

WATER-SURFACE PROFILE AND FLOOD BOUNDARIES FOR THE COMPUTED 100-YEAR FLOOD, MUDDY CREEK, NORTHERN CHEYENNE INDIAN RESERVATION, MONTANA

By
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INTRODUCTION

Areas that would be inundated by a peak discharge having a recurrence interval of 100 years (the 100-year flood) along streams in the Northern Cheyenne Indian Reservation are of interest to the Northern Cheyenne Tribe because of the potential for development of the land. Knowledge of the extent of the flood plain also is needed to control flood damage in the Northern Cheyenne Reservation. An area of concern is the flood plain of Muddy Creek (fig. 1).

One approach for decreasing flood damage is controlling land use adjacent to the stream by planned development and management of flood-hazard areas. Delineation of flood-hazard areas will allow selection of the type of desired development that is compatible with the flood risk.

The U.S. Geological Survey, in cooperation with the Northern Cheyenne Tribe, conducted a hydrologic and hydraulic analysis of Muddy Creek to identify areas along the creek subject to flooding. The specific objective of the study was to determine the extent of flooding that would result from a 100-year flood. This report presents the results of the study.

The magnitude of the 100-year flood was determined using techniques described in Omang (1992), and by Parrett and others (1987). Forty-three channel and flood-plain cross sections were surveyed and 14 cross sections were synthesized along a 6.7-mi reach. Physical dimensions of the hydraulic structures were measured. Manning's roughness coefficients were determined at each cross section. Field survey data and a computer model were used to calculate water-surface elevations for the 100-year flood at each cross section. These elevations were used to determine the inundated area for the 100-year flood.

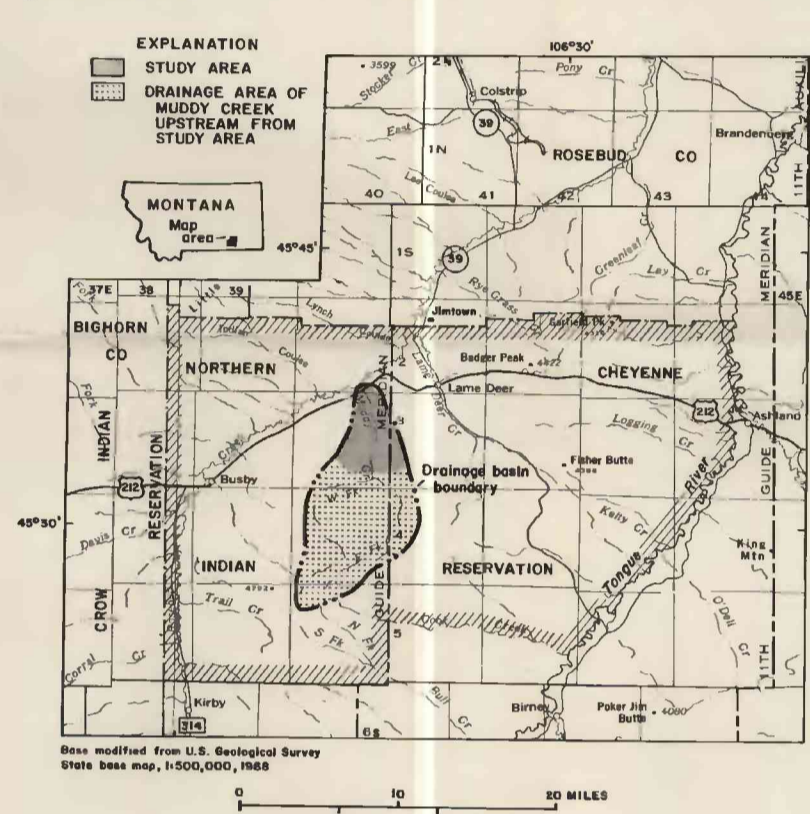


Figure 1.--Location of study area.

General Description of the Area

From its origin about 7 mi southwest of the town of Lane Deer, Muddy Creek flows north from the confluence of the East Fork and West Fork of Muddy Creek. It joins Rosebud Creek, 5 mi west of Lane Deer. The study area includes Muddy Creek from the confluence of the East and West Forks downstream to near the mouth. The Muddy Creek Basin is sparsely populated and consists of gently rolling hills and narrow, steep valleys. Areas of the valley adjacent to the stream are densely vegetated with brush and trees. Elevations of the land surface range from about 3,240 to 4,770 ft in the study area.

The climate is semiarid with cold winters and warm summers. Mean daily temperatures in the area range from 90 °F in July to 8 °F in January. Average annual precipitation is about 15 in., with about half occurring from April through July. June is the wettest month, with an average of about 2.9 in. of precipitation; December, January, and February are the driest, with an average of 0.6 in. (U.S. Environmental Data Service, 1971, p. 10).

Streamflow Conditions and Flooding

Muddy Creek has perennial flow and all tributaries that enter the stream are intermittent or ephemeral. Most runoff results from snowmelt in the spring and from rainfall from thunderstorms during the summer. Occasionally, snowmelt and rain combine to produce runoff.

Muddy Creek had large discharges in May 1978 and in the 1940's, according to local residents. The magnitudes of the discharges are not known because the stream was not gaged during that period. A streamflow-gaging station is currently (1993) being operated on Muddy Creek by the Northern Cheyenne Tribe, but no large flow has occurred since stream gaging began.

METHODS OF ANALYSIS

Standard hydrologic and hydraulic methods were used to analyze the flood hazard for Muddy Creek. The magnitude of a flood that is expected to be equaled or exceeded once on the average during any 100-year period (recurrence interval) was selected by the Northern Cheyenne Tribe for analysis. The 100-year flood has a 1-percent chance of being equaled or exceeded in any given year. Although the recurrence interval represents the long-term average period between floods of a specific magnitude, rare floods could occur at shorter intervals or even within the same year. The analyses reported herein reflect flooding potentials based on conditions in the basin in 1991.

Hydrologic Analysis

Flood-discharge values for Muddy Creek are based on techniques developed to estimate flood-frequency information using basin characteristics and channel geometry. The 100-year flood discharge was determined using techniques described by Omang (1992), which relate the 100-year flood discharge to basin characteristics. This discharge was computed to be 1,730 ft³/s. The 100-year flood discharge also was determined by using techniques described by Parrett and others (1987), which relate the 100-year flood to channel width. This discharge was computed to be 1,480 ft³/s. The two 100-year flood discharges were weighted using methods described by Parrett and others (1987, p. 25), and the resultant 100-year flood was computed to be 1,630 ft³/s. This discharge was used to determine the water-surface elevation at each cross section.

Hydraulic Analysis

The hydraulic characteristics of the cross section along Muddy Creek were analyzed to determine the water-surface elevations of the 100-year flood. The method used to define hydraulic characteristics requires cross-section geometry data and estimates of the roughness coefficient (Manning's "n").

Cross-section data were obtained from field surveys conducted during the summer of 1991. Forty-three cross sections were surveyed and 14 were synthesized. The synthesized cross sections (sections 13, 18, 20, 22, 24, 29, 30, 32, 34, 37, 42, 43, and 56 on the principal map) were estimated from adjacent surveyed sections and topographic maps. Structural geometry data also were obtained for one bridge and one culvert. Cross sections were located upstream and downstream from the bridge and culvert to permit computation of the backwater effects of the bridge and culvert. Cross sections typical of channel and flood-plain conditions in the upstream and downstream parts of the study area are shown in figures 2 and 3, respectively. Figure 4 shows the channel condition at the bridge.

The roughness coefficient represents the resistance to flow. Factors that affect the roughness coefficient include the type and size of materials that compose the bed and banks of the channel and flood plain, shape of the channel, variation in dimensions of adjacent cross sections, vegetation, structures, and degree of meandering. Roughness coefficients (Manning's "n") used in the hydraulic computations were based on engineering judgment of onsite observations. Roughness values used range from 0.030 to 0.105 for the main channel and from 0.045 to 0.085 for the flood-plain areas.

Water-surface elevations for the 100-year flood were computed using a water-surface-profile computation model (WSPRO) developed by the U.S. Geological Survey for the Federal Highway Administration (Shearman and others, 1986; Shearman, 1990). WSPRO is a computer program that is used to analyze one-dimensional, gradually varied, steady flow in open channels with fixed boundaries. With this computer program, the surveyed and synthesized cross-section data were used to define the hydraulic characteristics of the channel. The location of each cross section was selected to represent the hydraulic characteristics of that part of the reach, and each section was surveyed to define its shape. The model uses the standard step method (Chow, 1959, p. 265) to determine changes in water-surface elevation from a downstream cross section with a known water-surface elevation to an upstream cross section by balancing the total energy head at the sections. To compute the 100-year flood profile for Muddy Creek, the starting water-surface elevation at cross section 1 at the downstream end of the study area was determined from a slope-conveyance computation of normal depth. The starting water-surface elevation at cross section 27 upstream from the culvert was determined using techniques developed by Bodhaine (1968) and Hulsing (1967).

The water-surface profile for the 100-year flood (fig. 5) was drawn for the entire reach within the study area. The profile shows the computed water-surface elevations, the streambed elevations, and the location of the bridge, culvert, and cross sections used in the hydraulic analysis.

The hydraulic analysis was based on unobstructed flow. The water-surface elevations shown on the profile thus are considered to be valid only if the hydraulic structures remain unobstructed and do not fail.

For the WSPRO assumption of gradually varied, steady flow to be valid, the water-surface elevations (subreach) needs to be short. As described by Davidson (1984, p. 20), no cross-section subreach should be longer than about 75 to 100 times the mean depth for the modeled discharge nor longer than about twice the width of the subreach flood plain. The number of surveyed cross sections for this study was limited by surveying costs to 43. Therefore, 14 additional cross sections were synthesized and added to the WSPRO input data set to decrease possible step-backwater computation errors. If the synthesized cross-section data are replaced with surveyed data, the computed water-surface elevations at cross sections could change.

Sea-level elevation was transferred from either U.S. Geological Survey or U.S. Coast and Geodetic Survey bench marks to cross sections and to reference marks established at convenient locations along Muddy Creek. Reference-mark locations are shown on the principal map and reference-mark descriptions are given in table 1.

FLOOD BOUNDARIES

The flood boundaries along the stream define an area that would be inundated by a 100-year flood. For this study, the 100-year flood boundaries were delineated using water-surface elevations computed at each cross section and cross sections, where survey data were unavailable, the flood boundaries were interpolated using the contour lines on topographic maps.

The 100-year flood boundaries are shown on the principal map. Small flood-plain areas within the flood boundaries may lie above the water-surface elevation, but cannot be shown owing to limitations of the map scale or lack of detailed topographic data.

Standard hydrologic and hydraulic methods were used to determine the flood-hazard area for Muddy Creek. The 100-year flood was selected as having special significance for flood-plain management.

The magnitude of the 100-year flood was determined for the reach of Muddy Creek extending from the confluence of the East and West Forks downstream to near the mouth. The 100-year flood discharge was determined to be 1,630 ft³/s.

Geometry and roughness coefficients used for 43 channel and flood-plain cross sections were obtained from field surveys of a 6.7-mi reach of the river. Fourteen additional cross sections were synthesized from adjacent surveyed sections and topographic maps. These data were used to compute the water-surface elevation for the 100-year flood at each cross section using WSPRO, a computer program.

The water-surface profile was drawn showing computed water-surface elevations of a 100-year flood. The profile also shows the streambed elevations and the location of the bridge, culvert, and cross sections used in the hydraulic analysis. Flood boundaries were delineated using the water-surface elevations computed at each cross section. Between cross sections, the flood boundaries were interpolated using the contour lines on topographic maps.

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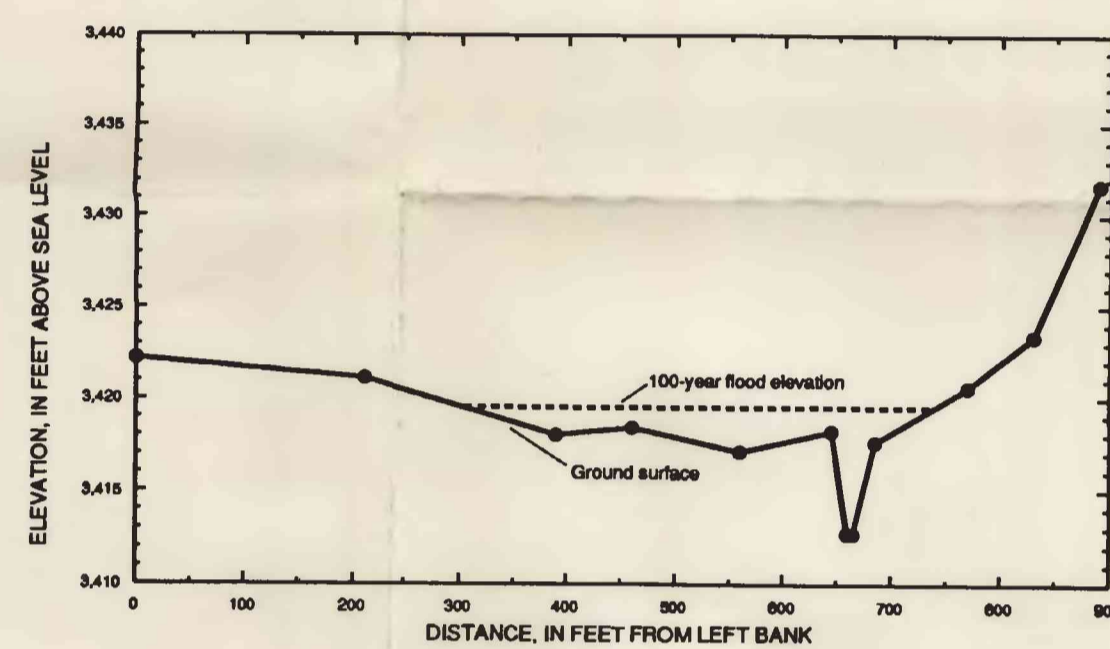


Figure 2.--Cross section 49, which is typical of channel and flood-plain conditions in the upstream part of the study area.

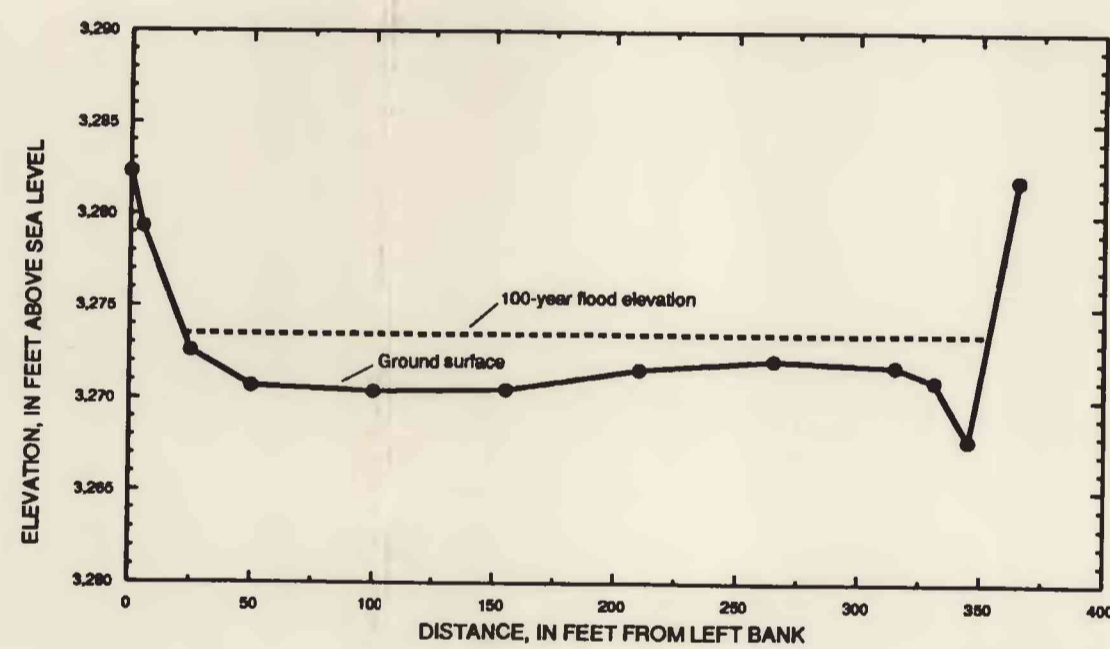


Figure 3.--Cross section 12, which is typical of channel and flood-plain conditions in the downstream part of the study area.

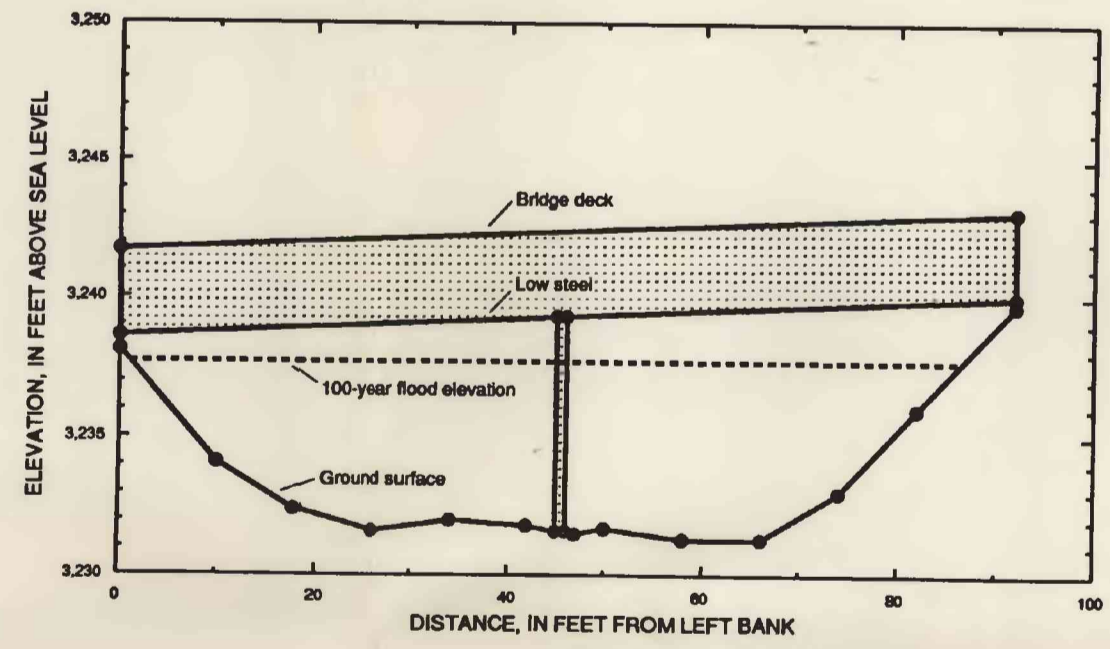
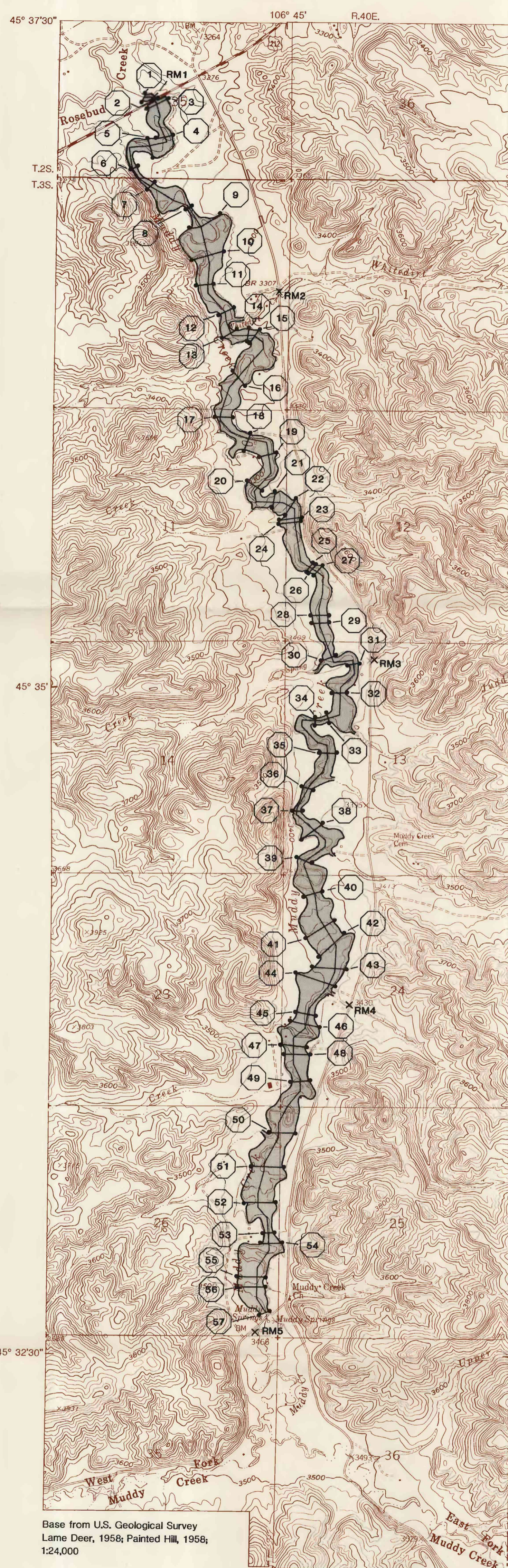
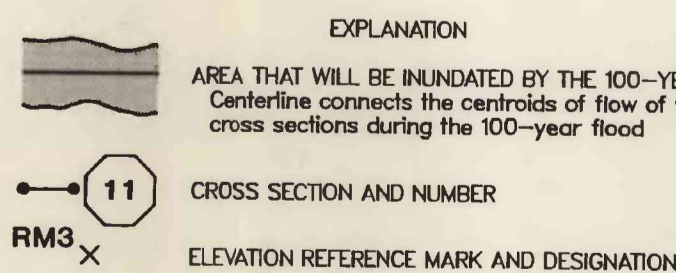


Figure 4.--Cross section 2, which shows the channel condition at the U.S. Highway 212 bridge crossing.



MAP SHOWING AREA INUNDATED BY THE 100-YEAR FLOOD, LOCATION OF CROSS SECTIONS, AND LOCATION OF ELEVATION REFERENCE MARKS

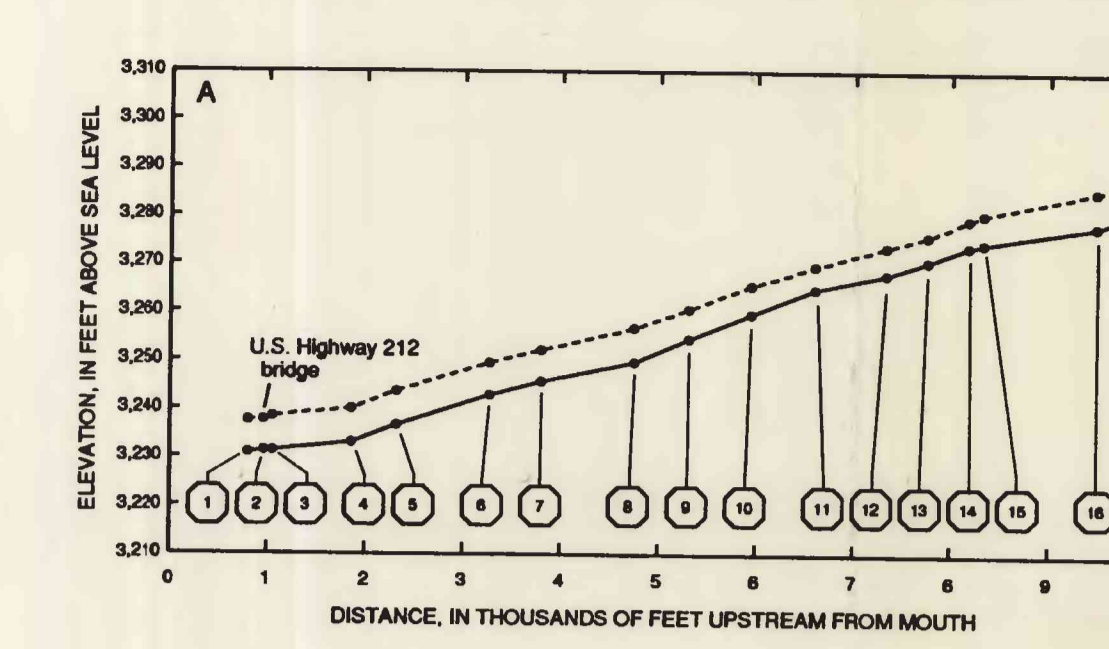


Table 1.--Elevation reference marks along Muddy Creek

Reference mark	Elevation (feet above sea level)	Description of location
RM1	3,244.84	Top of steel bolt, painted red and located in the east end of bridge crossing Muddy Creek, in the guardrail of the north side of U.S. Highway 212 bridge, is located 5.3 mi west of Lane Deer and 1,000 ft upstream from mouth of Muddy Creek.
RM2	3,308.97	Downstream crown of 8-ft corrugated metal culvert is located at crossing along Whiteshirt Creek, 1.1 mi southeast of U.S. Highway 212, at culvert along Muddy Creek Road, 1.3 mi upstream from mouth of Muddy Creek.
RM3	3,380.11	A standard U.S. Geological Survey aluminum disk, stamped "19 T30 1957-Reset 1973" and set in concrete post projecting 1 ft above ground, is located 2.8 mi southeast of U.S. Highway 212, along Muddy Creek Road, 65 ft northeast and 12 ft higher than road, 3.0 mi upstream from mouth of Muddy Creek.
RM4	3,427.72	Downstream crown of 8-ft corrugated metal culvert is located at crossing along unnamed tributary, 4.3 mi southeast of U.S. Highway 212, at culvert along Muddy Creek Road, 4.5 mi upstream from mouth of Muddy Creek.
RM5	3,466.06	A standard U.S. Geological Survey aluminum disk, stamped "20 T30 1957" and set in concrete post projecting 0.6 ft above ground, is located 5.8 mi south of U.S. Highway 212, along Muddy Creek Road, 400 ft southwest of Old Muddy Creek Road bridge.

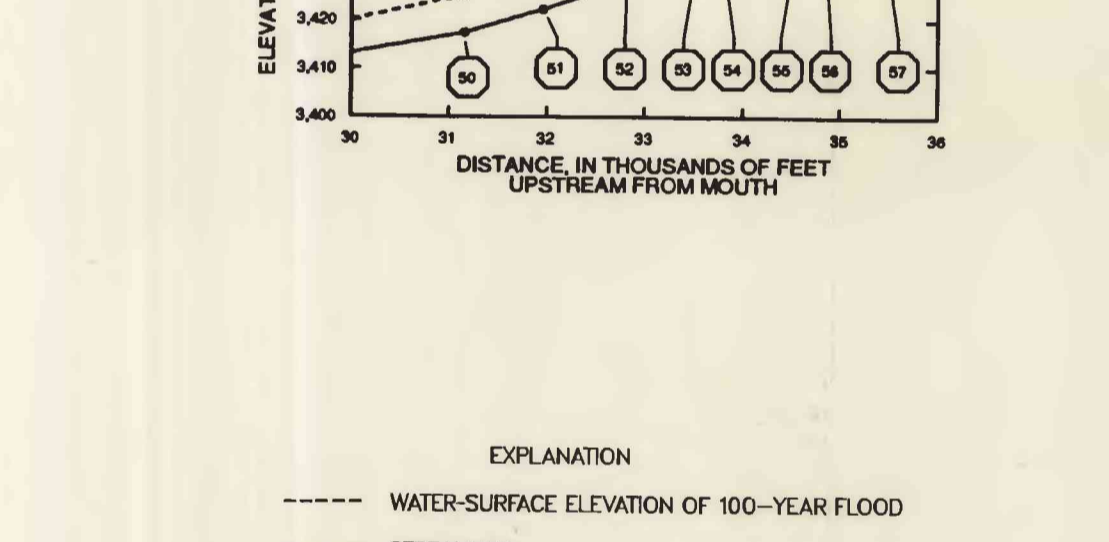
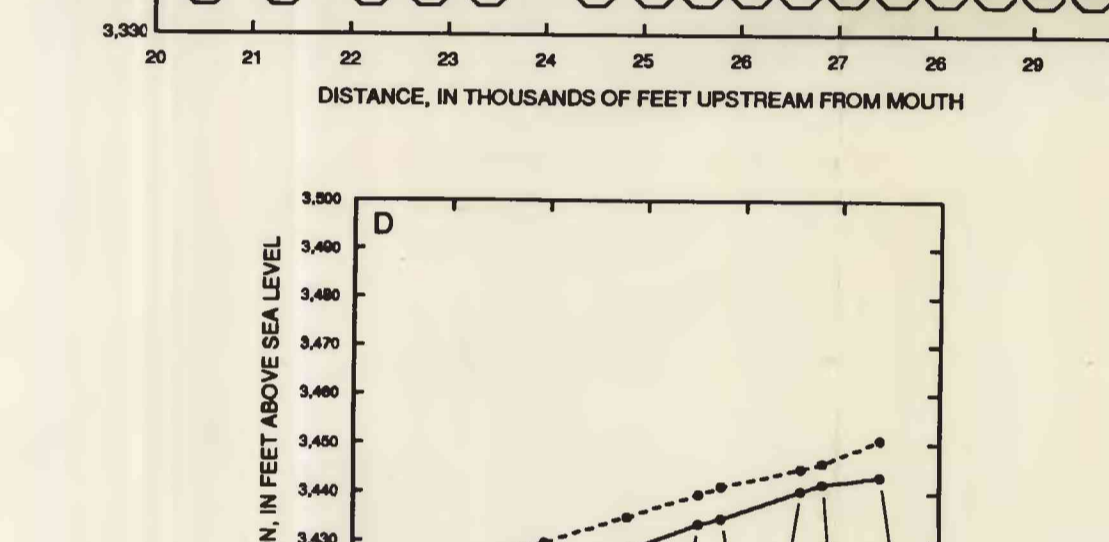
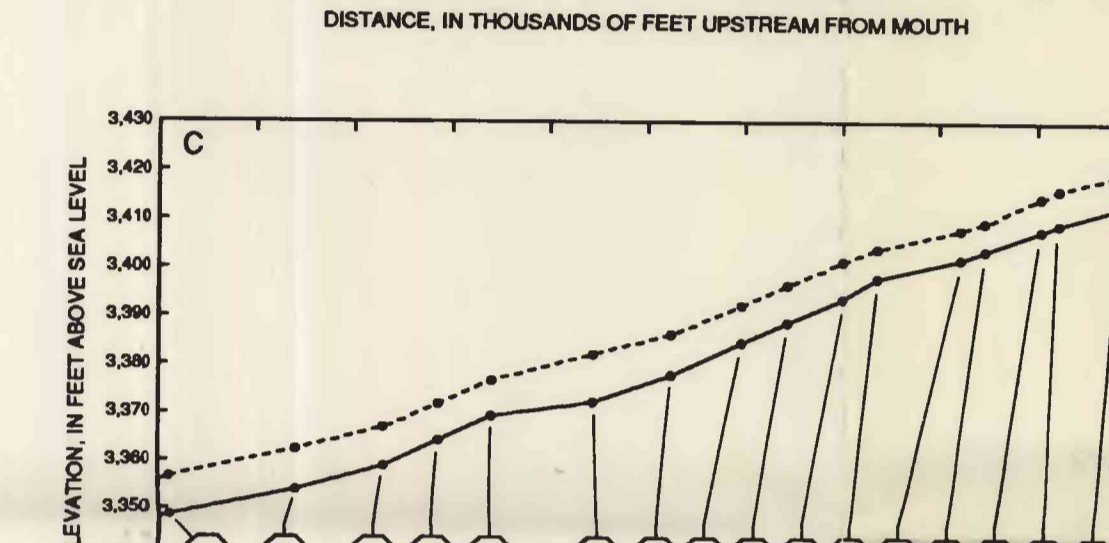
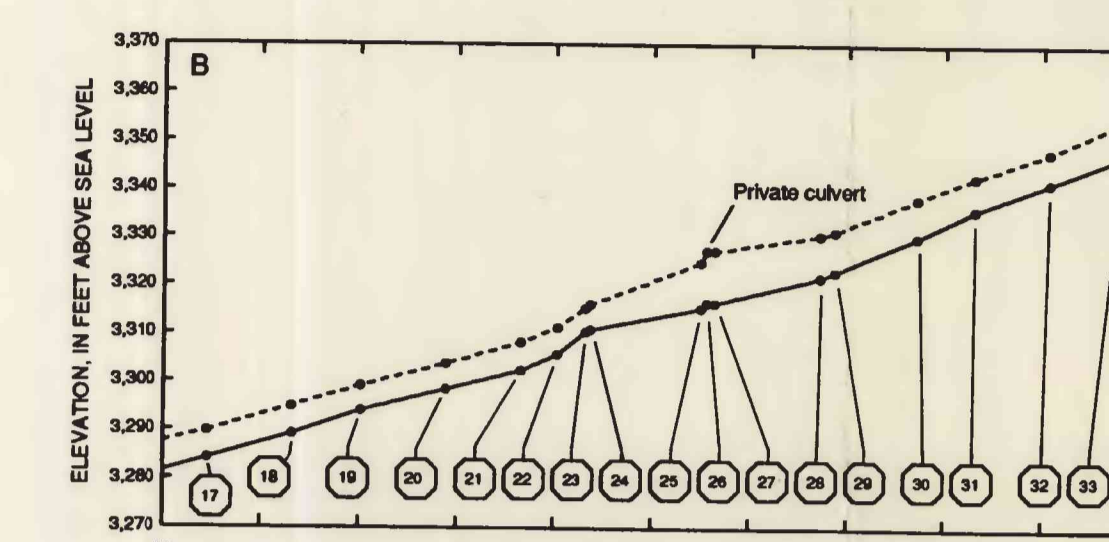


Figure 5.--Profile of computed water-surface elevations for the 100-year flood, streambed elevations, and location of cross sections.

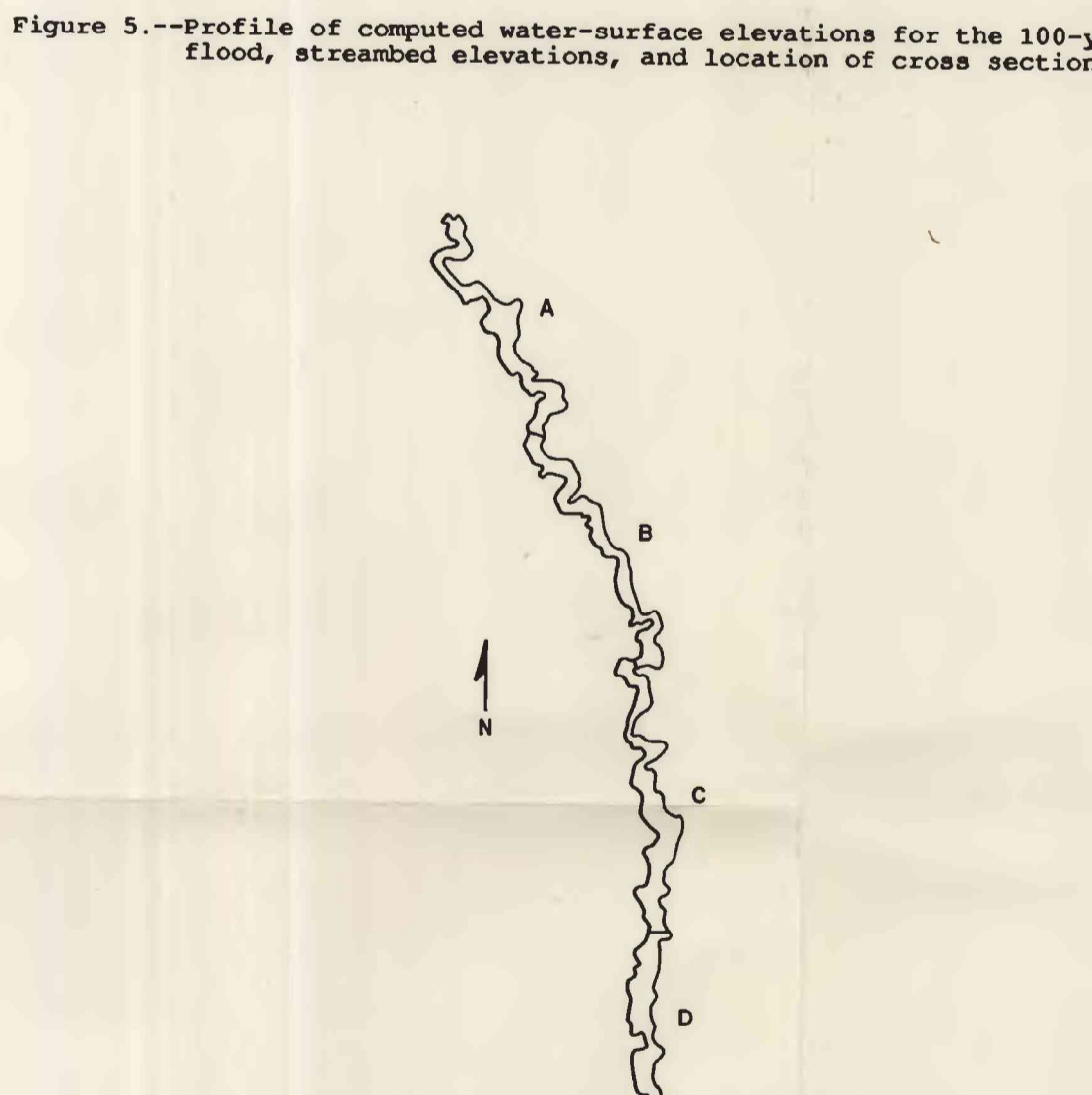


Diagram showing relative position of segments of the profile along Muddy Creek

Base from U.S. Geological Survey Lane Deer, 1958; Painted Hill, 1958; 1:24,000

SCALE 1:18,000
0 0.5 1 MILE
0 0.5 1 KILOMETER
CONTOUR INTERVAL 20 FEET
DATUM IS SEA LEVEL

For additional information write to:

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