,50A2))	OP1880
210	FORMAT(/' CUMULATIVE MASS BALANCE:')	OP1890
220	FORMAT(/' FAREA'/)	OP1900
230	FORMAT(/' SAREA'/)	OP1910
240	FORMAT(I5,3X,30F4.1/(8X,30F4.1))	OP1920
250	FORMAT(/' FRESHWATER LEAKAGE ACROSS TOP FROM OVERLYING ',	OP1930
	&' FRESHWATER')	OP1931
251	FORMAT(/' FRESHWATER LEAKAGE ACROSS TOP FROM OVERLYING SALTWATER')	OP1932
260	FORMAT(/' FRESHWATER LEAKAGE ACROSS BOT FROM UNDERLYING ',	OP1940
	&' FRESHWATER')	OP1941
261	FORMAT(/' FRESHWATER LEAKAGE ACROSS BOT FROM UNDERLYING ',	OP1942
	&' SALTWATER')	OP1943
270	FORMAT(/' SALTWATER LEAKAGE ACROSS TOP FROM OVERLYING SALTWATER')	OP1950
271	FORMAT(/' SALTWATER LEAKAGE ACROSS TOP FROM OVERLYING FRESHWATER')	OP1951
280	FORMAT(/' SALTWATER LEAKAGE ACROSS BOT FROM UNDERLYING ',	OP1960
	&' SALTWATER')	OP1961

281	FORMAT('/ SALTWATER LEAKAGE ACROSS BOT FROM UNDERLYING ',	OP1962
	&' FRESHWATER')	OP1963
290	FORMAT('/ FLOW RATES TO FRESHWATER CONSTANT HEAD NODES: '/	OP1970
	19X, 'I', 4X, 'J', 7X, 'RATE', 3(14X, 'I', 4X, 'J', 7X, 'RATE'))	OP1980
300	FORMAT(4(5X, 2I5, 3X, G11.4))	OP1990
310	FORMAT('/ FLOW RATES TO SALTWATER CONSTANT HEAD NODES: '/	OP2000
	19X, 'I', 4X, 'J', 5X, 'RATE', 3(14X, 'I', 4X, 'J', 5X, 'RATE'))	OP2010
	END	OP2020
C		OP2030
C	-----	MB 10
	SUBROUTINE MASBAL(NFH, NSH, K)	MB 20
C	-----	MB 30
C		MB 40
C	A SUBROUTINE TO CALCULATE AQUIFER AND CUMULATIVE MASS BALANCES	MB 50
C		MB 60
	IMPLICIT REAL*8 (A-H, O-Z)	MB 70
	PARAMETER (NR=40, NC=47, NL=5)	MB 80
	CHARACTER ICODE, IC, FR, M, S, TITLE*70	MB 90
C		MB 100
	COMMON /INTPAR/ NNR, NNC, NNL, NTSP, NP, NITP, NIT, NPUMP, IPUMP	MB 110
	1, ITMAX, NT, IWT(NR, NC), NFLAG, NCONT, NINT,	MB 120
	2LW, IW(10), JW(10), KW(10), NUP, NLK	MB 130
C		MB 140
	COMMON /CHAR/ICODE(NR, NC, NL), IC(10, 200), FR, M, S	MB 150
C		MB 160
	COMMON /VALUES/ DT, TFAC, ERR, RFAC, SGF, SGS, VIF, VIS, DEL, TSEC, TMAX	MB 170
	1, TPUMP, STST, WFAC, WITER	MB 180
C		MB 190
	COMMON /ARRAYS/ FKX(NR, NC, NL), FKY(NR, NC, NL),	MB 200
	1STORF(NR, NC, NL), STORS(NR, NC, NL), POR(NR, NC, NL), THCK(NR, NC, NL),	MB 210
	2ZBOT(NR, NC, NL), ZINT(NR, NC, NL), AQL(NR, NC, NL), RECH(NR, NC),	MB 220
	3PUMP(NR, NC, NL), PHIF(NR, NC, NL), PHIS(NR, NC, NL), DX(NC), DY(NR),	MB 230
	4WOPTF(10), TTFX(NR, NC, NL), TTFY(NR, NC, NL), TTSX(NR, NC, NL),	MB 240
	5TTSY(NR, NC, NL), DMAXF(200), DMAXS(200), THF(NR, NC, NL),	MB 250
	6THS(NR, NC, NL), HEAD(NR, NC), PHIFP(NR, NC, NL), PHISP(NR, NC, NL),	MB 260
	7WTOP(NR, NC, NL), WBOT(NR, NC, NL), PUMPF(NR, NC, NL), FAREA(NR, NC, NL),	MB 270
	8SAREA(NR, NC, NL), ZTOP(NR, NC, NL), BATH(NR, NC),	MB 280
	9FX(NR, NC, NL), FY(NR, NC, NL), SX(NR, NC, NL), SY(NR, NC, NL)	MB 290
C		MB 300
	COMMON /BAL/FIN(7), FOUT(5), SSIN(7), SOUT(5), CFIN(7, NL), CFOUT(5, NL),	MB 310
	1CSIN(7, NL), CSOUT(5, NL), FFLOW(200), SFLOW(200), IFL(200),	MB 320
	2JFL(200), ISL(200), JSL(200), FLKT(NR, NC), SLKT(NR, NC), FLKB(NR, NC),	MB 330
	3SLKB(NR, NC), FSTOR, CFSTR(10), SSTOR, CSSTR(10), TFIN, TCFIN, TSSIN,	MB 340
	4TCSIN, TFOUT, TCFOUT, TSOUT, TCSOUT, FSLKT(NR, NC), SFLKT(NR, NC),	MB 350
	5FDIF, CFDIF, SDIF, CSDIF, FRERF, FRERS, FSLKB(NR, NC), SFLKB(NR, NC),	MB 360
	6CFRERF, CFRERS, SRERF, SRERS, CSRERF, CSRERS	MB 370
C		MB 371
	COMMON /IUNITS/IUN(32), IOP(12)	MB 372
C		MB 380
C	INITIALIZE VARIABLES	MB 390
	IF(NFLAG.EQ.1) THEN	MB 400
	DO 11 L=1, NNL	MB 410
	CFSTR(L)=0.0	MB 420
	CSSTR(L)=0.0	MB 430

	DO 10 I=1,5	MB 440
	CFIN(I,L)=0.	MB 450
	CFOUT(I,L)=0.	MB 460
	CSIN(I,L)=0.	MB 470
	CSOUT(I,L)=0.	MB 480
10	CONTINUE	MB 490
	CFIN(6,L)=0.0	MB 491
	CFIN(7,L)=0.0	MB 492
	CSIN(6,L)=0.0	MB 493
	CSIN(7,L)=0.0	MB 494
11	CONTINUE	MB 495
	END IF	MB 500
	DO 20 I=1,5	MB 510
	FIN(I)=0.	MB 520
	FOUT(I)=0.	MB 530
	SSIN(I)=0.	MB 540
	SOUT(I)=0.	MB 550
20	CONTINUE	MB 560
	FIN(6)=0.	MB 561
	FIN(7)=0.	MB 562
	SSIN(6)=0.	MB 563
	SSIN(7)=0.	MB 564
	NFLAG=0	MB 570
	FSTOR=0.0	MB 580
	SSTOR=0.0	MB 590
	FRERF=0.0	MB 600
	SRERF=0.0	MB 610
	FRERS=0.0	MB 620
	SRERS=0.0	MB 630
	CFRERF=0.0	MB 640
	CSRERF=0.0	MB 650
	CFRERS=0.0	MB 660
	CSRERS=0.0	MB 670
	NFH=0	MB 680
	NSH=0	MB 690
C		MB 700
	DO 199 I=1,NNR	MB 710
	DO 199 J=1,NNC	MB 720
	FLKT(I,J)=0.	MB 730
	FSLKT(I,J)=0.	MB 731
	FLKB(I,J)=0.	MB 740
	FSLKB(I,J)=0.	MB 741
	SLKT(I,J)=0.	MB 750
	SFLKT(I,J)=0.	MB 751
	SLKB(I,J)=0.	MB 760
	SFLKB(I,J)=0.	MB 761
199	CONTINUE	MB 770
C	BEGIN CALCULATIONS	MB 780
	DO 200 I=2,NNR-1	MB 790
	DO 200 J=2,NNC-1	MB 800
	IF (FKX(I,J,K).EQ.0.) GO TO 200	MB 810
	AREA=DX(J)*DY(I)	MB 820
	AL=0.0	MB 830
	IF(K.EQ.NNL.AND.IWT(I,J).EQ.1)AL=1.0	MB 840

C		MB 850
C	COMPUTE FRESHWATER AND SALTWATER RATE CHANGES FOR TIME STEP	MB 860
C	CONSTANT FRESHWATER HEAD NODES	MB 870
	IF(STORF(I,J,K).GE.0.) GO TO 50	MB 880
	NFH=NFH+1	MB 890
	IFL(NFH)=I	MB 900
	JFL(NFH)=J	MB 910
	FFLOW(NFH)=0.0	MB 920
	IF(STORF(I,J-1,K).GE.0..AND.FAREA(I,J-1,K).GT.0.) FFLOW(NFH)=	MB 930
	1FFLOW(NFH)+(PHIF(I,J,K)-PHIF(I,J-1,K))*TTFX(I,J-1,K)*DY(I)	MB 940
	IF(STORF(I,J+1,K).GE.0..AND.FAREA(I,J+1,K).GT.0.) FFLOW(NFH)=	MB 950
	1FFLOW(NFH)+(PHIF(I,J,K)-PHIF(I,J+1,K))*TTFX(I,J,K)*DY(I)	MB 960
	IF(STORF(I-1,J,K).GE.0..AND.FAREA(I-1,J,K).GT.0.) FFLOW(NFH)=	MB 970
	1FFLOW(NFH)+(PHIF(I,J,K)-PHIF(I-1,J,K))*TTFY(I-1,J,K)*DX(J)	MB 980
	IF(STORF(I+1,J,K).GE.0..AND.FAREA(I+1,J,K).GT.0.) FFLOW(NFH)=	MB 990
	1FFLOW(NFH)+(PHIF(I,J,K)-PHIF(I+1,J,K))*TTFY(I,J,K)*DX(J)	MB1000
	IF(FFLOW(NFH).GE.0.) THEN	MB1010
	FIN(2)=FIN(2)+FFLOW(NFH)	MB1020
	ELSE	MB1030
	FOUT(2)=FOUT(2)-FFLOW(NFH)	MB1040
	END IF	MB1050
C		MB1060
C	CONSTANT SALTWATER HEAD NODES	MB1070
	50 IF(STORS(I,J,K).GE.0.) GO TO 80	MB1080
	NSH=NSH+1	MB1090
	ISL(NSH)=I	MB1100
	JSL(NSH)=J	MB1110
	SFLOW(NSH)=0.0	MB1120
	IF(STORS(I,J-1,K).GE.0..AND.SAREA(I,J-1,K).GT.0.) SFLOW(NSH)=	MB1130
	1SFLOW(NSH)+(PHIS(I,J,K)-PHIS(I,J-1,K))*TTSX(I,J-1,K)*DY(I)	MB1140
	IF(STORS(I,J+1,K).GE.0..AND.SAREA(I,J+1,K).GT.0.) SFLOW(NSH)=	MB1150
	1SFLOW(NSH)+(PHIS(I,J,K)-PHIS(I,J+1,K))*TTSX(I,J,K)*DY(I)	MB1160
	IF(STORS(I-1,J,K).GE.0..AND.SAREA(I-1,J,K).GT.0.) SFLOW(NSH)=	MB1170
	1SFLOW(NSH)+(PHIS(I,J,K)-PHIS(I-1,J,K))*TTSY(I-1,J,K)*DX(J)	MB1180
	IF(STORS(I+1,J,K).GE.0..AND.SAREA(I+1,J,K).GT.0.) SFLOW(NSH)=	MB1190
	1SFLOW(NSH)+(PHIS(I,J,K)-PHIS(I+1,J,K))*TTSY(I,J,K)*DX(J)	MB1200
	IF(SFLOW(NSH).GE.0.) THEN	MB1210
	SSIN(2)=SSIN(2)+SFLOW(NSH)	MB1220
	ELSE	MB1230
	SOUT(2)=SOUT(2)-SFLOW(NSH)	MB1240
	END IF	MB1250
C		MB1260
C	RECHARGE	MB1270
	80 FIN(1)=FIN(1)+AL*RECH(I,J)*AREA	MB1280
C		MB1290
C	PUMPAGE	MB1300
	IF(PUMP(I,J,K).EQ.0.) GO TO 140	MB1310
	PUMPS=PUMP(I,J,K)-PUMPF(I,J,K)	MB1320
	IF(PUMPF(I,J,K).LE.0.) THEN	MB1330
	FIN(3)=FIN(3)-PUMPF(I,J,K)	MB1340
	ELSE	MB1350
	FOUT(3)=FOUT(3)+PUMPF(I,J,K)	MB1360
	END IF	MB1370
	IF(PUMPS.LE.0.) THEN	MB1380

SSIN(3)=SSIN(3) - PUMPS	MB1390
ELSE	MB1400
SOUT(3)=SOUT(3)+PUMPS	MB1410
END IF	MB1420
C	MB1430
C STORAGE	MB1440
140 APROP=FAREA(I,J,K)+SAREA(I,J,K)-1.	MB1450
IF(APROP.GT.0.)THEN	MB1460
FST=AREA*((POR(I,J,K)*DEL*APROP+(POR(I,J,K)*AL	MB1470
1+STORF(I,J,K)*THF(I,J,K))*FAREA(I,J,K))*(PHIF(I,J,K)	MB1480
2-PHIFP(I,J,K))-POR(I,J,K)*(1.+DEL)*APROP*(PHIS(I,J,K)	MB1490
3-PHISP(I,J,K)))	MB1500
SST=AREA*((POR(I,J,K)*(1.+DEL)*APROP+STORS(I,J,K)	MB1510
1*THS(I,J,K)*SAREA(I,J,K))*(PHIS(I,J,K)-PHISP(I,J,K))	MB1520
2-POR(I,J,K)*DEL*APROP*(PHIF(I,J,K)-PHIFP(I,J,K)))	MB1530
ELSE	MB1540
FST=AREA*(STORF(I,J,K)*THF(I,J,K)+AL*POR(I,J,K))	MB1550
1*FAREA(I,J,K)*(PHIF(I,J,K)-PHIFP(I,J,K))	MB1560
SST=AREA*STORS(I,J,K)*THS(I,J,K)*SAREA(I,J,K)	MB1570
1*(PHIS(I,J,K)-PHISP(I,J,K))	MB1580
END IF	MB1590
C	MB1600
IF(STORF(I,J,K).LT.0.)FST=0.	MB1610
IF(STORS(I,J,K).LT.0.)SST=0.	MB1620
FSTOR=FSTOR+FST/DT	MB1630
SSTOR=SSTOR+SST/DT	MB1640
CFSTR(K)=CFSTR(K)+FST	MB1650
CSSTR(K)=CSSTR(K)+SST	MB1660
C	MB1670
C LEAKAGE	MB1680
K1=K-1	MB1690
K2=K+1	MB1700
C	MB1710
C ACROSS TOP	MB1720
AQLTOP=(1.-AL)*AQL(I,J,K)*AREA	MB1730
AQLBOT=0.0	MB1740
SGR=SGS/SGF	MB1750
IF(K.NE.1)AQLBOT=AQL(I,J,K1)*AREA	MB1760
WT1=1.0	MB1770
WT2=1.0	MB1780
WT3=0.0	MB1790
RT3=1.0	MB1800
WT4=1.0	MB1810
WT5=1.0	MB1811
IF(NLK.EQ.0)THEN	MB1820
WT2=0.0	MB1860
WT3=FAREA(I,J,K)	MB1870
WT4=0.0	MB1910
IF(K.NE.NNL)THEN	MB1821
DIRP=(SGR*PHIS(I,J,K+1)-PHIF(I,J,K))-(ZBOT(I,J,K2)+	MB1830
&ZTOP(I,J,K))/(2.*DEL)	MB1840
IF(DIRP.GT.0.0)WT1=0.0	MB1850
IF(ICODE(I,J,K2).EQ.' ')THEN	MB1860
WT1=1.0	MB1870

WT2=1.0	MB1880
IF(PHIF(I,J,K2).GT.PHIF(I,J,K).AND.BATH(I,J).LT.0.)WT5=0.0	MB1890
END IF	MB1900
END IF	MB1910
IF(K.NE.1)THEN	MB1920
DIRM=(PHIF(I,J,K1)-SGR*PHIS(I,J,K))+(ZBOT(I,J,K)+	MB1930
&ZTOP(I,J,K1))/(2.*DEL)	MB1940
IF(DIRM.LT.0.0)RT3=0.0	MB1950
END IF	MB1960
END IF	MB1965
IF(K.EQ.NNL)THEN	MB1970
FLKT(I,J)=(1.-AL)*FAREA(I,J,K)*AQLTOP*(HEAD(I,J)-PHIF(I,J,K))	MB1980
IF(BATH(I,J).LT.0.0)THEN	MB1981
IF(FLKT(I,J).GT.0.)THEN	MB1982
IF(NLK.EQ.0)THEN	MB1983
FLKT(I,J)=0.D0	MB1984
ELSE	MB1985
FSLKT(I,J)=FLKT(I,J)	MB1986
FLKT(I,J)=0.D0	MB1987
END IF	MB1988
END IF	MB1989
END IF	MB1990
SLKT(I,J)=(1.-AL)*(1.-FAREA(I,J,K))*AQLTOP*(HEAD(I,J)/SGR+	MB1991
1BATH(I,J)/(DEL+1.)-PHIS(I,J,K))	MB2000
ELSE	MB2010
IF(FAREA(I,J,K).GE.(1.-SAREA(I,J,K2)))THEN	MB2020
FLKT(I,J)=(1.-SAREA(I,J,K2))*AQLTOP*(PHIF(I,J,K2)-	MB2030
1PHIF(I,J,K))	MB2031
FSLKT(I,J)=WT1*(FAREA(I,J,K)-(1.-SAREA(I,J,K2)))*AQLTOP*(MB2032
2(SGR*PHIS(I,J,K+1)-PHIF(I,J,K))-(ZBOT(I,J,K2)+ZTOP(I,J,K))/	MB2033
3(2.*DEL))	MB2040
IF(FSLKT(I,J).LT.0.0)THEN	MB2041
FLKT(I,J)=FLKT(I,J)+FSLKT(I,J)	MB2042
FSLKT(I,J)=0.0	MB2043
END IF	MB2044
SLKT(I,J)=(1.-FAREA(I,J,K))*AQLTOP*(PHIS(I,J,K2)-	MB2050
1PHIS(I,J,K))	MB2060
ELSE	MB2070
FLKT(I,J)=FAREA(I,J,K)*AQLTOP*(PHIF(I,J,K2)-PHIF(I,J,K))*WT5	MB2080
SLKT(I,J)=SAREA(I,J,K2)*AQLTOP*(PHIS(I,J,K2)-PHIS(I,J,K))	MB2090
SFLKT(I,J)=WT2*((1.-FAREA(I,J,K))-	MB2100
&SAREA(I,J,K2))*AQLTOP*	MB2120
1(PHIF(I,J,K2)-SGR*PHIS(I,J,K)+(ZBOT(I,J,K2)+ZTOP(I,J,K)))/	MB2130
2(2.*DEL))	MB2140
IF(ICODE(I,J,K2).EQ.' ' .AND.BATH(I,J).LT.0.0)THEN	MB2150
IF(FLKT(I,J).GT.0.0)THEN	MB2151
FSLKT(I,J)=FLKT(I,J)	MB2152
FLKT(I,J)=0.0	MB2153
END IF	MB2154
SLKT(I,J)=SLKT(I,J)+SFLKT(I,J)	MB2155
SFLKT(I,J)=0.0	MB2156
END IF	MB2157
END IF	MB2160
END IF	MB2170

C		MB2180
C	ACROSS BOTTOM	MB2190
	IF(K.NE.1)THEN	MB2200
	IF(FAREA(I,J,K1).GT.(1.-SAREA(I,J,K)))THEN	MB2210
	FLKB(I,J)=(1.-SAREA(I,J,K))*AQLBOT*(PHIF(I,J,K1)-	MB2220
	1PHIF(I,J,K))+RT3*WT3*(FAREA(I,J,K1)-(1.-SAREA(I,J,K)))*	MB2230
	1AQLBOT*(PHIF(I,J,K1)-SGR*PHIS(I,J,K)+(ZBOT(I,J,K)+ZTOP(I,J,K1))/	MB2240
	2(2.*DEL))	MB2250
	SLKB(I,J)=(1.-FAREA(I,J,K1))*AQLBOT*(PHIS(I,J,K1)-	MB2251
	&PHIS(I,J,K))	MB2252
	SFLKB(I,J)=RT3*(1.-WT3)*(FAREA(I,J,K1)-	MB2260
	&(1.-SAREA(I,J,K)))*	MB2270
	1AQLBOT*(PHIF(I,J,K1)-SGR*PHIS(I,J,K)+(ZBOT(I,J,K)+ZTOP(I,J,K1))/	MB2280
	2(2.*DEL))	MB2290
	ELSE	MB2300
	FLKB(I,J)=FAREA(I,J,K1)*AQLBOT*(PHIF(I,J,K1)-	MB2310
	1PHIF(I,J,K))	MB2311
	FSLKB(I,J)=WT4*((1.-FAREA(I,J,K1))-SAREA(I,J,K))*AQLBOT*	MB2320
	2(SGR*PHIS(I,J,K1)-PHIF(I,J,K)-(ZBOT(I,J,K)+ZTOP(I,J,K1)))/(2.*DEL))	MB2330
	SLKB(I,J)=SAREA(I,J,K)*AQLBOT*(PHIS(I,J,K1)-PHIS(I,J,K))	MB2340
	END IF	MB2350
	END IF	MB2360
C		MB2370
	IF(STORF(I,J,K).LT.0.)THEN	MB2380
	FFLOW(NFH)=FFLOW(NFH)-FLKT(I,J)-FLKB(I,J)-FSLKT(I,J)	MB2390
	&-FSLKB(I,J)	MB2410
	IF(FLKT(I,J).LE.0.)THEN	MB2420
	FIN(2)=FIN(2)-FLKT(I,J)	MB2430
	ELSE	MB2440
	FOUT(2)=FOUT(2)+FLKT(I,J)	MB2450
	END IF	MB2460
	IF(FLKB(I,J).LE.0.)THEN	MB2470
	FIN(2)=FIN(2)-FLKB(I,J)	MB2480
	ELSE	MB2490
	FOUT(2)=FOUT(2)+FLKB(I,J)	MB2500
	END IF	MB2510
	IF(FSLKT(I,J).LE.0.)THEN	MB2420
	FIN(2)=FIN(2)-FSLKT(I,J)	MB2430
	ELSE	MB2440
	FOUT(2)=FOUT(2)+FSLKT(I,J)	MB2450
	END IF	MB2460
	IF(FSLKB(I,J).LE.0.)THEN	MB2470
	FIN(2)=FIN(2)-FSLKB(I,J)	MB2480
	ELSE	MB2490
	FOUT(2)=FOUT(2)+FSLKB(I,J)	MB2500
	END IF	MB2510
	END IF	MB2520
	IF(STORS(I,J,K).LT.0.)THEN	MB2530
	SFLOW(NSH)=SFLOW(NSH)-SLKT(I,J)-SLKB(I,J)-SFLKT(I,J)	MB2540
	&-SFLKB(I,J)	MB2560
	IF(SLKT(I,J).LE.0.)THEN	MB2570
	SSIN(2)=SSIN(2)-SLKT(I,J)	MB2580
	ELSE	MB2590
	SOUT(2)=SOUT(2)+SLKT(I,J)	MB2600

END IF	MB2610
IF(SLKB(I,J).LE.0.)THEN	MB2620
SSIN(2)=SSIN(2)-SLKB(I,J)	MB2630
ELSE	MB2640
SOUT(2)=SOUT(2)+SLKB(I,J)	MB2650
END IF	MB2660
IF(SFLKT(I,J).LE.0.)THEN	MB2570
SSIN(2)=SSIN(2)-SFLKT(I,J)	MB2580
ELSE	MB2590
SOUT(2)=SOUT(2)+SFLKT(I,J)	MB2600
END IF	MB2610
IF(SFLKB(I,J).LE.0.)THEN	MB2620
SSIN(2)=SSIN(2)-SFLKB(I,J)	MB2630
ELSE	MB2640
SOUT(2)=SOUT(2)+SFLKB(I,J)	MB2650
END IF	MB2660
END IF	MB2670
C	MB2680
C SUM VALUES	MB2690
IF(FLKT(I,J).GE.0.)THEN	MB2700
FIN(4)=FIN(4)+FLKT(I,J)	MB2710
ELSE	MB2720
FOUT(4)=FOUT(4)-FLKT(I,J)	MB2730
END IF	MB2740
IF(FSLKT(I,J).GE.0.)THEN	MB2741
FIN(6)=FIN(6)+FSLKT(I,J)	MB2742
ELSE	MB2743
FOUT(4)=FOUT(4)-FSLKT(I,J)	MB2744
END IF	MB2745
IF(FLKB(I,J).GE.0.)THEN	MB2750
FIN(5)=FIN(5)+FLKB(I,J)	MB2760
ELSE	MB2770
FOUT(5)=FOUT(5)-FLKB(I,J)	MB2780
END IF	MB2790
IF(FSLKB(I,J).GE.0.)THEN	MB2791
FIN(7)=FIN(7)+FSLKB(I,J)	MB2792
ELSE	MB2793
FOUT(5)=FOUT(5)-FSLKB(I,J)	MB2794
END IF	MB2795
IF(SLKT(I,J).GE.0.)THEN	MB2800
SSIN(6)=SSIN(6)+SLKT(I,J)	MB2810
ELSE	MB2820
SOUT(4)=SOUT(4)-SLKT(I,J)	MB2830
END IF	MB2840
IF(SFLKT(I,J).GE.0.)THEN	MB2841
SSIN(4)=SSIN(4)+SFLKT(I,J)	MB2842
ELSE	MB2843
SOUT(4)=SOUT(4)-SFLKT(I,J)	MB2845
END IF	MB2846
IF(SLKB(I,J).GE.0.)THEN	MB2850
SSIN(7)=SSIN(7)+SLKB(I,J)	MB2860
ELSE	MB2870
SOUT(5)=SOUT(5)-SLKB(I,J)	MB2880
END IF	MB2890

IF(SFLKB(I,J).GE.0.)THEN	MB2891
SSIN(5)=SSIN(5)+SFLKB(I,J)	MB2892
ELSE	MB2893
SOUT(5)=SOUT(5)-SFLKB(I,J)	MB2894
END IF	MB2895
200 CONTINUE	MB2900
C	MB2910
C COMPUTE CUMULATIVE FRESHWATER AND SALTWATER VOLUME CHANGES	MB2920
DO 210 I=1,5	MB2930
CFIN(I,K)=CFIN(I,K)+FIN(I)*DT	MB2940
CFOUT(I,K)=CFOUT(I,K)+FOUT(I)*DT	MB2950
CSIN(I,K)=CSIN(I,K)+SSIN(I)*DT	MB2960
CSOUT(I,K)=CSOUT(I,K)+SOUT(I)*DT	MB2970
210 CONTINUE	MB2980
CFIN(6,K)=CFIN(6,K)+FIN(6)*DT	MB2981
CFIN(7,K)=CFIN(7,K)+FIN(7)*DT	MB2982
CSIN(6,K)=CSIN(6,K)+SSIN(6)*DT	MB2983
CSIN(7,K)=CSIN(7,K)+SSIN(7)*DT	MB2984
C	MB2990
C COMPUTE TOTALS FOR EACH AQUIFER	MB3000
TFIN=FIN(1)+FIN(2)+FIN(3)+FIN(4)+FIN(5)+FIN(6)+FIN(7)	MB3010
TCFIN=CFIN(1,K)+CFIN(2,K)+CFIN(3,K)+CFIN(4,K)+CFIN(5,K)+	MB3011
&CFIN(6,K)+CFIN(7,K)	MB3020
TSSIN=SSIN(1)+SSIN(2)+SSIN(3)+SSIN(4)+SSIN(5)+SSIN(6)+SSIN(7)	MB3030
TCSIN=CSIN(1,K)+CSIN(2,K)+CSIN(3,K)+CSIN(4,K)+CSIN(5,K)+	MB3031
&CSIN(6,K)+CSIN(7,K)	MB3040
TFOUT=FOUT(2)+FOUT(3)+FOUT(4)+FOUT(5)	MB3050
TCFOUT=CFOUT(2,K)+CFOUT(3,K)+CFOUT(4,K)+CFOUT(5,K)	MB3060
TSOUT=SOUT(2)+SOUT(3)+SOUT(4)+SOUT(5)	MB3070
TCSOUT=CSOUT(2,K)+CSOUT(3,K)+CSOUT(4,K)+CSOUT(5,K)	MB3080
C	MB3090
C COMPUTE AQUIFER MASS BALANCE ERROR	MB3100
FDIF=TFIN-TFOUT	MB3110
SDIF=TSSIN-TSOUT	MB3120
CFDIF=TCFIN-TCFOUT	MB3130
CSDIF=TCSIN-TCSOUT	MB3140
FERR=FSTOR-FDIF	MB3150
SERR=SSTOR-SDIF	MB3160
CFERR=CFSTR(K)-CFDIF	MB3170
CSERR=CSSTR(K)-CSDIF	MB3180
IF(TFIN.NE.0.)FRERF=FERR/TFIN*100.	MB3190
IF(TSSIN.NE.0.)SRERF=SERR/TSSIN*100.	MB3200
IF(FSTOR.NE.0.)FRERS=FERR/FSTOR*100.	MB3210
IF(SSTOR.NE.0.)SRERS=SERR/SSTOR*100.	MB3220
IF(TCFIN.NE.0.)CFRERF=CFERR/TCFIN*100.	MB3230
IF(TCSIN.NE.0.)CSRERF=CSERR/TCSIN*100.	MB3240
IF(CFSTR(K).NE.0.)CFRERS=CFERR/CFSTR(K)*100.	MB3250
IF(CSSTR(K).NE.0.)CSRERS=CSERR/CSSTR(K)*100.	MB3260
C	MB3270
RETURN	MB3280
END	MB3290