

Notable Local Floods of 1942-43

GEOLOGICAL SURVEY WATER-SUPPLY PAPER 1134



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Flood of August 4-5, 1943 in Central West Virginia

H. M. ERSKINE

with a Summary of Flood Stages and Discharges in West Virginia

NOTABLE LOCAL FLOODS OF 1942-43

GEOLOGICAL SURVEY WATER-SUPPLY PAPER 1134-A

*Prepared in cooperation with the Corps
of Engineers and the State of West
Virginia*



UNITED STATES DEPARTMENT OF THE INTERIOR

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NOTABLE LOCAL FLOODS OF 1942-43

FLOOD OF AUGUST 4-5, 1943, IN CENTRAL WEST VIRGINIA

By H. M. ERSKINE

ABSTRACT

During the night of August 4-5, 1943, a violent thunderstorm of unusual intensity occurred in parts of Braxton, Calhoun, Gilmer, Ritchie, and Wirt Counties in the Little Kanawha River Basin in central West Virginia. Precipitation amounted to as much as 15 inches in 2 hours in some sections. As a result, many small streams and a reach of the Little Kanawha River in the vicinity of Burnsville and Gilmer reached the highest stages known. Computations based on special surveys made at suitable sites on representative small streams in the areas of intense flooding indicate that peak discharges closely approach 50 percent of the Jarvis scale.

Twenty-three lives were lost on the small tributaries as numerous homes were swept away by the flood, which developed with incredible rapidity during the early morning hours. Damage estimated at \$1,300,000 resulted to farm buildings, crops, land, livestock, railroads, highways, and gas- and oil-producing facilities. Considerable permanent land damage resulted from erosion and deposition of sand and gravel.

INTRODUCTION

The flood of August 4-5, 1943, in the Little Kanawha River Basin in central West Virginia was of short duration and high intensity. The area affected (fig. 1) was about 50 miles long and 10 miles wide extending northwest along the major axis of the roughly diamond-shaped Little Kanawha River Basin. Many small streams in Braxton, Calhoun, Gilmer, Ritchie, and Wirt Counties reached stages much higher than previously known as a result of precipitation that amounted to as much as 15 inches in 2 hours. Little Kanawha River reached the highest stages of record in the vicinity of Burnsville and Gilmer, but farther downstream the peak stages were not exceptional. In less than one-fourth of the area of the basin was precipitation during the storm in excess of 4 inches.

Twenty-three lives were lost as a result of the flood. Although there was considerable flooding of residential property in the communities along the Little Kanawha River, all loss of life occurred along relatively small tributaries. There in the small valleys the water rose with great rapidity during the early morning hours, carrying away many homes.

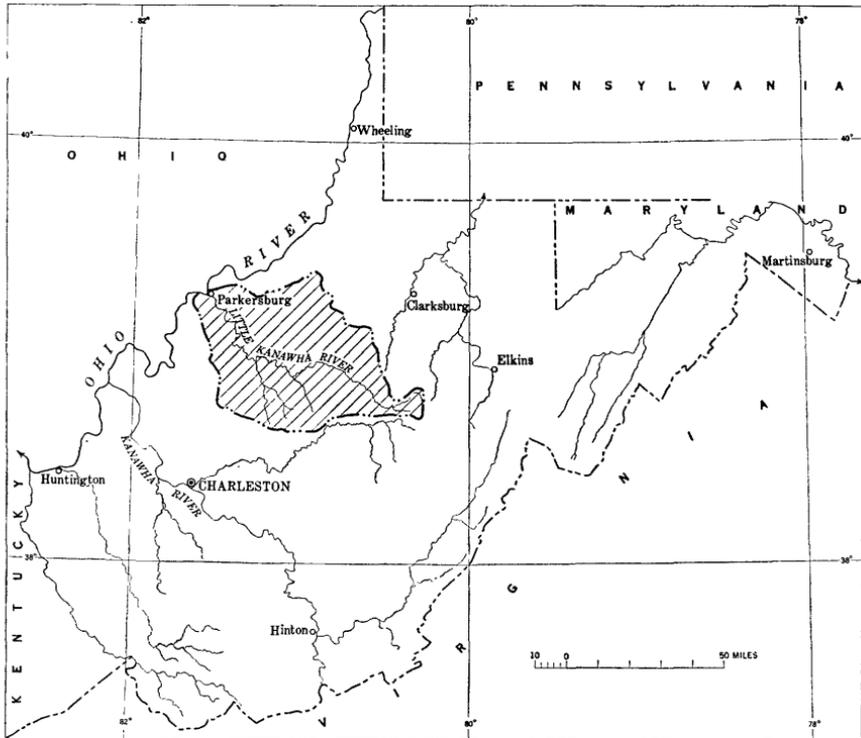


FIGURE 1.—Map of West Virginia showing location of Little Kanawha River basin.

Property damages were high considering the limited extent to which the area is settled. Crops and agricultural land, farm homes and farm buildings, railroads, highways and rural roads, and gas- and oil-producing facilities suffered heavy damages. As a result of a survey conducted by its office at Huntington, W. Va., the Corps of Engineers arrived at the following summary of damages:

Agricultural lands, farm buildings, livestock, etc.....	\$511, 750
Urban damages.....	29, 500
Baltimore & Ohio Railroad Co.....	566, 935
State Road Commission.....	158, 950
Gas- and oil-producing companies.....	26, 938
Total.....	1, 294, 073

Severe flash floods, resulting from violent thunderstorms covering relatively small areas, are known to have occurred previously in West Virginia, but little or no factual information is available relative to the maximum rainfall and its intensity or to the maximum rate of discharge from the small areas affected. Notable among previous floods of this general type in West Virginia are the floods on Cabin

Creek and Coal River in Kanawha and Boone Counties during August 1916, when 40 lives were lost and the estimated property damage was \$5,000,000, and the floods on Paint Creek and Armstrong Creek in Fayette and Kanawha Counties during July 1932, which caused the loss of 19 lives and property damage estimated at \$1,000,000 (Congressional Doc., 1935).

The outstanding features of this flood made it appear highly desirable that factual information relative to it and the storm that produced it be collected and assembled in report form. Accordingly, the Geological Survey undertook to make special determinations of the peak discharges by means of slope-area measurements based on surveys at the most favorable sites available on representative small streams in the most severely flooded areas. The selection of these sites was coordinated closely with the data on rainfall that were assembled soon after the storm.

Although there were several regularly operated precipitation stations in the storm area, none of these were in the localities where the rainfall was intense. The wide variations in rainfall over relatively small areas made it essential that additional records be obtained to define accurately the amount and distribution of precipitation.

Representatives of the Corps of Engineers, the Weather Bureau, and the West Penn Power Co. made a thorough investigation of the area soon after the storm and interviewed many local residents regarding pertinent features of the storm. Numerous miscellaneous records of rainfall were thus obtained from the amount of water collected in pails, tubs, jars, and other containers that were uncovered and in the open during the storm.

It is the aim of this report to bring together in suitable form the data collected during those special investigations and the data collected at regular gaging and precipitation stations.

ADMINISTRATION AND PERSONNEL

This report was prepared in the Water Resources Division of the Geological Survey under the general administrative direction of G. L. Parker, Chief Hydraulic Engineer, until his death on February 12, 1946, and since that time by his successor, C. G. Paulsen. The field and office work was performed and the original report prepared by H. M. Erskine, district engineer, Charleston district, assisted by his staff. Hollister Johnson, hydraulic engineer, reviewed the computations of peak discharge.

ACKNOWLEDGMENTS

The general stream-measurement program in West Virginia at the time of the flood and during the subsequent investigation was carried on by the Geological Survey cooperating with the Corps of Engineers,

and with the State of West Virginia through its Geological and Economic Survey, Public Service Commission, Health Department, and Water Commission. Acknowledgment is made to those cooperating agencies and also to the Weather Bureau and to J. E. Stewart, hydraulic engineer, West Penn Power Co., who furnished much valuable material and many helpful suggestions.

DESCRIPTION OF THE STORM AND FLOOD

The heavy rainfall of August 4 to 5 and the resultant floods were caused by a large supply of convectively unstable, moist, tropical maritime air, transported from the general region of the Gulf of Mexico, which released its potential energy with explosive violence upon interaction with a cold air mass that had moved into the area from the northwest. Storms of this type are limited to the hot summer period and are characterized by intense rainfall, accompanied by thunder and lightning. This general type of storm is described in considerable detail elsewhere (Eisenlohr, 1951).

The U. S. Weather Bureau (1943) has described this storm as follows:

Thundershowers, mostly of short duration, occurred about dusk on August 4, throughout the Little Kanawha River Basin. However, these showers were locally heavy in the Burnsville-Copen area. They were followed about 3 hours later by record-breaking rains accompanied by one of the worst, if not the worst, electrical storms of record. The excessive rains began to fall in the McFarlan-Girta area about 11 p. m., August 4, and progressed southeastward into the Saltlick Creek Basin where the excessive rains began about 1 a. m., August 5. These rains continued in most places for from 1 to 2 hours and were generally continuous, although quite a number of persons reported brief slackenings of the hard rains. There were two main peaks of excessive rainfall, one over the Burnsville-Copen-Cedarville area and the other over the Nobe-Brohard area.

The resulting flood developed with incredible speed and, coming as it did at night on the tributaries, gave no opportunity for warning the people residing along the normally small streams. The violence of the storm and the roar of the streams awakened many in time for them to seek refuge on higher ground. However, many were either not awakened in time or, if awake, did not realize the danger and remained in their homes, which in many cases were destroyed by the onrushing waters. Eight persons were drowned in the vicinity of Heaters on the O'Brien Fork Saltlick Creek, eight in the vicinity of Copen on Copen Run (fig. 2), five at Girta on Island Run, and two above Tanner on Tanner Creek. Of these 23 who lost their lives more than half were small children.

The area affected by the flood ranges from hilly to mountainous, with elevations varying from about 600 to 1,500 feet except for a few points in the southeastern part where elevations in excess of 2,000



FIGURE 2.—Residence of the Yeager family where six persons perished during the flood. The house was swept $\frac{1}{2}$ mile down Copen Run and came to rest in the creek bed. Courtesy of Corps of Engineers.

feet are reached. The hillsides are generally steep, the valleys narrow, and the profiles of the tributary streams moderately steep. Heavy showers had fallen over most of the area during the week preceding the flood. These circumstances tended to produce a situation favorable to high rates of runoff.

The principal tributaries that had high rates of discharge—the highest stages known to local residents—were Saltlick Creek, Copen Run, Cedar Creek, Tanner Creek, Laurel Fork, Yellow Creek, and Leatherbark Creek. At Burnsville, Little Kanawha River reached a crest stage about 25 feet above extreme low water and 0.5 foot higher than the flood of March 1918, which had been the highest of record. At Gilmer, the 1918 maximum was exceeded by 0.1 foot. At Glenville, the crest stage was 30.73 feet compared to 33.6 feet in November 1926, which is the highest of record. The crest stage became lower as it advanced downstream.

The water-supply system for the town of Burnsville was put out of operation. Other municipal supplies were not affected but two-thirds of the wells in the flood area were reported contaminated by the flood waters. Emergency water-purification measures and public clinics for immunization against typhoid fever were promptly set up under the general direction of the State Health Department. The Red Cross and Civilian Defense workers were active in assisting with relief and rehabilitation work.

Probably the largest single property damaged by the flood was the 10-mile length of Baltimore & Ohio Railroad Co. tracks between Heaters and Burnsville where six railroad bridges in the Saltlick Creek basin were destroyed, much track washed out or moved, and

the roadbed generally damaged. (See fig. 3.) About 5 miles of track along Copen Run between Copen and Gilmer were extensively damaged and out of operation for 11 days. (See fig. 4.) The primary State highways in the flood area closed temporarily because of slides and inundations and the washing out of small bridges and fills (fig. 5). Detours were provided where needed soon after the flood



FIGURE 3.—Residence near Heaters, which with its four occupants was washed a mile down O'Brien Fork of Saltlick Creek and lodged on a displaced section of railroad track. Courtesy of Baltimore & Ohio Railroad Co



FIGURE 4.—Washed-out railroad bridge on Copen Run. Courtesy of Corps of Engineers.



FIGURE 5.—Washed-out fill on State Route 5 at Jobs Run about 10 miles below Glenville. Courtesy of Corps of Engineers.

receded. The secondary road system suffered severely with many miles of roadway and dozens of small bridges and culverts completely destroyed.

Agricultural property was heavily damaged. In addition to the many farm homes and buildings demolished or badly damaged, much tillable land in small valleys was permanently damaged by erosion along the creek banks, the washing of topsoil from the cultivated bottom lands, and the depositing of heavy deposits of sand and gravel which came down every small run in great quantities (figs. 6, 7). The



FIGURE 6.—Rock and mud deposit from a hillside wash. This wash drains about 2 acres in Right Fork Saltlick Creek basin. The man is standing at site of five-room house that was swept away by flood. Courtesy of Corps of Engineers.

cleared hillside land, used mostly for grazing purposes, was frequently cut by deep gullies. Large slides and blow-outs were common. (See fig. 8.) There were many instances where hillside pastures lost at least one-third of their cover of sod as a result of these actions.



FIGURE 7.—Typical sand and gravel deposits. Right Fork Saltlick Creek basin.



FIGURE 8.—Typical hillside erosion. Right Fork Saltlick Creek basin.

The crops on the lowlands, particularly corn, hay, and garden truck, were almost a complete loss. (See fig. 9.) Many farmers faced the necessity of selling their livestock owing to the resulting shortage of feed.

Residential and business property in Burnsville and low-lying sections of Glenville suffered materially from inundation, but no lives were lost. Ninety percent of Burnsville was inundated during the crest of the flood. Gas- and oil-producing companies suffered damages to pipe lines, rigs, compressor stations, and company buildings.



FIGURE 9.—Cornfield destroyed by flood. Right Fork Saltlick Creek. Courtesy of Corps of Engineers.

RAINFALL

Precipitation had been high in central West Virginia during the latter part of July. Heavy rainfall July 28-30 resulted in high stream flows, particularly in the upper reaches of Little Kanawha River. The data of precipitation as recorded at Weather Bureau stations in and adjacent to Little Kanawha River basin during the period July 20 to August 6 are given in table 1, and the locations of the stations are shown in figure 2. Figure 10 also shows, by means of isohyetal lines, the total rainfall during the period July 26-30.

Although light thundershowers occurred generally over the flood area during the late afternoon of August 4, the downpour producing the flood did not begin until several hours later. In the northwestern part of the area the intense rain began about 11 p. m., while in the southeastern section it did not begin until about 1 a. m., August 5. The heavy rain continued for about 2 hours with little slackening. Although there were no recording precipitation gages in the areas where the rainfall was most intense, there were several near by. The

TABLE 1.—Daily precipitation, in inches, July 20 to Aug. 6, 1943, at stations in and adjacent to Little Kanawha River basin

[From U. S. Weather Bureau]

Station (see fig. 10)	July											August						Totals for storms		
	20	21	22	23	24	25	26	27	28	29	30	31	1	2	3	4	5	6	July 25-30	Aug. 4-5
Cairo 1.			0.44				0.39		0.71	2.33	0.70					1.20	2.55		4.13	2.55
Creston 2.			.52				.72	0.19	1.67	1.36	.25					.65	2.40		3.19	2.40
Elizabeth 2.			.95				.63		1.64	1.07	.22					.21	1.65		3.46	1.65
Freemansburg 3.			.62				.96		2.50	.68	.02				0.30	1.33			3.26	
Gassaway 3.			.65				1.10		2.50	.77					.45	.64	.97		3.87	1.17
Glenwill 2.	0.12		.45				.27	.13	1.76	1.68	1.52				Tr.	.77	3.70	Tr.	5.36	3.70
Inland 1.			.60				1.67		1.31	1.15	1.46				Tr.	.80	2.45		5.59	2.45
Manchester 3.			.54				.02		1.77	.55	.29				.55	5.15	.63		2.91	5.03
Parkersburg 3.			.46				.05		1.22	.80					.68	1.34			2.17	.98
Parkersburg 2.			.30				.82		1.05	1.58	2.00				.13	.47	.03		7.23	.63
Smithburg 2.	.14		.40				.22		1.70	.55	.02				.10	1.18	.63	.01	5.19	.63
Speer 2.			.44				.33	.10	1.27	3.20	.29				.22	.94	1.61		2.37	1.61
St. Marys 2.			.42				.54		.12	1.21	.94				.08	.93	.67		3.46	1.46
Stumptown 3.			.32				.52			1.21	.94				.16	.90	1.65		3.85	1.65
Sutton 1.			.63				1.31		1.44	1.10										

1 Precipitation generally measured in late afternoon; amount recorded is for the 24 hours ending at the time of observation.

2 Precipitation measured in morning; amount then recorded is for preceding 24 hours.

3 Precipitation is for the 24-hour period midnight to midnight.

4 Incomplete; no record 6 a. m. to 2 p. m.

5 Total for period 3 p. m. 27th to 12 p. m. 30th.

records obtained from them are indicative of the distribution of the rainfall with respect to time and are in substantial agreement with the testimony of local residents in the storm area. The hourly precipitation at the recording gages during the storm period is listed in table 2.

TABLE 2.—*Hourly precipitation, in inches, for storm of Aug. 4-5, 1943, at recording gages*

[From U. S. Weather Bureau. Eastern Standard Time]

Hour ending	Freemans- burg	Gassaway	Macfarlan	Parkers- burg	Smithburg	Stump- town
<i>Aug. 4</i>						
4 p. m.				0.02		
5	0.02		0.01	.07		
6	.41		.04	.01	0.02	
7	.01	0.02				0.31
8		.43				.04
9						
10	.14		.66	.82	.29	
11	.16		3.70	.06		.02
12			.59			.42
<i>Aug. 5</i>						
1 a. m.		.67	.01			.61
2		.03	.02			.03
3		.02				
4						.02
5						.01
Total	0.74	1.17	5.03	0.93	0.31	1.46

It was apparent soon after the storm that the areas receiving the most intense rainfall were relatively small and were so scattered that the records for the regular precipitation stations did not give the complete picture. Accordingly, soon after the flood, field parties visited the area where intense rainfall was indicated and made a search for quantitative data. Thus, rainfall measurements were obtained at 118 additional points. The location of these points, amount of rainfall measured, and remarks regarding the reliability of each record are given in table 3. Figure 11 is an isohyetal map based on all available records for the storm.

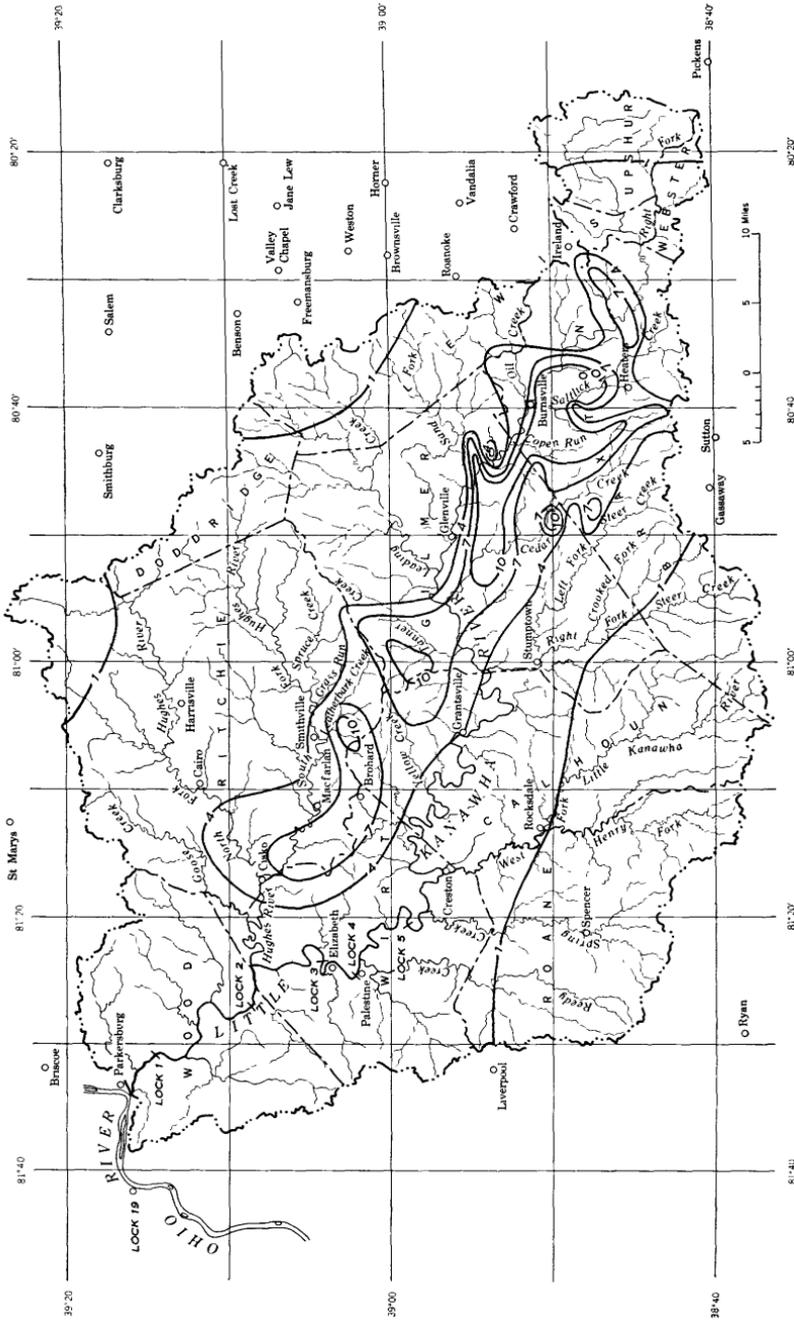


FIGURE 11.—Isohyetal map of Little Kanawha River basin showing total rainfall, August 4-5, 1943.

TABLE 3.—Miscellaneous measurements of precipitation during storm of Aug. 4-5, 1943

[Measurements taken from miscellaneous gages and from containers other than gages]

Measurement No.	Location			Elevation (feet above msl)	Rainfall (inches)	Estimated accuracy of measurement			
	Latitude		Longitude						
	°	'	"	°	'	"			
1	39	01	11	81	13	33	800	1 5.5	Fair.
2	39	02	04	81	10	39	930	9.4	Good.
3	39	02	08	81	10	34	950	8.0	Do.
4	39	01	31	81	10	31	880	4.5	Fair.
5	39	02	32	81	10	00	800	2 2.0	Good.
6	38	55	21	81	07	51	710	5.0	Do.
7	39	04	32	81	05	42	700	5.5	Do.
8	39	03	53	81	05	26	840	10.4	Do.
9	39	00	49	81	05	03	900	6.0+	Fair.
10	38	59	44	81	04	41	1,020	5.0	Do.
11	38	59	36	81	04	17	1,020	2 2.0	Good.
12	38	58	15	81	03	08	960	9.4+	Do.
13	38	57	50	81	02	31	1,080	10.5+	Do.
14	38	58	43	81	02	23	1,100	6.8+	Do.
15	38	59	03	81	02	16	1,100	9.5	Do.
16	38	55	30	81	00	44	1,100	2 2.0	Do.
17	39	01	23	80	59	38	960	6.0	Do.
18	39	00	38	80	59	07	880	6.8+	Do.
19	39	01	12	80	57	38	900	8.1+	Fair.
20	38	59	02	80	56	31	750	5.1	Good.
21	38	55	05	80	51	50	760	13.0+	Do.
22	38	51	31	80	51	05	860	6.1	Do.
23	38	50	13	80	49	35	800	6.0	Fair.
24	38	50	42	80	49	10	820	7.0+	Reliable.
25	38	50	08	80	49	07	900	10.4	Good.
26	38	50	08	80	48	54	900	6.5	Fair.
27	38	52	15	80	48	44	860	7.2	Good.
28	38	52	10	80	48	13	860	4.5	Fair.
29	38	50	02	80	48	02	840	4.9	Good.
30	38	47	12	80	48	00	870	6.9+	Fair.
31	38	52	35	80	47	59	920	9.2	Good.
32	38	47	47	80	47	24	1,050	4.5	Do.
33	38	49	23	80	47	18	800	5.3	Do.
34	38	47	23	80	46	21	960	6.4	Fair.
35	38	49	59	80	46	13	880	7.8+	Good.
36	38	52	59	80	46	12	1,175	13.0	Do.
37	38	51	35	80	45	57	830	5.3	Do.
38	38	51	31	80	45	44	890	5.0	Do.
39	38	47	52	80	45	42	880	4.7+	Fair.
40	38	48	56	80	45	21	860	6.5+	Good.
41	38	54	57	80	45	10	720	10.8	Do.
42	38	51	36	80	44	59	860	6.4	Do.
43	38	52	07	80	44	43	830	11.0+	Fair.
44	38	53	07	80	44	41	750	5.4	Good.
45	38	53	45	80	44	18	850	3.1	Seems too low.
46	38	54	09	80	44	15	740	3.6	Questionable.
47	38	46	03	80	44	09	890	6.1	Good.
48	38	54	24	80	44	09	830	15.0+	Fair.
49	38	54	38	80	44	08	770	5.2+	Good.
50	38	53	40	80	43	49	760	7.6	Do.
51	38	50	15	80	43	47	860	2 9.3	Do.
52	38	46	00	80	43	46	920	7.0	Do.
53	38	50	52	80	43	35	830	7.1	Fair.
54	38	47	23	80	43	18	1,040	8.0	Good.
55	38	48	24	80	43	13	970	9.0	Do.
56	38	45	38	80	43	04	940	7.7	Do.
57	38	50	46	80	43	03	850	9.9	Do.
58	38	49	49	80	42	33	960	9.4+	Do.
59	38	46	22	80	42	31	950	10.5+	Do.
60	38	44	31	80	42	28	975	4.6	Do.
61	38	45	02	80	42	23	950	4 7.0	Do.
62	38	44	25	80	42	10	990	5.0	Do.
63	38	43	21	80	42	07	1,075	3.8	Do.
64	38	47	24	80	41	58	1,110	9.2+	Do.
65	38	48	33	80	41	54	980	8.0	Do.
66	38	42	03	80	41	49	975	5.0	Do.
67	38	45	02	80	41	48	940	8.5	Do.
68	38	44	54	80	41	44	1,020	7.0+	Do.
69	38	44	54	80	41	44	1,100	6.1+	Do.
70	38	51	26	80	41	31	800	10.0+	Do.
71	38	47	15	80	41	19	1,130	5.0	Do.
72	38	48	21	80	41	19	950	6.9+	Do.
73	38	47	06	80	41	10	1,180	4.8	Do.
74	38	47	59	80	41	0	1,020	9.2+	D

TABLE 3.—Miscellaneous measurements of precipitation during storm of Aug. 4-5, 1943—Continued

Measurement No.	Location						Elevation (feet above msl)	Rainfall (inches)	Estimated accuracy of measurement
	Latitude			Longitude					
	°	'	"	°	'	"			
75	38	49	46	80	40	52	790	12.5	Good.
76	38	43	41	80	40	52	1,160	7.9+	Do.
77	38	48	39	80	39	56	800	8.5+	Fair.
78	38	51	41	80	39	45	800	5.3	Good.
79	38	51	36	80	39	46	800	7.6+	Do.
80	38	48	10	80	39	37	870	3.3	Questionable.
81	38	47	48	80	39	37	1,080	7.2+	Good.
82	38	47	54	80	39	28	980	6.2	Fair.
83	38	45	21	80	39	18	890	4.2	Good.
84	38	51	31	80	39	17	780	8.9+	Do.
85	38	45	40	80	39	16	910	5.5	Fair.
86	38	43	55	80	39	14	980	4.6	Do.
87	38	51	41	80	39	12	880	14.8	Do.
88	38	51	03	80	38	56	780	10.0	Good.
89	38	45	45	80	38	33	880	6.0	Do.
90	38	46	41	80	38	26	820	6.9+	Fair.
91	38	46	25	80	38	15	950	7.7	Good.
92	38	48	45	80	38	06	960	³ 12.0	Do.
93	38	45	43	80	38	04	920	4.8	Do.
94	38	50	41	80	37	33	780	10.3+	Fair.
95	38	48	23	80	37	07	840	10.0+	Good.
96	38	51	18	80	37	02	1,010	6.5	Do.
97	38	52	18	80	36	58	810	4.4	Fair.
98	38	49	38	80	36	54	740	3.5+	Do.
99	38	49	38	80	36	54	740	5.4	Do.
100	38	45	51	80	36	48	860	6.2	Good.
101	38	49	21	80	36	47	820	9.6+	Fair.
102	38	48	57	80	36	46	820	5.0	Do.
103	38	50	33	80	36	44	790	6.4	Good.
104	38	50	33	80	36	44	800	5.8+	Do.
105	38	49	05	80	36	43	850	9.3+	Do.
106	38	52	50	80	36	39	850	4.7	Do.
107	38	47	00	80	36	34	900	5.5	Do.
108	38	50	45	80	36	34	820	6.1+	Do.
109	38	50	51	80	36	28	825	6.7	Do.
110	38	45	58	80	36	09	820	6.2	Do.
111	38	45	53	80	35	53	900	6.5	Do.
112	38	46	12	80	34	02	785	4.9	Do.
113	38	46	12	80	34	02	785	4.6	Do.
114	38	44	56	80	33	44	1,225	9.5	Do.
115	38	46	31	80	32	48	825	7.6	Do.
116	38	46	02	80	31	31	980	7.8	Do.
117	38	48	02	80	29	54	1,025	³ 8.0	Do.
118	38	48	02	80	29	54	1,025	8.0	Do.

NOTE.—Where the record is incomplete owing to overflowing of the container, the total amount is followed by +. All measurements include the fall from a shower or showers that occurred about dusk on August 4, 1943, and it is estimated that this fall did not exceed 1 inch at any place.

¹ Approximate.

² Considerably plus.

³ Slightly plus.

⁴ Estimated.

FLOOD DISCHARGE

MEASUREMENT OF FLOOD DISCHARGE

The usual method of determining stream discharge at gaging stations is by the application of the stage-discharge rating to the stages recorded. The rating is defined by current-meter discharge measurements through as much of the range between the extremes of low and high water as is practicable.

Many difficulties are encountered when attempts are made to obtain current-meter measurements at high stages, particularly at

gaging stations on the relatively small and flashy streams. Impassable roads resulting from slides, debris deposits, washouts, and inundation; washed-out or inaccessible bridges or cableways from which measurements are usually made; huge quantities of drift in the water and very swift and turbulent flow as well as rapidly changing stages are among the obstacles encountered. When it is impracticable or impossible to obtain current-meter measurements upon which to base the upper end of a rating, it must be extended upward on the basis of special studies.

Information on flood flows may be desirable at points other than at the regular gaging stations. This is particularly true of flash floods resulting from intense rainfall over relatively small areas as in the flood of August 1943. Although it is usually impossible to determine the total flood runoff at such points, a reasonably accurate value for the peak discharge may be computed where field conditions are favorable. The methods employed in determining the maximum discharges given in this report are: Extension of rating curves at the gaging stations, computation of flow by the critical-depth method, computation of flow through contracted openings, and computation of flow by the slope-area method. A brief statement concerning each of these methods is given below; fuller descriptions are given elsewhere (Corbett and others, 1943, pp. 98-108).

Extension of rating curves.—This method is based essentially on the shape of the upper part of the rating curve as defined by current-meter measurements and a knowledge of the channel conditions and other pertinent factors that may affect the shape of the rating above the highest measurements. Logarithmic plotting with suitable adjustments for overflow areas and other factors that may affect the channel capacity at high stages is frequently used.

Critical-depth method.—This method has limited usage as it depends on the flow at the selected section being under critical-depth conditions. Proof that this condition prevailed usually depends upon an independent determination of peak flow by other methods, therefore it is generally used in conjunction with determinations made by some other method.

Contracted-opening method.—Computations using this method are based on the conversion of head into velocity while the flow is passing through a restricted section of the channel such as a narrow opening between bridge abutments.

Slope-area method.—This method has the widest application to conditions as they are usually found in natural streams. It was used in computing peak discharges at all but one of the sites where special

discharge determinations were made for this flood. The computations were based on the Manning formula.

The slope, area of cross section, and hydraulic radius were computed from data obtained by field surveys soon after the flood had subsided. Where necessary, owing to the relative difference in conveyance properties, the cross sections were divided into appropriate parts and a value of n assigned to each. Where cross sections were reasonably uniform, one value of n was assigned to the entire section. The value selected for n in each case was arrived at on the basis of the Geological Survey's background of experience gained through flood studies involving channels of various types and through reference to engineering texts and manuals dealing with the subject.

DISCHARGES AT GAGING STATIONS DURING THE FLOOD PERIOD

Records of stage and discharge for the flood period at the nine gaging stations in operation in the Little Kanawha River Basin are given on succeeding pages. These records consist of a station description, a table giving the daily mean discharge during July and August 1943, and a table showing the gage height and discharge at indicated time in sufficient detail to permit reasonably accurate plotting of the hydrographs during the flood period. The station description gives information regarding the location of the station, the drainage area, the character of the gage height and discharge records, the maximum during the flood of August 1943, and the greatest known flood prior to 1943. The table of gage height and discharge at indicated time is omitted for the stations on Leading and Steer Creeks because complications due to backwater prevented the computation of discharge with sufficient accuracy to warrant this refinement. Where backwater or other factors affected the normal stage-discharge relation, notation of the special methods used is made under "Discharge records."

In nearly all cases the stage-discharge relation is fairly well defined by current-meter measurements, up to the maximum for the August 1943 flood. Slope-area measurements have been used to aid in defining the upper ends of the ratings for the stations in the Hughes River Basin. Water-stage recorders functioned satisfactorily at all stations during the flood period. At nonrecording stations the gage readers rendered excellent service by obtaining special gage readings, which made it possible to construct accurate hydrographs for the flood period. Figures 12 and 13 are hydrographs showing the discharge of Little Kanawha River and its tributaries during the period July 26 to August 9.

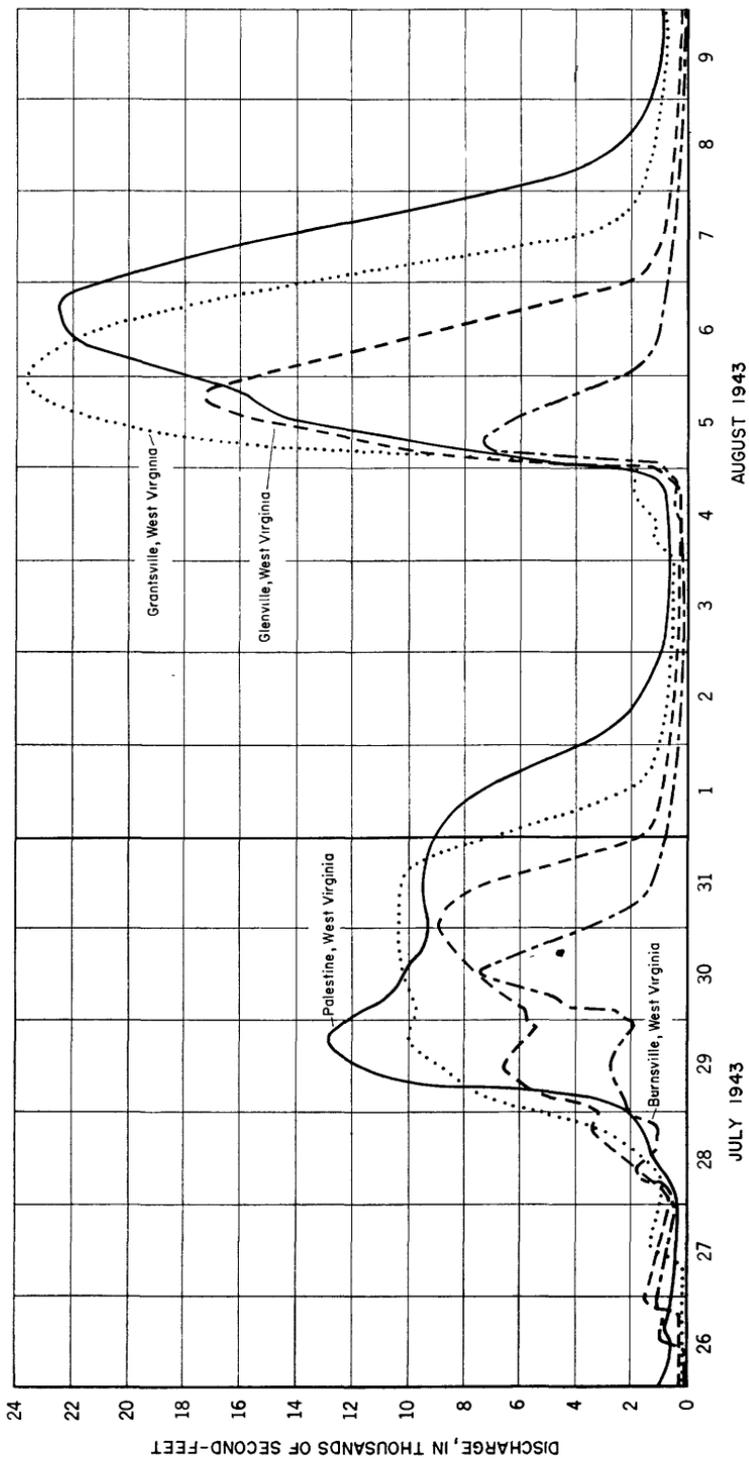


Figure 12.—Discharge hydrographs at stream-gaging stations on Little Kanawha River July 26 to Aug. 9, 1943.

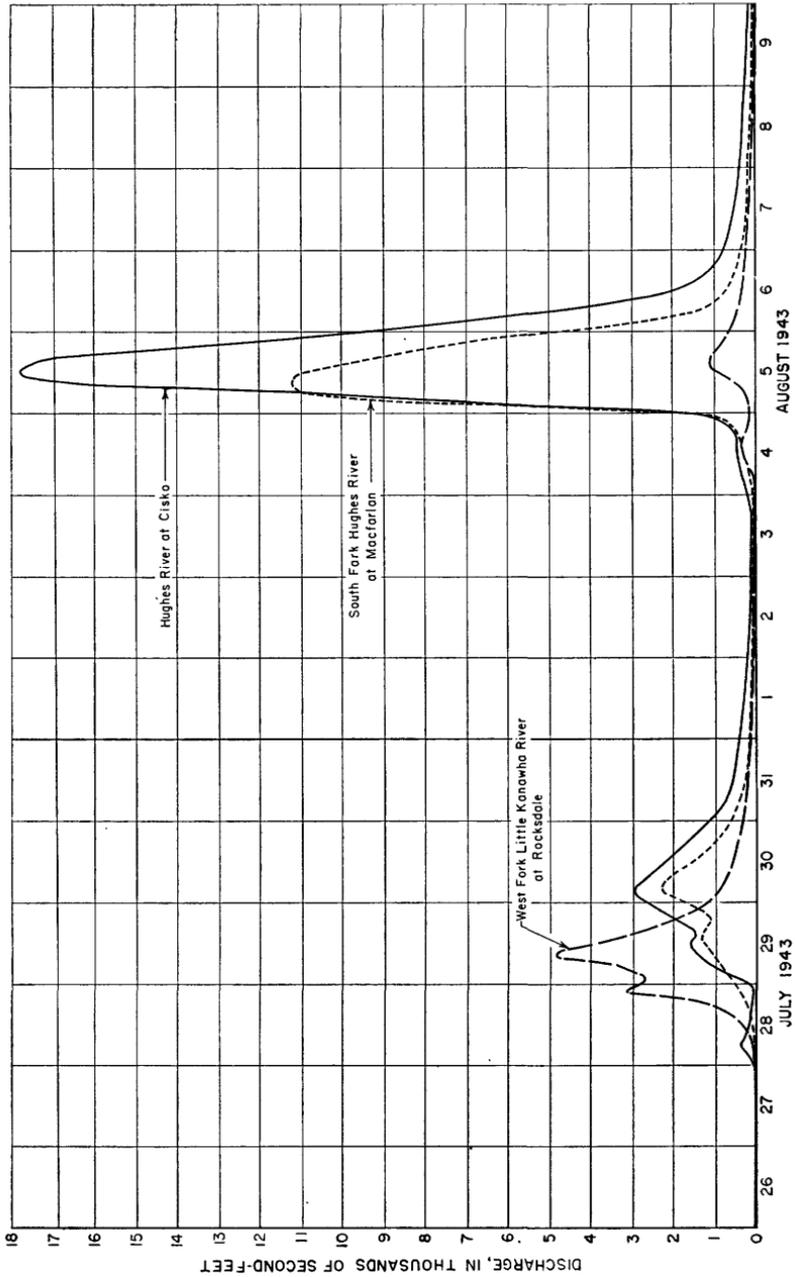


Figure 13.—Discharge hydrographs at stream-gaging stations on tributaries of Little Kanawha River July 26 to Aug. 9, 1943.

LITTLE KANAWHA RIVER NEAR BURNSVILLE

LOCATION.—Lat. 38°49'25'', long. 80°35'35'', at bridge on State Highway 5, 0.1 mile downstream from Knawl Creek and 4 miles southeast of Burnsville, Braxton County. Datum of gage is 756.09 feet above mean sea level (levels by Corps of Engineers).

DRAINAGE AREA.—155 square miles.

GAGE-HEIGHT RECORD.—Water-stage recorder graph except for period 9 a. m. to 4 p. m. July 30 for which a graph was drawn based on a floodmark.

DISCHARGE RECORD.—Stage-discharge relation defined by current-meter measurements up to 6,500 second-feet and extended above. Gage heights used to half-tenths between 3.5 and 4.6 feet; hundredths below and tenths above these limits.

MAXIMA.—1943: Discharge, 7,370 second-feet 1 p. m. July 30 (gage height 17.62 feet from floodmark); 7,290 second-feet 6 a. m. Aug. 5 (gage height, 17.46 feet).

1938-42: Discharge, 9,200 second-feet Feb. 3, 1939 (gage height, 19.04 feet, observed at crest).

The flood of Mar. 13, 1918, reached a stage of 19.7 feet, from floodmark (discharge about 9,800 second-feet).

Mean discharge, in second-feet, 1943

Day	July	August	Day	July	August	Day	July	August	Day	July	August
1.....	69	510	9.....	146	172	17.....	42	495	25.....	36	37
2.....	52	288	10.....	93	121	18.....	52	300	26.....	517	33
3.....	41	177	11.....	57	106	19.....	54	177	27.....	619	50
4.....	38	308	12.....	44	75	20.....	45	115	28.....	1,120	147
5.....	35	5,050	13.....	76	107	21.....	34	80	29.....	2,280	104
6.....	134	1,180	14.....	93	447	22.....	38	64	30.....	5,190	61
7.....	84	495	15.....	76	705	23.....	78	53	31.....	1,560	46
8.....	69	282	16.....	62	408	24.....	50	44			
Monthly mean discharge, in second-feet.....										416	395
Runoff, in inches.....										3.09	2.94

Gage height, in feet, and discharge, in second-feet, at indicated time, 1943

Hour	Gage height	Discharge	Hour	Gage height	Discharge	Hour	Gage height	Discharge
July 26:			July 26—Con.			July 28—Con.		
1 a. m.---	1.58	34	10.....	5.38	1,010	8.....	7.78	1,810
2.....	1.58	34	11.....	5.40	1,010	9.....	7.94	1,840
3.....	1.57	33	12.....	5.43	1,010	10.....	7.80	1,810
4.....	1.57	33	July 27:			11.....	7.43	1,670
5.....	1.56	33	2 a. m.---	5.34	980	Noon.....	6.98	1,530
6.....	1.55	32	3.....	5.00	890	2 p. m.---	5.82	1,130
7.....	1.55	32	4.....	4.70	800	3.....	5.22	950
8.....	1.75	44	5.....	4.40	710	4.....	5.41	1,010
9.....	2.12	88	6.....	4.15	635	5.....	5.45	1,010
10.....	3.15	342	7.....	3.95	575	6.....	7.15	1,600
11.....	4.20	650	8.....	3.48	435	7.....	8.46	2,070
Noon.....	4.77	830	9.....	3.17	348	July 29:		
1 p. m.---	5.07	920	July 28:			4 a. m.---	8.93	2,200
2.....	5.11	920	1 a. m.---	3.12	334	5.....	9.30	2,360
3.....	4.90	860	2.....	3.08	323	Noon.....	9.83	2,560
4.....	4.62	770	3.....	3.05	314	1 p. m.---	9.86	2,600
5.....	4.48	740	4.....	3.02	306	2.....	9.63	2,480
6.....	4.75	830	5.....	3.23	364	3.....	9.24	2,320
7.....	5.10	920	6.....	5.50	1,040	4.....	8.67	2,120
8.....	5.30	980	7.....	7.15	1,600	5.....	8.00	1,840
9.....	5.35	995				6.....	8.87	2,200

Gage height, in feet, and discharge, in second-feet, at indicated time, 1943—Con.

Hour	Gage height	Discharge	Hour	Gage height	Discharge	Hour	Gage height	Discharge
<i>July 30:</i>			<i>Aug. 3:</i>			<i>Aug. 5—Con.</i>		
2 a. m.	9.70	2,520	Noon	2.57	172	5	17.40	7,210
4	13.78	4,620	12 p. m.	2.52	160	6	17.46	7,290
6	14.32	4,940	<i>Aug. 4:</i>			7	17.43	7,210
8	15.50	5,740	2 a. m.	2.55	167	8	17.35	7,210
10	16.6	6,570	4	2.58	174	9	17.23	7,050
Noon	17.5	7,290	6	2.64	189	10	17.04	6,890
1 p. m.	17.62	7,370	8	2.71	208	11	16.78	6,730
2	17.5	7,290	10	2.78	227	Noon	16.47	6,490
4	16.72	6,650	Noon	3.12	323	3 p. m.	15.07	5,460
6	15.77	5,950	1 p. m.	3.27	366	6	13.34	4,320
8	14.60	5,130	2	3.33	384	9	11.20	3,190
10	13.40	4,380	3	3.37	395	12	9.40	2,400
12	12.17	3,700	4	3.37	396	<i>Aug. 6:</i>		
<i>July 31:</i>			5	3.32	381	6 a. m.	6.40	1,300
6 a. m.	8.46	2,040	6	3.27	366	Noon	5.37	1,000
Noon	6.17	1,240	7	3.24	358	6 p. m.	4.76	825
6 p. m.	5.22	945	8	3.21	349	12	4.28	675
12	4.52	735	9	3.29	372	<i>Aug. 7:</i>		
<i>Aug. 1:</i>			10	3.82	525	8 a. m.	3.82	525
6 a. m.	4.02	585	11	4.15	630	4 p. m.	3.55	450
Noon	3.70	495	12	4.52	735	12	3.29	372
6 p. m.	3.49	432	<i>Aug. 5:</i>			<i>Aug. 8:</i>		
12	3.30	375	1 a. m.	4.74	795	Noon	2.94	271
<i>Aug. 2:</i>			2	8.10	1,880	12 p. m.	2.74	216
Noon	2.96	277	3	15.60	5,810	<i>Aug. 9:</i>		
12 p. m.	2.76	221	4	17.08	6,970	Noon	2.55	167
						12 p. m.	2.40	133

LITTLE KANAWHA RIVER AT GLENVILLE

LOCATION.—Lat. 38°56'00'', long. 80°50'20'', at highway bridge at Glenville, Gilmer County, and about 1,000 feet upstream from Sycamore Run. Datum of gage is 697.79 feet above mean sea level, adjustment of 1912. Auxiliary gage on highway bridge at mouth of Leading Creek, 2.7 miles downstream. Datum of auxiliary gage is 700.23 feet above mean sea level, adjustment of 1912.

DRAINAGE AREA.—386 square miles.

GAGE-HEIGHT RECORD.—Water-stage recorder graph at the base gage. Graph drawn based on four readings a day of the auxiliary staff gage from 6 a. m. July 29 to 3 p. m. July 31 and 8 a. m. Aug. 5 to 6 p. m. Aug. 6.

DISCHARGE RECORD.—Stage-discharge relation defined by current-meter measurements for entire range of stage. Gage heights used to half-tenths between 3.5 and 5.5 feet; hundredths below and tenths above these limits. Stage-fall-discharge relation defined by current-meter measurements for entire range of stage, used during periods of backwater from Leading Creek 6 a. m. July 29 to 3 p. m. July 31 and 8 a. m. Aug. 5 to 6 p. m. Aug. 6.

MAXIMA.—1943: Discharge, 17,300 second-feet 6 p. m. Aug. 5 (gage height 30.73 feet).

1915-22, 1928-42: Discharge, 20,400 second-feet Apr. 16, 1939, from graph of discharge adjusted for changing stage; gage height, 33.22 feet Apr. 17, 1939, from floodmarks.

The flood of Nov. 16, 1926, reached a stage of 33.6 feet; discharge not determined, but probably less than that of Apr. 16, 1939.

Mean discharge, in second-feet, 1943

Day	July	August	Day	July	August	Day	July	August	Day	July	August
1.....	114	911	9.....	172	296	17.....	65	1,400	25.....	75	70
2.....	83	426	10.....	136	208	18.....	69	560	26.....	333	59
3.....	56	262	11.....	90	167	19.....	83	308	27.....	1,060	61
4.....	44	318	12.....	70	146	20.....	75	204	28.....	1,970	215
5.....	41	13,000	13.....	88	118	21.....	65	150	29.....	5,500	229
6.....	292	8,670	14.....	132	302	22.....	56	120	30.....	7,400	142
7.....	256	984	15.....	98	770	23.....	69	98	31.....	6,090	94
8.....	138	474	16.....	70	578	24.....	96	80			
Monthly mean discharge, in second-feet.....										803	1,014
Runoff, in inches.....										2.40	3.03

Gage height, in feet, and discharge, in second-feet, at indicated time, 1943

Hour	Gage height	Discharge	Hour	Gage height	Discharge	Hour	Gage height	Discharge
<i>July 26:</i>			<i>July 30:</i>			<i>Aug. 5—Con.</i>		
2 a. m.	2.24	61	2 a. m.	16.30	5,720	4.....	21.07	8,780
6.....	2.22	59	4.....	16.82	6,050	5.....	22.68	10,000
Noon.....	2.28	66	6.....	17.40	6,480	6.....	23.98	11,000
2 p. m.	2.30	69	8.....	17.98	6,850	7.....	24.98	11,800
4.....	2.32	72	10.....	18.50	7,250	8.....	25.84	12,000
6.....	2.95	184	Noon.....	18.95	7,580	9.....	26.59	12,200
8.....	5.50	1,020	2 p. m.	19.34	7,830	10.....	27.44	13,000
10.....	6.56	1,430	4.....	19.72	8,100	11.....	28.19	13,900
12.....	6.85	1,510	6.....	20.05	8,400	Noon.....	28.82	14,600
<i>July 27:</i>			8.....	20.32	8,590	2 p. m.	29.85	16,200
6 a. m.	6.21	1,280	10.....	20.55	8,790	4.....	30.51	16,900
Noon.....	5.70	1,100	12.....	20.67	8,900	6.....	30.73	17,800
6 p. m.	4.92	805	<i>July 31:</i>			8.....	30.68	17,200
12.....	4.32	595	2 a. m.	20.65	8,820	10.....	30.25	16,400
<i>July 28:</i>			4 a. m.	20.46	8,700	12.....	29.73	15,700
2 a. m.	4.17	542	6.....	20.06	8,500	<i>Aug. 6:</i>		
4.....	4.05	508	8.....	19.50	8,020	2 a. m.	29.00	14,600
6.....	4.80	770	10.....	18.62	7,310	4.....	28.05	13,500
8.....	5.48	1,020	Noon.....	17.50	6,620	6.....	26.90	12,200
10.....	6.62	1,430	2 p. m.	12.55	3,770	8.....	25.51	11,000
Noon.....	7.80	1,880	12.....	7.57	1,800	10.....	24.00	10,100
2 p. m.	9.28	2,430	<i>Aug. 1:</i>			Noon.....	22.17	9,030
4.....	10.62	2,940	4 a. m.	6.05	1,210	2 p. m.	20.10	7,820
6.....	11.55	3,350	6.....	5.42	988	4.....	17.95	6,800
8.....	11.72	3,390	Noon.....	5.00	840	6.....	15.50	5,440
10.....	11.55	3,350	2 p. m.	4.52	665	8.....	13.15	4,030
12.....	11.22	3,180	12.....	4.20	560	10.....	10.70	2,980
<i>July 29:</i>			<i>Aug. 2:</i>			12.....	8.62	2,170
2 a. m.	11.90	3,470	Noon.....	3.76	410	<i>Aug. 7:</i>		
4.....	13.50	4,180	12 p. m.	3.45	320	6 a. m.	5.73	1,100
6.....	14.90	5,400	<i>Aug. 3:</i>			Noon.....	5.08	877
8.....	15.95	5,850	Noon.....	3.23	256	6 p. m.	4.62	700
10.....	16.80	6,380	12 p. m.	3.11	224	12.....	4.34	612
Noon.....	17.23	6,620	<i>Aug. 4:</i>			<i>Aug. 8:</i>		
2 p. m.	17.48	6,600	6 a. m.	3.16	237	6 a. m.	4.12	525
4.....	17.37	6,400	Noon.....	3.40	305	Noon.....	3.91	458
6.....	17.00	6,120	6 p. m.	3.48	329	6 p. m.	3.74	410
8.....	16.45	5,770	12.....	5.53	1,020	12.....	3.60	365
10.....	15.73	5,270	<i>Aug. 5:</i>			<i>Aug. 9:</i>		
12.....	16.19	5,700	1 a. m.	9.39	2,470	Noon.....	3.37	296
			2.....	15.66	5,310	12 p. m.	3.19	245
			3.....	19.22	7,440	<i>Aug. 10:</i>		
						Noon.....	3.05	208
						12 p. m.	2.93	180

LITTLE KANAWHA RIVER AT GRANTSVILLE

LOCATION.—Lat. 38°55'20", long. 81°05'50", at bridge on State Highway 16 at Grantsville, Calhoun County, about 1,200 feet downstream from Philip Run. Datum of gage is 652.83 feet above mean sea level, adjustment of 1912.

DRAINAGE AREA.—913 square miles.

GAUGE-HEIGHT RECORD.—Water-stage recorder graph.

DISCHARGE RECORD.—Stage-discharge relation defined by current-meter meas-

urements. Gage heights used to half-tenths between 8.0 and 11.0 feet; hundredths below and tenths above these limits.

MAXIMA.—1943: Discharge, 23,700 second-feet 10:30 p. m. Aug. 5 (gage height, 34.95 feet).

1928-42: Discharge, 34,300 second-feet Apr. 17, 1939 (gage height, 43.10 feet).

Mean discharge, in second-feet, 1943

Day	July	August	Day	July	August	Day	July	August	Day	July	August
1	288	2,920	9	367	899	17	110	2,800	25	128	148
2	172	810	10	428	644	18	99	1,470	26	119	170
3	126	530	11	239	512	19	94	776	27	738	343
4	99	1,340	12	156	422	20	108	512	28	1,820	467
5	112	17,900	13	126	467	21	106	330	29	8,530	593
6	156	20,500	14	158	374	22	101	264	30	9,970	422
7	571	5,750	15	184	1,070	23	101	201	31	9,700	259
8	380	1,330	16	140	1,370	24	112	167			
Monthly mean discharge, in second-feet										1,146	2,121
Runoff, in inches										1.45	2.68

Gage height, in feet, and discharge, in second-feet, at indicated time, 1943

Hour	Gage height	Discharge	Hour	Gage height	Discharge	Hour	Gage height	Discharge
<i>July 26:</i>			<i>July 30-Con.</i>			<i>Aug. 5-Con.</i>		
2 a. m.	6.96	131	12	21.18	10,300	9	29.55	18,300
Noon	6.93	124	<i>July 31:</i>			10	30.30	19,000
12 p. m.	6.87	110	4 a. m.	21.23	10,300	11	31.00	19,700
<i>July 27:</i>			8	21.23	10,300	Noon	31.60	20,300
2 a. m.	6.87	110	Noon	21.03	10,200	1 p. m.	32.10	20,800
4	6.86	108	4 p. m.	20.51	9,700	2	32.70	21,400
6	6.86	108	8	19.50	8,800	3	33.22	21,900
8	6.88	112	12	17.83	7,270	4	33.75	22,500
10	8.50	565	<i>Aug. 1:</i>			5	34.10	22,800
Noon	9.84	1,180	4 a. m.	15.66	5,380	6	34.40	23,100
2 p. m.	10.03	1,290	8	13.35	3,500	7	34.62	23,300
4	10.02	1,260	Noon	11.60	2,240	8	34.80	23,500
6	9.96	1,240	6 p. m.	10.26	1,400	9	34.90	23,600
8	9.86	1,180	12	9.68	1,100	10	34.94	23,600
10	9.76	1,130	<i>Aug. 2:</i>			11	34.95	23,700
12	9.63	1,080	Noon	9.06	790	12	34.91	23,600
<i>July 28:</i>			12 p. m.	8.68	645	<i>Aug. 6:</i>		
2 a. m.	9.50	1,000	<i>Aug. 3:</i>			2 a. m.	34.75	23,500
4	9.35	925	Noon	8.35	512	4	34.50	23,200
6	9.48	1,000	2 p. m.	8.30	495	6	34.13	22,800
8	9.38	950	4	8.25	478	8	33.70	22,400
10	9.52	1,020	6	8.20	460	10	33.10	21,800
Noon	10.13	1,350	8	8.28	495	Noon	32.40	21,100
2 p. m.	10.71	1,670	10	8.35	512	4 p. m.	30.60	19,300
4	11.32	2,040	12	8.28	495	8	28.35	17,200
6	11.83	2,370	<i>Aug. 4:</i>			12	25.40	14,300
8	12.75	3,060	2 a. m.	8.74	665	<i>Aug. 7:</i>		
10	13.92	3,880	4	9.58	1,050	2 a. m.	23.65	12,600
12	15.30	5,020	6	9.87	1,180	4	21.85	10,900
<i>July 29:</i>			8	9.83	1,180	6	19.90	9,160
6 a. m.	18.20	7,630	10	9.70	1,100	8	17.95	7,450
Noon	19.50	8,800	Noon	9.85	1,180	10	16.00	5,650
2 p. m.	19.96	9,250	2 p. m.	10.33	1,460	Noon	14.40	4,240
4	20.38	9,610	4	10.78	1,730	4 p. m.	12.30	2,580
6	20.70	9,880	6	10.94	1,820	8	11.52	1,990
8	20.82	9,970	8	10.93	1,820	12	11.15	1,790
10	20.80	9,970	10	10.90	1,790	<i>Aug. 8:</i>		
12	20.62	9,790	12	10.85	1,760	2 a. m.	11.00	1,660
<i>July 30:</i>			<i>Aug. 5:</i>			Noon	10.45	1,330
2 a. m.	20.46	9,700	1 a. m.	11.00	1,850	12 p. m.	9.93	1,080
4	20.46	9,700	2	16.20	5,830	<i>Aug. 9:</i>		
6	20.60	9,790	3	17.85	7,270	Noon	9.51	878
8	20.74	9,880	4	20.70	9,880	12 p. m.	9.16	737
10	20.85	9,970	5	23.70	12,700	<i>Aug. 10:</i>		
Noon	20.91	10,100	6	25.90	14,800	Noon	8.89	644
4 p. m.	21.00	10,200	7	27.50	16,300	12 p. m.	8.63	560
8	21.08	10,200	8	28.70	17,500			

SUPPLEMENTAL RECORD.—Aug. 5, 10:30 p. m., gage height, 34.93 feet; discharge, 23,700 second-feet.

LITTLE KANAWHA RIVER AT PALESTINE

LOCATION.—Lat. 39°02'00'', long. 81°24'20'', in lower pool at lock 4 at Palestine, Wirt County, 0.9 mile downstream from Reedy Creek. Datum of gage is 596.075 feet above mean sea level, adjustment of 1912. Auxiliary water-stage recorder in upper pool at lock 3, 5.5 miles downstream from lock 4. Datum of gage is 590.51 feet above mean sea level, adjustment of 1912.

DRAINAGE AREA.—1,510 square miles.

GAGE-HEIGHT RECORD.—Water-stage recorder graphs. Record from auxiliary gage used during periods of no backwater from Ohio River.

DISCHARGE RECORD.—State-discharge relation defined by current-meter measurements up to 18,000 second-feet and extended above. Stage-fall-discharge

Mean discharge in second-feet, 1943

Day	July	August	Day	July	August	Day	July	August	Day	July	August
1.....	417	7,040	9.....	624	1,320	17.....	205	2,430	25.....	181	182
2.....	375	1,900	10.....	538	968	18.....	246	2,760	26.....	644	346
3.....	326	784	11.....	481	776	19.....	222	1,400	27.....	329	475
4.....	283	784	12.....	378	661	20.....	186	907	28.....	877	707
5.....	276	12,200	13.....	309	651	21.....	155	466	29.....	9,800	743
6.....	464	21,600	14.....	264	820	22.....	165	363	30.....	10,300	798
7.....	1,030	14,909	15.....	222	798	23.....	158	283	31.....	9,400	440
8.....	730	3,000	16.....	210	1,100	24.....	141	234			
Monthly mean discharge, in second-feet.....										1,288	2,640
Runoff in inches.....										0.98	2.02

Gage height, in feet, and discharge, in second-feet, at indicated time, 1943

Hour	Gage height	Discharge	Hour	Gage height	Discharge	Hour	Gage height	Discharge
<i>July 25:</i>			<i>July 29—</i>			<i>Aug. 5—</i>		
4 a. m.	9.50	162	Con.			Con.		
8.....	9.52	172	8.....	15.10	12,800	10.....	15.14	12,800
Noon.....	9.50	162	12.....	14.87	12,200	Noon.....	15.38	13,900
2 p. m.	9.48	155	<i>July 30:</i>			6 p. m.	15.77	15,300
4.....	9.47	152	8 a. m.	14.43	10,600	12.....	16.45	17,600
6.....	9.47	152	4 p. m.	14.18	10,000	<i>Aug. 6:</i>		
8.....	9.50	162	12.....	14.01	9,400	2 a. m.	16.62	18,400
10.....	9.60	210	<i>July 31:</i>			4.....	16.98	20,000
12.....	10.20	655	Noon.....	13.95	9,400	6.....	17.18	20,800
<i>July 26:</i>			12 p. m.	13.88	9,100	8.....	17.34	21,200
2 a. m.	10.50	1,010	<i>Aug. 1:</i>			10.....	17.48	22,000
4.....	10.40	880	8 a. m.	13.58	8,230	Noon.....	17.56	22,400
6.....	10.24	697	4 p. m.	12.90	6,340	2 p. m.	17.60	22,400
8.....	10.12	581	12.....	11.90	3,810	4.....	17.62	22,400
10.....	10.08	547	<i>Aug. 2:</i>			6.....	17.60	22,400
Noon.....	10.10	563	8 a. m.	11.14	2,110	8.....	17.55	22,400
2 p. m.	10.20	655	4 p. m.	10.74	1,370	10.....	17.47	22,000
4.....	10.20	655	12.....	10.50	1,010	12.....	17.34	21,200
6.....	10.15	609	<i>Aug. 3:</i>			<i>Aug. 7:</i>		
8.....	10.08	547	Noon.....	10.32	784	8 a. m.	16.45	17,600
10.....	10.04	514	12 p. m.	10.22	676	4 p. m.	15.00	12,500
12.....	9.98	466	<i>Aug. 4:</i>			12.....	13.22	7,130
<i>July 27:</i>			4 a. m.	10.20	655	<i>Aug. 8:</i>		
Noon.....	9.76	309	8.....	10.20	655	Noon.....	11.32	2,490
12 p. m.	9.67	252	12 p. m.	10.26	718	12 p. m.	10.70	1,300
<i>July 28:</i>			2 p. m.	10.29	750	<i>Aug. 9:</i>		
4 a. m.	9.66	246	4.....	10.30	760	Noon.....	10.48	984
8.....	9.97	458	6.....	10.32	784	12 p. m.	10.33	796
Noon.....	10.40	880	8.....	10.35	820	<i>Aug. 10:</i>		
6 p. m.	10.72	1,330	10.....	10.65	1,220	Noon.....	10.22	676
12.....	11.07	1,970	12.....	11.36	2,580	12 p. m.	10.12	581
<i>July 29:</i>			<i>Aug. 5:</i>			<i>Aug. 11:</i>		
4 a. m.	12.75	5,950	2 a. m.	12.63	5,690	Noon.....	10.06	530
8.....	13.95	9,400	4.....	13.37	7,670	12 p. m.	9.98	466
Noon.....	14.68	11,500	6.....	13.90	9,100			
4 p. m.	14.98	12,500	8.....	14.65	11,200			

relation defined by current-meter measurements up to 25 feet and extended above. Gage heights used to half-tenths between 11.5 and 13.0 feet; hundredths below and tenths above these limits.

MAXIMA.—1943: Discharge, 22,400 second-feet 4 p. m. Aug. 6 (gage height 17.62 feet).

1939-42: Discharge, 40,500 second-feet Dec. 30, 1942 (gage height, 27.5 feet, from graph based on gage readings).

The flood of April 17, 1939, reached a stage of 32.25 feet, from floodmarks (discharge, about 53,000 second-feet).

LEADING CREEK NEAR GLENVILLE

LOCATION.—Lat. 38°57'45", long. 80°52'05", 200 feet upstream from Big Run, 1.4 miles above mouth, and 2¼ miles northwest of Glenville, Gilmer County. Datum of gage is 700.23 feet above mean sea level, adjustment of 1912. Auxiliary gage is at site of abandoned highway bridge at mouth 1.3 miles downstream. Datum of auxiliary gage is same as base gage.

DRAINAGE AREA.—144 square miles.

GAGE-HEIGHT RECORD.—Base staff gage read twice a day to hundredths. Graphs drawn based on all available gage readings July 5-10, 27-31, Aug. 4-7, 14-15, 27-28. Auxiliary staff gage read four times a day July 29-31 and Aug. 5-6; graphs drawn based on all available readings for these days.

DISCHARGE RECORD.—Stage-discharge relation defined by current meter-measurements up to 5 feet. Gage heights used to half-tenths between 2.5 and 4.0 feet; hundredths below and tenths above these limits. Stage-fall-discharge relation defined by current-meter measurements between 5.0 and 24.0 feet, and used during periods of backwater from Little Kanawha River, July 29-31 and Aug. 5-6.

MAXIMA.—1943: Discharge, about 3,500 second-feet 8:00 p. m. Aug. 5 (gage-height, 24.1 feet, observed at crest).

1938-42: Discharge not determined; gage height, 27.5 feet Apr. 17, 1939.

REMARKS.—Gage heights given in the paragraph on maxima are affected by backwater from Little Kanawha River.

Mean discharge, in second-feet, 1943

Day	July	August	Day	July	August	Day	July	August	Day	July	August
1	18	139	9	202	62	17	7.0	65	25	11	11
2	11	64	10	94	45	18	7.3	64	26	8.2	9.9
3	6.7	34	11	36	34	19	7.9	34	27	36	20
4	6.7	456	12	22	26	20	7.9	22	28	426	206
5	21	1,420	13	15	24	21	9.7	18	29	1,170	58
6	160	2,130	14	12	30	22	32	15	30	1,150	29
7	90	323	15	9.4	77	23	16	13	31	681	19
8	104	92	16	7.9	43	24	14	12			
Monthly mean discharge, in second-feet										142	180
Runoff, in inches										1.14	1.45

STEER CREEK NEAR GRANTSVILLE

LOCATION.—Lat. $38^{\circ}51'45''$, long. $81^{\circ}02'05''$, at highway bridge 500 feet upstream from Rush Run, 2.2 miles above mouth and 5.5 miles southeast of Grantsville, Calhoun County. Datum of gage is 678.00 feet above mean sea level, adjustment of 1912. Auxiliary gage is the water-stage recorder at Grantsville (see page 22).

DRAINAGE AREA.—166 square miles.

GAGE-HEIGHT RECORD.—Water-stage recorder graphs.

DISCHARGE RECORD.—Stage-discharge relation defined by current-meter measurements up to 6,000 second-feet and extended above. Gage heights used to half-tenths between 4.6 and 6.5 feet; hundredths below and tenths above these limits. Stage and discharge ratios of the base and auxiliary gages were used in computing discharge for periods of backwater from Little Kanawha River Aug. 5-7.

MAXIMA.—1943: Discharge, 5,580 second-feet 1 p. m. Aug. 5; gage height 18.72 feet 3 p. m. Aug. 5.

1938-42: Discharge 12,400 second-feet Apr. 16, 1939; gage height 28.15 feet Apr. 16, 1939, from graph based on gage readings.

REMARKS.—All gage heights given in the paragraph on Maxima are affected by backwater from Little Kanawha River.

Mean discharge, in second-feet, 1943

Day	July	August	Day	July	August	Day	July	August	Day	July	August
1.....	32	80	9.....	34	54	17.....	9.1	471	25.....	10	16
2.....	17	49	10.....	17	42	18.....	8.0	114	26.....	13	107
3.....	11	35	11.....	11	61	19.....	9.3	56	27.....	17	150
4.....	8.6	119	12.....	8.6	50	20.....	12	40	28.....	186	143
5.....	12	3,070	13.....	32	80	21.....	8.6	32	29.....	1,260	60
6.....	34	1,060	14.....	44	148	22.....	6.8	26	30.....	401	40
7.....	50	162	15.....	22	169	23.....	17	20	31.....	169	30
8.....	36	84	16.....	13	193	24.....	19	17			
Monthly mean discharge, in second-feet.....										81.5	219
Runoff, in inches.....										0.57	1.52

WEST FORK LITTLE KANAWHA RIVER AT ROCKSDALE

LOCATION.—Lat. $38^{\circ}50'35''$, long. $81^{\circ}13'20''$, at highway bridge about 50 feet downstream from Henrys Fork, 800 feet downstream from Rocksdale, Calhoun County, and 9 miles southwest of Grantsville. Datum of gage is 657.85 feet above mean sea level, adjustment of 1912.

DRAINAGE AREA.—205 square miles.

GAGE-HEIGHT RECORD.—Wire-weight gage read to hundredths twice a day. Graphs drawn based on all available readings July 5-9, 27-31, Aug. 3-8, 12-18, and 26-28.

DISCHARGE RECORD.—Stage-discharge relation defined by current-meter measurements up to 12,000 second-feet. Gage heights used to half-tenths between 5.8 and 7.6 feet; hundredths below and tenths above these limits.

MAXIMA.—1943: Discharge, 1,100 second-feet 3 p. m. Aug. 5 (gage height 9.72 feet from graph based on gage readings).

1928-31, 1938-42: Discharge 20,200 second-feet Apr. 16, 1939 (gage height 30.3 feet, from floodmarks), from rating curve extended above 13,000 second-feet.

Mean discharge, in second-feet, 1943

Day	July	August	Day	July	August	Day	July	August	Day	July	August
1	39	90	9	144	51	17	12	510	25	10	16
2	24	61	10	57	42	18	11	150	26	9.3	55
3	17	45	11	35	38	19	14	70	27	23	85
4	15	193	12	26	32	20	11	44	28	831	47
5	121	640	13	20	182	21	9.6	32	29	2,980	30
6	212	348	14	18	115	22	10	25	30	634	21
7	154	148	15	17	119	23	9.3	22	31	210	15
8	238	80	16	15	170	24	12	19			
Monthly mean discharge, in second-feet										192	113
Runoff, in inches										1.08	0.63

Gage height, in feet, and discharge, in second-feet, at indicated time, 1943

Hour	Gage height	Discharge	Hour	Gage height	Discharge	Hour	Gage height	Discharge
<i>July 26</i>			<i>July 30</i>			<i>Aug. 5—</i>		
1 a. m.	5.08	9.2	6 a. m.	8.75	785	Con.		
Noon	5.05	8.0	Noon	8.15	576	8	7.60	390
6 p. m.	5.09	9.6	6 p. m.	7.75	448	10	8.40	645
12	5.30	20	12	7.40	334	Noon	9.20	925
<i>July 27:</i>			<i>July 31:</i>			<i>Aug. 5—</i>		
6 a. m.	5.40	25	Noon	6.90	206	2 p. m.	9.70	1,100
Noon	5.30	20	12 p. m.	6.50	131	4	9.70	1,100
6 p. m.	5.30	20	<i>Aug. 1:</i>			6	9.35	995
12	5.55	34	Noon	6.20	90	8	9.00	855
<i>July 28:</i>			<i>Aug. 2:</i>			<i>Aug. 6:</i>		
6 a. m.	6.25	96	Noon	5.85	57	4 a. m.	7.90	478
Noon	7.50	362	12 p. m.	5.76	50	8	7.60	390
2 p. m.	8.10	543	<i>Aug. 3:</i>			Noon	7.40	334
4	8.80	785	Noon	5.68	44	6 p. m.	7.12	254
6	9.94	1,180	6 p. m.	5.65	42	12	6.90	206
8	12.05	2,050	12	5.73	47	<i>Aug. 7:</i>		
10	14.2	3,140	<i>Aug. 4:</i>			8 a. m.	6.70	166
12	13.5	2,780	4 a. m.	6.10	80	4 p. m.	6.45	124
<i>July 29:</i>			8	6.75	176	12	6.30	102
2 a. m.	13.3	2,680	Noon	7.35	320	<i>Aug. 8:</i>		
4	14.2	3,140	4 p. m.	7.30	307	Noon	6.10	80
6	15.4	3,480	8	6.80	185	12 p. m.	5.90	61
8	16.9	4,830	12	6.70	166	<i>Aug. 9:</i>		
10	16.7	4,690	<i>Aug. 5:</i>			Noon	5.77	51
Noon	15.6	3,960	2 a. m.	6.70	166	12 p. m.	5.67	43
6 p. m.	12.4	2,230	4	6.85	196			
12	9.8	1,140	6	7.15	267			

SUPPLEMENTAL RECORD.—Aug. 5, 3 p. m., gage height, 9.72 feet; discharge, 1,100 second-feet.

SOUTH FORK HUGHES RIVER AT MACFARLAN

LOCATION.—Lat. 39°04'40'', long. 81°11'25'', at highway bridge 0.4 mile east of Macfarlan, Ritchie County, 0.5 mile upstream from Dutchman Run and 1.5 miles upstream from Macfarlan Creek. Datum of gage is 635.28 feet above mean sea level, adjustment of 1912.

DRAINAGE AREA.—210 square miles.

GAGE-HEIGHT RECORD.—Wire-weight gage read to hundredths twice a day, more frequently during high water. Graphs drawn based on all available gage heights on days of fluctuating stage.

DISCHARGE RECORD.—Stage-discharge relation defined by current-meter measurements up to 6,000 second-feet and extended above on basis of slope-area measurements at 21.7 and 27.91 feet. Gage heights used to half-tenths between 4.6 and 5.4 feet; hundredths below and tenths above these limits.

MAXIMA.—1943: Discharge 11,200 second-feet at 8:45 a. m. Aug. 5 (gage height 27.91 feet, observed at crest).

1915-22, 1938-42: Discharge 11,000 second-feet Jan. 22, 1917 (gage height 25.7 feet).

A flood of unknown date, but prior to 1915, reached a stage of about 29 feet, from information by local residents (discharge about 13,000 second-feet).

Mean discharge, in second-feet, 1943

Day	July	August	Day	July	August	Day	July	August	Day	July	August
1	2.8	104	9	9.1	74	17	2.8	51	25	3.4	13
2	2.2	60	10	7.6	56	18	9.4	53	26	3.7	11
3	1.8	37	11	11	45	19	18	44	27	3.2	12
4	1.5	374	12	10	34	20	7.9	37	28	140	58
5	5.4	8,570	13	11	31	21	4.5	29	29	1,070	65
6	14	1,190	14	7.0	64	22	5.0	24	30	1,510	44
7	23	216	15	4.5	181	23	5.0	19	31	295	30
8	14	116	16	3.2	75	24	4.0	15			
Monthly mean discharge, in second-feet										104	378
Runoff, in inches										0.57	2.08

Gage height, in feet, and discharge, in second-feet, at indicated time, 1943

Hour	Gage height	Discharge	Hour	Gage height	Discharge	Hour	Gage height	Discharge
<i>July 26:</i>			<i>July 31:</i>			<i>Aug. 5—</i>		
Noon	3.12	3.7	Noon	5.00	246	Con.		
12	3.11	3.6	12	4.60	150	2 p. m.	25.90	10,100
<i>July 27:</i>			<i>Aug. 1:</i>			4		
6 a. m.	3.10	3.4	Noon	4.40	108	6	24.50	9,380
Noon	3.08	3.2	12	4.20	74	8	22.90	8,500
6 p. m.	3.06	2.9	<i>Aug. 2:</i>			10	21.10	7,500
12	3.20	5.0	Noon	4.10	60	12	18.70	6,300
<i>July 28:</i>			<i>Aug. 3:</i>			<i>Aug. 6:</i>		
6 a. m.	3.48	13	Noon	3.90	38	2 a. m.	12.50	3,200
Noon	4.20	74	6 p. m.	3.85	34	4	10.40	2,210
6 p. m.	5.00	246	12	4.28	87	6	8.40	1,390
12	6.00	532	<i>Aug. 4:</i>			8	7.50	1,060
<i>July 29:</i>			6 a. m.			10	6.90	846
6 a. m.	6.95	870	Noon	4.80	196	Noon	6.50	710
Noon	8.00	1,230	6 p. m.	5.20	300	4 p. m.	5.90	520
4 p. m.	8.10	1,270	8	5.45	370	8	5.55	430
8	7.85	1,150	12	6.00	532	12	5.30	342
12	9.20	1,710	<i>Aug. 5:</i>			<i>Aug. 7:</i>		
<i>July 30:</i>			2 a. m.			Noon		
4 a. m.	10.60	2,300	4	15.50	4,700	12 p. m.	4.80	206
8	10.00	2,030	6	23.60	8,880	<i>Aug. 8:</i>		
Noon	8.80	1,550	8	27.20	10,900	Noon	4.40	112
6 p. m.	7.40	1,010	10	27.90	11,200	12 p. m.	4.30	94
12	6.30	628	Noon	27.85	11,200	<i>Aug. 9:</i>		
				27.20	10,900	Noon	4.18	75
						12 p. m.	4.10	65

SUPPLEMENTAL RECORD.—Aug. 5, 8:45 a. m., gage height, 27.91 feet; discharge, 11,200 second-feet.

HUGHES RIVER AT CISKO

LOCATION.—Lat. 39°07'45'', long. 81°17'10'', 200 feet downstream from county footbridge at Cisco, Ritchie County, and 1 mile downstream from confluence of North and South Forks and 4½ miles south of Petroleum. Datum of gage is 605.35 feet above mean sea level, adjustment of 1912.

DRAINAGE AREA.—453 square miles.

GAGE-HEIGHT RECORD.—Staff gage read to hundredths twice a day, more frequently during high water. Graphs drawn based on gage readings on days of fluctuating stage.

DISCHARGE RECORD.—Stage-discharge relation defined by current-meter measurements up to 21 feet, extended to 27.5 feet on the basis of slope-area measure-

ments at 24.5 and 27.5 feet. Gage heights used to half-tenths between 3.5 and 4.8 feet; hundredths below and tenths above these limits.

MAXIMA.—1943: Discharge, 17,800 second-feet noon Aug. 5 (gage height, 24.5 feet, from floodmark).

1915-22, 1929-31, 1939-42: Discharge about 25,700 second-feet Jan. 22, 1917 (gage height, 30.25 feet).

Mean discharge, in second-feet, 1943

Day	July	August	Day	July	August	Day	July	August	Day	July	August
1	10	307	9	45	196	17	17	133	25	18	31
2	9.4	173	10	85	133	18	215	142	26	16	24
3	8.4	120	11	45	105	19	51	142	27	18	22
4	9.4	537	12	33	85	20	35	93	28	164	29
5	58	12,600	13	18	74	21	22	66	29	1,440	104
6	110	3,190	14	23	84	22	24	49	30	2,150	94
7	40	502	15	20	353	23	25	36	31	652	69
8	41	292	16	16	208	24	22	35			
Monthly mean discharge, in second-feet.....										175	646
Runoff, in inches.....										0.45	1.64

Gage height, in feet, and discharge, in second-feet, at indicated time, 1943

Hour	Gage height	Discharge	Hour	Gage height	Discharge	Hour	Gage height	Discharge
<i>July 26:</i>			<i>July 30:</i>			<i>Aug. 5:</i>		
1 a. m.	2.52	17	4 a. m.	10.10	2,920	2 a. m.	13.0	4,900
Noon	2.50	16	8	9.64	2,610	4	16.4	7,660
12 p. m.	2.62	22	Noon	8.90	2,200	6	19.6	10,900
<i>July 27:</i>			6 p. m.	7.90	1,700	8	23.3	15,800
Noon	2.51	16	12	6.70	1,170	10	24.2	17,200
6 p. m.	2.47	15	<i>July 31:</i>			Noon	24.5	17,800
12 p. m.	2.90	39	6 a. m.	5.60	732	2 p. m.	24.4	17,600
<i>July 28:</i>			Noon	5.20	587	4	24.2	17,200
2 a. m.	3.65	133	12 p. m.	4.74	434	6	23.0	15,300
4	4.25	277	<i>Aug. 1:</i>			8	21.4	13,100
6	4.42	322	Noon	4.35	307	10	19.7	11,000
8	4.30	292	12 p. m.	4.03	221	12	18.1	9,300
10	4.03	221	<i>Aug. 2:</i>			<i>Aug. 6:</i>		
Noon	3.82	162	Noon	3.83	173	6 a. m.	13.2	5,040
4 p. m.	3.50	108	12 p. m.	3.75	152	Noon	8.6	2,040
8	3.35	86	<i>Aug. 3:</i>			6 p. m.	6.50	1,090
12	3.95	196	Noon	3.50	108	12	5.70	770
<i>July 29:</i>			6 p. m.	3.45	100	<i>Aug. 7:</i>		
2 a. m.	4.80	450	12	3.95	196	8 a. m.	5.10	552
4	5.80	808	<i>Aug. 4:</i>			4 p. m.	4.75	434
6	6.60	1,130	6 a. m.	4.60	385	12	4.58	385
8	7.20	1,380	Noon	4.80	450	<i>Aug. 8:</i>		
10	7.80	1,650	2 p. m.	4.84	450	8 a. m.	4.36	307
Noon	7.70	1,600	4	4.88	483	4 p. m.	4.20	262
2 p. m.	7.40	1,470	6	5.00	517	12	4.10	234
4	7.50	1,520	8	5.25	587	<i>Aug. 9:</i>		
6	8.10	1,790	10	6.20	965	Noon	3.95	196
8	8.70	2,100	12	10.0	2,850	12 p. m.	3.80	162
10	9.20	2,370						
12	9.70	2,670						

SUMMARY OF FLOOD DISCHARGES

Data from all flood measurements made in the flood area are summarized in table 4. The locations of points where the discharges were measured are shown in figure 14, the number shown for each point being the corresponding number in the first column of table 4.

The areal distribution of rainfall during the storm of August 4-5 was such that the maximum amount of rainfall fell on only a small part of the drainage area above any gaging station. In no case did rainfall amounting to 4 inches or more fall on more than half the drainage area above a gaging station. As a result, the gaging-station records do not give a good indication of the peak discharges on the smaller streams whose drainage areas were wholly within the region of heavy rainfall. The general location of each peak-discharge measurement on a small stream was selected after an examination of the region and a study of the rainfall records in order to obtain representative measurements in areas where the peak runoff rates were unusually high. It is believed that the measurements showing the highest unit-runoff rates with respect to a drainage area of given size are very close to the maximum rates that occurred on drainage areas of corresponding size anywhere in the flood area.

The maximum unit discharges, in second-feet per square mile, are plotted against the drainage area in square miles in figure 15. The

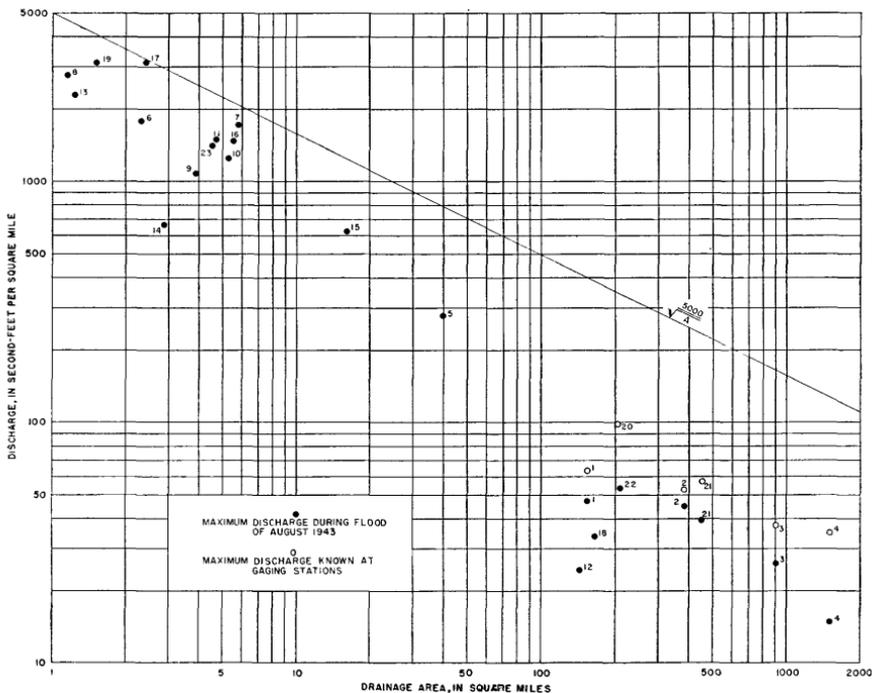


FIGURE 15.—Relation of unit discharges in Little Kanawha River basin to size of drainage basin.

numbers shown near the plotted points correspond to those in the first column of table 4 and on figure 14. At gaging stations where the flood of August 1943 was not the maximum known, the discharge for the maximum flood previously known is also plotted.

TABLE 4.—Summary of flood discharges in Little Kanawha River basin, for the flood of August 1943

No.	Stream and place of determination	Drainage area (square miles)	Period of record	Maximum flood previously known				Maximum during present flood			
				Date	Gage height (feet)	Discharge	Time	Gage height (feet)	Second-foot	Discharge	Second-foot per square mile
1	Little Kanawha River near Burnsville ¹	155	1938-42	Mar. 13, 1918	19.7	9,800	63.4	Aug. 5, 6 a. m.	17.46	7,290	47.0
2	Little Kanawha River at Glenville ¹	386	{1915-22 1928-43}	{Apr. 16, 1939 ² Apr. 17, 1939	{333.22 43.10	{20,400 34,300	{52.8 37.6	{Aug. 5, 6 p. m. Aug. 5, 10:30 p. m.	{30.73 34.95	{17,300 23,700	{44.8 26.0
3	Little Kanawha River at Palestine ¹	913	1928-43	Apr. 17, 1939	43.10	34,300	37.6	Aug. 5, 10:30 p. m.	34.95	23,700	26.0
4	Little Kanawha River at Palestine ¹	1,510	1939-43	do.	32.25	53,000	35.1	Aug. 6, 4 p. m.	17.62	22,400	14.8
5	Satlick Creek above Gem ⁴	39.8								11,000	276
6	Berry Fork at Heaters ⁴	2.30								4,100	1,780
7	Right Fork Satlick Creek at Gem ⁴	5.74								9,800	1,710
8	Hyers Run near Burnsville	1.15								3,200	2,780
9	Copen Run above Copen ⁵	3.87								4,300	1,080
10	Copen Run at Copen ⁶	5.27								6,600	1,250
11	Duskcamp Run near Stouts Mill ⁴	4.67								6,900	1,480
12	Leading Creek near Glenville ¹	1.23		Apr. 17, 1939	27.5			Aug. 5, 8 p. m.	24.1	3,500	24.3
13	Walker Fork at Flower ⁴	2.87								2,800	2,280
14	Spruce Fork near Glenville ⁴	16.1								1,900	660
15	Tanner Creek at Tanner ⁴	5.47								10,000	621
16	Trace Fork at Revere ⁴	2.42								8,000	1,460
17	Laurel Fork above White Pine ⁴	166	1938-43	Apr. 16, 1939	28.15	12,400	74.7	Aug. 5, 1 p. m.	18.72	7,400	3,080
18	Steer Creek near Grantsville ¹	1.51								5,580	33.6
19	North Fork Yellow Creek near Big Spring ⁴	205	{1928-31 1934-43}	{Apr. 16, 1939 (⁷)	{30.3 28.0	{20,200 13,000	{98.5 61.9	{Aug. 5, 3 p. m. Aug. 5, 8:45 a. m.	{9.72 27.91	{1,100 11,200	{5.4 53.3
20	West Fork Little Kanawha River at Rocksdale ¹	210	{1915-22 1938-43}	{Jan. 22, 1917 Jan. 22, 1917	{30.25 30.25	{25,700 25,700	{56.8 56.8	{Aug. 5, 12 m. Aug. 5, 12 m.	{24.5 24.5	{17,800 17,800	{39.3 39.3
21	South Fork Hughes River at Macfarlan ¹	453	1939-43							6,300	1,400
22	Hughes River at Cisko ¹	4.50									
23	Island Run at Girta ⁴										

¹ Gaging-station record.² Greater flood occurred Nov. 16, 1926. Gage height, 33.6 feet.³ Apr. 17, 1939.⁴ Slope-area method.⁵ Contracted-opening and critical-depth methods.⁶ Slope-area and contracted-opening methods.⁷ Prior to 1915.

FLOOD CRESTS

The heights reached by great floods are matters of primary importance to be considered in planning many of man's activities. Adequate knowledge of flood heights over long periods of time is essential to the solution of problems involving the design and location of structures and works in the river valleys and also to the control of floods.

Records of the stages reached by floods in recent years are available at a number of gaging stations and at several miscellaneous points where floodmarks recorded by local residents have later been correlated with sea-level datum by surveys. These data are summarized in table 5.

TABLE 5.—*Flood crests reached by major floods at points on Little Kanawha River*
[Feet above mean sea level]

Location	Miles above mouth	Elevation (in feet) of flood crests on indicated date						
		March 1913	March 1918	November 1926	January 1937	February 1939	April 1939	August 1943
Burnsville, USGS gage near	127.1		775.8	773.7		775.1	774.4	773.6
Burnsville	121.5		766.8	764.0		764.0	765.8	767.3
Gilmer—Braxton Co. line	116.6		755.8				755.2	755.9
Stouts Mill	113.5		748.8				748.8	748.7
Sand Fork	110.2		741.7	741.8			742.7	741.3
Glenville, USGS gage	103.7		730.4	731.4	722.9	727.4	731.0	728.5
Leading Creek	100.9						727.4	724.0
Dekalb	92.6		721.7	721.7				717.0
Steer Creek	83.3		701.5				701.5	694.4
Grantsville, USGS gage	78.0		695.5	692.7	686.2	689.8	695.9	687.8
Creston, USWB gage	48.7	642.1	652.5	645.4	644.5		654.9	641.4
Lock and Dam 5, upper gage	41.2	635.0	641.5	636.6	633.6		643.5	632.8
Lock and Dam 4, upper gage	30.7	621.7	628.7	622.8	625.4		629.2	619.6
Lock and Dam 3, upper gage	25.3	621.3	621.7	614.6	622.1		622.5	608.1
Lock and Dam 2, upper gage	14.9	620.9	609.9	603.6	618.4		609.6	598.4
Lock and Dam 1, upper gage	3.8	620.5	596.9	591.6	617.0		596.8	587.2
Parkersburg, USWB gage	0	620.5	596.8	588.9	616.7	595.1	596.1	575.6

¹ Highest gage reading, may not be the peak.

The flood profile in figure 16 was constructed from the data in table 5 supplemented by elevations at a number of other locations where the information was available for only one or two floods.

It is of interest to note in figure 16 that, although the flood of April 1939 was higher over a greater length of river than any other flood, the profile of recorded maximum stages, in the relatively short length of 127 miles, is made of sections of five major floods: March 1913, March 1918, November 1926, April 1939, and August 1943.

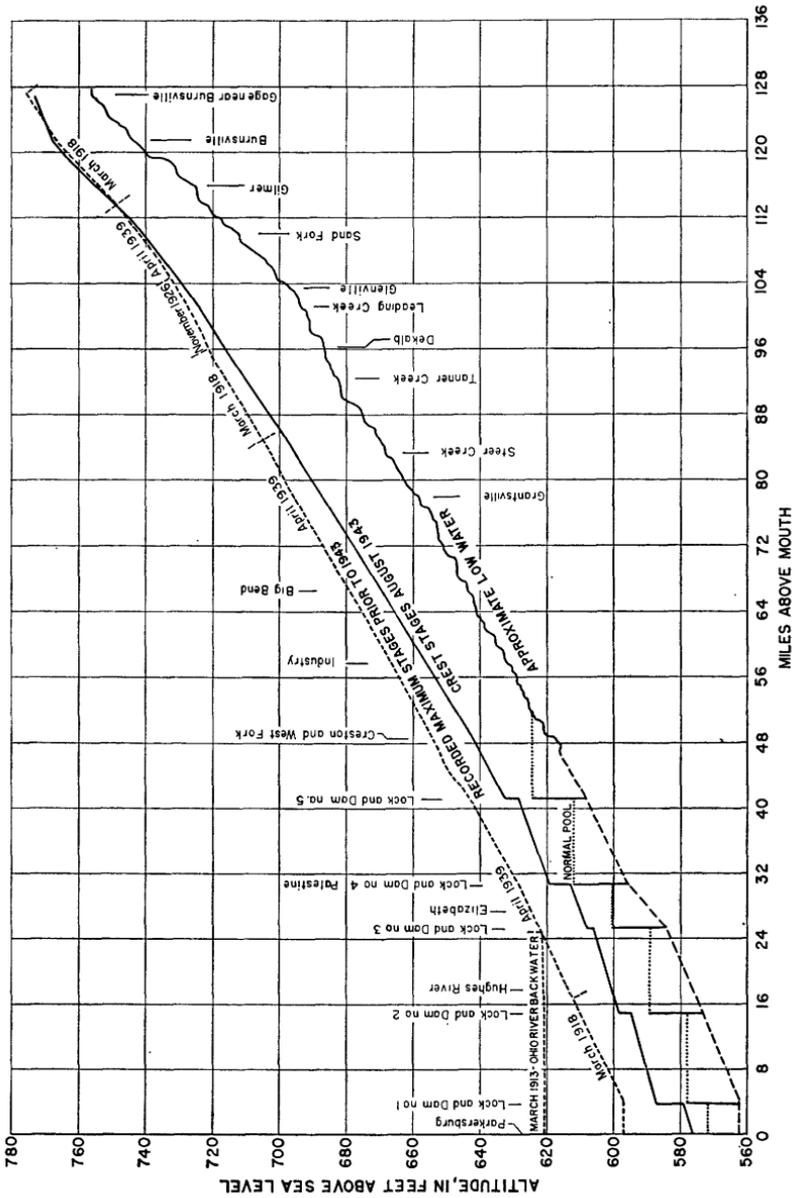


FIGURE 16.—Flood profiles of Little Kanawha River.

PREVIOUS FLOODS

FLOODS PRIOR TO APRIL 1939

Several large floods have occurred in the Little Kanawha River Valley since 1900 when systematic records were started. Fairly good information is available for most of these floods but relatively little is known about the floods prior to that time. A brief description of the floods prior to 1943, for which we have some information, is given below.

April 1852.—The meager information available indicates that this was probably the greatest flood on Little Kanawha River since white settlers first located in its valley. At Prices Ripple (mile 85) the river rose 38 feet above low water and at Dekalb the peak was 36 feet above low water. These points indicate stages approximately 2 feet higher than were reached in April 1939 in the section of river between Glenville and Grantsville.

September 1861.—The only record is one obtained from an aged resident of Burnsville, who stated that the floods of 1861 and 1875 exceeded that of 1918. No doubt this flood in the Little Kanawha River Valley was a great flood as records for Elk River show that the greatest flood known on that river occurred at this time.

August 1875.—This flood exceeded the 1918 flood at Burnsville (see above flood of September 1861); it was the greatest flood known on Middle Island Creek at Little, reaching a stage of 33.5 feet. Thus a great flood is on record on the lower reaches of the Little Kanawha River as well as in the headwaters.

February 1884.—There is no evidence of particularly high stage in the upper part of the basin during this flood but there were undoubtedly high stages in the lower section owing to backwater from the Ohio River, which reached the highest stage known prior to the great flood of 1913; the stage at Parkersburg reached 53.9 feet.

July 1888.—No record of this flood has been found, but the extremely high flood recorded in the West Fork, Tygart, and Elk River Basins indicates that a major flood probably occurred also in the upper part of the Little Kanawha Valley.

March 1913.—The lowest 23-mile stretch of Little Kanawha River reached the highest known stage as the result of backwater from the Ohio River that reached a stage of 58.9 feet at Parkersburg, the highest known stage at that place. The flood was relatively small on that part of Little Kanawha River above the effect of backwater.

The Ohio River flood of 1913 is described by Horton and Jackson (1913).

March 1918.—This was the greatest flood known in some sections of the Little Kanawha River valley between Grantsville and Glenville and above Burnsville. A rainfall of 4.8 inches in 24 hours was reported at Sutton, a few miles outside the upper end of the Little Kanawha Basin. Probably 2 to 5 inches of rain fell on the basin.

November 1926.—This was the highest flood of record at Glenville and in a reach of the river extending about 10 miles downstream. The rainfall for 1 day at Glenville was 3.50 inches. The heaviest rainfall was apparently concentrated near Glenville and in the Leading Creek area. The rise was quick and the runoff high.

January 1937.—Moderately high floods occurred all along the river as the result of general heavy rainfall. The most severe part of this flood was in the lower reaches where backwater from the Ohio River was the principal factor, as indicated by the stage of 55.4 feet recorded at Parkersburg.

The 1937 flood is described by Grover (1938).

February 1939.—This flood was the result of heavy rainfall in the headwaters of the Little Kanawha, Elk, Gauley, and Tygart River basins. A rainfall of 3.5 inches was recorded at Sutton. Rainfall was less in the middle and lower reaches of the Little Kanawha Valley resulting in a moderate flood in those sections. At the gaging station near Burnsville the stage reached was the highest recorded during the period of continuous records, exceeded, as far as is known, only by the flood of 1918.

FLOOD OF APRIL 1939

The flood of April 1939 was the greatest flood of record in sections of the river extending about 4 miles above and below Sand Fork and from the mouth of Steer Creek to Lock 3. (See fig. 16.) The maximum flood of record occurred in 70 of the 127 miles between the Burnsville gaging station and Parkersburg. In view of the magnitude of this flood (figs. 17, 18) and the records of precipitation, stage, and discharge available for the period, it is described in greater detail than the previous floods.

Rainfall during the period April 14-18 ranged from about 3 inches along the northern edge of the basin in the headwaters of the North Fork Hughes River to nearly 5 inches along the southwest edge in the headwaters of Spring Creek. The records for the precipitation stations in the basin and adjacent thereto are given in table 6. The isohyetal map (fig. 19) illustrates the areal distribution of the rainfall.



FIGURE 17.—Business section of Burnsville during flood of April 16, 1939. Stage is about 1.5 feet lower than the maximum August 5, 1943. Courtesy of Corps of Engineers.



FIGURE 18.—Part of flooded section of Glenville during flood of April 1939. Stage is about 2 feet below the crest of April 1939 flood.

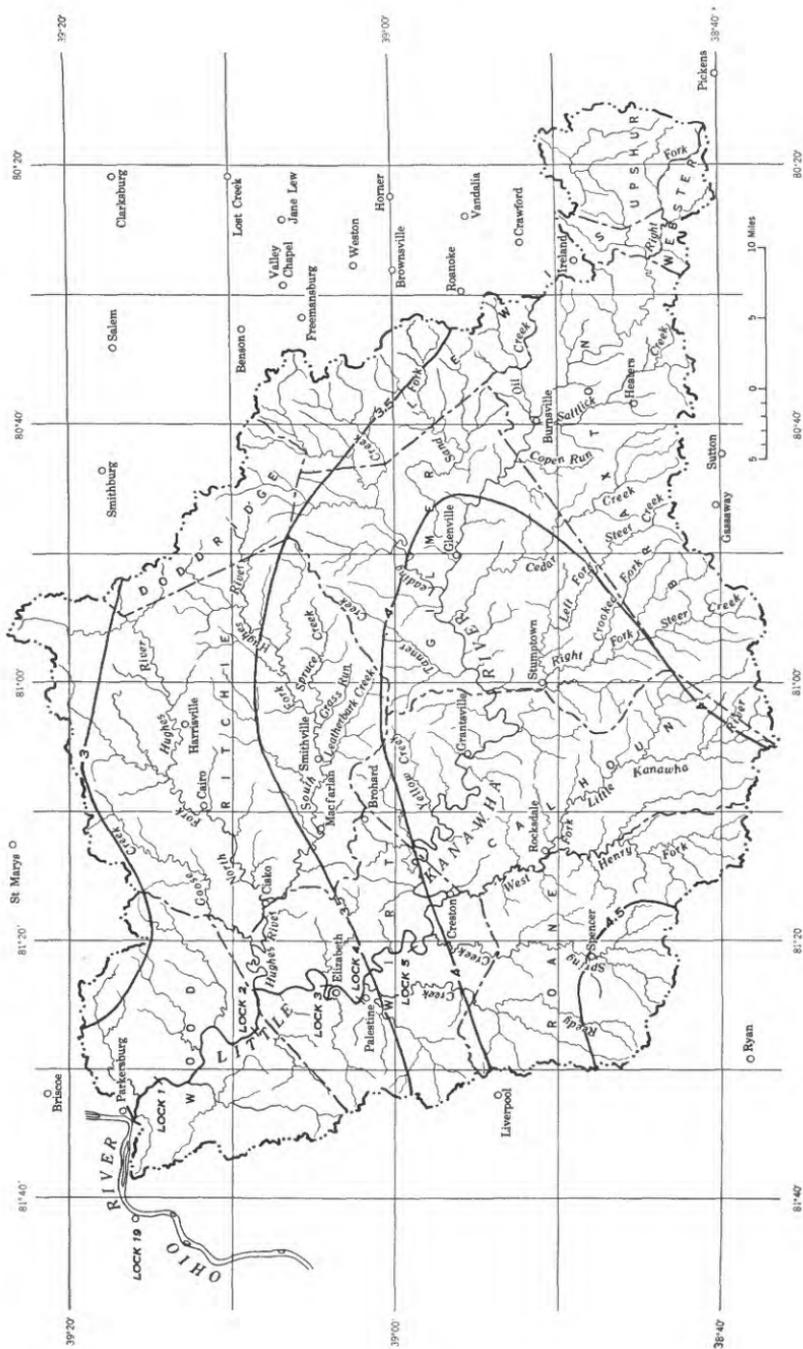


FIGURE 19.—Isohyetal map of Little Kanawha River basin showing total rainfall April 14-18, 1939.

The available recording precipitation gage records and the notes by observers at the standard precipitation gages indicate that the time during which there was rainfall was quite uniform over the basin. Figure 20 shows the hourly rainfall at recording gages in and near Little Kanawha River Basin. The gages at Valley Head and Weston are in the Monongahela River Basin a few miles from the boundary of the Little Kanawha Basin at its southeastern and central eastern edges, respectively. In general, light rain fell over the basin during the afternoon and evening of April 14. About midnight on the 14th steady rain began, continuing with only short interruptions until

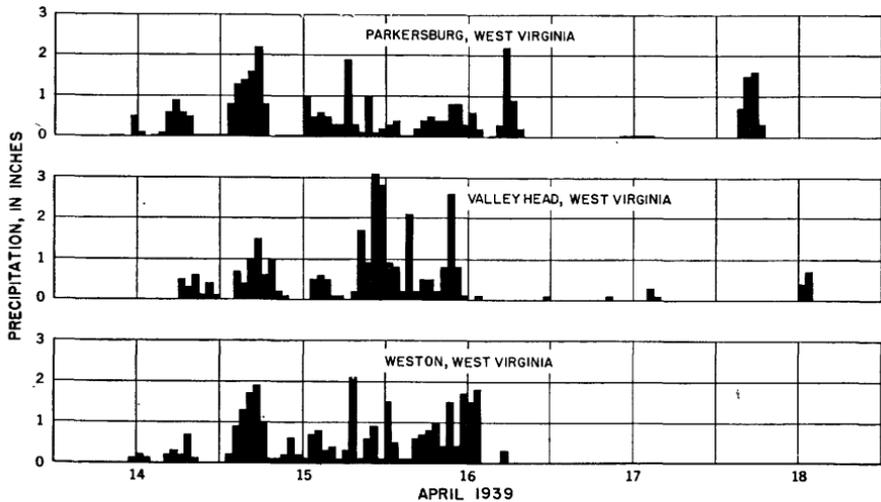


FIGURE 20.—Hourly precipitation at recording rain gages April 14-18, 1939.

the afternoon of the 16th when the period of steady rainfall ended. Light rains were reported at some stations on the 17th and 18th.

Antecedent conditions were favorable for high percentages of runoff. Precipitation at Glenville during March amounted to 5.34 inches, 1.19 inches of which fell March 28-31. An additional 0.69 inch was recorded April 23. It may be seen from table 6 that general rainfall over the basin during the period April 6-12 ranged from about 1.5 to 2 inches with some rainfall reported nearly every day.

The maximum stage and discharge, precipitation, runoff, and retention for each gaging station are summarized in table 7. The rainfall for each drainage area was determined from the isohyetal map (fig. 19) based on all rainfall records in and adjacent to the basin. The direct runoff was computed as the observed discharge during and following the storm minus the base flow. The method is illustrated in figure 21, showing the discharge graph for Grantsville.

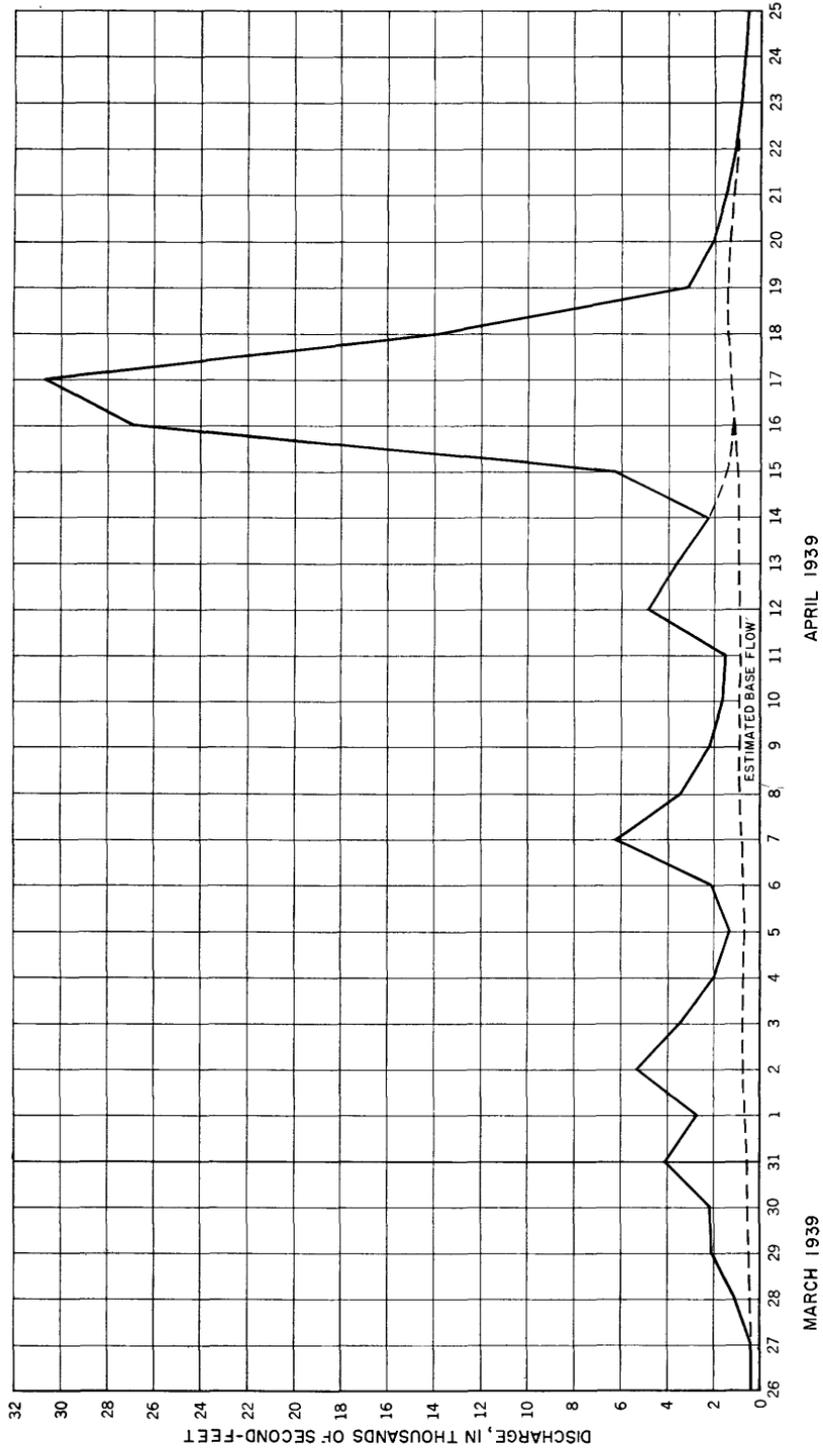


FIGURE 21.—Graph of daily mean discharge of Little Kanawha River at Grantsville, March 26 to April 25, 1939, showing estimated base flow.

TABLE 6.—Daily precipitation in inches, April 5-20, 1939

[Data from U. S. Weather Bureau]

Station	April										Storm total, Apr. 14-18					
	5	6	7	8	9	10	11	12	13	14		15	16	17	18	19
Cairo ¹	1.13	0.02	0.12	0.14	0.25	0.39	T	0.08	1.26	1.47	0.27	0.25	0.05	T	0.05	3.33
Creston ²	.45	.15	.50	.05	.12	.57	T	.40	.75	1.80	.87	.28	.10	T	.10	4.10
Elizabeth ²	.66	.40	.40	.08	.12	.40	T	.40	1.08	1.35	.45	.36	.10	.02	.10	3.24
Glenville ²	.39	.44	T	.06	.09	.71	T	.40	1.06	1.98	.86	.17	.12	.04	.12	4.07
Ireland ¹	.77	.16	.32	.06	.34	.11	T	.40	1.28	2.40	.01	.41	T	.04	T	3.69
Parkersburg ³	0.08	.94	.03	.10	.28	.04	.33	.33	1.49	.89	.01	.41	T	.04	T	3.13
Pickens ¹	.06	.10	.40	.06	.21	.02	.33	.33	1.28	2.25	.06	.25	.12	.01	.12	3.92
Spencer ²	.38	.35	.10	.06	.14	.55	.09	.14	.64	2.52	1.08	.25	.12	.01	.12	4.49
St. Marys ²	.75	.36	.09	.30	.13	.55	.41	.33	1.02	.86	.65	.35	.15	.01	.15	2.88
Sutton ¹	.70	.20	.20	.09	.13	.41	.41	.33	1.02	2.62	.65	.35	.15	.01	.15	3.70
Weston ³	.80	.10	.15	.09	.62	.08	.08	.18	1.47	1.29	.15	.15	.01	.01	.01	2.94

¹ Precipitation generally measured in late afternoon; amount recorded is for the 24 hours ending at the time of observation.

² Precipitation measured in morning; amount then recorded is for preceeding 24 hours.

³ Precipitation is for the 24-hour period midnight to midnight.

TABLE 7.—Summary of crest stage and discharge, precipitation, runoff, and retention, at gaging stations for the flood of April 15-17, 1939

Stream	Location	Drainage area (square miles)	Crest stage (feet)	Maximum discharge		Precipitation (inches)	Direct runoff (inches)	Retention (inches)
				Second-feet	Second-feet per square mile			
Little Kanawha River	Near Burnsville	155	18.3	8,680	55.0	3.65	2.65	1.0
Do.	Glenville	386	33.22	20,400	52.8	3.7	3.3	.4
Do.	Grantsville	913	43.10	34,300	37.6	3.85	3.05	.8
Do.	Palestine	1,510	32.25	53,000	34.1	4.2	3.25	.95
West Fork Little Kanawha River	Rocksdale	205	30.3	20,200	98.5	3.4	2.75	.65
Hughes River	Cisco	453	23.5	15,900	35.1	3.4	2.75	.65
South Fork Hughes River	Macfarlan	210	23.97	10,200	48.5	3.55	2.65	.90

The base flow, consisting essentially of effluent from ground-water storage, is shown by the dashed line. The area between the dashed and the solid line represents the direct runoff resulting from the storm. The method used in computing the base flow and runoff is explained in detail in previous water-supply papers (Scofield, 1938, p. 488; Youngquist and Langbein, 1941, p. 76).

The retention, computed as the difference between rainfall and runoff, ranged from 0.4 inch at Glenville to 1.0 inch near Burnsville. The average for all stations is 0.8 inch.

DISTRIBUTION AND FREQUENCY OF FLOOD EVENTS

The recorded annual maximum stage, and floods exceeding the established flood stages at Creston and Glenville, are listed in table 8 for the periods of record for the gages near Burnsville, at Glenville, at Grantsville, and at Creston.

The data collected at Glenville—43 years of gage-height record and 14 years of discharge record—appear to be best suited to illustrate the annual and seasonal distribution of past floods and the frequency with which they have occurred.

The recorded floods exceeding the flood stage of 23 feet at Glenville are shown in figure 22. Probably the relatively few minor floods recorded during the years 1901-12 is due to some extent to the fact that the practice of making special readings of gage height at or near flood crests was not generally in use for minor floods prior to 1913. It may be of some significance that no floods exceeding 30 feet were recorded prior to 1918 but during the years 1918-43 four such floods were recorded.

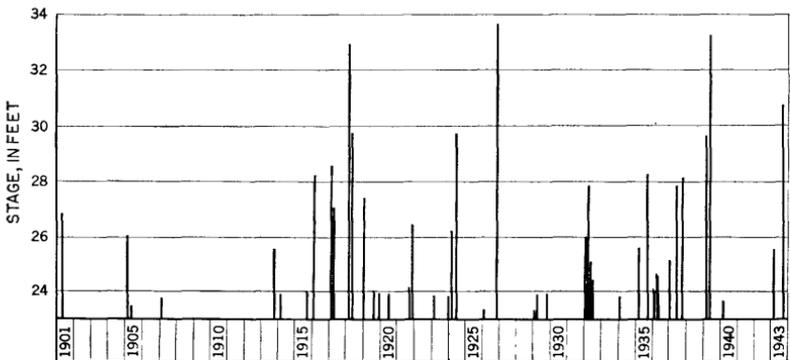


FIGURE 22.—Stages reached by floods exceeding 23 feet at Glenville, 1901-43.

TABLE 8.—Recorded flood-crest stages, in feet, at stations on Little Kanawha River.

[For annual maximum flood and other floods for which the gage height was greater than 23 feet at Glenville or 20 feet at Creston]

Year	Burnsville ¹		Glenville ²		Grantsville ³		Creston ⁴	
	Date	Gage height	Date	Gage height	Date	Gage height	Date	Gage height
1901			Apr. 4	26.8			Apr. 4	23.5
1901			Apr. 20	22.0			Apr. 20	25.8
1902			Jan. 27	16.2			Jan. 27	21.9
1902			Feb. 26	15.0			Feb. 26	21.7
1903			Feb. 28	21.1			Mar. 1	21.5
1904			Mar. 23	11.5			Mar. 24	13.0
1904							Apr. 28	16.0
1905			Feb. 9	12.5			Feb. 10	20.0
1905			Mar. 10	26.0			Mar. 10	23.5
1905			May 12	23.5			May 12	20.0
1906			Mar. 16	16.5			Mar. 16	19.5
1906			Dec. 18	17.1			Dec. 18	18.5
1907			Jan. 9	23.7			Jan. 9	16.6
1907			Jan. 13	20.9			Jan. 13	22.0
1907			Jan. 17	22.1			Jan. 18	21.2
1908			Feb. 7	6.3			Feb. 6	16.0
1908			May 10	17.5			May 11	12.8
1909			Feb. 16	14.5			Feb. 17	15.0
1909			May 1	14.5			May 1	18.3
1910			Jan. 7	17.5			Jan. 7	13.6
1910			Jan. 19	15.5			Jan. 19	16.6
1911			Jan. 30	22.6			Jan. 30	19.5
1912			Feb. 27	15.1			Feb. 27	16.0
1912			July 22	19.1			July 23	12.6
1913			Mar. 27	22.5			Mar. 28	20.4
1913			Nov. 16	25.5			Nov. 17	23.1
1914			Feb. 19	23.9			Feb. 20	22.3
1915			Oct. 1	24.0			Oct. 2	19.8
1915			Dec. 18	22.0			Dec. 19	20.7
1916			Jan. 12	28.2			Jan. 12	24.0
1916			Feb. 13	22.5			Feb. 13	20.6
1917			Jan. 22	28.5			Jan. 22	24.1
1917			Mar. 12	27.0			Mar. 12	23.2
1917			May 29	22.3			May 29	24.0
1918			Jan. 29	22.7			Jan. 29	32.0
1918	Mar. 13	19.7	Mar. 13	32.9	Mar. —	42.7	Mar. 14	32.0
1918			May 26	29.7			May 26	17.5
1919			Jan. 2	27.4			Jan. 2	24.6
1919			June 26	23.0			June 27	17.5
1919			July 17	24.0			July 17	17.0
1919			Dec. 7	23.9			Dec. 7	18.6
1920			Jan. 23	22.3			Jan. 23	19.7
1920			July 25	23.9			July 26	12.0
1921			Nov. 29	24.1			Nov. 29	20.3
1921			Dec. 24	26.4			Dec. 24	21.0
1922			Mar. 15	22.7			Mar. 16	18.3
1922			June 18	22.8			June 19	10.3
1923			Feb. 2	23.8			Feb. 2	19.4
1923			Dec. 31	23.8				
1924			Feb. 20	26.2			Jan. 1	17.8
1924			May 12	29.7			Feb. 20	21.8
1924							May 13	23.8

TABLE 8.—Recorded flood-crest stages, in feet, at stations on Little Kanawha River—
Continued

[For annual maximum flood and other floods for which the gage height was greater than 23 feet at Glenville or 20 feet at Creston]

Year	Burnsville ¹		Glenville ²		Grantsville ³		Creston ⁴	
	Date	Gage height	Date	Gage height	Date	Gage height	Date	Gage height
1925			May 12	22.5			May 12	15.4
1925			Oct. 25	22.2			Oct. 26	15.6
1926			Jan. 22	23.3			Jan. 22	17.3
1926	Nov. —	17.6	Nov. 16	33.6	Nov. —	39.9	Nov. 17	23.7
1927			Feb. 19	22.8			Feb. 20	16.8
1927			May 1	21.5			May 1	16.9
1928			Apr. 30	19.5			Apr. 30	17.5
1928			June 30	19.6			June 30	16.4
1929			Feb. 26	19.2	Feb. 26	28.4	Feb. 27	18.1
1929			Mar. 6	23.9	Mar. 5	27.2	Mar. 6	17.4
1929			Oct. 3	23.9	Oct. 3	24.7	Oct. 4	13.4
1930			Feb. 5	16.8	Feb. 5	23.7	Feb. 5	14.0
1931			Apr. 1	14.6	Apr. 2	21.9	Apr. 2	14.7
1931			Aug. 22	18.0	Aug. 22	22.8	Aug. 23	10.5
1932			Jan. 30	26.0	Jan. 30	35.8	Jan. 30	22.0
1932			Feb. 5	27.8	Feb. 5	31.2	Feb. 5	21.4
1932			Mar. 17	25.0	Mar. 16	28.9	Mar. 18	20.7
1932			Mar. 28	24.4	Mar. 28	30.7	Mar. 28	21.3
1933			Feb. 15	13.8	Feb. 15	30.4	Feb. 16	12.4
1933			Mar. 19	21.7	Mar. 19	28.3	Mar. 19	19.1
1934			Jan. 7	23.8	Jan. 7	29.4	Jan. 8	19.5
1935			Mar. 12	25.5	Mar. 12	33.5	Mar. 12	23.4
1935			Aug. 8	28.2	Aug. 8	34.7	Aug. 8	21.0
1936			Jan. 3	24.1	Jan. 3	⁶ 28.0	Jan. 3	21.3
1936			Feb. 15	24.0	Feb. 15	30.4	Feb. 15	23.9
1936			Mar. 17	24.6	Mar. 18	31.8	Mar. 17	21.4
1936			Apr. 6	23.4	Apr. 6	31.8	Apr. 6	21.9
1937			Jan. 23	25.1	Jan. 23	33.4	Jan. 23	22.8
1937			June 22	27.8	June 22	30.1	June 22	17.6
1937			Oct. 28	28.1	Oct. 29	32.1	Oct. 29	20.7
1938	May 21	11.0	May 20	21.5	May 21	30.0	May 22	19.2
1939	Feb. 3	19.0	Feb. 4	29.6	Feb. 4	37.0	Feb. 4	25.7
1939	Apr. 16	18.3	Apr. 16	33.2	Apr. 17	43.1	Apr. 17	33.2
1940	Mar. 31	9.6	Mar. 31	23.6	Mar. 31	30.3	Mar. 31	19.6
1940	Apr. 20	10.8	Apr. 20	22.5	Apr. 20	31.2	Apr. 20	21.4
1941	June 5	8.5	June 4	18.6	June 4	25.5	June 5	15.3
1942	Dec. 30	9.8	Dec. 30	22.2	Dec. 30	31.6	Dec. 30	23.8
1943	Mar. 20	11.5	Mar. 20	25.5	Mar. 20	34.2	Mar. 20	23.6
1943	July 30	17.6	July 31	20.7	July 31	21.3	July 29	12.0
1943	Aug. 5	17.5	Aug. 5	30.7	Aug. 5	35.0	Aug. 6	19.7

¹ Records from floodmarks prior to 1938, two or more gage readings daily, 1938, 1939, and water-stage recorder charts beginning Feb. 26, 1940. Gage is 4 miles upstream from Burnsville.

² Records from U. S. Weather Bureau prior to 1929 consisting of one daily reading, which was supplemented by special readings during floods beginning in 1913; Weather Bureau records and twice-daily gage readings by Geological Survey 1929-34, water-stage recorder charts beginning Dec. 14, 1934. All records reduced to the present gage datum.

³ Records from floodmarks prior to 1929; two or more gage readings daily, 1929-34; water-stage recorder charts beginning Nov. 21, 1934.

⁴ Records from U. S. Weather Bureau. Gage read once daily supplemented by special readings during floods beginning in 1913. Datum of gage is 612.71 feet above mean sea level, adjustment of 1912.

⁵ May not be the maximum for year; no record Apr. 1 to July 11, 1904.

⁶ Estimated from records at other stations.

The seasonal distribution of floods recorded at Glenville is shown in figure 23. Eleven of the 48 recorded floods exceeding 23 feet in the 43-year period occurred during March. The next highest in rates of occurrence were January with nine floods and February with seven.

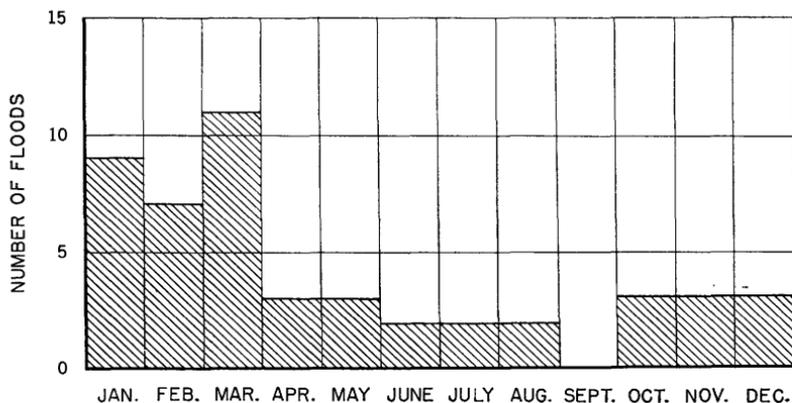


FIGURE 23.—Monthly distribution of floods exceeding 23 feet at Glenville, 1901-43.

No floods have been recorded in September but from two to three occurred in each of the other eight months of the year.

The frequency with which floods of various magnitudes have occurred above a base stage of 23 feet at Glenville during the period of record is shown in figure 24. The recurrence interval was computed

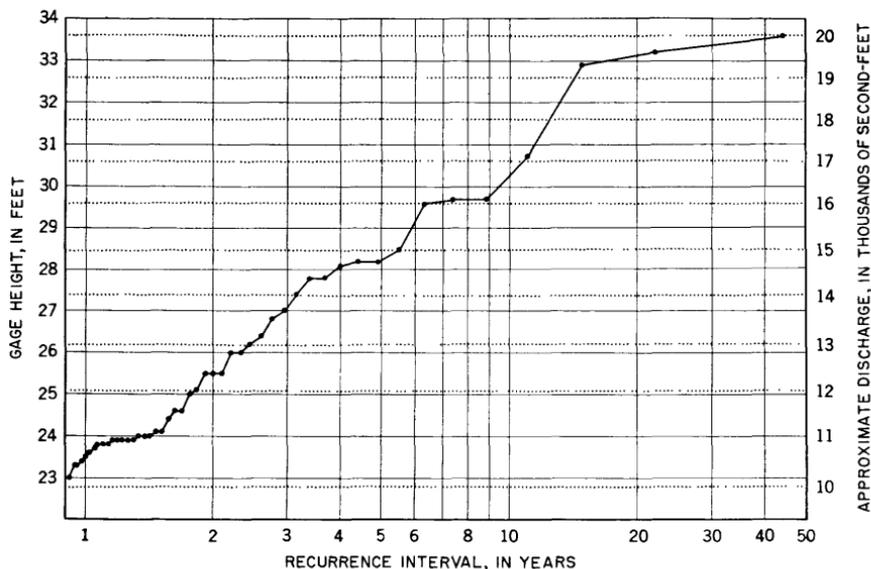


FIGURE 24.—Recurrence interval of floods on Little Kanawha River at Glenville, 1901-43.

from the formula $\frac{N+1}{M}$, in which N is the number of years of record, and M is the relative magnitude of the event beginning with the highest as one. The trend of the plotted points indicates that floods reaching a stage of 23½ feet and discharge of 10,500 second-feet have been equaled or exceeded on an average of once a year during period of record, and floods reaching or exceeding a stage of 28 feet and discharge of 14,500 second-feet have occurred on an average of once every 4 years.

Flood-frequency studies have a very important place in establishing road-bed levels, bridge elevations and clearances, channel capacities, and in other engineering and economic problems where costs of the works must be balanced against probable damages and liabilities that are a function of the frequency of flooding. Anyone attempting to use flood-frequency data should understand its limitations. The relative shortness of the record as well as deficiencies in the application of the statistical theory tend to introduce errors, particularly in the recurrence interval for larger floods.

Much has been written on the subject of flood-frequency analysis, and several methods have been summarized (Jarvis and others, 1936). A more recent approach which is viewed with favor by some students of the subject is that presented by Gumbel (1945, pp. 833-839).

SUMMARY OF FLOOD STAGES AND DISCHARGES IN WEST VIRGINIA

A summary of the known peak stages and discharges at gaging stations and at certain other points on streams in West Virginia and on its boundaries is given in table 9.

Except as otherwise noted, the discharge data given in this table are from the published reports or from the unpublished data in the files of the Geological Survey. Records of peak stage have been taken from the U. S. Weather Bureau publications in those cases where the gage-height record was collected by that agency. The records of peak stages for most of the gages at locks and dams were taken from the reports of the Corps of Engineers.

The summary includes data for practically all points on West Virginia streams where systematic records of stage and discharge have been collected for periods of a few years or more, with the exception of abandoned navigation locks and dams and certain points on Ohio River. A number of old navigation structures were aban-

doned and removed from Kanawha and Monongahela Rivers when modern locks and dams were constructed. In these cases the records are given for the present locks but not for those that were removed. Stage and discharge records for Ohio River along the border of West Virginia have been summarized only at stations where discharge records were secured. There are numerous other locations where records of stage are available. Records of flood stages for these points have been summarized in previous water-supply papers (Horton, 1913, Grover, 1937, 1938).

Different gages have been used from time to time at or near the same site at some stations. Insofar as practicable, the records have been reduced to the datum of the gage last used. The datum of the gage in terms of feet above mean sea level is given where this information is available.

The period of known floods is not necessarily the same as the period for which continuous and systematic records have been collected. In many cases records of flood heights have been extended for a number of years prior to the establishment of gages on the basis of reliable floodmarks that were pointed out by local residents.

Wherever practicable, the approximate discharge for great floods outside the period of continuous discharge record have been determined by the extension of rating curves developed in recent years. In some cases this procedure was not applicable because of changes in channel conditions. In others it would have been necessary to extend the rating curve so far beyond the limit to which it was defined that a reasonable degree of accuracy in the result could not be assured. Under these conditions the maximum known stage is given and also the peak discharge for the greatest flood during the period of continuous discharge record.

The reference numbers assigned to the points listed in table 9 may be used in finding its location on the map, plate 1.

Figure 25 shows the peak discharge in second-feet per square mile plotted against the corresponding drainage area for each point where the discharge was given in table 9. Any study attempting to arrive at a probable peak discharge for a drainage basin by comparison with this chart should take into consideration the physical characteristics of the basin such as topography, shape, soil, vegetal cover, and channel conditions, as well as the drainage area.

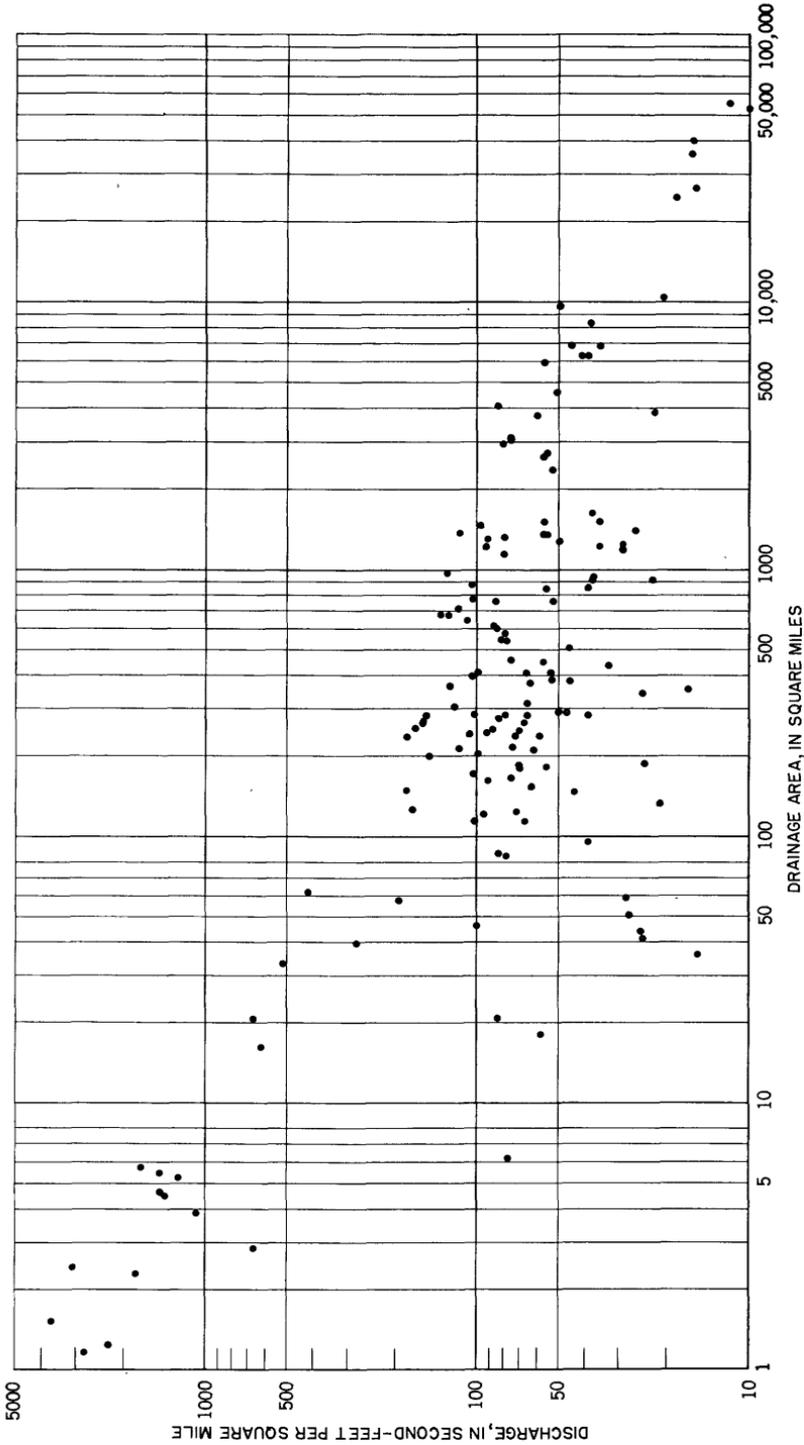
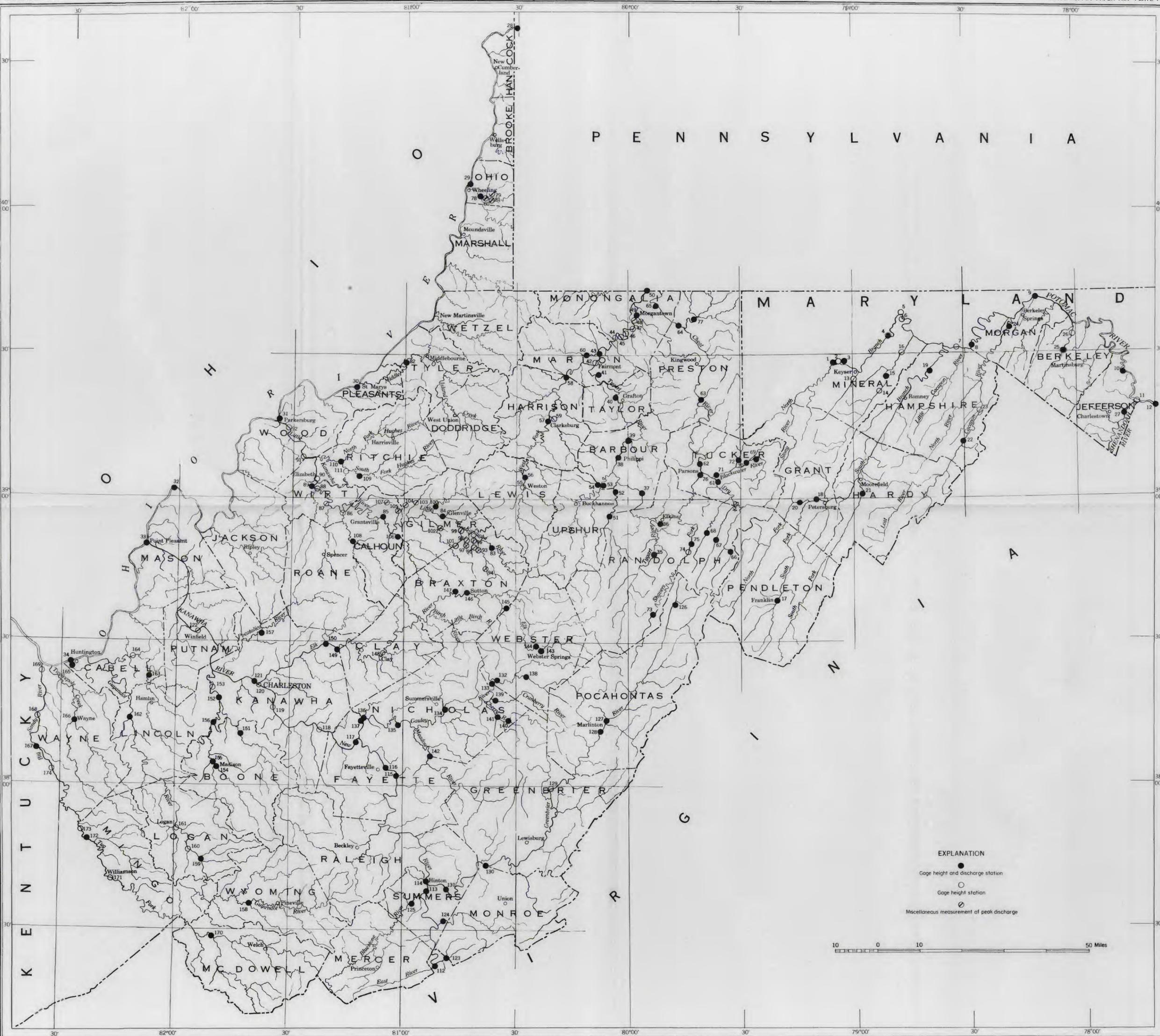


FIGURE 25.—Relation of unit discharges in table 9 to size of drainage basin.



MAP OF WEST VIRGINIA SHOWING LOCATION OF STAGES AND DISCHARGES LISTED IN TABLE 9

LITERATURE CITED

- Congressional Documents, 1935, Kanawha River, W. Va., Va., and N. C.: 74th Cong., 1st sess., H. Doc. 91.
- Corbett, D. M., and others, 1943, Stream-gaging procedure, a manual describing methods and practices of the Geological Survey: U. S. Geol. Survey Water-Supply Paper 888.
- Eisenlohr, W. S., 1951, Floods of July 18, 1942, in north-central Pennsylvania: U. S. Geol. Survey Water-Supply Paper 1134-B. (In press.)
- Grover, N. C., 1937 [1938], The floods of March 1936, part 3, Potomac, James, and upper Ohio Rivers: U. S. Geol. Survey Water-Supply Paper 800.
- 1938 [1939], Floods of Ohio and Mississippi Rivers, January-February 1937: U. S. Geol. Survey Water-Supply Paper 838.
- Gumbel, E. J., 1945, Floods, estimated by probability method: Eng. News-Record, vol. 134.
- Horton, A. H., and Jackson, H. J., 1913, The Ohio Valley flood of March-April 1913, including comparisons with some earlier floods: U. S. Geol. Survey Water-Supply Paper 334.
- Jarvis, C. S., and others, 1936, Floods in the United States, magnitude and frequency: U. S. Geol. Survey Water-Supply Paper 771.
- Scofield, C. S., 1938, Quality of water of the Rio Grande Basin above Fort Quitman, Tex.: U. S. Geol. Survey Water-Supply Paper 839.
- U. S. Weather Bureau, 1943, Monthly Weather Review.
- Youngquist, C. V., and Langbein, W. B., 1941, Flood of August 1935 in the Muskingum River Basin, Ohio: U. S. Geol. Survey Water-Supply Paper 869.

TABLE 9.—Maximum known flood stages and discharges in West Virginia

No.	Stream and place of determination	Drainage area (square miles)	Datum of gage in feet above mean sea level	Period of known floods	Peak stage and discharge			Remarks
					Date	Stage in feet	Discharge	
					Second-foot	Second-foot per square mile		
<i>Potomac River basin</i>								
1	North Branch Potomac River at Bloomington, Md.	287	1 951.98	1924-43	Mar. 29, 1924	29,000	101	Gaging-station record.
2	North Branch Potomac River at Luke, Md.	402		1899-1906, 1936	Mar. 17, 1936	40,000	99.5	Discharge by slope-area method. Flood of Mar. 19, 1924, reached a stage about 1.5 feet higher. Discharge by slope-area method.
3	North Branch Potomac River at Piedmont, W. Va.	410		1924-43	do.	40,000	98.5	Discharge by slope-area method.
4	North Branch Potomac River at Pinto, Md.	596	3 648.23	1889-1943	Mar. 29, 1924	55,000	92.3	Gaging-station record.
5	North Branch Potomac River at Cumberland, Md.	619		1889-1943	Mar. 18, 1936	53,600	86.6	Discharge by slope-area method.
6	North Branch Potomac River near Cumberland, Md.	875	3 585.22	1889-1943	June 1, 1889	89,000	102	Gaging-station record.
7	Potomac River at Okonoko, W. Va.	2,983		1936-43	Mar. 18, 1936	239,000	80.2	Discharge by slope-area method.
8	Potomac River at Paw Paw, W. Va.	3,109	3 487.88	1889-1943	do.	240,000	75.2	Gaging-station record.
9	Potomac River at Hancock, Md.	4,073	1 383.46	1889-1943	do.	340,000	83.4	Do.
10	Potomac River at Shepherdstown, W. Va.	5,936	1 281.00	1889-1943	Mar. 19, 1936	335,000	56.4	Do.
11	Potomac River at Harpers Ferry, W. Va.	9,372	4 245.53	1889-1943	do.	36,53		From U. S. Weather Bureau.
12	Potomac River at Point of Rocks, Md.	9,651	1 200.54	1889-1943	do.	41.03	49.7	Gaging-station record.
13	New Creek at Keyser, W. Va.	46.4			Mar. 17, 1936	4,600	100	Discharge by slope-area method.
14	Patterson Creek at Headsville, W. Va.	161	3 631.04	1936-43	do.	14,600	90.6	Do.
15	Patterson Creek near Headsville, W. Va.	216			do.	16,000	74	Gaging-station record.
16	Patterson Creek at Alaska, W. Va.	249			do.	17,400	69.9	Discharge by slope-area method.
17	South Branch Potomac River at Franklin, W. Va.	182	3 1,092.5	1936-43	March 1936	10,100	55.5	Discharge not determined.
18	South Branch Potomac River near Petersburg, W. Va.	642	1 692.00	1877-1943	May 16, 1942	54,000	84	Gaging-station record.
19	South Branch Potomac River near Springfield, W. Va.	1,471	1 562.02	1877-1943	Mar. 18, 1936	143,000	97.2	Do.
20	North Fork of South Branch Potomac River at Cabins, W. Va.	314	3 1,050.13	1936-43	Mar. 17, 1936	20,600	65.6	Do.
21	South Fork of South Branch Potomac River near Moorefield, W. Va.	283	3 861.51	1928-43	do.	30,400	107	Do.
22	Cacapon River at Yellow Springs, W. Va.	306	3 858.51	1936-43	Oct. 15, 1942	36,700	120	Do.
23	Cacapon River at Cacapon Bridge, W. Va.	367		1924-36	Mar. 17, 1936	46,000	125	Discharge by slope-area method.
24	Cacapon River near Great Cacapon, W. Va.	677	3 456.78	1889-1943	Mar. 18, 1936	87,600	128	Gaging-station record.

TABLE 9.—Maximum known flood stages and discharges in West Virginia—Continued

No.	Stream and place of determination	Drainage area (square miles)	Datum of gage in feet above mean sea level	Period of known floods	Peak stage and discharge			Remarks
					Date	Stage in feet	Discharge	
						Second-foot	Second-foot per square mile	
<i>Monongahela River basin—Continued</i>								
54	Big Run at Volga, W. Va.	6.19	1,387.37	1941-43	Oct. 15, 1942	473	76.5	Gaging-station record.
				1888-1943	July 1888			From Treatise on water resources, Scotland G. Highland.
55	West Fork River at Weston, W. Va.	133	4 995.6	1903-43	Apr. 16, 1939	12,600	69.6	From U. S. Weather Bureau. Gaging-station record.
56	West Fork River at Butcherville, W. Va.	181	9 993.00	1888-1943	July 1888			From Treatise on water resources, Scotland G. Highland.
57	West Fork River at Clarksburg, W. Va.	384	6 931.82	1923-43	Apr. 17, 1939	13,500	35.2	Gaging-station record.
58	West Fork River at Enterprise, W. Va.	759	3 869.45	1888-1943	do	10 21.57		Do.
59	Etik Creek near Clarksburg, W. Va.	107	11 955.01	1910-18	July 25, 1912	15		Discharge not determined.
60	Buffalo Creek at Barrackville, W. Va.	115	1 882.42	1907-08	July 1912	18	101	Gaging-station record.
61	Dry Fork at Hendricks, W. Va.	345	11 698.76	1940-43	May 16, 1942	6.75	24.4	Do.
62	Cheat River near Parsons, W. Va.	718	12 1,590.70	1888-1943	July 10, 1888	20.5	118	Do.
63	Cheat River at Rowlesburg, W. Va.	972	7 1,369.8	1844-1943	July 6, 1844	16.7	129	Do.
64	Cheat River near Pisgah, W. Va.	1,354	1 875.68	1927-43	Oct. 28, 1937	24.28	55.1	Do.
65	Cheat River near Morgantown, W. Va.	1,380	19 822.28	1888-1925	Feb. 3, 1939			Do.
66	Gandy Creek at Horton, W. Va.	36		1924-26	July 10, 1888	18.7	116	Do.
67	Laurel Fork at Wymer, W. Va.	44		1924-26	Sept. 30, 1924	2.70	550	Do.
68	Gladly Fork at Evenwood, W. Va.	41		1924-26	Oct. 25, 1925	3.30	25.0	Do.
69	Blackwater River above Beaver Creek near Davis, W. Va.	58.7		1929-32	do	3.10	24.6	Do.
70	Blackwater River at Davis, W. Va.	86.2	13 3,038.87	1921-43	Feb. 5, 1932	5.0	28.0	Do.
71	Blackwater River at Hendricks, W. Va.	148		1911-18	Mar. 29, 1924	13.2	83.2	Do.
72	North Fork Blackwater River at Douglas, W. Va.	17.9		1929-31	Mar. 12, 1917	8.37		Discharge not determined.
73	Shavers Fork at Cheat Bridge, W. Va.	57.5		1896-1926	Oct. 2, 1929	3.81	58.0	Gaging-station record.
74	Shavers Fork at Bernis, W. Va.	115		1918-25	July 22, 1906	14	191	Do.
75	Shavers Fork at Flint, W. Va.	124	13 2,407.82	1924-32	March 1918	15.3	66.8	Discharge not determined.
					May 12, 1924	11.0	71.0	Gaging-station record.
					June 20, 1928	9.31	70.2	Do.
					Feb. 4, 1932	9.54		Do.
76	Shavers Fork at Parsons, W. Va.	214	12 1,631.70	1880-1943	July 10, 1888	12.5	117	Gaging-station record. Datum of gage in use since 1941 is 1634.87 feet above mean sea level.
					July 17, 1907			

77	Big Sandy Creek at Rockville, W. Va.	1888-1943	July 10, 1888	20 to 25,000 to 30,000	150	Gaging-station record.
<i>Whedling Creek basin</i>						
78	Wheeling Creek at Elm Grove, W. Va.	1940-43	Dec. 30, 1942	13.67	78.4	Do.
79	Little Wheeling Creek above Elm Grove, W. Va.		July 10, 1937		665	Discharge by slope-area method. From Corps of Engineers, U. S. Army.
80	Little Wheeling Creek at Elm Grove, W. Va.		do		417	Discharge by slope-area method. From Corps of Engineers, U. S. Army.
81	Middle Wheeling Creek at Elm Grove, W. Va.		do		513	Do.
<i>Middle Island Creek basin</i>						
82	Middle Island Creek at Little, W. Va.	1875-1943	August 1875	33.5	74.3	Gaging-station record.
<i>Little Kanawha River basin</i>						
83	Little Kanawha River near Burnsville, W. Va.	1918-43	March 1918	19.7	63.2	Do.
84	Little Kanawha River at Glenville, W. Va.	1901-43	(Nov. 16, 1928)	33.6		Discharge not determined; probably less than peak discharge Apr. 16, 1939.
85	Little Kanawha River at Grantsville, W. Va.	1918-43	Apr. 16, 1939	33.22	20,400	Gaging-station record.
86	Little Kanawha River at Creston, W. Va.	1901-43	Apr. 17, 1939	43.10	34,300	Gaging-station record.
87	Little Kanawha River at Lock 5, W. Va.	1913-43	do	33.2		From U. S. Weather Bureau.
88	Little Kanawha River at Lock 4, W. Va.	1913-43	April 1939	26.7		From Corps of Engineers, U. S. Army.
89	Little Kanawha River at Palestine, W. Va.	1915-43	Apr. 17, 1939	26.55		Upper-pool gage.
90	Little Kanawha River at Lock 3, W. Va.	1913-43	do	32.25	53,000	From Corps of Engineers, U. S. Army.
91	Little Kanawha River at Lock 2, W. Va.	1913-43	do	32.0		Upper-pool gage.
92	Little Kanawha River at Lock 1, W. Va.	1913-43	do	40.3		From Corps of Engineers, U. S. Army.
93	Saltlick Creek above Gem, W. Va.	1913-43	March 1913	51.1		Do.
94	Berry Fork at Heaters, W. Va.	1943	Aug. 5, 1943		276	Discharge by slope-area method.
95	Right Fork Saltlick Creek at Gem, W. Va.		do		4,100	Do.
96	Hyers Run near Burnsville, W. Va.		do		9,800	Do.
97	Copen Run above Copen, W. Va.		do		2,780	Do.
98	Copen Run at Copen, W. Va.		do		1,080	Discharge by critical-depth and contracted-opening methods.
99	Duskcamp Run near Stout's Mill, W. Va.		do		1,250	Discharge by slope-area and contracted-opening methods.
100	Walker Creek near Glenville, W. Va.	1938-43	do		1,480	Discharge by slope-area method.
101	Trace Fork at Flower, W. Va.		Apr. 17, 1939	27.5		Discharge not determined.
102	Spruce Run near Glenville, W. Va.		Aug. 5, 1943		2,280	Discharge by slope-area method.
103	Tanner Creek at Tanner, W. Va.		do		1,900	Do.
104	Trace Fork at Revers, W. Va.		do		621	Do.
105	Laurel Creek above White Pine, W. Va.		do		8,000	Do.
106	Steer Creek near Grantsville, W. Va.	1938-43	Apr. 16, 1939	28.15	74.7	Gaging-station record.

Footnotes at end of table

TABLE 9.—Maximum known flood stages and discharges in West Virginia—Continued

No.	Stream and place of determination	Drainage area (square miles)	Datum of gage in feet above mean sea level	Period of known floods	Peak stage and discharge			Remarks
					Date	Stage in feet	Discharge	
						Second-foot	Second-foot per square mile	
<i>Little Kanawha River basin—Continued</i>								
107	North Fork Yellow Creek near Big Spring, W. Va.	1.51			Aug. 5, 1943.	4,700	3,100	Discharge by slope-area method.
108	West Fork Little Kanawha River at Roeksdale, W. Va.	205	1,657.85	1928-43	Apr. 16, 1939	20,200	98.5	Gaging-station record.
109	South Fork Hughes River at Macfarlan, W. Va.	210	1,635.28	1915-43	Uncertain.	13,000	61.9	Gaging-station record. Peak stage sometime prior to 1915.
110	Hughes River at Cisko, W. Va.	453	1,605.35	1915-43	Jan. 22, 1917	25,700	56.8	Gaging-station record.
111	Island Run at Girta, W. Va.	4.50			Aug. 5, 1943.	6,300	1,400	Discharge by slope-area method.
<i>Kanawha River basin</i>								
112	New River at Glenlyn, Va.	3,768	61,489.76	1927-43	Aug. 14, 1940	226,000	60.0	Gaging-station record.
113	New River near Hinton, W. Va.	4,600	61,368.49	1901-43	{ Apr. 21, 1901 May 25, 1901	294,000	50.8	Do.
			{ 41,348.2	1878-1943	Sept. 13, 1878	260,000	41.6	U. S. Weather Bureau staff gage at site of lower ferry.
114	New River at Hinton, W. Va.	6,257	61,355.18	1936-43	Aug. 15, 1940	246,000	39.3	Gaging-station record. U. S. G. S. recording gage.
115	New River at Caperton, W. Va.	6,926	6,938.44	1928-43	do	244,000	35.8	Gaging-station record.
116	New River at Fayette, W. Va.	6,860	6,838.44	1878-1943	Sept. 13, 1878	310,000	45.3	U. S. G. S. recording gage in forebay.
117	Kanawha River at Kanawha Falls, W. Va.	8,367	6,623.20	1861-1943	Sept. 14, 1878	320,000	38.2	U. S. G. S. recording gage in forebay.
118	Kanawha River at London Dam, W. Va.	8,490	{ 500.00 580.00	{ 1935-43 1935-43	{ Aug. 15, 1940 do	{ 23.3 42.9	{ 20.7 40.8	{ U. S. G. S. recording gage in forebay. U. S. G. S. recording gage in forebay.
119	Kanawha River at Marmet Dam, W. Va.	8,816	{ 560.00 560.00	{ 1935-43 1822-1943	{ do Sept. 29, 1861	{ 40.8 46.9	{ 20.7 20.7	{ U. S. G. S. recording gage in forebay. U. S. G. S. recording gage in forebay. From Corps of Engineers, U. S. Army.
120	Kanawha River at Charleston, W. Va.	8,881	6,538.87	1822-1943	Aug. 15, 1940	216,000	20.7	Gage on South Side bridge. Record- ing gage at old Look 6.
121	Kanawha River at Charleston, W. Va.	10,420	6,548.00	1939-43	{ Aug. 15, 1940 Dec. 31, 1942	{ 38.25 38.77	{ 20.7 20.7	{ U. S. G. S. recording gage in forebay. U. S. G. S. recording gage in forebay.
122	Kanawha River at Winfield Dam, W. Va.	11,810	{ 550.00 565.94	{ 1941-43 1941-43	{ Mar. 13, 1943 do	{ 1,390 4,560	{ 27.5 24.1	{ U. S. G. S. recording gage in afterbay Gaging-station record.
123	Rich Creek at Peterstown, W. Va.	50.6	61,565.94	1941-43	Mar. 13, 1943	1,390	27.5	Do.
124	Indian Creek at Indian Mills, W. Va.	189	61,472.54	1908-16	do	4,560	24.1	Do.
125	Bluestone River at Lilly, W. Va.	438	61,433.7	1929-43	{ Mar. 25, 1935 do	{ 16,000 16,000	{ 37.9 37.9	{ Do. Do.

FLOOD OF AUGUST 4-5, 1943, IN WEST VIRGINIA

126	Greenbrier River at Durbin, W. Va.	134	6 2, 699.71	1943	Mar. 13, 1943.	21.1	2,830	Do.
127	Greenbrier River at Marlinton, W. Va.	406	1908-16	1908-16	Mar. 27, 1913.	53.4	21,700	Do.
128	Greenbrier River at Buckeye, W. Va.	6 2, 085.89	1929-43	1929-43	Feb. 5, 1932.	76.9	41,500	Do.
129	Greenbrier River at Renick, W. Va.	1 853.4	1889-1943	1889-1943	Mar. 14, 1918.	57.1	77,500	From U. S. Weather Bureau in 1928.
130	Greenbrier River at Alderson, W. Va.	6 1, 529.42	1895-1943	1895-1943	Mar. 14, 1918.	37.4	60,800	Gaging-station record.
131	Greenbrier River at Hilldale, W. Va.	6 1, 388.66	1836-43	1836-43	Mar. 18, 1936.			From data furnished by U. S. Corps of Engineers.
132	Gauley River at Camden-on-Gauley, W. Va.	12 003.28	1901-43	1901-43	July 4, 1932.	180	42,500	Gaging-station record.
133	Gauley River at Allendale, W. Va.	12 003.28	1908-34	1908-34	July 4, 1932.	27.38	42,500	Do.
134	Gauley River near Summersville, W. Va.	11 880.90	1913-30	1913-30	do	169	92,000	Do.
135	Gauley River above Leander, W. Va.	1 981.17	1913-30	1913-30	Mar. 13, 1918.	135.5	92,000	Do.
136	Gauley River at Belva, W. Va.	1 669.0	1928-43	1928-43	July 5, 1932.	34.0	112,000	Do.
137	Gauley River at Belva, W. Va.	1 663.53	1928-43	1928-43	Jan. 30, 1911.	28.60	105,000	Do.
138	Williams River at Dyer, W. Va.	12 193.46	1929-43	1929-43	July 4, 1932.	19	37,000	Do.
139	Cranberry River at Woodbine, W. Va.	1 98.18	1929-43	1929-43	Nov. 18, 1929.	172	22,000	Do.
140	Cherry River at Richwood, W. Va.	1 2, 088.94	1908-16	1908-16	Oct. 1, 1915.	8.25	38.8	Do.
141	Cherry River at Fenwick, W. Va.	1 1, 869.47	1929-43	1929-43	June 27, 1940.	9.0	6,600	Do.
142	Meadow River at Naallen, W. Va.	1 1, 869.47	1928-43	1928-43	Mar. 5, 1934.	15.2	27,300	Do.
143	Elk River at Webster Springs, W. Va.	1 171	1908-16	1908-16	Jan. 29, 1911.	11.0	17,300	Do.
144	Elk River below Back Fork at Webster Springs, W. Va.	242	1929-43	1929-43	July 4, 1932.	12.98	26,000	Do.
145	Elk River at Centralia, W. Va.	6 931.89	1934-43	1934-43	Feb. 3, 1939.	14.72	18,300	Do.
146	Elk River at Sutton, W. Va.	6 807.19	1918-43	1918-43	Mar. 13, 1918.	37.2	44,000	Do.
147	Elk River at Cassaway, W. Va.	1 798.31	1908-18	1908-18	do	44	46,000	Do.
148	Elk River at Clay, W. Va.	1 670.9	1918-43	1918-43	do	32.4		From U. S. Weather Bureau.
149	Elk River at Queen Shoals, W. Va.	6 604.13	1918-43	1918-43	July 5, 1932.	39.2	91,300	Gaging-station record.
150	Elk River at Clendenin, W. Va.	1 290	1888-16	1888-16	1888.	32	64,000	Do.
151	Coal River at Ashford, W. Va.	6 622.46	1908-16	1908-16	Aug. 9, 1916.	35.0	40,700	Do.
152	Coal River at Fugs, W. Va.	1 849	1911-16	1911-16	do	36.6	47,300	Do.
153	Coal River at Tornado, W. Va.	1 576.9	1908-31	1908-31	Aug. 1916.	35.3		Discharge not determined.
154	Little Coal River at Madison, W. Va.	1 667.92	1930-41	1930-41	Nov. 18, 1929.	16.3	33,500	Gaging-station record.
155	Little Coal River at Danville, W. Va.	6 661.12	1939-43	1939-43	Feb. 3, 1939.	27.35	42,800	Do.
156	Little Coal River at McCorkie, W. Va.	1 947.91	1915-22	1915-22	do	30.2	42,800	Do.
157	Pocotalico River at Sissonville, W. Va.	1 594.56	1908-43	1908-43	Aug. 9, 1916.	28.57	24,000	Do.
					June 27, 1910.	32.4	17,000	Do.
					Apr. 16, 1939.	34.4	14,100	Datum of gage questionable.
158	Guyandot River at Baileysville, W. Va.	354	1929-31	1929-31	Nov. 16, 1929.	13.64	5,950	Gaging-station record.
159	Guyandot River at Man, W. Va.	762	1928-43	1928-43	Mar. 3, 1934.	19.11	40,000	Do.
160	Guyandot River at Wilber, W. Va.	1 170.88	1915-22	1915-22	Jan. 28, 1918.	24.8		Discharge not determined.
161	Guyandot River at Logan, W. Va.	4 639.08	1907-43	1907-43	do	27.0	43,500	From U. S. Weather Bureau.
162	Guyandot River at Branchland, W. Va.	1 947.91	1907-43	1907-43	1907.	44.0	33.5	Gaging-station record.
163	Mud River near Milton, W. Va.	1 572.64	1938-43	1938-43	Feb. 3, 1939.	28.35	14,500	Do.
164	Mud River at Yates, W. Va.	318	1915-22	1915-22	Uncertain.	23.0		Discharge not determined. Peak stage was some time prior to 1915.

Footnotes at end of table.

TABLE 9.—Maximum known flood stages and discharges in West Virginia—Continued

No.	Stream and place of determination	Drainage area (square miles)	Datum of gage in feet above mean sea level	Period of known floods	Peak stage and discharge			Remarks	
					Date	Stage in feet	Discharge		
					Second-foot	Second-foot per square mile			
165	<i>Fourpole Creek basin</i> Fourpole Creek at Huntington, W. Va.	20.9	2 520.23	1940-43	Apr. 19, 1943	7.96	1,750	83.7	Gaging-station record. Higher stages recorded as result of backwater from Ohio River.
166	<i>Twelvepole Creek basin</i> Twelvepole Creek at Wayne, W. Va.	291	1 574.92	1915-43	June 30, 1928	28.3	14,000	48.1	Gaging-station record.
167	<i>Big Sandy River basin</i> Big Sandy River at Louisa, Ky.	3,870	3 516.81	1875-1943	Apr. 3, 1908 Feb. 5, 1920	48.4	85,000	22.2	Gaging-station record. Lower gage at Lock 3.
168	Big Sandy River at Lock 2, Ky.	4,198	3 506.0	1908-43	Mar. 6, 1890 Jan. 27, 1937	51.7			From Corps of Engineers, U. S. Army. Lower-pool gage.
169	Big Sandy River at Lock 1, Ky.	4,275	3 489.6	1908-43	do	69.5			Do.
170	Tug Fork at Litwat, W. Va.	502	4 936.36	1903-43	Mar. 25, 1935	10.0	23,000	45.8	Gaging-station record.
171	Tug Fork at Williamson, W. Va.	941	4 624.6	1901-43	Jan. 24, 1918	38.1			From U. S. Weather Bureau.
172	Tug Fork near Kermit, W. Va.	1,185	4 381.82	1934-43	Feb. 3, 1939	33.9	34,400	29.0	Gaging-station record. Discharge not determined. Peak stage was some time prior to 1915.
173	Tug Fork at Kermit, W. Va.	1,240	4 574.44	1915-43	Uncertain	46.9			Gaging-station record.
174	Tug Fork at Lock 1, Ky.	1,514	3 527.6	1908-43	Jan. 29, 1918 Apr. 3, 1908	39.0 43.0	36,000	29.0	From Corps of Engineers, U. S. Army. Lower-pool gage.

1 Adjustment of 1912.

2 About.

3 From Corps of Engineers, or bench mark of that agency.

4 From U. S. Weather Bureau.

5 Sandy Hook diatum.

6 Occurred January 2, 1943.

7 Datum of 1928.

7 Baltimore & Ohio R. R. bench mark.

8 Adjustment of 1907.

9 West Virginia State Road Commission bench mark.

10 Greater flood occurred in 1888 at stage of about 33 feet.

11 City of Clarksburg bench mark.

12 Adjustment of 1903.

13 Level 6 by West Virginia Power and Transmission Co.

14 Occurred April 17, 1938.

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Floods of July 18, 1942 in North-Central Pennsylvania

By W. S. EISENLOHR, JR.

Notable Local Floods of 1942-43

GEOLOGICAL SURVEY WATER-SUPPLY PAPER 1134-B

*Prepared in cooperation with the
States of New York and Pennsylvania*



With a section on
Descriptive Details of the
Storm and Floods

By J. E. STEWART

UNITED STATES DEPARTMENT OF THE INTERIOR

Oscar L. Chapman, *Secretary*

GEOLOGICAL SURVEY

W. E. Wrather, *Director*

PREFACE

The floods of July 18, 1942, in north-central Pennsylvania were extraordinary. The enormous volume of rain that fell during the storm and produced these floods seems almost unbelievable. For that reason considerable space has been devoted to descriptions of previous historic storms and floods of the same type. The similarity of the eye-witness accounts is striking. It is hoped that this documentation will contribute to a more reliable knowledge of these extraordinarily great floods.

The field work and collection and tabulation of the basic information on stages and discharges for the flood was done in the Harrisburg, Pa., and Albany, N. Y., districts of the Surface Water Branch under the direction of J. W. Mangan and A. W. Harrington, district engineers, respectively. Hollister Johnson, hydraulic engineer, at that time assigned to the Albany district, had immediate supervision of the miscellaneous measurements of peak discharge made in that district, and reviewed most of those made in Pennsylvania. He also obtained the information about previous floods at Salamanca, N. Y. W. S. Eisenlohr, Jr., assisted in some of the measurements of flood flow and prepared the report in the Technical Coordination Branch under the general direction of R. W. Davenport, chief; he also supplied photographs for the report.

Substantial and important contributions to the collection of field information and to the report were made, as noted, by James E. Stewart, hydraulic engineer, now with the West Penn Power Co., who as a young engineer with the Geological Survey was greatly interested in floods. The great number of miscellaneous rainfall measurements are largely the result of his efforts, and his contributions to this report are gratefully acknowledged. Thanks are also due the West Penn Power Co. for making available the results of Mr. Stewart's work.

The Corps of Engineers and the Weather Bureau cooperated in the collection of meteorologic information and made it available for these reports; the Corps of Engineers also furnished the results of four peak-discharge measurements.

The stream-gaging work in the two districts of the Geological Survey was performed in cooperation with the Pennsylvania Department of Forests and Waters and the New York State Department of Public Works.

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FLOODS OF JULY 18, 1942, IN NORTH-CENTRAL PENNSYLVANIA

By WILLIAM S. EISENLOHR, JR.

ABSTRACT

The floods of July 1942 in north-central Pennsylvania and adjacent areas in New York were record-breaking on most of the smaller streams. They followed unprecedented rains that amounted to as much as 35 inches at some points during a storm that for the most part lasted less than 12 hours at any point. In the area of heavy rainfall, peak flood discharges were much greater than for the floods of March 1936. The storm was centered over the headwaters of three major drainage basins—Susquehanna, Allegheny, and Genesee—with the result that flood flows in the lower reaches of those streams were not outstanding. The estimated property damage exceeded \$10,000,000 and 15 persons lost their lives. Description of previous storms and floods show that quite similar conditions have occurred in other areas in the past. The isohyetal map in the report is based on more than 400 miscellaneous observations of rainfall. Gage heights and discharges during the flood period are given for 14 gaging stations, and peak discharges are given for 47 other points on streams in the flood area. The maximum discharge was 117,000 second-feet in West Branch Susquehanna River at Renovo, and the maximum in relation to drainage area was 2,100 second-feet per square mile from 11.4 square miles in Annin Creek near Turtle Point. The report also contains a table of flood-crest elevations.

INTRODUCTION

On July 18, 1942, north-central Pennsylvania and adjacent areas in New York were visited by destructive floods of unprecedented magnitude. The principal areas affected were in Elk, Cameron, McKean, and Potter Counties in Pennsylvania, and Cattaraugus, Allegany, and Stueben Counties in New York. (See fig. 26.) Rainfalls of more than 30 inches were reported in several localities, and on about 200 square miles the rainfall was as much as 20 inches. The rainfall was greater than 10 inches on more than 2,000 square miles.

The resulting floods produced the highest crest stages ever recorded on First Fork and Driftwood Branch of Sinnemahoning Creek and on upper Allegheny and Clarion Rivers. Flood discharges on small drainage areas—less than 100 square miles—were as great in relation to drainage area (see fig. 43) as any ever recorded on streams in Pennsylvania and adjacent areas in New York. Allegheny River at Red

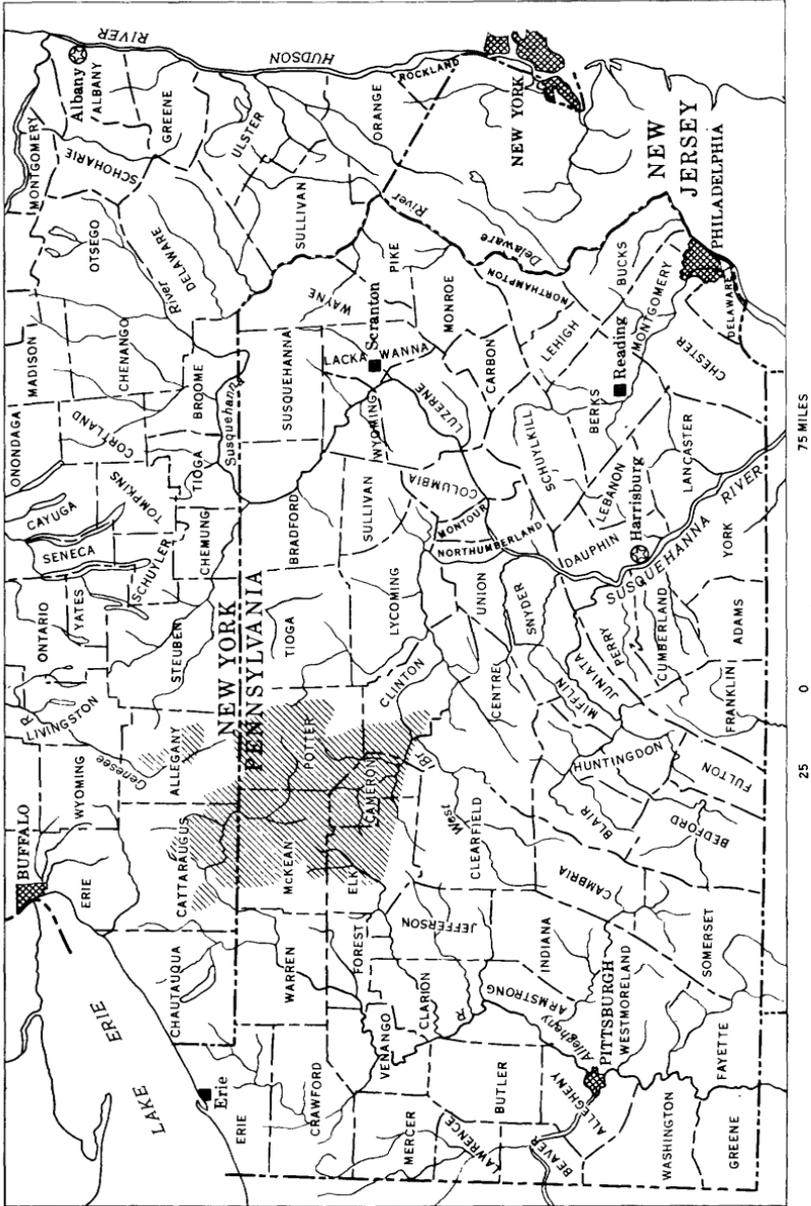


Figure 26.—Index map showing location of flood area.

House, N. Y., was nearly a foot higher than the previous maximum in 40 years of record, and at Eldred, Pa., it was more than 9 feet higher than the previous maximum in 27 years of record, with more than four times the discharge. Driftwood Branch Sinnemahoning Creek was nearly 3 feet higher than in March 1936 with more than one and one-half times the maximum discharge.

Practically all the towns along the streams were inundated to a damaging extent. The dikes at Emporium, Pa., were overtopped and about 60 percent of the town was flooded; the water was 5 feet deep in the middle of Broad Street Square. At Austin, Pa., events were reminiscent of the historic flood disaster of September 30, 1911, when the dam broke and 80 people were killed. The dam of the Williamson Pulp & Paper Co.'s reservoir—with a capacity 65,000,000 gallons at spillway crest—was breached (fig. 27), pouring floodwaters upon the



FIGURE 27.—Dam of Williamson Pulp & Paper Co., Austin, Pa., breached by flood.

town of Austin; buildings were knocked off their foundations (fig. 28) and water was 4 to 5 feet deep on the main street. Port Allegany probably suffered the greatest damage per capita of any town in the area. Virtually every business building was flooded 3 to 20 feet. More than 150 people attending a conference in the Free Methodist



FIGURE 28.—Buildings in Austin, Pa., destroyed by flood.

Church were rescued from treetops, roofs, and floating wreckage. The church was washed against a nearby silk mill; it caught fire, and both buildings—as well as others adjacent—were destroyed.

Many other towns were inundated to a considerable extent: Coudersport, Smethport, Eldred, Bradford, and Ridgway, Pa., and Portville, Olean, and Salamanca, N. Y. Above Johnsonburg, Pa., a dam broke, and a wall of water 8 feet high was reported as having swept through the town; 18 families had to be rescued with makeshift rafts, and many houses were moved off their foundations and pushed together (fig. 29). All these communities supported industries that suffered heavily, at a time when capacity production was needed for the war effort.

Public utilities and transportation systems were badly damaged. Water, gas, and electric services were disrupted in many communities. Railroads, highways, and bridges were washed out. A passenger train was marooned by the collapse of a bridge near Ridgway. It was estimated that 75 miles of Pennsylvania Railroad track was damaged. The Coudersport and Port Allegany Railroad between Roulette and Port Allegany, and the Baltimore & Ohio Railroad between Sinemahoning and Austin were so badly damaged that they were abandoned. (See figs. 30 and 31.). Over all the 27 miles of this stretch of the Baltimore & Ohio Railroad it is doubtful if a section as



FIGURE 29.—Houses in Johnsonburg, Pa., swept off their foundations and pushed together.



FIGURE 30.—Bridge of Coudersport & Port Allegany R. R. damaged by flood.



FIGURE 31.—Baltimore & Ohio R. R. tracks along First Fork Sinnemahoning Creek torn up by flood.

long as a tenth of a mile anywhere was left undamaged; at one place the track was left in the middle of the stream.

Fifteen persons lost their lives in this storm, and many others were made homeless. The damages sustained were extremely large for the size and population of the area. That they were not larger, however, is probably because three separate drainage basins were involved: the Susquehanna, flowing into Chesapeake Bay; the Allegheny, head of the Ohio River system; and the Genesee, tributary to St. Lawrence River. This distribution may explain why the floods were not severe in the lower courses of the major streams. Had the storm that produced these floods been centered over only one major drainage basin, it seems likely that the floods would have been much more severe on the larger streams. The estimated total damage, according to figures compiled by the Weather Bureau, was \$10,121,550; this was divided approximately as follows:

Railroads.....	\$3, 106, 200	Communications and power lines.....	\$123, 000
Commercial property.....	2, 050, 000	Agriculture.....	750, 000
Residential property.....	1, 650, 000	Relief.....	100, 000
Industrial property.....	1, 470, 000		
Highways.....	872, 350	Total.....	10, 121, 550

Two pictorial booklets describing the flood were published by W. H. Greenhow Co., Hornell, N. Y., and Nelsen Enterprises, Emporium, Pa.

The storm that produced these floods was of less than 24 hours' duration and at many places lasted little, if any, more than 12 hours. Rainfall measured at regular precipitation stations did not exceed 8 inches, except at Coudersport and Emporium where 8.22 and 8.10 inches, respectively, were recorded. From the magnitude of the floods, James E. Stewart, hydraulic engineer of the West Penn Power Co., concluded that rainfalls greatly in excess of these amounts must have fallen within the flood area. Being concerned over the capacity of the spillways designed for power dams of his company, Stewart visited the flood area and obtained evidence that verified his conclusions. A container was found that caught 30.8 inches of rain, and small streams everywhere had moved enormous quantities of rock. This information was brought to the attention of the Corps of Engineers, the Weather Bureau, and the Geological Survey.

At a joint conference held in Pittsburgh, it was agreed that the Corps of Engineers and the West Penn Power Co. would cooperate in a program to obtain all available precipitation data for the storms. This information would be turned over to the Weather Bureau for study in connection with the records for regular weather stations. The Geological Survey would collect additional information on flood runoff to complement the rainfall data. This program was carried out. A detailed meteorological analysis of the storm was made by the Weather Bureau, and a report was prepared as a hydrologic bulletin supplement (U. S. Weather Bureau, 1944).

As soon as field work was completed, the Geological Survey cooperated with the Pennsylvania Department of Forests and Waters in preparing and publishing a report (Pennsylvania Department of Forests and Waters, 1943) on the floods. The purpose of that report was to place in the hands of the public at the earliest practicable time the flood data then available. The present report by the Geological Survey, delayed in preparation owing partly to situations arising from the war emergency, contains additional runoff data as well as records revised since the Pennsylvania report was published. The rainfall records and an isohyetal map also are included so that all base data for rainfall-runoff studies will be contained in a single report. All clock times given are referred to eastern war time; to convert to standard time, subtract 1 hour.

DESCRIPTIVE DETAILS OF THE STORM AND FLOODSBy JAMES E. STEWART¹**DESCRIPTION OF THE STORM**

A rainstorm, unusual in the amount of precipitation, and remarkable in its other characteristics, occurred in north-central Pennsylvania and adjacent areas of New York on July 17-18, 1942. The forerunners of the storm may have arrived as early as 6 a. m., Friday, July 17, when there were short, intense showers at such recording rainfall stations as Wellsboro and Jackson Summit, Pa. Nevertheless, the first definite warning of exceptional meteorologic conditions was a 10- or 15-minute hailstorm that occurred about 3 p. m., Friday, in the region of Keating Summit, Pa., to the east of that point, and perhaps at other locations.

At 8:35 p. m., Friday, the rainstorm burst furiously near Cohocton, N. Y. From that time until 8 p. m., Saturday, it raged almost continuously but shifted from place to place and back and forth in the storm area. Generally, however, the areas of heavy precipitation gradually shifted southwestward until, in the main storm area, the last of the storm's explosive energy was expended in the vicinity of Ridgway, Pa. After 8 p. m., Saturday, in this main storm area, there was no overhead evidence of the previous tumult except for some very light, intermittent showery conditions that continued through Sunday, July 19.

The heavy storm rainfall covered a period of slightly less than 24 hours, but at no one location was there more than about 16 hours between the beginning and ending of the heavy rain; in fact, at many places this period was confined to about 14 hours. But even a 14- or 16-hour period gives a false idea of the storm's intensity, because in most locations the rainfall was concentrated in a very few periods of heavy precipitation. It is probably conservative to say that at any given spot more than 50 percent of the rain fell during high-intensity periods, in which the total elapsed time was 6 hours or less. (See fig. 36.) In fact, at many locations more than 80 percent of the rain is known to have fallen in periods of 5 hours or less.

In the area near Cohocton, N. Y., and in the main storm area west of Hornell, N. Y., the heavy rain had stopped by 7 a. m., Saturday. In the main storm area northeast of Coudersport, Pa., most of the rain fell before 7 a. m., Saturday. In much of the remaining, indeed the major part, of the storm area the deluge occurred between 7 a. m. and 1 p. m., Saturday. In a few small sections of this area, however, the rainfall before 7 a. m. was as great or nearly as great as after 7 a. m. From Emporium, Pa., westward and northwestward there was heavy

¹ Hydraulic engineer, West Penn Power Co.

rain after 1 p. m., Saturday, and from the head of Clarion River southwestward to Ridgway, Pa., most of the rain occurred after that hour.

Where the total storm rainfall is known to have exceeded 12 inches, and many times for lesser amounts, people complained of the following:

1. Cellars being filled or partly filled with water—far away from and above streams—where flood water inflow either had never occurred before or had never exceeded the cellars' drainage capacities.
2. Roofs leaking badly that had never leaked before.
3. Water coming out the stovepipe holes in the chimneys.

Generally there was wind at the start of the storm and sometimes at the end, but usually very little or no wind during the storm. Wind directions were erratic; for example, at two different locations they were in exactly opposite directions within a distance of 1 mile. Quite often the wind seemed to be up or down a hollow, irrespective of the direction of cloud travel. There were many reports of thunderstorms coming first from one direction, then returning from another; of storms that traveled perpendicular to the path of previous storms; storms that seemed to mill or circle; and of storms that seemed to come together. As far as lightning was concerned, there was uniform testimony that a much higher percentage of lightning strokes occurred between clouds rather than between cloud and ground as compared with ordinary storms.

In the Austin, Pa., region, many people said that the rain did not come down in drops, and they had the visual impression, at least, that the rain came down in streams which they likened to strings and ropes. This phenomenon appears to be a characteristic of intense cloudburst storms as described in the accounts of previous storms given later. Although it could have been an optical illusion, it seems more likely to have been an actual occurrence. Also, in that region, there were people who had not been able to reach shelter and were afraid they would drown during the downpour through lack of oxygen. One man expressed the effect by saying it was just like breaking water after a long dive, lungs bursting, with water pouring down over his head.

The observer who recorded more than 30.8 inches of rain in $4\frac{3}{4}$ hours stated that it seemed to fall at a tremendous rate, but quite uniformly, for the greater part of the time. Also, the drops seemed to be exceptionally large and very close together. From her statement and the record of total rainfall at that point, it may be assumed that the rainfall at no time exceeded a rate of about 10 inches per hour and that there was no "streaming" for that rate and for that size of drop.

On the other hand, there were regions, particularly around Austin, where the rainfall rate for very short periods ranged from 15 to nearly 40 inches per hour. For such high rates of rainfall it would appear that the drops would be so close together they would tend to coalesce

into streams and sheets as a result of mutual mass attraction. If streaming actually occurs at extremely high rainfall rates, then the question arises as to its effect on rainfall catch and the deductions therefrom. It would seem that such streams of water, although moving about, might result in the catch at any given point being materially different from one only a few feet away.

PREVIOUS EXTRAORDINARY STORMS

STORM OF JULY 26, 1819

Storms of the intensity of the one that occurred in July 1942 in Pennsylvania are extremely rare. That this storm was not unique, however, is shown by the following quotations from reports of apparently similar storms that have occurred in the past. The earliest record is the account by Dwight (1822) of the storm of July 26, 1819, at Catskill, N. Y., from which the following quotations are taken:

About half past five [p. m.] another dense and black cloud accompanied by a fresh wind arose from the southwest. * * * About the same time, or immediately after, a very thick and dark cloud rose up rapidly from the northeast. They met immediately over the town. At this instant a powerful rain commenced. The air soon became so obscure, that trees and buildings, and other large objects, could not be discerned at the distance of a few yards. The obscurity did not appear to arise from a fog, of the usual kind; but from the abundance of the rain and the low descent of the clouds, which appeared to rest upon the ground or to hang a little above it. After the clouds met, the wind became very variable, and blew for short periods from almost every point of the compass. At times it came with so much force as to drive the rain in a very unusual manner, through the crevices in doors and windows, and the roofs of dwelling houses. Many houses which had never before been known to leak at this time admitted great quantities of water. In several instances the wind suddenly abated, and a calm of a few minutes ensued. The lightning and thunder were unusually severe. The thunder frequently resembled a violent crash, and was as sudden and of as short continuance as the sound occasioned by the firing of a cannon, or by the snapping of a whip. The rain descended at times in very large drops; and at times in streams, and sheets.

During the storm four or five intermissions each of about 8 or 10 minutes occurred, also in the rain. In each instance it excited a hope that the storm was approaching its termination, but this hope was soon dissipated by the appearance of fresh torrents. The extreme violence of the rain terminated before half past six o'clock, though it continued to descend with considerable briskness until about nine; and moderately until about ten, and it did not entirely cease until about eleven. The quantity which fell from the commencement to the termination of the storm is difficult to ascertain with exactness. It seems probably from the facts hereinafter mentioned, that it exceeded 15 inches on a level. Some remarkable phenomena occurred in various places.

At the Point, just before the clouds met, two sloops were observed sailing before the wind, under a full press of sail, one sailing rapidly up stream, and the other more rapidly down. They met near the north end of the island, when the north-east wind prevailed.

Further on, Dwight gives the account of

a gentleman who * * * observed the phenomena of the storm with more exactness than any other person with whom I have conversed. His account is as follows: * * * "The descent of rain was most copious between a quarter before 6 o'clock, and a quarter after 6. In this half hour he estimates the descent of water to have exceeded 12 inches upon a level."

Dwight continues:

The whole quantity of water which fell at the Point, is estimated to have exceeded 15 inches upon a level. * * * Should we then estimate the whole tract, on which the rain descended with peculiar violence, and in quantities never before known, in this section of the country, since its first settlement at 80 square miles, we probably should not be very wide from the truth; and on this tract, I am persuaded that the water fell full 15 inches upon a level. On a considerable part of the tract there is reason to believe that the quantity exceeded 18 inches."

STORM OF AUGUST 5, 1843

Another storm of similar magnitude occurred August 5, 1843, in Delaware County, Pa. That storm was the subject of a report by a special committee of the Delaware County Institute of Science (1910 pp. 7-18), from which the following descriptions are taken:

No general description of the *heavy rain* which * * * caused the inundation, will exactly apply to any two neighborhoods—much less to the whole extent of the county. In the time of its commencement and termination—in the quantity of rain which fell—in the violence and direction of the wind, there was a remarkable want of correspondence between different parts of the county. It may be observed, however, that comparatively little rain fell along its southern and south-eastern borders. * * *

In those sections of the county where its greatest violence was expended, the character of the storm more nearly accorded with that of a tropical hurricane, than with anything which appertained to this region of country. The clouds wore an unusually dark and lowering appearance, of which the whole atmosphere appeared in some degree to partake, which circumstance, no doubt, gave that peculiarly vivid appearance to the incessant flashes of lightning which was observed by every one. The peals of thunder were loud and almost continuous. The clouds appeared to approach from different directions, and to concentrate at a point not very distant from the zenith of the beholder. In many places there was but very little wind, the rain appearing to fall in nearly perpendicular streams; at other places it blew a stiff breeze, first from the east or northeast; and suddenly shifting to the southwest; while at a few points it blew in sudden gusts with great violence, accompanied with whirlwinds, which twisted off and prostrated large trees, and swept every thing before it. * * *

As observed by Joel Evans, at his residence in Springfield * * * the heaviest rain fell between five and six o'clock. The direction of the wind during the day, until the heavy fall of rain commenced, being generally from the S. E., though at some periods throughout the day it was variable, shifting from E. S. E., to S. S. E. The atmosphere at a considerable height above the earth's surface, appeared to be in a very unsettled and agitated state, from 12 o'clock, M. to 5 o'clock P. M., which was indicated by contrary and opposite currents of wind prevailing, carrying with them light clouds, which he observed several times in the afternoon; he being induced to go out to make observations on the state of the

weather, from its very unusual and threatening appearance. During the fall of the very heavy rain, and as nearly as he can recollect, about half past five o'clock, the wind suddenly commenced blowing with great force from the east, which soon increased to a violent gale, prostrating fences, and some trees in its course. Its velocity was such that with the immense quantity of water falling (which it carried with it in one continuous sheet, as it were), rendered it impossible to see a distance of more than fifty yards. After blowing in this way for fifteen or twenty minutes, the wind almost as suddenly veered to S. W. (nearly the opposite point of the compass), and for a short time (perhaps not more than from five to ten minutes) blew with equal violence, leveling in that direction on his farm, a number of panels of fence, and one or two apple trees. The wind subsided about six o'clock or very soon after, and was succeeded by a calm. * * *

As observed by Professor John F. Frazer * * * in the upper end of Chester township, the heavy rain commenced late in the afternoon, about half past five or six o'clock, and continued perhaps half an hour or more. During the rain there was no wind, the streams (for it fell more in streams than in drops) were, apparently quite vertical. Professor F. was unprepared to measure the quantity of rain which fell, but it exceeded anything which he had ever witnessed. * * *

Mr. Joseph Edwards, who resides in Middletown township, within half a mile of the center of the county, observed a phenomenon during the last heavy shower of rain, which does not appear to have been noticed in any other part of the county. He remarks that during the last shower which continued, say twenty minutes, and in which there fell a greater quantity of water than during any equal space of time during the afternoon—unlike any other shower he had witnessed—the distant woods and other objects were not obscured in any sensible degree by the falling rain. This extraordinary appearance was a subject of remark by all present, and created considerable surprise. At the time there was an impending mass of dense clouds, without any apparent motion in the air. This particular shower approached from the south, unaccompanied by wind.

STORM OF JULY 5, 1939

More recently a similar storm occurred July 5, 1939, in eastern Kentucky, which has been described in a special report (Schrader, 1945):

The characteristics of the storm are well defined because its unusual aspects attracted the attention of many people. Although generally considered as a single storm, it actually consisted of a series of thunderstorms accompanied by almost continuous lightning and thunder, which were noticeable for a considerable time before the storm. The lightning was described as continuous lightning and sheet lightning and by several observers as the most persistent they had ever witnessed. The thunder preceding the storm was a low rumble and at the height of the storm is reported to have shaken the earth. The lightning was so continuous that, despite the fact that the storm occurred at night in most localities, the cloud formations could be viewed without difficulty in what is described as a purplish hue. Although in a turbulent state, these cloud formations had a distinct outline and could be seen approaching from the north at a rapid rate.

On the edge of the storm area winds reached gale proportions such that buildings were damaged and in several places were entirely destroyed. Reports indicate that there was relatively little wind outside of the storm area, and observers at the storm centers reported that little or no wind accompanied the rainfall. According to information obtained from weather stations, the prevailing direction of

the wind was to the southwest on the northeast side of the storm area, to the northeast on the southwest side, and variable on the northwest and southeast sides. * * *

The rates of precipitation produced by this storm were extremely high and exceeded any known to have occurred previously in the region affected. The rainfall is described by observers as having been so intense that it was impossible to see objects only a few feet away. People caught out in the storm were compelled to stop before reaching shelter because of the lack of visibility. Numerous accounts were given of runoff that covered the sloping ground in sheets. Although the rain did not fall at a continuous high rate throughout the storm, high rates apparently were maintained in some localities for periods as long as an hour. These periods of sustained intensity occurred near the end of the storm. During such periods the rainfall was alternately in the form of large drops and of continuous sheets of water similar to those coming from the eaves of a roof.

The remarkable similarity in the descriptions of each of these storms is most striking. It indicates that they were all of the same general type.

COLLECTION OF RAINFALL RECORDS

Radio and newspaper reports of the storm of July 17-18, 1942, were exceedingly interesting. Accordingly, as rapidly as they became available, the official records of rainfall were studied. Although those records indicated rainfalls of as much as 8 inches, it did not appear reasonable that even that considerable amount of rain would have sufficed to cause the flood and resulting damage, particularly as it followed a drought. Therefore, the writer asked for and received permission from the West Penn Power Co. to make a preliminary investigation of the rainfalls that had occurred. A field trip was made August 5 to 9, inclusive, and rainfall records were obtained from about 50 amateurs. Among these was one reliable overflow record showing more than 30.8 inches of rainfall. That record, along with many other overflows and three total-storm records of 17.5, 18.5, and 19.0 inches, respectively, indicated that the storm had been one of major intensity and that its rainfall warranted a thorough investigation. On another trip to the storm district August 15 and 16 the writer obtained 23 more records.

Meanwhile in Pittsburgh the original findings were called to the attention of the Weather Bureau, Geological Survey, and Corps of Engineers. It was suggested that interested agencies should meet and plan a thorough field and office rainfall-investigation program that would avoid duplication of work. Also, that other hydrologic phases of the storm and storm damage, already under investigation by one or more of these Federal bureaus, be discussed at the meeting and coordinated with the rainfall program. This suggestion met with approval.

A meeting was held on August 17 in Pittsburgh with representatives of the interested agencies present. Those attending the meeting came to unanimous conclusions as follows:

1. All the subsequent runoff determinations should be made by the Geological Survey, but a close liaison should be established between the Survey and the collectors of precipitation data so that good and sufficient field data on rainfall and runoff would become available for correlation studies.

2. All available miscellaneous precipitation records should be collected by the Corps of Engineers and the West Penn Power Co. through a cooperative and systematic coverage of the storm area.

3. All rainfall data should be pooled and finally turned over to the Weather Bureau.

The Corps of Engineers and the West Penn Power Co. thoroughly searched the main storm area for rainfall records, except for a small region west of Hornell, N. Y., which was only partly covered owing to bad winter weather. The rainfall records obtained through this further investigation, with regular and miscellaneous records previously obtained, resulted in nearly 500 records of precipitation being made available for the storm study.

It is worth while to point out to those who may sometime be interested in collecting miscellaneous rainfall records that the country, rather than the city, is the place to obtain records. This is because the farmer often has empty containers set out in the open, whereas the city inhabitant rarely does. In northern Pennsylvania a record was probably obtained for every 12 or 15 contacts in the country, whereas there might not have been obtained 1 in 200 or 300 contacts in the towns and cities.

It was found necessary to visit every farmer in the storm area, inasmuch as apparently one farmer rarely knew of a record that his neighbor had. In fact, often within a family only one person might know of the record. This was due not to reticence, but to lack of importance of the record to the observer.

At first, no data were obtained except as to the actual rainfall, but later a form was gradually evolved which brought in much pertinent information, such as location of container, direction of wind and clouds, presence or absence of hail and sheet runoff, beginning and ending times for rain periods, time any adjacent stream was highest, etc. A copy of the questionnaire in its final form is shown on figure 32. They were filled out through personal interviews.

The more than 400 records of rainfall thus obtained were used to draw the isohyetal map contained in this report. If the precipitation had not been so great—causing overflows—and the distribution had been somewhat better, it would have been an excellent isohyetal map.

STORM OF JULY 17-18, 1942, IN NORTHERN PENNSYLVANIA

1. Hour and date this questionnaire was made out?
2. Auto mileage and on what stream, or distance and direction to a map location?
3. Did you have out any empty bucket, tub, washboiler, milk can, jar, oil drum, or watering tank during the storm? If so, what was it?
4. Size of container?
5. Location of container and distance from nearest obstacle?
6. Depth of water in container?
7. Was this depth of water for all of storm or only part? If only part, what part?
8. Did container overflow?
9. If it overflowed, how much additional rain is estimated to have fallen?
10. Approximate time that heavy rain began Friday or Friday night and ended Saturday. Also times for any periods that rain was stopped or nearly so?

11. Direction of both storm movement (clouds) and ground wind, if any, at beginning, during, and at end of storm.

12. Was there any hail before or during the storm, and if so when?
13. Time and length of period minor streams were highest on Saturday?
14. Did side-hill streams carry stones, and if so when?
15. Describe electrical features of storm.
16. Describe effects of heavy rainfall on roofs, cellars, chimneys, etc.
17. What was the appearance of the water flowing down the hillsides?
18. Name and Post Office address of person furnishing data?
19. Remarks, estimated accuracy of observations, reliability of observer, etc. Use back of sheet if necessary.

20. **Depth of rainfall.**

FIGURE 32.—Questionnaire used for recording miscellaneous rainfall information.

As it is, the map should represent fairly well the rainfall centers and isohyets. There undoubtedly are areas of low rainfall not found, however, because they are in a large and practically uninhabited forest reserve in one section of the storm area. A preliminary isohyetal map

made in the offices of the West Penn Power Co. involved months of work in drawing and redrawing isohyets and in reviewing partial records and other data. The preliminary map was used in the drawing of the map prepared by the United States Weather Bureau.

FLOOD WAVES

Where the total precipitation was greatest over considerable areas, the intense rains started about 8 a. m. Saturday. They fell on ground completely saturated by 4 inches or more of rain that had fallen the previous night. Not only was the ground saturated, but the streams were very high and in some cases far out of their banks. With such high stages, the streams were overwhelmed by the sudden inrush of water from the tremendous Saturday forenoon rain. As a consequence, flood waves from 1 to 3 feet in height formed on top of the previous flood waters of many streams. These waves swept the full length of streams having drainage areas of less than 100 square miles.

Near Port Allegany and north therefrom to the New York State line, a single rolling wave about 1 to 3 feet high probably occurred in the fairly large streams: Sartwell, Lillibridge, Twomile, Annin, and Newell Creeks, Barden Brook, and Rock, McCrea, Kings, and Bells Runs. Furthermore, for most of these streams the maximum stage set by the wave was at least maintained, and in many cases increased somewhat, during the period of heavy rain.

Where the total rainfall was the greatest in the Port Allegany region, the larger streams seemed to remain near the maximum stage for hours. In some cases, even the outpouring of the small hillside gulches seemed to be at a tremendous but fairly steady rate. On the other hand, near the edges of the heaviest rainfall areas, where there were definite intervals between heavy downpours, there were also definite recessions between flood peaks for small streams such as Taylor Brook—a tributary of Bells Run—and Champion Hollow—a tributary of Kings Run.

Many streams in other sections also had flood waves: Freeman Run, Dexter Run, East Branch Clarion River, Straight Creek, South Fork Straight Creek, and upper Portage Creek. There probably were other streams on which the flood waves were unrecorded. Particularly is this true of smaller streams in the State Forest which flow into First Fork Sinnemahoning Creek and the headwaters of Sinnemahoning Portage. On East Branch Clarion River at Glenn Hazel, where the drainage area is about 80 square miles, there were three rolling waves, each apparently about 1½ feet high and about 100 yards apart. The waves on this particular stream were a result of intense rains Saturday afternoon.

DEBRIS MOVEMENTS, BLOW-OUTS AND SLIDES

The tremendous rainfalls concentrated into a short period resulted in certain physical and geologic phenomena. For example, all steep-sloped streams did a great deal of eroding and carried along large quantities of detritus. The streams ranged from those in small gulches, with drainage areas less than 0.01 square mile, to mountain rivers, such as First Fork Sinnemahoning Creek, draining several hundred square miles—Allegheny River and some of its tributaries in their middle and lower reaches, such as Oswayo, Potato, and Portage Creeks, are not considered steep-sloped streams. Tremendous quantities of rock and gravel poured out from small gulches and streams with a few square miles of drainage area. From the small gulches these outpourings lodged on the alluvial cone at the mouth of each gulch, whereas along the small streams most of the material was carried to a point near their mouths where backwater from the main stream caused the material to deposit. The largest deposits noted were at the mouths of streams tributary to First Fork Sinnemahoning Creek near Wharton.

In the regions of exceptionally heavy rainfall around Port Allegany, particularly heavy erosion occurred in small side gulches where there was less than a square mile of drainage area. As a consequence, there were tremendous outflows of rock in proportion to the drainage area, but in total volume, outflows could not compare with those from the small streams in First Fork Sinnemahoning Creek district where the slopes and drainage areas were greater.

The erosion and rock movement caused by the flood of July 18, 1942, substantiates the belief that, instead of long time attrition, infrequent but tremendous floods are the principal eroding agents where steep topography, bedrock, and rock detritus are involved. During this flood, peak flows of 100 to 500 second-feet plunged down small gulches where there were irregular slopes of 10 percent or more. Vivid evidence that great forces had been at work was found in small gulches near Port Allegany. There, apparently great quantities of actual bedrock had been ripped out and carried down to the alluvial cones and deposited (fig. 33).

The same process probably occurred on a smaller scale in the larger streams. For example, on First Fork Sinnemahoning Creek there was considerable evidence that during the flood all the channel material down to bedrock had been in motion and in many places the channel itself had shifted. This is the usual process whereby streams erode their channel through rock. Many streams of relatively flat gradient often are not considered as lowering their channels because floods of sufficient magnitude to do the cutting are so extremely rare. That tremendous but infrequent floods are the princi-



FIGURE 33.—Upstream end of debris cone on Taylor farm, formed during flood. Note that stream channel is dry. Photographed August 26, 1942.

pal land-leveling agent is supported by the appearance of the old alluvial cones at the mouth of small gulches.

Inspection of the trenches cut in those old cones by the flood of July 18, 1942, clearly showed that they consisted mainly of great masses of broken-up bedrock, embeded in soil, that could have been brought down only by tremendous floods of the past. However, from the first settlement of that territory until July 18, 1942, the rock detritus cones were covered with grass and soil and a little rivulet coursed across each one in rainy weather; hence they gave no hint to the settler as to their underlying significance. In fact, they blended in with the general topography to such a degree that their true nature would have been apparent only to the trained eye of a geologist. Of course, when trails or roads were built along the valleys of the main streams, those early highways went up and over the alluvial cones. Naturally, the settlers built their homes on the cones in preference to the hillsides between the gulches, and no doubt they congratulated themselves that they had also obtained excellent protection from floods by their increased elevation above the main creek into which the gulches debouched. This sense of security was rudely shaken by the

tremendous water and rock outflows from the gulches during the 1942 flood. Fortunately, so far as known, no loss of life resulted from this rather dangerous location for homes, although narrow escapes were common.

An especially interesting effect of the torrential rainfalls was an exceedingly great number of what, for want of a better expression, have been termed "blow-outs." Where the rainfall exceeded 10 inches, these blow-outs, or large holes, were found on most of the hill-sides. The soil from the blow-out holes, mostly in a semiliquid form, had run and slid down to the bottom of the hills but had scarcely disturbed the ground surface or anything thereon. When the first holes were seen, it was thought that perhaps in each case the excessive surface runoff had started a small hole and rapidly enlarged it to the size noted. However, this thought was quickly discarded owing to the fact that there was little or no erosion either at the upper or lower side of the holes (fig. 34), and to the fact that normally they occurred in a series and all in one plane. Examination of individual holes indicated



FIGURE 34.—Blow-out hole near Port Allegany. Note absence of erosion around rim.

that they had been caused by flows of water from within the hillsides, a method of formation confirmed by interrogation of persons who had seen the blow-outs burst forth.

The holes occurred where zones of shattered rock would have formed outcrops on hillsides but for the shallow soil mantle. It is thought that great pressures were built up at this point by the large quantities of water that reached the zone of shattered rock by infiltration from the extremely heavy rainfall on higher ground. The dense ground surface, usually sod-covered, acted as a dam which blew out at the weakest spots when enough pressure developed. Each of these blow-outs acted as a safety valve to protect a considerable length of ground surface in front of the shattered rock layer on either side of that hole. Thus, a series of perhaps only a dozen large holes would occur along a mile or two of hillside. Manifestly, each line of holes clearly defined the location of a shattered rock stratum.

In addition to the blow-outs, there were many true slips and slides, although even those may very well have been due to water from shattered rock strata wetting and lubricating the slipped material. Some of these slides plunged down through forested areas, carrying everything before them. One swept from near the top of a hill to its bottom in Kettle Creek, a distance of probably more than a mile. Although small at the top, the slide appeared from a distance to be nearly a quarter of a mile wide at the bottom. Some of the slides, with the trees upright, moved for hundreds of feet. Some of the trees continued to grow in the new location as though nothing had happened.

METEOROLOGY OF THE STORM

DESCRIPTION

The following description is taken from the analysis contained in the Hydrologic Bulletin Supplement (U. S. Weather Bureau, 1944), omitting all references to the detailed charts and diagram used in that analysis as they are beyond the scope of this report.

The phenomenally heavy rains that fell over Elk, Cameron, McKean, and Potter Counties, Pa., and adjoining sections of New York on July 17-18, 1942, resulted from the recurrent thunderstorm activity associated with an atmospheric flow pattern that has been named Type V in Hydrometeorological Report No. 2: "Maximum possible precipitation over the Ohio River basin above Pittsburgh, Pa." Notable among heavy storms of this type are the Newcomerstown, Ohio, storm of August 6-7, 1935 (Showalter, 1941), and the eastern Kentucky storm of July 4-5, 1939 (Schrader, 1945). The July 1942 storm is classed as a 24-hour storm although durations were generally less than 24 hours at any one place. Nevertheless, the point-rainfall measurements of July 17-18, 1942, exceeded by several hundred per-

cent the known 24-hour amounts of May 30–31, 1889 (the Johnstown flood). They also exceeded the all-time Pennsylvania 24-hour record of 16.0 inches at Concord Township, Delaware County, on August 5, 1843 (see p. 148), the United States 12-hour record of 24.0 inches at Ewan, N. J., on September 1, 1940 (see p. 151), and approached the United States 24-hour record of 38.2 inches at Thrall, Tex., on September 9–10, 1921.²

It cannot be said, however, that amounts of rainfall like those of July 17–18, 1942, have never occurred before in the United States, even within a narrowly but reasonably defined period of record. The only certainty is that those amounts are among the highest for such durations that have been measured. It is notable, also, that this storm was one in an unusually extended series of excessive rainfalls that occurred in Pennsylvania between early in March and the latter part of August 1942.

Meteorological conditions near the earth's surface on July 17–18 were characterized by a warm anticyclone centered over the Southeastern States and the adjoining ocean. The flood area throughout was south of a quasi-stationary front extending eastward from Minnesota through the Great Lakes and then southward through eastern New York and New Jersey. The frontal system advanced slowly northeastward during the 18th.

The warm anticyclonic circulation extended above 15,000 feet. Around the western and northern periphery of the warm anticyclone there was the usual flow of maritime tropical air. The moist air stream was continuous from the Gulf of Mexico to Buffalo, N. Y. Over Buffalo on July 18 the air was unusually warm and moist even for July 1942, which was not a dry month. Except in the surface layer, where passage over the Great Lakes had induced an inversion, the air showed conditional and convective instability up to 15,000 feet and neutral equilibrium above that level. The precipitable water content up to about 16,000 feet was computed to be 1.95 inches. This is close to the maximum values of long record in the vicinity—for example, 2.13 inches at Cleveland. A concurrent sounding of the upper air at Sault Sainte Marie showed unusual warmth, moisture, and instability extending even that far north.

Soundings of the upper air in the Mississippi Valley region indicated decreasing pressure gradient with altitude and smooth anticyclonic flow without much change in curvature. Over the New York-Pennsylvania region, however, there was an increasing pressure gradient with altitude and a rapid change of curvature in the flow from anticyclonic to cyclonic. This was true at the 5,000-, 10,000-, and 15,000-foot levels, indicating a deep zone of convergence. An

² These United States 12-hour and 24-hour records supersede those previously published.

area of saturation coincident with the heavy rainfall zone also was apparent on one of the upper air charts prepared for this analysis.

The persistence of the zone of convergence over the region can be considered the major cause of the heavy rain. Other factors, which of themselves were not entirely effective impulses but which must be given consideration as adding to the effect, were the orographic lift of the unstable air mass from about the 600-foot elevation of Lake Erie to the more than 2,000-foot elevations of the mountain ridges of Pennsylvania and New York and the regenerating influence of the locally formed dense, cold air mass in the zone of heavy rain. The latter effect is an unusual illustration of how a thunderstorm zone maintains its own existence.

In general, the areas of lowest temperature coincided with the heavy-rain zone. Active cooling within the rain zone can be attributed to a combination of causes other than reflection of solar radiation from cloud tops. There was cooling of the air by conduction from cold rain and by melting of hail. Airplane pilots' reports during the period indicated cloud elevations well above the freezing level. In addition, however, the air was cooled by evaporation of rain. This is thought to be the dominant cause of cooling. As a result of such evaporation, the surface air would cool to its wet-bulb temperature, but the temperatures observed in the rain zone were as low as 63° F., a temperature lower than the wet-bulb temperature of any surface air that could have moved into the region horizontally. The indications are that in a process of such convective intensity as the rainfall amounts imply there was a large-scale exchange of air between the surface and aloft. Examination of the radiosondes from Detroit, Mich., and Huntington, W. Va., both representative of some of the air undergoing convergence over the area, shows wet-bulb temperatures aloft such as to produce, by descent of air through heavy rainfall, surface temperatures as low as 63° F.

The cooling thus accomplished produced a mass of cold, dense air at the surface. A slight ridge of high pressure was associated with the cold mass probably as a result of both the denser air and downward accelerations which also contribute to a pressure rise. The demarcation between cold and warm air became, in effect, a frontal zone, the cold air spreading laterally to lift the warm air and also acting as a barrier which the warm air overran, renewing and spreading thunderstorm activity by both methods.

The frontal zone became pronounced at about sunset of July 17, when heavy thunderstorms began over Cohocton, N. Y., and then

spread fanwise, reaching Ridgway, Pa., between 2 and 3 a. m., July 18. A careful study of the data available shows that, while the individual thunderstorms moved to the southeast in harmony with the northwest winds aloft, the thunderstorm area was propagated towards the southwest also. Such a propagation—to the right across the mean upper-air current—has been noted in previous thunderstorm studies and has been attributed to the hydrodynamical principle that cyclonic vortices move (or propagate) to the right across the current in which they are imbedded.

Between 3 and 6 a. m. July 18 the major extension of the cold-air mass seemed to be to the northeast. The heavy-rain zone spread with it, diminishing in the southwest. After 6 a. m. the edge of the cold air, acting as a front, moved to the southwest again, bringing a second period of heavy rain to the regions within its path.

Over the area of heavy rainfall the total precipitation resulted from a succession of three downpours coinciding with the oscillations of the cold air mass, the first and last outbursts being the most intense. The greatest amounts were centered between Emporium, Coudersport, Shinglehouse, and Smethport—unfortunately a region containing no official rain gages. The isohyetal map (pl. 2) based largely on miscellaneous records, shows the effect of the persistent, mainly northwesterly, winds in elongating the isohyetal pattern in a southeastward direction. Added to this effect was the southwestward-propagating effect plus the spreading of the cold, dense air-mass formed at the surface. It should be noted that equally plausible interpretations of the miscellaneous rainfall reports could lead to different isohyetal values near the storm center. Material differences in resulting duration-depth data would become negligible for the larger areas but any duration-depth computations for areas under 100 square miles should be classed as doubtful.

RAINFALL RECORDS

All available rainfall records for this storm, other than those few that were rejected because their accuracy was too uncertain, are given in tables 1 and 2. Table 1 contains the records for regular precipitation stations and is based on the Weather Bureau report (1944). The data have also been plotted in figure 35 to show the storm periods at the different locations and their timing. The data have been revised where necessary to correct known errors. The geographical positions are as accurate as could be determined, but they may still be subject to slight error for a few stations.

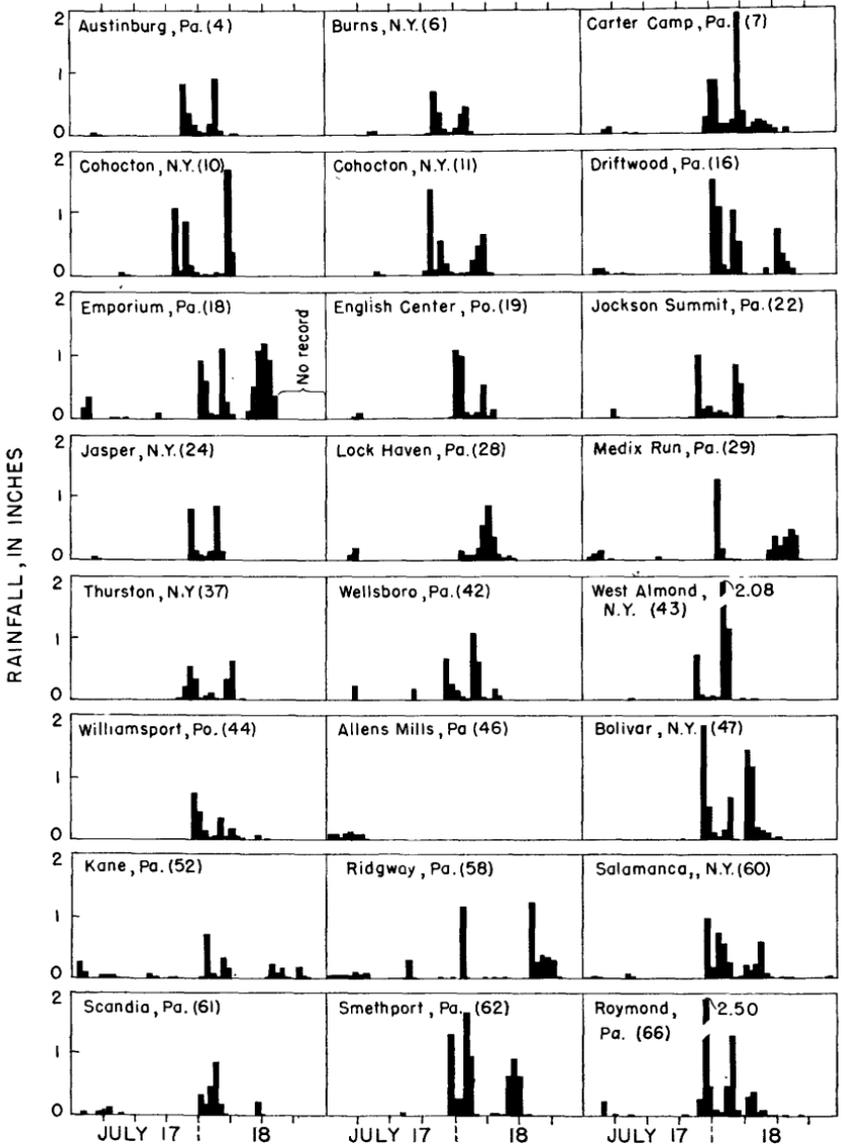


FIGURE 35.—Distribution of rainfall at recording-gage stations.

TABLE 1.—Rainfall at precipitation stations, July 17-19, 1942

[R, recording gage; amount shown is for 24-hour period ending at midnight. Other gages read in the morning except as noted. T, Trace. Records furnished as follows: C of E, Corps of Engineers; SCS, Soil Conservation Service; F-S FFS, Federal-State flood forecasting service of Pennsylvania; all others, Weather Bureau]

No. on pl. 2	Station	County	Latitude (deg., min.)	Longi- tude (deg., min.)	Elevation	July			Storm total 1
						17	18	19	
Susquehanna River drainage basin									
1	Addison, N. Y.	Steuben	42 06	77 14	990	0.10	2.50	1.05	3.55
2	Alfred, N. Y.	Allegany	42 16	77 47	1,780	T	3.35	0	3.35
3	Ansonia, Pa.	Tioga	41 45	77 26	1,136	.20	2.44	.42	2.86
4	Austinburg, Pa. (C of E, T-3) R.	do	42 00	77 32	1,790	1.35	1.26	.01	2.61
5	Bradford, N. Y. (C of E, H-26)	Steuben	42 22	77 08	1,430	0	1.67	0	1.67
6	Burns, N. Y. (C of E, N-19) R.	Allegany	42 25	77 44	1,620	1.19	.80	.80	1.99
7	Carter Camp, Pa. (F-S FFS) R.	Potter	41 38	77 43	---	.39	4.85	0	5.24
8	Cedar Run, Pa.	Lycoming	41 31	77 27	800	.38	3.20	.12	3.32
9	Clearfield, Pa.	Clearfield	41 01	78 27	1,260	.28	6.63	.55	1.18
10	Colcocton, N. Y. (SCS, R-1) R.	Steuben	42 30	77 29	1,570	2.29	2.23	.07	4.52
11	Colcocton, N. Y. (SCS-10) R.	do	42 32	77 27	1,900	2.35	1.47	.13	3.82
12	Colcocton, N. Y. (SCS, S-43)	do	42 30	77 31	1,520	0	2.69	0	2.69
13	Colcocton, N. Y. (SCS, S-60)	do	42 28	77 30	1,460	0	2.83	.05	2.88
14	Corning, N. Y.	do	42 08	77 03	1,925	T	1.58	.07	1.65
15	Covington, Pa.	Tioga	41 50	77 05	1,193	.80	1.45	0	2.25
16	Driftwood, Pa. (F-S FFS) R.	Cameron	41 20	78 08	820	.29	5.59	.17	5.88
17	Du Bois, Pa. (near) 2	Clearfield	41 06	78 38	1,670	.68	.27	.26	1.27
18	Emporium, Pa.	Cameron	41 30	78 14	1,033	.59	3.44	4.23+	7.67+
19	Emporium, Pa. (Z) (F-S FFS) R.	do	41 30	78 14	1,033	.67	17.43	(3)	8.10
20	English Center, Pa. (F-S FFS) R.	Lycoming	41 26	77 17	881	.15	3.08	0	3.23
21	Galeton, Pa. 2	Potter	41 44	77 39	1,330	.46	3.78	0	3.93
22	Haskinville, N. Y.	Steuben	42 25	77 34	1,620	0	1.88	.05	1.78
23	Jackson Summit, Pa. (C of E, G-4) R.	Tioga	41 57	77 01	1,700	1.54	1.74	0	3.28
24	Jasper, N. Y. (C of E, N-9) 2	Steuben	42 06	77 33	1,610	0	1.82	0	1.82
25	Jasper, N. Y. (C of E, N-8) R.	do	42 09	77 35	2,160	.97	1.20	0	2.17
26	Karthauss, Pa.	Clearfield	41 07	78 07	860	.40	.95	.83	1.78
27	Lawrenceville, Pa. 2	Tioga	42 00	77 08	966	.12	3.39	0	3.39
28	Lock Haven, Pa.	Clinton	41 08	77 26	557	.30	1.82	.29	2.11
29	Lock Haven, Pa. (No. 2) (F-S FFS) R.	do	41 07	77 27	600	.28	2.40	0	2.68
30	Medix Run, Pa. (F-S FFS) R.	Elk	41 17	78 23	---	.42	3.37	.08	3.79

See footnotes at end of table.

TABLE 1.—Rainfall at precipitation stations, July 17-19, 1942—Continued

No. on pl. 2	Station	County	Latitude (deg., min.)	Longi- tude (deg., min.)	Elevation	July			Storm total ¹
						17	18	19	
Susquehanna River drainage basin—Continued									
30	Middlebury, Pa. (C of E, T-4)	Tioga	41 54	77 20	1,780	0	2.00	0.06	2.06
31	Prattsburg, N. Y. (C of E, H-25)	Steuben	42 33	77 17	1,580	0	2.12	0	2.12
32	Pump Station Tower, Pa.	Lycoming	41 28	77 34	2,150	0	1.70	.30	3.00
33	Renovo, Pa.	Clinton	41 19	77 46	645	.31	1.90	2.69	4.59
34	Richmond, Pa. (C of E, T-2)	Tioga	41 50	77 04	1,580	0	1.95	0	1.95
35	Smith-Elliott Tower, Pa.	Jefferson	41 06	78 32	2,230	0	.62	1.08	1.70
36	Tamarack Tower, Pa.	Clinton	41 24	77 51	2,220	.34	2.70	3.35	6.05
37	Thurston, N. Y. (C of E, H-2) R.	Steuben	42 12	77 20	1,640	1.14	1.25	0	2.39
38	Troupsburg, N. Y. (C of E, N-16) ²	do.	42 04	77 30	1,945	0	2.26	0	2.26
39	Wallace, N. Y. (SCS, S-49)	do.	42 26	77 28	1,600	0	1.89	0	1.89
40	Wayland, N. Y. (SCS, S-52) ²	Livingston	42 35	77 30	1,600	0	2.92	0	2.92
41	Wellsboro, Pa.	Tioga	41 45	77 17	1,319	.26	3.03	.25	3.28
42	Wellsboro, Pa. (C of E, T-1) R.	do.	41 45	77 15	1,600	1.32	2.13	0	3.45
43	Williamsport, Pa.	Lycoming	41 15	77 00	550	.20	1.83	.32	2.15
	Williamsport, Pa. (Z) (F-S FFS) R.	do.	41 15	77 00	542	.74	1.42	0	2.16
Allegheny River drainage basin									
44	Allegheny State Park, N. Y. ²	Cattaraugus	42 06	78 45	1,500	0.18	3.92	0.32	3.92
45	Allens Mills, Pa. (F-S FFS) R.	Jefferson	41 12	78 54	1,580	.70	0	.16	.70
46	Bolivar, N. Y. (near) (F-S FFS) R.	Allegheny	42 03	78 11	1,560	2.32	4.02	.52	6.34
47	Bradford, Pa. (Reservoir No. 1) ²	McKean	41 57	78 44	1,680	.17	4.70	.02	4.70
48	Coudersport, Pa. (No. 1) ⁴	Potter	41 46	78 01	1,653	.26	8.22	0	8.22
49	Franklinville, N. Y. ⁴	Cattaraugus	42 21	78 27	1,600	0	3.21	.28	3.21
50	Kane, Pa. (near, No. 3)	McKean	41 41	78 48	1,800	0	1.51	1.68	3.19
51	Kane, Pa. (near) (F-S FFS) R.	Elk	41 36	78 44	1,680	.62	2.95	.15	3.57
52	Knapp Creek, N. Y. ³	Cattaraugus	42 01	78 31	2,370	.54	6.32	.75	6.77
53	Little Valley, N. Y. ⁶	do.	42 15	78 48	1,575	.23	3.61	.07	3.61
54	Mount Alto Airport, Pa. ²	McKean	41 48	78 38	2,155	0	4.64	0	4.64
55	Olean, N. Y.	Cattaraugus	42 04	78 26	1,440	.17	6.00	1.70	7.70
56	Raymond, Pa. (F-S FFS) R.	Potter	41 52	77 52	2,220	3.00	3.17	.04	6.17
57	Red House, N. Y.	Cattaraugus	42 07	78 48	1,382	0	4.48	.98	5.46
58	Ridgeway, Pa.	Elk	41 26	78 44	1,393	.80	1.68	2.62	4.68
	Ridgeway, Pa. (Z) (F-S FFS) R.	do.	41 26	78 44	1,371	.80	3.59	.31	4.39

59	Saint Mary's, Pa. (near).....do.....	41 24	78 33	1,664	0	1.70	3.63	5.33
60	Salamanca, N. Y. (F-S FFS) R.....	42 11	78 43	1,750	1.15	2.95	.35	4.10
61	Seandla, Pa. (near) ⁷ R.....	41 55	79 01	2,040	.28	2.21	.39	2.49
62	(Smetport, Pa. (Z) (F-S FFS) R.....do.....	41 48	78 27	1,510	T	4.35	2.41	6.76
	(Smetport, Pa. (Z) (F-S FFS) R.....do.....	41 48	78 27	1,510	1.34	5.35	.30	6.69

Genesee River drainage basin

63	Andover, N. Y. ²do.....	42 10	77 47	1,670	0	4.10	0	4.10
64	Angelica, N. Y. ⁴do.....	42 18	78 01	1,420	.05	4.00	.11	4.00
65	Dansville, N. Y. ⁴do.....	42 34	77 42	700	.13	2.44	0	2.44
66	Scio, N. Y. ²do.....	42 10	77 59	1,440	.16	4.56	0	4.56
67	West Almond, N. Y. (C of E) R.....do.....	42 18	77 53	1,960	.84	3.31	.11	4.15
68	Wiscony, N. Y. ²do.....	42 30	78 05	1,200	.06	3.37	.10	3.37

¹ Does not include rainfall measured by recording gages July 19.

² Measured in the afternoon, 4 to 6 p. m.

³ No record 3 p. m. July 18 to 6 a. m. July 20.

⁴ Measured in the evening, 7 to 8 p. m.

⁵ Observations at 1:30 and 7:30 by Airway observer; amount given is for 24 hours ending at 1:30 a. m.

⁶ Measured at 2 p. m.

⁷ Observations by L. D. Anderson.

Table 2 contains the rainfall records that J. E. Stewart, with the Corps of Engineers cooperating, was instrumental in collecting, as described on pages 71 to 74. They are arranged in order from north to south by major drainage basins. The geographical positions were checked thoroughly and should be as accurate as the maps available. Unfortunately, much of the area is sparsely settled, and topographic maps, based on surveys using modern methods and with the culture reasonably up to date, were available for only very small parts of the area. The quadrangle designation is given only for purposes of cross reference to the Weather Bureau report. For that reason the quadrangle designation for each record is the same in both reports, except for obvious typographical errors such as for No. 398, even though some designations were found to be incorrect. The rainfall measured represents the volume of the catch divided by the area of the opening through which it was caught. At many locations, the reporter or resident furnished information concerning more than one container. The rainfall caught in each one is listed separately in table 2. When several measurements were made using the same container, the rainfall measured each time is given in italics in table 2. The total amount measured in each of these containers is then given immediately below in roman type; the time period for the total is not given as it is merely the sum of the elapsed time for the separate measurements. Under Remarks is given information on the intensity of precipitation and factors that affected the accuracy of the catch.

Many of the records in table 2 were for parts of the storm period only. They are useful to some extent in that they show an amount by which the total precipitation is known to have been exceeded. They would be more useful if they could be used as a basis for estimating the total rainfall at that point. Several methods were investigated by Stewart, who finally adopted the relation shown in figure 36. The maximum rainfall for various intervals of time was picked off the charts for several representative recording gages. These amounts were expressed in percent of total storm rainfall and plotted in figure 36. The heavy line in that figure represents a weighted average that was used to estimate the storm rainfall from the partial records. The procedure was to assume that the amount of rainfall measured was the maximum that occurred during the storm for that length of time. The measured rainfall was divided by the percentage of the total rainfall occurring during the same length of time, obtained from the average curve in figure 36. More than 50 estimates of total rainfall given in table 2 were obtained by this method.

Records of rainfall of less than 1 hour duration were not used to estimate total rainfall. The estimates were interpreted as the minimum amount that could be expected for the total rainfall; the assumption that the measured rainfall was the maximum was considered

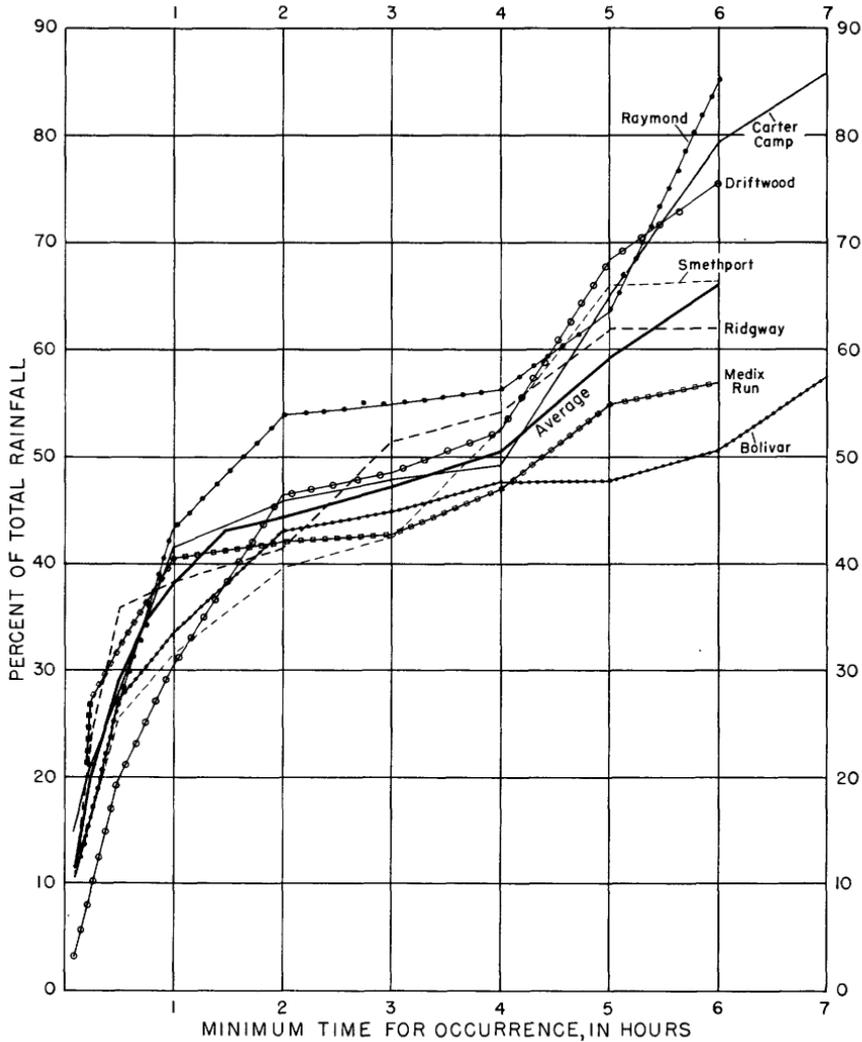


FIGURE 36.—Relation between percent of total rainfall and minimum time required for its occurrence.

unlikely in many instances. The method was tested by comparing the estimated total rainfall, computed from a partial catch and figure 36, with the observed total catch in the same container. The results were reasonably satisfactory. Admittedly the process is somewhat crude, but with the tests made and the relatively small variations shown by the observed data in figure 36, it was concluded that the estimates obtained were of sufficient accuracy to be given weight in drawing the isohyetal map (pl. 2).

Table 2 in manuscript form was checked by Mr. Stewart against the original field notes so as to eliminate all errors in compiling the table, insofar as possible. Several errors in previous work were eliminated in that way.

TABLE 2.—Miscellaneous measurements of rainfall, July 18, 1942

[DN, During night; *, container overflowed at unknown time or leaked badly; E, estimated on basis of rainfall measured; + or - with time of rain indicates a variation from time shown up to 15 minutes. Italic figures indicate more than one measurement in the same container; total for the container is given immediately below in roman type]

No. on pl. 2	Latitude (deg., min., sec.)	Longitude (deg., min., sec.)	Quadrangle designation 1	Reporter or resident	Rainfall between times shown			Storm period between times shown			Remarks	
					July 17 P. m.	July 18		July 17 P. m.	July 18			Total rainfall measured (inches)
						A. m.	P. m.		A. m.	P. m.		
West Branch Susquehanna River drainage basin												
1	41 51 36	77 49 18	<i>Pennsylvanica</i> Gonsee 6a.....	W. Cutler.....	11:30	12 m.	8.6	11:30	12 m.	8.6	Very heavy rain 11:30 p. m. 17 to 9 a. m. 18.	
2	41 50 22	77 50 33	5c.....	L. Hawkes.....	12	12 m.	9.5	12	12 m.	9.5	Very heavy rain 12 p. m. 17 to 7 a. m. 18.	
3	41 49 51	77 48 20	9a.....	J. Griffins.....	11	10:30	6.8	11	10:30	6.8	Heavy rain 11 p. m. 17 to 6:30 a. m. 18.	
4	41 47 35	77 45 52	9b.....	P. Bowen.....	12	9	6.5	12	9	6.5	Hard rain 12 p. m. 17 to 8 a. m. 18.	
5	41 47 05	77 53 11	8L.....	Blough.....	11	---	*7.4	11	---	---	Heavy rain 11 p. m. 17 to 10:30 a. m. 18.	
6	41 45 56	77 52 43	8J.....	Dr. Nixon.....	11:30	---	17.5	11:30	1:30	17.5	Hard rain 11:30 p. m. 17 to 7 a. m. 18 and 8 a. m. to 1:30 p. m. 18.	
7	41 44 49	78 00 06	Emporium 3a.....	F. Kellnor.....	9	6	4.1	9	---	---	11.4 inches catch probably small as container leaked to south with wind from northwest. Reporter estimated as much rain fell after 9 a. m. 18 as before. Container 2 feet from house with eaves extending 8 inches and probably reducing catch. A little hail about 10 p. m. 17. No rain 8 to 9 a. m. 18.	
8	41 44 14	78 02 00	3b.....	Grumm.....	11	9	11.4	11	---	11.4		
9	41 44 04	77 48 56	Short Run 3a.....	J. O'Neill.....	---	---	*17.2	---	---	---		
10	41 44 03	77 59 17	1b.....	Weiss.....	10	4	*13.9	10	---	2	E21.2	
11	41 43 49	77 58 54	1c.....	L. Lättlefeld.....	11	4	*10.0	11	---	---	E>15.1	
12	41 43 36	77 59 15	1c.....	Evien.....	10	11:30	*10.0	10	11:30	2		
					6	6	*5.4	6	6	6	9 a. m. 18.	
					11:30	11:30	*10.8	11:30	11:30	11:30	to 9 a. m. 18. No rain 8 to 9 a. m. 18.	

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13	41 43 33	77 57 45	If.....	12.8+	12.8+	12 m.	12 m.	12.8+	Container leaked slightly.
14	41 43 20	77 58 06	U. Buttner.....	4.2	4.2	8:30	8:30	4.2	Hail 2 to 2:10 p. m. 17.
15	41 43 17	78 00 49	S. Kelly.....	16.0	16.0	12	12	16.0	Short thunderstorm about 4 p. m. 17. Heaviest rain 7:15 a. m. to 1:30 p. m. 18. Heaviest rain 8:30 a. m. to 3:30 p. m. 18.
16	41 42 27	78 05 00	A. Haekett.....	21.2	21.2	DN	DN	19.0	Container empty 17; emptied a few days after storm.
17	41 42 24	78 05 35	J. Smith.....	19.0	19.0	11:45	11:45	19.0	About 4 hours heavy rain after container filled the second time.
18	41 42 19	77 57 46	E. Vergason.....	*9.1	*9.1	11:30	11:30	*9.1	Amount of rain may include that for hailstorm 3 to 3:45 p. m. 17.
19	41 42 15	78 05 14	R. Haekett.....	*8.1	*8.1	7	7-15 to 9:15	E18.3	Heaviest rain 12 p. m. 17 to 6:30 a. m. 18. Reporter estimated rain enough to fill container 2 more times. Shower with hail 3 to 3:10 p. m. 17.
20	41 42 10	77 58 28	G. Girardin.....	*16.2	*16.2	11	12 m.	12:30	Downpour started 9 a. m. 18. Hailed 4 to 4:30 p. m. and about 11:30 p. m. 17. Heavy rain 8:30 a. m. to 12 m. and 12:30 to 3:30 p. m. 18. Reporter estimated container would have filled again after 10 a. m.
21	41 42 04	78 05 40	Mitcheltree.....	*10.2	*10.2	6	6	12:30	Some hail 3:45 to 4:05 p. m. 11:30 to 11:45 p. m. 17; and 2 to 2:15 p. m. 18. Copious heavy rain 11:30 p. m. 17 to 3:30 p. m. 18. Container sheltered by dense foliage of maple tree.
22	41 41 53	77 56 15	O. Watson.....	*1.7	*1.7	7:30	7:30	4.1	Hailed about 1 minute at beginning of storm; heaviest rain 3 m. to 5:30 p. m. 18. No rain 7 a. m. 17; very heavy rain 8 a. m. to 2 p. m. 18.
23	41 41 31	78 05 20	L. Tyler.....	*9.6	*9.6	12	12	4.1	Reporter estimated at least 3/4" rain after 12 m. 18. Very little rain 9 to 9:30 a. m.
24	41 41 30	78 07 11	R. Kio.....	*5.6	*5.6	6	6	4.1	
				*3.3	*3.3	6-10	6-10	4.1	
				*8.9	*8.9			4.1	
25	41 40 55	78 07 33	J. MacMartin.....	4.1	4.1	11:30	11:30	4.1	
26	41 40 43	77 56 23	M. Kio.....	5.7	5.7	8	8	5.7	
27	41 40 39	78 11 19	E. Sebring.....	*10.0	*10.0	9	9	5.7	
28	41 40 30	78 02 15	Stephens.....	*6.0	*6.0	2-8	2-8	E15.0	
				*6.0	*6.0	8-12	8-12	E15.0	
				*12.0	*12.0			E15.0	

See footnote at end of table.

TABLE 2.—Miscellaneous measurements of rainfall, July 18, 1942—Continued

No. on pl. 2	Latitude (deg., min., sec.)	Longitude (deg., min., sec.)	Quadrangle designation, ¹	Reporter or resident	Rainfall between times shown			Storm period between times shown			Remarks	
					July 17 P. m.	July 18		July 17 P. m.	July 18			Total rainfall measured (inches)
West Branch Susquehanna River drainage basin—Continued												
29	41 40 02	78 12 06	<i>Pennsylvania</i> —Con. Emporium 1h.....	C. Perrigo.....	8-12	---	11.0	DN	---	P. m.	E21.7	Reporter estimated that container would have filled again 5.5" more had it been emptied at noon 18. Heaviest rain 8 a. m. to 1 p. m. 18.
30	41 39 32	78 08 28	5b.....	L. Goodwin.....	11	12 m.	*8.8	11	---	P. m.	---	Some hail p. m. 17; heavy rain 10 p. m. 17 to 6 a. m. 18; no rain 6:30 to 7:45 a. m.; and very heavy rain 10:30 a. m. to 12 m. 18.
31	41 39 31	78 09 10	5a.....	E. Ludwig.....	10	---	*12.5	10	---	3:30	---	Heavy hail storm about 3 p. m. 17; heaviest rain about 12 m. 18.
32	41 39 11	78 09 21	5c.....	H. Andrews.....	DN	10	---	DN	---	P. m.	---	A little hail on 18. Reporter estimated more rain after 8 a. m. 18 than before.
33	41 39 06	78 09 19	5d.....	G. Earl.....	DN	8	---	DN	---	1	---	Heavy rain 1 to 7 a. m. and 8 a. m. to 12 m. 18. No rain 7 to 8 a. m. 18.
34	41 38 57	77 58 03	Short Run 4a.....	E. Reed.....	8-12	---	*6.0	---	1	1:30	E11.8	Hard rain period 10:30 p. m. 17 to 10:30 a. m. 18.
35	41 38 18	77 59 10	4b.....	Pierce.....	8	P. m.	10.1 *5.1	9:30	---	P. m.	---	Hailed about 1 a. m. 18.
36	41 37 27	77 42 43	Gaileton 4a.....	C. Heuser.....	9:30	10:30	*13.2 3.6	10:30	12 m.	---	---	Reporter estimated 25% more rain after 11 a. m. Contain-er carried away by flood about 12 m. 18.
37	41 37 17	78 02 53	Emporium 6a.....	D. Rees.....	1-12	---	*9.4	---	1	P. m.	---	Heaviest rain 8:30 a. m. to 12 m. 18. Catch measured Sept. 8 probably small.
38	41 36 50	78 04 02	6b.....	C. Cooney.....	1-11	---	18.0	---	1	P. m.	---	
39	41 36 35	77 53 28	Short Run 5a.....	A. Walker.....	DN	---	*12.4	DN	---	5	---	

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40	41	36	04	78	12	50	Emporium 4b.....	V. Cannon.....	11:15	A. m.		*7.2	11:15	4:30		Container nearly full early a. m. 18. Reporter estimated enough rain to fill container twice.
41	41	35	34	77	53	57	Short Run 5b.....	C. Lloyd.....		1	1:30	8.5		1:30	8.5	Some hail about 1 a. m. 18, heaviest rain 8:30 a. m. to 1:30 P. m. 18. Container at end of house with 10-inch eaves.
42	41	35	14	77	54	08	5c.....	Williams.....		1	P. m.	6.5		P. m.	6.5	Heavy rain 1 to 6:15 and 6:30 to 11:30 a. m. 18. Container may have been emptied about 10 a. m. as usual but probably not.
43	41	34	45	77	54	20	Emporium 7a.....	Casbear.....	12		2	6.0	12	2	6.0	Reporter states container filled in about 5 minutes and then overflowed, but time may have been longer.
44	41	34	43	78	13	22	Emporium 7a.....	C. Dodge.....				6.5				Heaviest rain started between 10:30 and 11 a. m. 18. Reporter states container filled in about 5 minutes and then overflowed, but time may have been longer.
45	41	34	22	78	12	05	7b.....	C. Howard.....		11:30	4	5.4	DN	4		Heaviest rain on 18 started about 11 a. m.
46	41	33	54	77	55	56	Short Run 7a.....	G. McFall.....	DN		5	4.0	DN	5	4.0	Light shower at 5 p. m. 17.
47	41	33	51	77	56	18	7b.....	Berglund.....		12:30 to 1:30	4	6.2	DN	4	E 16.3	Heavy rain 8:15 a. m. to 2 p. m. 18, heaviest 12 m. to 2 p. m.
48	41	33	35	78	02	03	Emporium 9a.....	Card.....		9:30 to 9:55	1	9.5		1		Heaviest rain with steady intensity 9 a. m. to 12 m. 18.
49	41	33	31	78	01	45	9b.....	D. Williams.....		1-8	2	2.9 *6.3 *9.3		1		Intermittent heavy downpours 8:30 a. m. to 2 p. m. 18.
50	41	33	24	77	57	26	Short Run 7c.....	F. Decker.....		9 to 9:20	4	4.4	12	4		Very heavy rain 8 a. m. to 1 p. m. 18.
51	41	33	03	78	01	21	Emporium 9c.....	Greeley.....	DN			*7.1	11	2		Heavy rain 7 a. m. to 2 p. m. 18.
52	41	32	47	77	47	14	Short Run 8a.....	Kutcherbocker.....	11			*1.7	12	2:30	20.4	Hard rain 12 to 5 a. m. 18, and 12 m. to 2:30 p. m. 18, Downpour 10 a. m. to 12 m. 18.
53	41	32	26	77	58	51	7d.....	J. Fester.....	12		6	5.4	12			Hard rain 11:30 p. m. 17 to 3 a. m. 18. Downpour 11 a. m. to 12 m. 18.
54	41	32	00	78	00	52	Emporium 9d.....	T. Mancuso.....	12		2:30	20.4	12			Container set out D N 17 and found full on 18.
55	41	31	57	78	17	59	Colegrove 9a.....	F. Lockwood.....	11:30		P. m.	*6.7	11:30	P. m.		
56	41	31	53	78	18	00	9b.....	W. Timblin.....				*8.7				

See footnote at end of table.

TABLE 2.—Miscellaneous measurements of rainfall, July 18, 1942—Continued

No. on pl. 2	Latitude (deg., min., sec.)	Longitude (deg., min., sec.)	Quadrangle designation ¹	Reporter or resident	Rainfall between times shown			Storm period between times shown			Remarks	
					July 17 P. m.	July 18		July 17 P. m.	July 18			Total rainfall measured (inches)
						A. m.	P. m.		A. m.	P. m.		
West Branch Susquehanna River drainage basin—Continued												
57	41 31 34	78 03 55	Pennsylvania—Con. Emporium 9e.	B. Abel	9:50 ^a to 10		6.5	12	5		Container, set out 8 feet from house about 10 a. m. 18, overflowed in 10 min. Cloudburst from about 10 a. m. to 12 m. 18. No rain 6 to 8 a. m. 18.	
58	41 30 02	78 02 07	9f.	J. Lewis		*11.9					Hailed about 5 min. about 10 a. m. 18; heaviest rain 8:30 a. m. to 2:30 p. m. 18.	
59	41 29 57	78 02 15	Driftwood 3a.	W. Deibler	7	7.8		DN	2:30	E 12.8	Very hard rain 12 m. to 4:30 p. m. 18.	
60	41 29 20	78 02 53	3b.	C. Logue	11 A. m.	*7.4		11	2:30		D. m. 18.	
61	41 29 14	78 26 08	Caledonia 1a.	G. Mahonlich, Jr.	DN		*9.3	DN	4:30		Downpour 10 a. m. to 3 p. m. 18; container only 3 1/2 feet from eaves of house. No rain 7 to 8 a. m. 18.	
62	41 29 08	78 17 14	3b.	M. Judd	12 9		2.4	12	6	E > 12.5	Heaviest rain 8 a. m. to 12 m. 18.	
63	41 29 07	78 13 42	Driftwood 1a.	J. Sage	8	6	*6.5	8	6		Accuracy of record in doubt. Hail 1:45 to 2 p. m. 18.	
64	41 29 05	78 17 35	Caledonia 3a.	S. Crawford	8	3	10.5				Container 15 feet from lee side of barn. Downpour 2:30 to 4:30 p. m. 18.	
65	41 29 00	78 27 42	1b.	P. Herzog	11	4:30	5.3	11	4:30	5.3	Very heavy rain 12 m. to about 6 p. m. 18.	
66	41 28 57	78 13 47	Driftwood 1b.	J. Ostrum	12 6		3.4	12	8		Container 14 feet from porch, under dense evergreen. Heavy rain between 10 a. m. and 3 p. m. 18.	
67	41 28 53	77 49 22	Birumen 3a.	S. Jones	10 8		3.3	10	2:30		No rain 7 to 8 a. m.; heavy rain 8 a. m. to 3:30 p. m. 18;	
68	41 28 51	78 25 12	Caledonia 1c.	L. Carter	DN	3	4.9	DN	3		downpour 12 m. to 3:30 p. m. 18. It is possible that container overflowed.	
69	41 28 40	78 15 39	3c.	C. Lyon	12	3:30	7.2	12	3:30	7.2		

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70	41 28 34	78 11 03	Driftwood 1c.	H. Hicks	10:30	4:30	7.6	10:30	4:30	7.6	Container under maple tree with thin foliage.
71	41 28 04	78 16 35	Caledonia 3d.	E. Mumford	10:30	3	*6.9			4.7	Heavy rain 1 to 5 p. m.; very heavy rain 5 to 6 p. m. 18.
72	41 28 03	78 29 26	Id.	J. Brentel	12 m.	6	4.4	12	6	8.1±	Very hard rain 12 m. to 4 p. m. 18. Moderate rain 7 to 8 a. m. and light rain 5:30 to 6 p. m. estimated as 0.3 in.
73	41 28 00	78 61 51	3e.	P. Lyon	1-7 8	5:30	3.9 7.8		1		
74	41 27 48	77 50 42	Bitumen 2a	F. Caldwell	DN	P. m.	*9.8	DN	P. m.		Heavy rain 6 to 8 a. m. 18.
75	41 27 42	78 03 28	Driftwood 3c.	R. Keller	11		*12.1	11	3		May include rain from previous storms. Heavy rain to 6 a. m. and 6:40 a. m. to 2 p. m. 18.
76	41 27 33	78 17 30	Caledonia 3f.	J. Reed	12	7	16.5	12	7	16.5	Downpour 3:30 to 4:30 p. m. 18.
77	41 26 08	77 53 17	Bitumen 2b.	Peters	10		*7.3	10	3	E14.3	Heavy rain 5 a. m. to 12 m. 18.
78	41 25 52	78 22 34	Caledonia 2a	G. Sprankle	7:30 11	6	*8.2	10	6		Heaviest rain between 10 a. m. and 12 m. 18.
79	41 25 45	77 54 50	Bitumen 2c.	D. Crane	9 to 10	6	2.9 3.6	10	2:30		Heaviest rain DN 17. Container filled in 20 min.
80	41 25 42	77 54 48	2d.	H. Craner	11	1	*7.5	12	2	E16.9	Very heavy rain 11 a. m. to 1 p. m. 18.
81	41 25 32	77 54 04	2e.	Summerson	8	3	8.0		3	E11.4	Container 6' from end eaves. Poor location due to sheltering.
82	41 25 19	77 54 05	2f.	C. Calhoun	10	3	8.6	10	3	8.6	Very heavy rain 8:30 a. m. to 2 p. m. 18.
83	41 24 46	78 02 12	Driftwood 6a.	C. Peno	12:30	2	*8.9	12:30	2		Heavy rain 12 m. to 4 p. m. 18. Container overflowed 3 times. Emptied 6 a. m. and 12 m. 18.
84	41 24 35	78 11 56	4a.	S. Barr	DN	4	*7.3	DN	4		Very heavy rain 12:30 to 3:15 p. m. 18.
85	41 24 17	77 55 14	Bitumen 4a.	McCoy		12:30 to 3:15	3.3	11	3:15	E6.5	Heaviest rain about 11 p. m. 17.
86	41 24 04	78 14 44	Caledonia 6a.	J. Mix	9	5:30	3.9	9	P. m.		Container overflowed near end of storm.
87	41 23 46	78 01 10	Driftwood 6b.	P. Johnson	9		*8.2				Downpour 11 a. m. to 3 p. m. 18.
88	41 22 53	78 01 29	6c.	S. Kuppelweiser	9	2	9.7	9	4		No rain 7 to 8 and 10:30 to 11 a. m. 18. Very heavy rain 9 to 10:30 a. m. and 11 a. m. to 3:30 p. m. 18.
89	41 22 43	77 55 28	Bitumen 4b.	D. Schoonover		4-5	15.5 3.3			E9.2	

See footnote at end of table.

TABLE 2.—Miscellaneous measurements of rainfall, July 18, 1942—Continued

No. on pl. 2	Latitude (deg., min., sec.)	Longitude (deg., min., sec.)	Quadrangle designation ¹	Reporter or resident	Rainfall between times shown		Storm period between times shown		Remarks			
					July 17 p. m.	July 18 A. m. P. m.	July 17 p. m.	July 18 A. m. P. m.		Total rainfall measured (inches)		
West Branch Susquehanna River drainage basin—Continued												
90	41 22 22	78 09 10	<i>Pennsylvan</i> —Con. Driftwood 5a.....	H. Jordan.....	DN DN	8	3	7.9 5.4	DN	3	Container in poor location. Very hard rain 1 to 3 p. m. 18.	
91	41 21 39	78 08 17	5b.....	J. Johnson.....		1-7 7	P. m.	5.2 9.5	1	P. m.	9.5 Very heavy rain 7:30 a. m. to 2 p. m. 18.	
92	41 21 23	78 03 12	6d.....	Miller.....		1-7		3.1	1	P. m.	Very hard rain 10:30 a. m. to 2 p. m. 18.	
93	41 20 25	77 50 33	Bitumen 5b.....	Desmond Mine.....				11.8 10.9 7.2				
94	41 20 20	77 52 22	5a.....	Janet Mine.....				10.0 3.1			Heavy rain 2:30 to 3 p. m. 18. Heaviest rain between 5 and 6 a. m. 18; drizzled after 3 p. m. to 8 p. m. 18.	
95	41 18 05	78 03 06	Driftwood 6a.....	C. Pits.....	10	5:30		8.3	10	3	Hard rain 6 to 7 a. m. and 11 a. m. to 2 p. m. 18. No rain 7 to 9 a. m.; light rain 2 to 9 p. m. 18.	
96	41 17 52	77 50 46	Bitumen 8a.....	F. Morton.....	11	10:30			11	3		
97	41 16 49	77 53 14	8b.....	H. Anderson.....		1:45 to 6:30		1.5	1:45	2	3.0	
98	41 15 31	77 54 32	8c.....	A. Stinson.....		2	2	4.2	2	2	4.2	Very hard rain noon to 2 p. m. 18. Container under big limbs of a tree.
Allegheny River drainage basin												
99	42 11 48	78 33 28	<i>New York</i> Salamanca 38.....	J. Wiley.....	9	11		8.0	9	11	8.0	Heavy downpours 7 to 11 a. m. 18.
100	42 11 36	78 28 16	Olean la.....	G. Wilber.....	DN	11		6.8	DN	11	6.8	1 cloudburst after another 6:30 to 11 a. m. 18. Very little rain after 11 a. m. 18.

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101	42	11	25	78	33	25	Salamanca 3b.....	B. Potter.....	DN	12 m. 12 m.	4	4.7 2.8 7.5	DN	-----	4	7.5	Downpour 5 to 9 a. m. 18.
102	42	11	22	78	29	38	Olean 1b.....	J. Whiteher.....	11	A. m.	-----	7.6	11	-----	-----	-----	Heavy rain 11 p. m. 17 to about 7:30 a. m. 18.
103	42	10	45	78	26	26	1c.....	R. Rogers.....	DN	8	-----	*7.6	DN	-----	-----	-----	Downpour 6:30 to 8 and hard rain 8 to 10 a. m. 18.
104	42	10	45	78	24	29	2a.....	F. Gille.....	-----	6:30 to 10 6:30 to 10	-----	*5.3	11:30	-----	E>12.6	-----	Very heavy rain 11:30 p. m. 17 to 1:30 a. m. 18, downpour 6:30 to 9 a. m. 18.
105	42	10	30	78	28	57	1d.....	M. Hitchcock.....	7	-----	1	*9.4	7	-----	1	-----	Heavy rain 5 to 11 a. m. and downpour 11 a. m. to 1 p. m. 18.
106	42	10	27	78	22	24	3b.....	D. Wing.....	10:30	-----	-----	*8.1	10:30	-----	-----	-----	Downpour 7:30 to 9 a. m.; not much rain after 9 a. m. 18.
107	42	10	19	78	21	58	-----	C. Wagner.....	11:30	9	-----	4.7	11:30	-----	-----	4.7	A little hail DN 17. Hard rain 11:30 p. m. 17 to 12:30 a. m. 18. Downpour 8 to 9 a. m. 18. Container sheltered by tree.
108	42	09	54	78	23	12	Olean 5a.....	G. Bell.....	DN	-----	1	14.1	DN	-----	1	14.1	Downpour 8:30 to 9:30 a. m. 18.
109	42	09	32	78	23	45	5b.....	E. Saylor.....	12	12 m.	-----	6.0	12	-----	-----	6.0	Hard rain 12 to 7:30 a. m. and downpour 7:30 to 9:30 a. m. 18.
110	42	09	27	78	26	36	4a.....	C. Adams.....	10	12 m.	-----	*2.5	10	-----	-----	-----	A little hail and downpour for 1/2 hour shortly after 8 a. m. 18.
111	42	09	24	78	30	12	Salamanca 6a.....	G. Wood.....	-----	7 to 11	-----	6.0+	Before 12	-----	-----	E9.6	No rain 5 to 7 a. m. 18. 3/4 hr. downpour between 7 and 8 a. m. 18. Hard bursts of rain 8 to 11 a. m. 18. Slight leak in container.
112	42	09	23	78	25	20	Olean 4b.....	A. Swart.....	10	11	-----	*6.6	10	-----	-----	-----	No rain 5 to 7:30 a. m. 18. Downpour started 7:30 a. m.; thereafter heavy bursts of rain until 11 a. m. 18.
113	42	09	10	78	17	38	6a.....	C. Kraatts.....	10	9	-----	*4.8	10	-----	-----	-----	Excessive rain 7:30 to 8:30 a. m. 18.
114	42	09	03	78	17	45	6b.....	A. Subject.....	10:45	-----	2:30	5.4	10:45	-----	2:30	5.4	Excessive rain 7 to 8:30 a. m. 18.
115	42	08	40	78	21	16	5c.....	J. Bump.....	9	10	-----	5.9	9	-----	-----	5.9	No rain 5 to 7 a. m.; downpour 8 to 10 a. m. 18.
116	42	08	24	78	24	30	5d.....	E. Hurlburt.....	11	11	-----	*9.2	11	-----	-----	-----	Downpour 8:45 to 11 a. m. 18.

See footnote at end of table.

TABLE 2.—Miscellaneous measurements of rainfall, July 18, 1942—Continued

No. on pl. 2	Latitude (deg., min., sec.)	Longitude (deg., min., sec.)	Quadrangle designation ¹	Reporter or resident	Rainfall between times shown			Storm period between times shown			Remarks	
					July 17 P. m.	July 18		July 17 P. m.	July 18			Total rainfall measured (inches)
						A. m.	P. m.		A. m.	P. m.		
Allegheny River drainage basin—Continued												
117	42 08 20	78 21 05	New York—Con. Olean 5c.	F. Miller.		6:30 to 10		*1.7	11	12		Downpour 7 to 11 a. m. 18.
118	42 08 06	78 20 21	5g.	A. Dehukam.		10		*6.6				Downpour 8 to 8:30 a. m. 18. Container noticed overflowing 9:30 or 10 a. m. 18. Very hard rain 6 or 7 to 9:30 a. m. 18.
119	42 08 04	78 20 50	5f.	C. Shaffer.	9	11		*7.9	9	11		Downpour 7:30 to 8:30 a. m. 18, followed by another slightly later.
120	42 07 57	78 28 52	4c.	F. Miller.	DN	2		*7.0	DN	2		Excessive rain 7:30 to 9 a. m. 18.
121	42 07 56	78 18 11	6e.	M. Manley.	11	9		3.5	11	9		Steady hard rain 5:30 to 10 a. m.; heaviest rain 6 to 9:30 a. m. 18. Good record of amount of rainfall after 8:30 a. m.
122	42 07 46	78 28 35		A. Hausel.		8:30 to 12		2.6	DN	12 m.		Steady hard rain 12 to 8 a. m. and downpour 8 to 8:30 a. m. 18.
123	42 07 46	78 22 44	Olean 5h.	W. Alexander.	DN	10		*7.4	DN	10		Downpour 6:30 to 8:30 a. m. and hard rain 8:30 until nearly noon 18.
124	42 07 40	78 24 48	5i.	F. Phelps.	DN	12 m.		9.5	DN	12 m.		Hard bursts of rain 8 to 12 noon 18.
125	42 07 38	78 14 33	Belmont 4a.	C. Congdon.	DN	10 to 10 to 12 m.		4.5	DN	12 m.		Heavy rain for an hour about 9 p. m. 17; excessive rain 8:30 to 11 a. m. 18.
126	42 07 20	78 20 50	Olean 5j.	E. Miller.	8:30	9		*9.9	8:30	11		Heavy rain 11:30 p. m. 17 to 12:30 a. m. 18. Downpour 7:45 to 10 and hard rain 10 to 11:30 a. m. 18.
127	42 07 08	78 18 56		A. Forsythe.	11:30 to 11:30	11:30 to 11:30		*7.3	11:30	11:30		
								*7.2				

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128	42 07 07	78 23 01	Olean 5l	J. Zink	DN	12 m.	11.2	DN	12 m.	11.2	Hard rain 4 to 8 and excessive rain 8:30 to 9 a. m. 18. Container may have leaked at beginning of storm. Excessive rain 7:45 to 10 a. m. 18.
129	42 07 05	78 18 58	6d	D. Wagner	11:30	11:30	*7.4	11:30	11:30		Excessive rain 8 to 8:30 and hard rain 8:30 to 10:30 a. m. 18.
130	42 07 00	78 24 08	5k	F. Smith	10:30	7	13.4	10:30	10:30	13.4	Succession of heavy thunderstorms 7:30-12 a. m. 18. Downpour 7:30 a. m. to 10 a. m. 18.
131	42 06 53	78 15 01	6c	H. Higby	10:45	7	3.8	10:45	P. m.		Hard showers 11 p. m. 17 to 6:30 a. m. 18. Downpour 6:30 to 7:30 and hard rain 7:30 to 11 a. m. 18.
132	42 06 49	78 14 57	Belmont 4b	F. Walbur	1:30	7	5.7	10:45	1:30	E12.5	Downpour 7:30 to 10:30 a. m. 18.
133	42 06 44	78 23 15	Olean 5m	Bush	11	8	*6.0	11	11		Intermittent heavy rain DN 17. No rain 7 to 7:30, excessive rain 7:30 to 9:30, and hard rain 9:45 to 11:30 a. m. 18.
134	42 06 40	78 19 53	6f	O. Crawford	11	8-11	*2.6 *9.1				Hardest rain about 8 a. m. 18.
135	42 06 40	78 12 58	Belmont 4e	C. Fisk	11	12 m.	3.4	11	12 m.	3.4	Downpour 7:30 to 10:30 a. m. 18.
136	42 06 39	78 22 36	Olean 5n	C. Brown	9:45	7 to 11:30	*3.6 *10.9	9:45	11:30	E>22.1	Intermittent heavy rain DN 17. No rain 7 to 7:30, excessive rain 7:30 to 9:30, and hard rain 9:45 to 11:30 a. m. 18.
137	42 06 38	78 14 26	Belmont 4c	J. Aud	DN	6 6-11	*14.5 7.0	DN	11	14.0	Downpour 8 to 9 a. m. 18. Succession of heavy rains DN 17 to 5:45 a. m. 18. Cloud-burst 8 to 10 a. m. 18.
138	42 06 36	78 13 30	4d	C. Foster	11	12 m.	14.0	11	12 m.	8.9	Downpour 7 to 8 a. m. 18.
139	42 06 12	78 14 00	4f	C. Beckwith	10:15	10	8.9 11.2	10:15	10	11.2	Amount measured is night rain only.
140	42 05 54	78 15 14	Olean 6h	H. Robinson	DN	5:30	2.8	DN	11		Excessive rain 7:30-12 a. m. 18. A little hail within this period.
141	42 05 53	78 14 20	Belmont 4g	F. Barbet	DN	4	6.0	4	12 m.		Most of rain came in cloud-burst betw 7:18 and 9 a. m. 18
142	42 05 49	78 19 07	Olean 6g	C. Willover	4	12 m.	*9.1	4	12 m.		Excessive rain 8 to 10 or 10:30 a. m. 18.
143	42 05 29	78 14 53	Belmont 4h	F. Foster	DN	10:30	*6.9	DN	10:30		Time container was in use is somewhat indefinite; reporter estimated enough rain fell to fill container two more times.
144	42 05 27	78 21 35	Olean 5o	C. Fredrick	DN	7 to 8	*7.9	DN	8	E>21.6	
145	42 05 14	78 21 34	5p	D. Kayes			*8.2	DN			
146	42 05 14	78 14 31	Belmont 4i	L. Childs			*4.0				

See footnote at end of table.

TABLE 2.—Miscellaneous measurements of rainfall, July 18, 1942—Continued

No. on pl. 2	Latitude (deg., min., sec.)	Longitude (deg., min., sec.)	Quadrangle designation 1	Reporter or resident	Rainfall between times shown			Storm period between times shown			Remarks	
					July 17 p. m.	July 18		July 17 p. m.	July 18			Total rainfall measured (inches)
					A. m.	P. m.	A. m.	P. m.	A. m.	P. m.		
Allegheny River drainage basin—Continued												
<i>New York—Con.</i>												
147	42 05 06	78 17 44	Olean 6l	J. Hewitt	DN	9:30	DN	9:30	DN	9:30	Downpour 7 to 9:30 a. m. 18	
148	42 04 55	78 15 41	9b	H. Burett	7	12 m.	7	12 m.	7	12 m.	Heavy rain 7 to 9 and down pour 9 to 11 a. m. 18.	
149	42 04 50	78 19 50	9a	R. Blakeslee	10:30	12 m.	10:30	12 m.	10:30	12 m.	Downpour 7:30 to 12 m. 18; some hail in this period.	
150	42 04 47	78 25 56		Times Herald	DN	11:30	10:30		10:30		Hardest rain about 11 a. m. 18 when it was almost as dark as night.	
151	42 04 46	78 11 59	Belmont 7a	P. Hoffman	10:30		10:30		10:30		Cloudburst 6:30 to 8 a. m. 18.	
152	42 04 42	78 15 04	Olean 9c	M. Neu	11	9	11	9	11	9		
153	42 04 37	78 12 26	Belmont 7b	B. Frost	DN	6	6	6	DN	6		
						6-8						
154	42 04 30	78 16 17	Olean 9f	L. Lewis	11	12 m.	11	12 m.	11	12 m.	Downpour 7 to 9 a. m. 18.	
155	42 04 24	78 16 37	9c	F. Payne	11	6	11	12 m.	11	12 m.	Downpour 7:30 to 11 a. m. 18.	
						6-12						
156	42 04 23	78 17 48	9d	O. Merrick		7:45			DN	10	Downpour 7:45 to 9 a. m. 18.	
157	42 04 19	78 14 56	Belmont 7c	W. Keller	7	11	7	11	7	11	Hard rain 7 p. m. 17 to 9 a. m. 18.	
158	42 04 13	78 16 58	Olean 9g	O. Ecker	9:30	10	9:30	10	9:30	10	6.0+	
											Catch small, owing to some leakage and end of catch trough being sheltered.	
											Hard rain all night; down-pour 6:30 to 8:30 a. m. 18.	
159	42 04 10	78 13 43	Belmont 7d	A. Chaffee							Very heavy rain 7:20 to 8 and downpour 8 to 10 a. m. 18.	
160	42 03 44	78 22 04	Olean 8a	F. Withereil	9	11	9	11	9	11	Downpour started about 8 a. m. 18.	
161	42 03 37	78 18 25	9h	E. Bradford	10:30	12 m.	10:30	12 m.	10:30	12 m.	Heavy rain 11:15 p. m. 17 to 11 a. m. 18; heaviest 7:30 to 9 a. m. 18.	
162	42 03 33	78 14 45	Belmont 7e	H. Woodard	11:15	12 m.	11:15	12 m.	11:15	12 m.	9.5	

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163	42	03	25	78	15	02	Olean 9i	J. Coffey	DN	11		*9.5	DN	11			Downpour 8:30 to 9:30 and hard rain 9:30 to 11 a. m. 18.
164	42	03	14	78	21	48	8c	W. Boxer	8	12 m.		*13.1	8	12 m.			5 cloudbursts forenoon of 18.
165	42	03	09	78	21	52	8b	B. Martin	DN	12 m.		*5.0	DN	12 m.			Downpour 8-9 and hard rain 9-12 a. m. 18.
166	42	03	03	78	12	37	Belmont 7i	G. Hulbert	10	12 m.		4.8	10	12 m.		4.8	Hard rain 10 p. m. 17 to 8 a. m. 18; heaviest between 7 and 8 a. m.
167	42	02	35	78	10	22	7h	C. Rigby	11	11		1.3	11	11		1.3	Cloudburst 8 to 10 a. m. 18.
168	42	02	27	78	11	22	7g	J. Cartwright	7	5		6.3	7	10		6.3	Cloudburst before 9 a. m. 18.
169	42	02	18	78	22	39	Olean 8d	F. Vanyo	DN	12 m.		*6.6	DN	12 m.		*6.6	Hard rain 9 p. m. 17 to 11 a. m. 18.
170	42	02	01	78	10	25	Belmont 7i	M. Burrows	9	11		3.0	9	11		3.0	Hardest rain 6:30 to 8 a. m. 18. Containner in poor location.
171	42	01	41	78	09	10		F. Bates				*8.2	DN	12 m.		*8.2	Downpour 7 to 8 a. m. 18. Heavy rain all night 17. Excessive rain 8-12 a. m. 18.
172	42	01	24	78	13	20	Belmont 7j	M. Lamphere	DN	8		6.4	DN	9		6.4	Hard rain all night 17.
173	42	01	12	78	21	48	Olean 8c	M. Coleman	10:45	12 m.		5.6	10:45	12 m.		5.6	Hard rain all night 17.
174	42	00	52	78	12	54	Belmont 7k	C. Giddings	11	11		6.8	11	11		6.8	Hard rain 11 p. m. 17 to 4:30 a. m. 18 and 8:30 to 11 a. m. 18. Downpour 5:30 to 8:30 a. m. 18.
175	42	00	50	78	09	23	8a	D. Grimes				3.6				3.6	Hardest part of rain between 8 and 9 a. m. 18. Reporter estimated that 0.87" fell between 7:30 and 8 a. m. 18, making a total of 5.57" from 7:30 to noon 18.
176	42	00	34	78	15	52	Olean 9j	R. Loucks	DN	8-12 12 m.		5.0 *5.0	DN	12 m.		E9.4	No rain 6 to 8 a. m.; heavy rain 8 to 11:30 a. m. 18. Reporter estimated total rain as about 15 inches.
177	41	59	59	78	16	45	<i>Pennsylvania</i> Smethport 3a	F. Pitzrick	DN	11:30		*9.3	DN	11:30			Downpour 6 to 8 a. m. 18. Succession of downpours 5-12 a. m. 18.
178	41	59	57	78	16	16	3b	L. Shaw	11:30	A. m.		4.3	12	8+		4.3	Downpour 6:30 to 10 a. m. 18.
179	41	59	57	78	10	55	Coudersport 1a	P. Gillmer	12	10		*7.4	12	12 m.		*7.4	Downpour 7 to 10 a. m. 18.
180	41	59	50	78	20	55	Smethport 2a	L. Evens	11:30	12 m.		6.7	11:30	12 m.		6.7	Downpour 7:45 to 10 a. m. 18. Containner may have held rain from previous storms, but probably not more than 2 inches. Reporter stated no water in containner prior to 17.
181	41	59	50	78	11	18	Coudersport 1b	D. Green	11	10+		*8.3 *8.7	11	10+		*8.3 *8.7	Downpour 7:45 to 10 a. m. 18.
182	41	59	27	78	15	09	Smethport 3c	E. Turner	10:30	10+		*7.8	10:30	10+		*7.8	Downpour 7:45 to 10 a. m. 18.
183	41	59	22	78	20	58	2b	C. Baldwin	12	12 m.		*10.6	12	12 m.		*10.6	Downpour 7:45 to 10 a. m. 18.
184	41	59	18	78	11	08	Coudersport 1c	R. James	11:45	12 m.		23.0	11:45	12 m.		23.0	Downpour 7:45 to 10 a. m. 18.

See footnote at end of table.

TABLE 2.—Miscellaneous measurements of rainfall, July 18, 1942—Continued

No. on pl. 2	Latitude (deg., min., sec.)	Longitude (deg., min., sec.)	Quadrangle designation 1	Reporter or resident	Rainfall between times shown		Storm period between times shown				Remarks	
					July 17 p. m.	July 18 A. m. P. m. (inches)	July 17 p. m.	July 18 A. m. P. m.	July 18 A. m. P. m. (inches)			
Allegheny River drainage basin—Continued												
185	41 59 17	78 17 52	Pennsylvania—Con. Smetthport 3d.....	E. Mills.....	10	2	*5.6	10	2	-----	Downpour by "bursts" 7 a. m. to 2 p. m. 18. Downpour 8-12 a. m. 18.	
186	41 59 09	78 21 03	2c.....	D. Bufkun.....	12	8	*5.8 9.0	12	12 m.	-----	-----	
187	41 58 47	78 13 02	Coudersport 1d.....	H. Blanchard.....	DN	11+	*14.8	DN	11+	-----	Heavy rain 7:30 to 11 a. m. 18. Downpour about 11:30 a. m. 18.	
188	41 58 44	78 19 28	Smetthport 3 c.....	Shoemaker.....	DN	1	10.5 8.5	DN	1	-----	-----	
189	41 58 43	78 22 22	2d.....	Mason.....	DN	1	10.5	DN	1	-----	Heavy rain 8:15 a. m. to 1 p. m. 18.	
190	41 58 41	78 17 05	3h.....	A. Scutt.....	11	10+	*7.2	11	10+	-----	Container nearly full before heavy storm of 18.	
191	41 58 36	78 20 13	2c.....	J. Soules.....	11	2	9.5+ *7.2	11	2	-----	Container for 9.5-inch catch 1 foot from end of house, probably leaked at beginning of catch. Hard rain 11 p. m. 17 to 3 a. m. 18; downpour 7:30 to 11 or 11:30 a. m. 18.	
192	41 58 36	78 08 39	Coudersport 2a.....	L. Drake.....	11	5	*2.3	-----	-----	-----	Container overflowed before 5 a. m. 18.	
193	41 58 34	78 19 20	Smetthport 3f.....	S. Peasley.....	11:30	12 m.	*7.1	11:30	12 m.	-----	Heavy rain 7 to 8 a. m. 18. Hard rain 10 p. m. 17 to 6 a. m. 18 and 7 to 8 a. m. 18.	
194	41 58 34	77 59 24	Genesee 1a.....	F. Walker.....	10	7	5.0	10	9	-----	-----	
195	41 58 33	78 14 32	Coudersport 1e.....	L. Holly.....	9	1-9	*4.5	9	1	1:30	Heavy rain 9 a. m. to 1:30 p. m. 18.	
196	41 58 33	78 12 15	1f.....	P. Baker.....	9	2	*9.9	9	2	-----	Downpour 8 a. m. to 2 p. m. 18.	
197	41 58 32	78 19 14	Smetthport 3g.....	A. Cole.....	-----	9	6.5	-----	-----	-----	Container (wood-stave barrel) probably leaked materially at beginning of catch.	
198	41 58 31	78 23 01	2f.....	McNaughton.....	-----	1	6.3	11	4	-----	Heavy rain 8 a. m. to 1 p. m. 18.	
199	41 58 22	78 23 08	2g.....	F. Johnson.....	DN	12 m.	*6.1	DN	12 m.	-----	Rained 1/11 to 11:40 a. m. 18.	

200	41 58 03	78 14 25	Coudersport 1g	M. Maason	2 to 11	*7.3	2 to 12 m.			Downpour period 8 to 10:30 a. m. 18.
201	41 57 58	78 16 34	Smethport 3i	G. Taylor	11:30	9.7	11:30			Downpour 7:45 to 11 a. m. 18. The reporter estimated that the rainfall for that period alone was enough to fill the container, which overflowed.
202	41 57 50	78 12 35	Coudersport 1h	J. Kemp	10	12.5	10	1	12.5	Downpour 7:30 to 10 a. m. 18.
203	41 57 45	78 20 48	Smethport 2i	R. Leet	6:30 to 9:30	8.0			E21.1	Heavy rain 8:30 to 11 a. m. 18. Container overflowed 9:30 a. m. 18 or before. Presumably no rain between 6:30 and 8:30 a. m. 18.
204	41 57 44	78 08 31	Coudersport 2b	A. Cronk	Before 12	3.0	Before 12		E8.2	Downpour 7-12 a. m. 18. Container lacked 0.57' of being full at noon. Reporter stated that there may have been enough rain after noon to finish filling the container.
205	41 57 43	78 20 06	Smethport 2j	Ewing	8:30 to 9	3.1				Heavy rain 8:30-12 a. m. 18.
206	41 57 42	78 22 00	2h	G. Eastman	8:15 to 9:30	9.2	10	11-15	E22.7	No rain 5:30 to 8:15 and heavy rain 8:15 to 11:15 a. m. 18.
207	41 57 30	78 16 10	3j			*7.9				Container set out during a. m. storm of 18.
208	41 57 24	78 10 37	Coudersport 1i	J. Rupert	7:30 to 11	*7.7	DN	11	E>15.8	Cloudburst 7:30 to 8:30 a. m. 18. No rain 6 to 7:30 a. m. 18.
209	41 56 56	78 14 12	Coudersport 1j	D. Stannard	8:30 to 9+	*7.7	6	2	E20.3	Downpour started 7:30 a. m.; heaviest rain 9 to 10 a. m. 18. Reporter estimated that there was enough rain to fill container almost 4 times. A very little hail night of 17.
210	41 56 50	78 10 43	1k	E. Salisbury	7	3.4	11	11		Hard rain 8 to 11 a. m. 18 of which the heaviest was 8 to 9 a. m.
211	41 56 47	78 09 35	2c	W. Bliss	12:30	12.8				Downpour 8 to 12 a. m. 18.
212	41 56 44	78 35 50	Bradford 2a	E. O'mara	About 12 m. to 9	13.8	12:30	12 m. + 9	13.8	Heavy rain 2 to 5:30 and 9 to 11 a. m. 18.
213	41 56 43	78 23 35	Smethport 2k	O. Ward	12:30	*14.0	5			Very heavy rain 8:30 to 9:30 a. m. 18.
214	41 56 35	78 10 43	Coudersport 1l	T. Elliot	9	12.2	DN		12.2	Downpour period 8 to 11:30 a. m. 18 (5 cloudbursts).
215	41 56 34	77 56 24	Genesee 1b	F. Kenyon		*9.6	11	11:30		

See footnotes at end of table.

TABLE 2.—Miscellaneous measurements of rainfall, July 18, 1942—Continued

No. on pl. 2	Latitude (deg., min., sec.)	Longitude (deg., min., sec.)	Quadrangle designation ¹	Reporter or resident	Rainfall between times shown			Storm period between times shown			Remarks	
					July 17 p. m.	July 18		July 17 p. m.	July 18			Total rainfall measured (inches)
						A. m.	P. m.		A. m.	P. m.		
Allegheny River drainage basin—Continued												
216	41 56 27	78 08 45	Pennsylvania—Con. Condersport 2d	E. McGairl.	11	12 m.		*12.5	11	12 m.		Downpour 7:30 to 11:30 a. m. 18.
217	41 56 26	78 10 35	1m.	B. Kellog.	11	12 m.		*10.5	DN	12 m.		Heavy rain 8 to 12 a. m. 18.
218	41 56 17	78 13 59	1n.	L. Chase.	11:45	9		9.8 9.2	11:45	10		Downpour 6:30 to 10 a. m. 18. Observer estimated that about 2" of rain fell after container was full at 9 a. m. 18.
219	41 56 15	78 05 35	2c.	F. Dunshie.	12		4	2.8	12		4	Very hard rain 9 a. m. to 1:30 p. m. 18.
220	41 56 07	78 14 46	1o.	L. Bridge.	10		P. m.	*8.1	10		P. m.	Downpour 7:30 a. m. to 1 p. m. 18.
221	41 56 04	78 13 30	1p.	A. Blanchard.	11		1	*9.4	11		1+	Downpour period 8:30 a. m. to 12 m. 18. A little hail a. m. 18.
222	41 56 00	78 15 22	Smethport 3k.	F. Matthews.	11:30	7:30		*6.7	11:30	About 12 m.		Extremely heavy rain from shortly after 8 to 9:30 a. m. 18.
223	41 55 51	78 15 51	3l.	Woodward.	11:30 10	7:30 12 m. 8		*6.7 *13.4 *9.4				Reporter estimated enough rain fell to have filled container again.
224	41 55 36	78 20 56	2n.	R. Kelley.		5:30 to 11 12 m.		*3.9	DN	11		A little hail about 9 a. m. 18.
225	41 55 33	78 21 24	2m.	W. Barnes.	11:45			8.5	11:45	12 m.		Heavy rain 9 a. m. to 12:30 p. m. 18. Container (capacity 6.1 inches) filled in a 15- or 20-minute period beginning 9 a. m. 18 except for estimated 1 in. remaining from previous night's storms.
226	41 55 28	78 22 30	2l.	Smith.	DN	9:15		6.1	DN	12:30		
227	41 55 02	78 06 48		Keir.		8 to 8:30		3.0				

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228	41 54 55	78 17 58	Smethport 6a	L. Jordan	11	8:30	1	*9.6	12	3	No rain 7 to 8:30 a. m. 18.
229	41 54 46	78 18 03	6b	C. Hubble		8:30	12:30	5.8	11		Heavy rain started 8:30 a. m. 18.
						8:30		*15.0			
230	41 54 45	78 06 16	Condersport 5a	P. Fisk	DN	7		*18.8	DN	2 or 3	
						7-10		5.2			
								*3.9			
231	41 54 38	78 13 36	4a	N. Austin	DN		2:30	*9.1	DN	2:30	Very heavy rain 8 a. m. to 2:30 p. m. 18. Light hail just before heavy rain.
								*7.3			Heaviest rain 10 a. m. to 12:30 p. m. 18.
232	41 54 27	78 23 45	Smethport 5a	A. Rice		11:30	12:30	*2.9	11:30	2	Reporter believed there was enough rain to have filled container again.
233	41 54 18	78 03 45	Condersport 6c	R. Haskins	DN	10		*8.4			No rain 6 to 8 a. m. 18.
											Very heavy rain 12 to 5:30 and 6:30 to 8:30 a. m. 18.
234	41 54 14	78 03 45	6a	A. Swift	10		P. m.	10.0	10	P. m.	Container may have leaked a little at beginning of storm.
235	41 54 12	78 07 18	5b	W. Stephenson	DN	5:30	1	*7.0	DN	1	Heavy rain 8:30 a. m. to 12:30 p. m. 18.
236	41 54 08	78 01 06	6d	F. Amidon	12	7 to 9:30		6.8	12		Hard rain 7:30-12 a. m. 18.
								6.4			A little hail about 10:45 a. m. 18.
237	41 54 07	78 04 18	6b	F. Rachick	9:30		2	8.4	9:30	2	Heavy rain beginning between 7:30 and 8:15 a. m. 18 and ending about 12:30 p. m. 18. Reporter stated more rain fell during heavy rain after container was full than before (7.7 inches).
238	41 53 48	78 19 42	Smethport 6c	Lewis	10		P. m.	*9.7	10	P. m.	Heavy rain started 8:30 a. m. 18.
239	41 53 45	78 20 40	5b	J. Cooney				*8.8			A little hail about 10:45 a. m. 18.
240	41 53 35	78 15 29	6d	H. Evans		7:45 to 10:15		7.7			Heavy rain started 8:30 a. m. 18.
241	41 53 31	78 20 57	5c	Glover		8:45 to 9:30		5.8	11	2	Heavy rain started 8:30 a. m. 18.
242	41 53 24	78 06 28	Condersport 5c	L. Shattuck	DN			*8.3			Heavy rain 8 to 11:30 a. m. 18.
243	41 53 23	78 19 02	Smethport 6e	Cantwell	DN	10	4	*8.1	DN	4	Heavy rain started 8:30 a. m. 18.
244	41 53 12	78 16 30	6f	A. McNeil		8 to 8:20		1.8	DN	12 m.	Heavy rain about 8-12 a. m. 18.
245	41 53 09	77 59 31	Genesee 4c	J. Hemple	DN	8	1	*6.6	DN	1	Container sheltered.
						8		*7.9			
						8		*8.5			
246	41 53 07	78 06 44	Condersport 5d	E. Fosmer	DN	8 to 9:30		3.4	DN	P. m.	Heaviest rain from 8:30-12 a. m. 18. Reporter estimated nearly as much rain after 9:30 a. m. 18 as before.
								9.7			

See footnote at end of table.

259	41 51 59	78 11 45	Coudersport 4d.....	C. Baum.....	12	6	12:30	2.9 2.3 8.6 14.2	12	12:30	8.6	Downpour 8 a. m. to 12:30 p. m. 18.
260	41 51 46	78 19 36	Smethport 6h.....	R. Simar.....							14.2	Downpour 7 to after 10 a. m. 18.
261	41 51 45	78 00 11	Coudersport 6e.....	A. Sturdevant.....	12	6:30 6:30 to 9 9-11		3.7 7.9 1.9 6.9 1.8	12	2		Very hard rain 12 to 2, 4:30 to 7, and 9 to 11 a. m. 18. Nearly all of storm rainfall measured.
262	41 51 38	78 15 14	Smethport 6i.....	G. Scott.....		11 to 11:20			12	12:30		Heavy rain 12 to 5 a. m. 18. A few large hail stones about 9:30 a. m. 18. Downpour period 9:30 to 11:30 a. m. 18. Amount is for part of downpour period on 18. Container may have overflowed and children dipped out part of catch. Downpour started at 9 a. m. 18. Hard showers 2:30 to 3 p. m. 17, also hail. Very heavy rain 11 p. m. 17 to 9 a. m. 18 and hard rain 10-12 a. m. 18. Amount small owing to small leak in container.
263	41 51 11	78 24 50	5i.....	Harris.....				*11.5				
264	41 51 04	78 13 20	Coudersport 4e.....	M. Thornshelley.....				6.7				
265	41 51 02	77 56 28	Genesee 4f.....	Aheara.....	2:30		P. m.	10.0	2:30	P. m.	10.0	
266	41 51 01	78 21 47	Smethport 5j.....	R. Teeter.....				17.2				
267	41 50 55	78 01 03	Coudersport 6g.....	F. Kenyon.....				*12.5				
268	41 50 54	78 13 47	4g.....	A. Burr.....	DN	12 m.		*13.8	DN	P. m.	17.2	
269	41 50 44	78 03 52	6f.....	R. Brock.....	11:20	6		5.7	11:20	12:30		Very hard rain from before 8 a. m. to 12 m. 18. Reporter estimated there was nearly enough rainfall to have half-filled container again.
270	41 50 42	78 18 23	Smethport 6j.....	S. Carlson.....	12	7		*7.3	12	1		Heaviest part of rain on 18 was over by 10:30 a. m. Hard rain 12 to 4 a. m. 18. Downpour period 6 a. m. to 1 p. m. 18. Reporter estimated there was enough rain to have filled container again.
271	41 50 42	78 06 05	Coudersport 5f.....	J. Biros.....	10		3:45	6.4	10	3:45	6.4	Excessive rain 7:30 a. m. to some time in p. m. 18.
272	41 50 32	78 14 40	4h.....	K. Cooke.....	12		3:30	*12.7	12	3:30		Downpour 7:30 a. m. to 2:30 p. m. 18. A little hail about noon 18.
273	41 50 28	78 04 03	6h.....	F. Snyder.....		9:30 to 10:30		2.5	DN	12 m.		

See footnote at end of table.

TABLE 2.—Miscellaneous measurements of rainfall, July 18, 1942—Continued

No. on pl. 2	Latitude (deg., min., sec.)	Longitude (deg., min., sec.)	Quadrangle designation 1	Reporter or resident	Rainfall between times shown			Storm period between times shown			Remarks		
					July 17 p. m.	July 18 A. m.	July 18 P. m.	July 17 p. m.	July 18 A. m.	July 18 P. m.		Total rainfall measured (inches)	
274	41 50 27	78 24 13	Pennsylvanian—Con. Smethport 5k.....	A. Austin.....		8	P. m.	3.3		12:30	P. m.	E7.1	No rain 4 to 9 a. m. 18. Heavy rain 12:30-4 a. m. and 9-12 a. m. 18. Heavy rain 7:45 a. m. to 12:30 p. m. 18. The heaviest rain was around noon. Hard rain 9 to 10:45 a. m. 18. Very heavy rain 7 to 11:30 a. m. 18.
275	41 50 24	78 17 09	6k.....	G. Appolt.....		7:45	About 1	*30.8	11		About 1		Amount greater than 6.1 inches by 8 a. m. 18. Reporter believed as much or more rain fell after 8 a. m. 18 as before. Heavy rain 9 a. m. to 1:30 p. m. 18.
276	41 50 22	78 24 11	5l.....	F. Wilcox.....		7:30	P. m.	*9.8	DN			E>22.4	Amount greater than 6.1 inches by 8 a. m. 18. Reporter believed as much or more rain fell after 8 a. m. 18 as before. Heavy rain 9 a. m. to 1:30 p. m. 18.
277	41 50 17	78 03 22	Coudersport 6j.....	M. Peet.....	11		2:30	*6.7	11		2:30		Amount greater than 6.1 inches by 8 a. m. 18. Reporter believed as much or more rain fell after 8 a. m. 18 as before. Heavy rain 9 a. m. to 1:30 p. m. 18.
278	41 50 15	78 03 42	6i.....	A. Thompson.....	11		1	*6.8	11		1		Amount greater than 6.1 inches by 8 a. m. 18. Reporter believed as much or more rain fell after 8 a. m. 18 as before. Heavy rain 9 a. m. to 1:30 p. m. 18.
279	41 50 06	78 23 30	Smethport 5m.....	K. Austin.....	12		1:30	*13.1	12		1:30		Amount greater than 6.1 inches by 8 a. m. 18. Reporter believed as much or more rain fell after 8 a. m. 18 as before. Heavy rain 9 a. m. to 1:30 p. m. 18.
280	41 49 59	78 17 39	6a.....	R. Hardes.....	DN		6	4.5					Amount greater than 6.1 inches by 8 a. m. 18. Reporter believed as much or more rain fell after 8 a. m. 18 as before. Heavy rain 9 a. m. to 1:30 p. m. 18.
281	41 49 57	78 21 33	8a.....	C. Strang.....	11		4:30	*12.1	11		4:30		Amount greater than 6.1 inches by 8 a. m. 18. Reporter believed as much or more rain fell after 8 a. m. 18 as before. Heavy rain 9 a. m. to 1:30 p. m. 18.
282	41 49 54	78 15 06	9c.....	H. Brown.....		12:30	12:30	*10.8		12:30	12:30		Amount greater than 6.1 inches by 8 a. m. 18. Reporter believed as much or more rain fell after 8 a. m. 18 as before. Heavy rain 9 a. m. to 1:30 p. m. 18.
283	41 49 53	78 19 17	9b.....	E. Strang.....		7	P. m.	6.5				E11.9	Amount greater than 6.1 inches by 8 a. m. 18. Reporter believed as much or more rain fell after 8 a. m. 18 as before. Heavy rain 9 a. m. to 1:30 p. m. 18.
284	41 49 52	77 51 05	Genesee 8a.....	G. Harvey.....	12	5:30	5:30	*12.5	12	12 m.	12 m.	18.5	Amount greater than 6.1 inches by 8 a. m. 18. Reporter believed as much or more rain fell after 8 a. m. 18 as before. Heavy rain 9 a. m. to 1:30 p. m. 18.
285	41 49 48	78 24 56	Smethport 8b.....	P. Olson.....	9:30	A. m.		4.4	9:30		3		Amount greater than 6.1 inches by 8 a. m. 18. Reporter believed as much or more rain fell after 8 a. m. 18 as before. Heavy rain 9 a. m. to 1:30 p. m. 18.
286	41 49 35	78 19 56	9d.....	G. Stoker.....	DN	11:30		*9.3	DN		3		Amount greater than 6.1 inches by 8 a. m. 18. Reporter believed as much or more rain fell after 8 a. m. 18 as before. Heavy rain 9 a. m. to 1:30 p. m. 18.
287	41 49 34	78 08 17	Coudersport 8a.....	E. Harned.....	DN		1:30	8.5	DN		1:30	8.5	Amount greater than 6.1 inches by 8 a. m. 18. Reporter believed as much or more rain fell after 8 a. m. 18 as before. Heavy rain 9 a. m. to 1:30 p. m. 18.

Allegheny River drainage basin—Continued

288	41 49 19	78 00 40	9a	W. Perry	11	2	*13.5 *9.9	11	2	Very hard rain 11 p. m. 17 to 3:30 a. m. and 6 to 9 a. m. 18. The 13.5 inches record was obtained at a poor location and therefore is somewhat questionable.
289	41 49 13	78 03 23	9b	W. Scott	DN	3	*6.5	DN	3	Very hard rain 9 to 12 a. m. 18.
290	41 49 08	78 11 20	7a	H. Baker	12	11	11.5	12	P. m.	Downpour 8 a. m. to 12 m. 18. Container full by or before 11 a. m.
291	41 49 08	78 08 36	8c	R. Green	11	10	*8.3 *8.0	11	2	Reporter estimated there was enough rain to have filled container again.
292	41 49 07	78 15 48	Smithport 9g	M. Manning	11	10		11	2	
293	41 49 06	78 08 35	Coudersport 8c	T. Gross	7:30	P. m.	*9.0 9.0			E15.1 9.0
294	41 49 04	78 25 09	Smithport 7a	W. Raymer	12:30	12:30	5.0		12:30	10.0
295	41 49 02	78 17 56	9k	L. Scherer	8:30 8:30	12 m. +	5.0 10.0 6.4			
296	41 49 02	78 22 54	8c	G. Smith	8	12:30	*12.0	DN	P. m.	6.4
297	41 49 00	78 17 13		O. Crossman		12:30	*9.5			E>18.9
298	41 49 00	78 16 52	Smithport 9f	E. Dolaway	8	12:30 to end of storm	1.7			
299	41 48 55	78 25 09	7b	E. Kibble			*11.2 8.0 5.8			8.0 5.8
300	41 48 53	77 53 10	Genesee 8b	A. Gleason	11:30	P. m.	*15.4	11:30	P. m.	Container may have been sheltered by house. Very heavy rain 11:30 p. m. 17 to 8 a. m. 18 and hard rain 8:30 a. m. to 1:30 p. m. 18.
301	41 48 47	78 17 02	Smithport 9h	Minnier	9:50	P. m.	18.2	11:30	P. m.	Very heavy rain 11:30 p. m. 17 to 4:30 a. m. 18 and hard rain 4:30-12 a. m. 18.
302	41 48 44	77 54 36	Genesee 8c	Butler	11:30	P. m.	11.0	11:30	P. m.	Heavy rain p. m. 17 to 10:30 a. m. 18.
303	41 48 15	77 53 30	8d	Abbey	11	7:30	*24.2	11	7:30	Hard rain 7 to 8:30 and downpour 8:30 to 10 a. m. 18.
304	41 48 13	78 17 11	Smithport 9i	J. Carlson	12:15	1	*14.6	12:15	1	Very heavy rain 12 to 6 a. m. and 7 a. m. to 1 p. m. 18.
305	41 48 09	77 54 10	Genesee 8e	E. Boucher	12		14.0			Amount for night of 17 only. Very heavy rain 11 p. m. 17 to 9 a. m. 18.
306	41 47 56	78 25 10	Smithport 7e	A. Moser	12 m.	12 m.	2.8	12:30	4:30+	6.5
307	41 47 44	77 56 45	Genesee 7a	R. Yentzer	11	12 m.	6.5	11	12:15	1
308	41 47 38	78 14 58	Coudersport 7b	E. Johnson	12:15 to 11	12:15 to 11	*9.2	12:15	1	Downpour 8 to 11 a. m. 18.

See footnote at end of table.

TABLE 2.—Miscellaneous measurements of rainfall, July 18, 1942—Continued

No. on pt. 2	Latitude (deg., min., sec.)	Longitude (deg., min., sec.)	Quadrangle designation ¹	Reporter or resident	Rainfall between times shown			Storm period between times shown			Remarks
					July 17 p. m.	July 18 A. m. P. m.	Amount measured (inches)	July 17 p. m.	July 18 A. m. P. m.	Total rainfall measured (inches)	
309	41 47 34	78 12 57	<i>Pennsylvanite</i> —Con. Coudersport 7d.....	F. Fuller.....		1 to 9:30	*7.7	1	12 m.	Downpour 5 to 10 a. m. 18. Reporter estimated there was enough rain to have filled container again.	
310	41 47 34	77 53 31	Genesee 8f.....	O. Knickerbocker.....	6		P. m.	6	P. m.	Heavy rain 8 p. m. 17 to 9:45 a. m. 18.	
311	41 47 33	78 08 56	Coudersport 8d.....	T. Fitzsimmons.....	12 12	8 11:30	1.8 *8.7	12	P. m.	Heavy rain started 10 a. m. 18. Reporter estimated there was enough rain to have filled container again.	
312	41 47 27	78 13 39	7c.....	V. Tyler.....	10:30	4 4-6 7:30 to 9:30	1.2 *5.5 *8.0	10:30	8	Downpour 4:30 to 5 a. m. and cloudburst 7:30 to 9:30 a. m. 18.	
313	41 47 19	78 16 32	Smethport 9j.....	T. Fitzsimmons.....		8:30 to 10:45 5:30	*14.7 *15.9	1 to 12 m.			
314	41 47 13	77 54 21	Genesee 8g.....	Blough.....	6		*6.2	6	1	Heavy rain 11 p. m. 17 to 6:30 a. m. 18.	
315	41 47 09	78 07 09	Coudersport 8b.....	C. Clark.....		1	*8.3	1	2:30	Standard rain gage operated Coudersport Water Co.	
316	41 47 07	78 18 45	Smethport 9c.....	Moss.....	8		P. m.	DN	P. m.	Downpour period 8-12 a. m. 18.	
317	41 47 06	78 01 42	Water Co.....	Water Co.....			7.7				
318	41 47 05	78 11 28	Coudersport 7g.....	O. Saiers.....	11:40		P. m.	11:40	P. m.	Hard rain 12 to 4:15 a. m. and 8:30 a. m. to 2 p. m. 18.	
319	41 47 04	78 14 43	7e.....	E. Anderson.....		A. m. after 8:30	*7.2	DN	2		
320	41 46 56	78 03 42	9d.....	O. Senak.....	11	9	*9.6			Very hard rain 5 to 9 a. m. 18.	
321	41 46 52	78 12 21	7f.....	R. McAlpine.....	DN	7	*11.9	DN	3:30	No rain 8 to 8:30 a. m. 18.	
322	41 46 50	78 16 00	Smethport 9l.....	M. Turner.....		4	*12.5	4	2	Downpour started 9 a. m. 18.	

Allegheny River drainage basin—Continued

FLOODS OF JULY 18, 1942, PENNSYLVANIA

355	41 42 00	78 30 00	Mount Jewett 3c.	F. Anderson.				8.0				8.0	Heavy rain 8:30 a. m. to 2 p. m. 18.
356	41 41 11	78 23 30	Colegrove 2c.	J. Ashby.	DN	3:30	7.1	DN	3:30			7.1	Container 6 or 8 feet from side of house, which may have decreased catch. Amount caught in 30 minutes a. m. 18.
357	41 41 08	78 10 20	Emporium 1c.	J. Barth.			1.2						Downpour 2 to 4 p. m. 18. Rain probably more than 9.0" as container had a pin hole leak when examined over 1 month later.
358	41 40 56	78 10 47	1f.	H. McCloud.	1-12		*9.7		1			9.0+	Hard rain 2 to 5 a. m. and 2 to 4 p. m. 18. Amount is for daytime 18 only and is small as container was wood-stave keg that leaked while being soaked up.
359	41 40 34	78 40 47	Mount Jewett 1a.	G. Jeffords.	12:45	4	9.0+		12:45	4			Hard rain 2 to 5 a. m. and 2 to 4 p. m. 18. Amount is for daytime 18 only and is small as container was wood-stave keg that leaked while being soaked up.
360	41 40 22	78 40 52	1b.	C. Weaver.	A. m.	P. m.	*5.5						Amount measured early a. m. 18. Reporter estimated as much rain fell p. m. 18, as in two other periods of which only the first was measured.
361	41 40 20	78 25 00	Colegrove 1b.	H. Minnier.		12:30	*12.0						Heavy rain 1 to 4:30 p. m. 18. No rain 5 a. m. to 1 p. m. 18. Amount is for 10 minutes about 3 p. m. 18.
362	41 40 17	78 23 14	2d.	F. Oviatt.		10 3	3.9	10	3			E8.5	Reporter estimated as much rain fell p. m. 18, as in two other periods of which only the first was measured.
363	41 40 14	78 24 47	1c.	G. Rawley.			*4.2						Heavy rain 1 to 4:30 p. m. 18. No rain 5 a. m. to 1 p. m. 18. Amount is for 10 minutes about 3 p. m. 18.
364	41 39 27	78 27 27	4a.	B. Hertzog.		1 to	*8.1						Reporter estimated as much rain fell p. m. 18, as in two other periods of which only the first was measured.
365	41 38 57	78 34 04	Mount Jewett 6a.	R. Howard.		4:30	*4.2						Heavy rain 1 to 4:30 p. m. 18. No rain 5 a. m. to 1 p. m. 18. Amount is for 10 minutes about 3 p. m. 18.
366	41 37 10	78 34 45	6b.	J. Szymanski.			8.0	12	3:30			E17.6	Reporter estimated as much rain fell p. m. 18, as in two other periods of which only the first was measured.
367	41 37 07	78 34 59	5a.	L. Allegretto.		1:20 to 6	4.6	DN	6			E7.8	Heavy rain 1 to 4:30 p. m. 18. No rain 5 a. m. to 1 p. m. 18. Amount is for 10 minutes about 3 p. m. 18.
368	41 36 14	78 34 34	6c.	Bodislow.	DN		18.5	DN				18.5	Reporter estimated as much rain fell p. m. 18, as in two other periods of which only the first was measured.
369	41 36 11	78 37 30	5c.	J. Surman.	DN	P. m.	*9.0	DN				8.1	Heavy rain 2 to 4 p. m. 18. Hard rain 2:30 to 6 p. m. 18. Amount is for 10 minutes about 3 p. m. 18.
370	41 36 08	78 38 18	50.	J. Clopp.	DN	A. m. 6	5.7	DN					Reporter estimated as much rain fell p. m. 18, as in two other periods of which only the first was measured.
371	41 36 00	78 37 02	5d.	R. Market.	DN	A. m. P. m.	8.1	DN				7.2	Heavy rain 2 to 4 p. m. 18. Hard rain 2:30 to 6 p. m. 18. Amount is for 10 minutes about 3 p. m. 18.
372	41 35 56	78 34 27	6d.	A. Smith.			*3.2	DN					Reporter estimated as much rain fell p. m. 18, as in two other periods of which only the first was measured.

391	42 20 01	77 53 22	Canasraga 5b.....	C. Almeter.....	10	6:30	5.0	10	6:30	5.0	Very heavy rain 4 to 5 a. m. 18.
392	42 19 58	77 55 11	7a.....	J. Herdman.....	10	5:30	5.1	10	5:30	5.1	Heaviest rain 3 to 4 a. m. 18.
393	42 19 58	77 54 19	8a.....	J. Almeter.....	10	5:30	7.9	9	6	7.9	Heavy rain started about 10 p. m. 17; heaviest rain 3 to 4 a. m. 18.
394	42 19 30	78 02 19	Angelia 9a.....	J. Stafer.....	9	A. m.	5.3	8		6.9	Reporter estimated that total storm rainfall did not exceed 6' or 7'.
395	42 19 23	78 00 51	9b.....	L. Graham.....	8		6.9				Very heavy rain 4 to 5 a. m. 18.
396	42 19 18	77 56 51	Canasraga 7b.....	J. Young.....		2 to 4	*4.9				Heaviest rain 4 to 5 a. m. 18.
397	42 19 13	77 57 06	7c.....	R. Jennings.....	11	8	5.1	11	8	5.1	Heaviest rain 4 to 5 a. m. 18.
398	42 17 33	77 55 05	7d.....	L. Ives.....	11	5	*8.2	11	5:45	6.8	Heaviest rain about 3 a. m. 18.
399	42 15 29	77 53 48	8b.....	F. Lonsberry.....	12	5:30	6.8	12	5:30	3.8	Heaviest rain about 3 a. m. 18.
400	42 14 45	77 57 35	Wellsville 1a.....	W. Keenan.....	12		3.8				Heaviest rain about 4 a. m. 18.
401	42 13 59	77 56 07	1b.....	R. Day.....	8:30	5	*15.5	8:30	5		Heaviest rain about 4 a. m. 18.
402	42 13 10	78 03 39	Belmont 3a.....	R. Stowell.....	10	6	5.0				Heaviest rain about 4 a. m. 18.; very little rain after 6 a. m. 18.
403	42 12 45	77 49 22	Wellsville 3a.....	L. Snyder.....		6	4.0			4.0	Heaviest rain about 3 a. m. 18.
404	42 10 28	77 51 15	2a.....	H. Beckwith.....	10:45		*14.1	10:45	6		Standard rain gage located near buildings. Heaviest rain about 1 a. m. 18.
405	42 09 38	77 51 26	5a.....	F. Mead.....	10	6+	9.0	10	6+	P. m.	Downpour 5 to 6 a. m. 18.
406	42 08 20	77 54 02	5b.....	C. Burdick.....	6+	7	4.0	6	7	4.0	Light hail for 10 or 15 minutes 3 p. m. 17.
407	42 07 04	77 56 53	4a.....	E. Rowe.....	5:30		3.5	5:30		8.8	Container held 6.2' at 8:30 a. m. 18.
<i>Pennsylvania</i>											
408	41 55 04	77 54 09	Genesee 2a.....	R. Storey.....	9:30	12 m.	32.7	9:30	12 m.	32.7	Heavy thundershowers 12 to 3 a. m. and heavy rain 8:30 to 9:30 a. m. 18.
409	41 54 44	77 55 17	4a.....	T. Trebik.....	10		3.6	10		3.6	Heavy rain 12 to 7 a. m. 18.
410	41 54 10	77 54 16	5a.....	James.....			8.8			8.8	
411	41 54 02	77 55 50	4b.....	E. Burch.....			5.6				
412	41 50 54	77 50 40	5b.....	A. Torok.....	12	6	*5.8	12	12 m.		

¹ See, Storm of July 17-18, 1942, New York-Pennsylvania, supplement to Daily and Hourly Precipitation compiled by Hydrologic Unit, U. S. Weather Bureau Office, Albany, N. Y.

THE ISOHYETAL MAP

The drawing of an isohyetal map is anything but an accurate procedure. The best precipitation gage can catch only the rain that falls at that particular spot. It is generally assumed that rainfall varies rather uniformly between points of measurement, except for possible topographic considerations. Such an assumption may not be true. An intermediate gage as little as a hundred feet from one of the points of measurement might have caught a rainfall significantly different from that indicated by the adjacent points of measurement for the same storm, especially if the rainfall were spotty, as during the storm of July 18, 1942. When an isohyetal map is to be based on miscellaneous observations of rainfall, such as given in table 2, the task of drawing the isohyetal lines is exceedingly difficult. Did a container that overflowed, barely overflow, or would it have been filled two or three times more if it had been emptied? Do two containers quite close together, but apparently catching radically different amounts of rainfall, represent an actual variation in rainfall, a poor location for a catch, or inaccurate information supplied by one or both reporters? In using miscellaneous observations of rainfall, the most satisfactory procedure is to accept all data at face value—and this was done. There are usually several ways of interpreting the same data, however, and it is frequently impossible to show that any one interpretation is better than another.

The center of greatest precipitation shown on plate 2 is at Port Allegany. There an isohyetal line was drawn to indicate a precipitation of 35 inches. That amount of rainfall is based on miscellaneous measurements 275 and 301. The rain at measurement point 275 was caught in the glass jar shown in figure 37, which shows also the exposure conditions of the jar as well as could be reenacted about a month after the rain. This jar was set out about 7:45 a. m.; it filled with rain for a catch of 30.8 inches, and then overflowed. Considering the unmeasured rainfall prior to 7:45 a. m. and the unknown amount lost by overflow, it would appear that the rainfall at this point was at least 35 inches. That a tremendous rainfall occurred is further indicated by the runoff conditions described under Flood Flows. Measurement 301 showed 18.2 inches of rain after 9:50 a. m. The latest that any rain was reported in the vicinity was at 2 p. m., for measurement 292. That would mean that the 18.2 inches fell in less than 4 hours and 10 minutes. The curve in figure 36 shows that 50.8 percent of the total rainfall occurred in that time, from which an estimated total rainfall of 35.8 inches was obtained for measurement point 301. If the 18.2 inches fell in less than 4 hours, the computation procedure would indicate an amount much greater than 35 inches. At the recording gage in Smethport, about 8 miles away, only 32 percent of the storm



FIGURE 37.—Glass jar (table 2, No. 275), on Appolt farm, that overflowed after catching 30.8 inches of rain. Scene was enacted about a month after the rain to show that nearby objects probably had little effect on the catch.

rainfall occurred after 10 a. m., and practically no rain fell after 1 p. m. That record thus indicates also that measurement 301 should be greater than 35 inches for the storm total. For computations of average rainfall the maximum precipitation was assumed to be 36 inches.

In addition to the precipitation center of more than 35 inches at Port Allegany, there are two centers of more than 30 inches at Turtlepoint and northeast of Coudersport. These centers are based on measurements 251 (34.5 inches) and 408 (32.7 inches), respectively. All three centers are within the main area of heavy precipitation. Off to the northeast, at Angelica, N. Y., there is an area of heavy precipitation that, from the appearance of the map, one might think was a separate storm. It should be noted, however, that the lowest isohyetal line drawn is for 4 inches. The two areas of heavy precipitation are in reality part of a general storm that covered practically the entire region shown on the map with precipitation of at least an inch.

The isohyetal map for the storm of July 18, 1942, given in plate 2, is the fourth such map to be compiled. It was drawn by the author

using the other three maps as basic references. The first map, drawn by Stewart, was based on precipitation records, topography, and relative erosion in small streams. For the last-mentioned, Stewart used an arbitrary scale of 0 to 10 to compare the relative amounts of eroded material at the mouths of the smaller streams. These data were helpful, particularly in the areas where rainfall measurements could not be obtained. Stewart's map was reviewed by the Weather Bureau and modified slightly to take into account the meteorologic characteristics of the storm. The map was reproduced in the Weather Bureau report. Subsequently, the Corps of Engineers made an extensive hydrologic analysis of the storm. As a result of that analysis, it was concluded by the Corps of Engineers that the Weather Bureau map showed too much total precipitation over the storm area for the runoff observed. The Corps of Engineers therefore prepared a new map, for use in the studies being made, that shows considerably less precipitation. The map for this report (pl. 2) is basically the Weather Bureau map redrawn in such a manner that wherever an acceptable interpretation of the data could be made showing less precipitation than the Weather Bureau map, that one was used. The resulting map is quite similar to the one prepared by the Corps of Engineers. The isohyetal lines were drawn to be fully consistent with the data in tables 1 and 2.

AREA-DEPTH RELATIONS

The areas enclosed within the several isohyetal lines on plate 2 were measured with a planimeter. The total area within the heavier lines, 25- and 8-inch, is 14 and 1,215 square miles, respectively. The 4-inch lines enclose a total area of 3,100 square miles. The greatest average precipitation over areas of various sizes is given in table 3. Differences between these values and similar ones based on the isohyetal map drawn by the Corps of Engineers are believed to be insignificant. No attempt was made to distribute the precipitation with respect to time; only the total storm was studied.

TABLE 3.—*Maximum average precipitation over indicated areas for storm of July 17-18, 1942*

Area (square miles)	Precipi- tation (inches)	Area (square miles)	Precipi- tation (inches)	Area (square miles)	Precipi- tation (inches)
1	35.8	50	25.1	500	16.4
5	32.6	100	22.6	1,000	13.5
10	30.5	200	20.0	2,000	10.5
20	28.3				

FLOOD FLOWS

Flood flows within the storm area were the greatest ever recorded on many of the streams, including those in Allegheny River basin above Kinzua, in the upper part of Clarion River drainage basin, Driftwood Branch Sinnemahoning Creek, and First Fork Sinnemahoning Creek (Pa.), and Karr Valley Creek (N. Y.). In the major stream channels leaving the area, however, the flood flows diminished rapidly in relation to previous maximum flows. The absence of outstanding floods in the lower reaches of the streams outside the storm area probably was because the heavy rainfall was divided among three major drainage basins.

Peak flows in the smaller streams in the area of intense precipitation must have been tremendous, as evidenced by the enormous erosion that took place. On the Appolt farm near Port Allegany, the only channel for draining the hillside before the storm was a worn place in the meadow hardly more than a foot wide (figure 38). The runoff from this storm cut a new channel about 5 feet deep and more than 10 feet wide. It was here that rainfall measurement 275 was obtained, as discussed on page 114. On the neighboring Taylor farm much the same events took place. The road between the two farms was blocked in several places by mud flows and slides. A view of the upper end of the debris cone formed as a result of the tremendous erosion is shown in figure 33.

The drainage basins for the channels on the Appolt and Taylor farms extend only a short distance on each side and to the top of the hill shown in figures 33 and 38. Although the hill had dense forest cover, it was evident that overland flow occurred under the trees. The forest litter had been cut up by a myriad of channels as the water from the intense rain flowed away. An attempt was made to estimate the maximum discharge in the channels draining these two areas. Both channels were scoured out to bedrock in many places and at several points the ledge of bedrock formed the head of a falls. At one of these places on the Appolt farm and at two on the Taylor farm, the channel was surveyed for computation of the flow by the critical-depth method. This method assumes that critical flow occurred at the brink of the falls where the cross-sectional area was surveyed. The discharge is then given by the formula (King, 1939, p. 373):

$$Q=5.67\sqrt{\frac{A^3}{T}}$$

in which

Q =discharge in second-feet,

A =cross-sectional area in square feet,

and

T =top width of cross section in feet.



FIGURE 38.—Old and new drainage channels on Appolt farm near Port Allegany, Pa. Man in dark suit points to previous channel, while man with cap aloft stands in channel cut July 18, 1942. Note party surveying critical-depth section, circled in right background.

The upper critical-depth section on the Taylor farm is shown in figure 39. The highwater marks used to obtain the depth of water in the channel were quite well defined. This indicates that they were made late in the storm and after the channel had been scoured out,



FIGURE 39.—Upper critical-depth section on Taylor farm. Photographed by R. C. Culler.

otherwise they would have been obliterated by the tremendous inflow. It is more than likely that velocities were much greater than critical and that the computed discharge is too small. These three measurements indicate the highest discharge in second-feet per square mile of any in the flood area. It should be realized however that the drainage areas are extremely small, and that the errors in measuring them on the small-scale map available may be quite large. Both channels were dry at the time of the author's visit on August 26.

Several parties searched the flood area to locate streams in which outstanding flood flows occurred. Wherever possible, slope-area, contracted-opening, and flow-over-dam measurement sites were chosen and surveyed for the computation of discharge. The slope-area reach on Nelson Run—a tributary of First Fork Sinnemahoning Creek in the area of high precipitation—is shown in the foreground of figure 40. This slope-area reach is about average for those used. Slope-area and other measurements of flow of these small streams made after the flood and under conditions that existed are not regarded as precise



FIGURE 40.—Slope-area reach on Nelson Run. (No. 789.60 in table 4.)

observations in any sense. They do provide, however, the best method of evaluating the discharge. The contracted opening at the new highway bridge at Port Allegany—destroyed by the flood just 3 days after it was opened to traffic—provided a site where a peak discharge of 77,000 second-feet was measured, the largest in Allegheny River drainage basin. (See fig. 41.) In addition to those made at gaging stations and the critical-depth measurements described above, about 50 miscellaneous measurements were made, including several by the Corps of Engineers. The results of those measurements are summarized in table 4. The methods and procedures used are described by Corbett and others (1943, pp. 98-109).

On many streams where flood flows are known to have been quite large, no sites for measurements could be found. Thus, the fact that a stream is shown on plate 2 with no point of measurement indicated does not mean that the flood flow in that stream was not excessive. On the other hand, some streams that produced little flood runoff were measured to show that they did not contribute to the excessive runoff in the larger streams to which they are tributary.

Following are some of the streams in which excessive flood flow occurred that were examined for measuring sites without success: Norcross, Loque, and Muley Runs tributary to First Fork Sinnema-



FIGURE 41.—Bridge opening on Allegheny River at Port Allegany, Pa., used for contracted-opening measurement. (No. 000.7 in table 4.)

honing Creek; Sevenmile Creek tributary to East Fork Clarion River; Dodge Creek, and Haskell Creek (Pa.), and the upper tributaries of Fivemile Creek in the vicinity of Olean, N. Y.; and Baker Creek near Angelica, N. Y.

In the headwaters of the larger streams, Driftwood Branch Sinnemahoning Creek, Allegheny River, and Clarion River, the flood of July 1942 was the largest known. As the floods progressed downstream and left the storm area they rapidly became smaller in relation to previous floods. That was true also in Genesee River although at the uppermost gaging station at Scio, N. Y., the peak discharge approached, but did not exceed, that for the flood of May 1919, the maximum in 26 years of record.

It is interesting to note the effect of the shape of the river valleys on the shape of the flood wave as it progressed downstream, as illustrated in figure 42. The streams in the Susquehanna River drainage basin flow through narrow valleys with very small flood plains. Consequently very little of the flood waters was held in channel storage, and the flood wave passed down the valley with little change in form. On the other hand, Allegheny River above Red House, N. Y., flows through a broad valley with large flood plains. During the flood of July 1942, those flood plains were inundated to a considerable depth with correspondingly large amounts of channel storage. As a result,

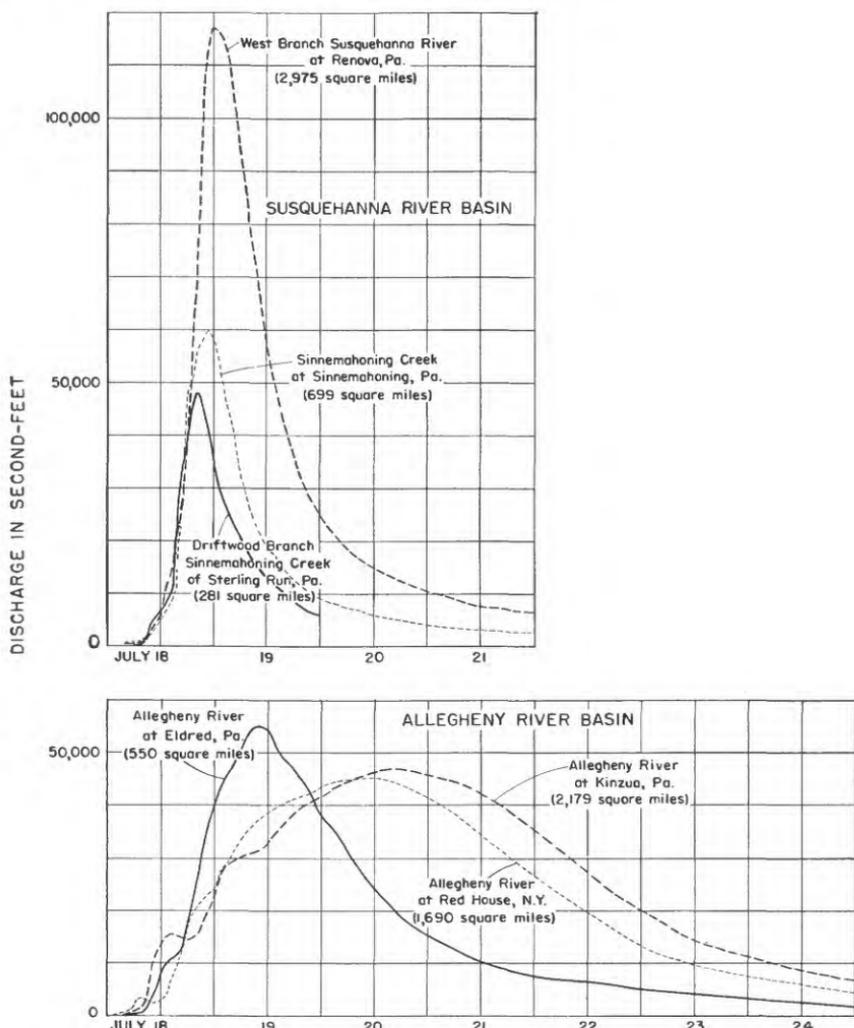


FIGURE 42.—Comparative hydrographs of flood flows in Susquehanna and Allegheny Rivers drainage basins.

the hydrograph of the flood wave at Red House was much flatter and broader than at Eldred. Below Red House, however, the valley of Allegheny River becomes much narrower, which probably accounts for the lack of change in the shape of the hydrograph between Red House and Kinzua. Other factors such as amount and distribution of precipitation have important effects on the size and shape of a flood hydrograph, but computations of channel storage for the flood of July 1942 between Eldred and Red House indicate that this factor alone could account for the diminution of the peak discharge between those two points.

STAGES AND DISCHARGES AT STEAM-GAGING STATIONS

Stage and discharge records at stream-gaging stations within and adjacent to the flood area are given on the following pages. For each station there is given a station description, a table of daily mean discharge for July and August 1942, and a table of gage heights and discharges at indicated times for use in plotting hydrographs of the flood. Daily mean discharges at these stations for the entire water year 1942 have been published in Water-Supply Papers 951, 953, and 954, Surface Water Supply of the United States 1942: Part 1, North Atlantic slope basins (includes Susquehanna River drainage basin); Part 3, Ohio River basin (includes Allegheny River drainage basin); and Part 4, St. Lawrence River basin (includes Genesee River drainage basin), respectively. Methods of obtaining the records are described briefly in those reports and at greater length in the manual, Stream-Gaging Procedure (Corbett and others, 1943).

Records of daily mean discharge alone are usually inadequate for making any detailed studies of a flood such as that of July 1942. To supply the data for the detailed studies necessary in connection with flood control and forecasting, channel improvement, bridge openings, and the design of hydraulic structures in relation to the flood channels of streams is the object of this report. The table of stages and discharges at indicated times has been included for that purpose. Some of the gage heights in those tables, and the discharges based on them, were not obtained from an actual gage-height record, perhaps because the gage used was a nonrecording one, because the recording-gage record was destroyed by the flood, or for some other reason. It is often possible to obtain a fair record of gage height and discharge by drawing a hydrograph through the plotted points representing observed data, using other available information such as observers' notes, meteorological information, studies of runoff at other gaging stations on the same or adjacent streams, interpreted in the light of intimate knowledge of the peculiar local conditions inherent to a particular gaging station. As Geological Survey engineers usually have access to more of such information than would normally be available to other users of the records, and to complete the report where it could be done with reasonable accuracy, they have made estimates of detailed records wherever necessary.

The station description and the tables are largely self-explanatory. The section headed "maxima" may need additional explanation: The first paragraph gives the maximum stage and discharge during the flood of July 1942; the second paragraph gives the maxima during the preceding period of stream-flow record; and the third paragraph

gives the maxima known outside the period of record, usually restricted to those greater than within the period of record.

The gaging-station records are arranged by parts in the order used in the annual reports on Surface Water Supply of the United States referred to previously. The stations are grouped by parts in numerical order of the parts and within each part are arranged in downstream order, the stations on the main stem being given first followed by each tributary in turn.

SUSQUEHANNA RIVER DRAINAGE BASIN

CANACADEA CREEK NEAR HORNELL, N. Y.

LOCATION.—Lat. 42°20'05'', long. 77°41'00'', 35 feet downstream from Morris Bridge, near city limits of Hornell, Steuben County, and 2 miles upstream from mouth.

DRAINAGE AREA.—58.7 square miles.

GAGE-HEIGHT RECORD.—Water-stage recorder graph, except for period 4 p. m. July 31 to 11 p. m. August 3.

DISCHARGE RECORD.—Stage-discharge relation defined by current-meter measurements up to 3,200 second-feet and extended to crest stage by logarithmic plotting. Discharge July 31 to August 3 computed on basis of records for nearby stations. Shifting-control method used August 4-31.

MAXIMA.—July 1942: Discharge, 6,080 second-feet 6:40 a. m. July 18 (gauge height, 7.07 feet).

1924-29, 1938 to June 1942: Discharge, 6,600 second-feet Mar. 17, 1942 (gauge height, 7.35 feet).

The flood of July 1935 reached a stage of about 12.3 feet at present site, obtained in 1940 from floodmarks (discharge, 21,000 second-feet by slope-area method).

Daily mean discharge, in second-feet, 1942

Day	July	Aug.	Day	July	Aug.	Day	July	Aug.	Day	July	Aug.
1.....	7.9	35	9.....	6.6	16	17.....	7.9	16	25.....	15	10
2.....	11	26	10.....	7.4	23	18.....	712	14	26.....	16	9.8
3.....	7.0	20	11.....	43	16	19.....	81	13	27.....	17	9.2
4.....	7.0	16	12.....	16	14	20.....	49	12	28.....	42	9.2
5.....	7.4	15	13.....	11	22	21.....	24	10	29.....	49	9.8
6.....	7.9	14	14.....	9.2	39	22.....	19	11	30.....	37	9.2
7.....	7.4	13	15.....	7.9	18	23.....	17	14	31.....	44	8.6
8.....	6.6	14	16.....	7.9	16	24.....	16	13			
										July	Aug.
Monthly mean discharge, in second-feet.....										42.5	15.7
Runoff, in inches.....										0.83	0.31

Gage height, in feet, and discharge, in second-feet, at indicated time, 1942

Hour	Gage height	Dis-charge	Hour	Gage height	Dis-charge	Hour	Gage height	Dis-charge	Hour	Gage height	Dis-charge
<i>July 17</i>			<i>July 18— Con.</i>			<i>July 18— Con.</i>			<i>July 19— Con.</i>		
6.....	1.11	7.0	3.....	1.18	14	2.....	2.28	352	6.....	1.43	99
11.....	1.12	7.9	4.....	1.28	26	3.....	2.18	309	N.....	1.35	77
6.....	1.12	7.9	5.....	1.40	44	4.....	2.10	276	6.....	1.29	60
8.....	1.11	7.0	6.....	5.95	4,170	5.....	2.01	242	12.....		48
10.....	1.11	7.0	7.....	6.20	4,570	6.....	1.94	217	<i>July 20</i>		
12.....	1.16	12	8.....	4.50	2,190	9.....	1.79	168			
<i>July 18</i>			9.....	3.67	1,320	12.....	1.68	137	3.....	1.45	81
			10.....	3.25	954	<i>July 19</i>			6.....		1.36
1.....	1.16	12	11.....	2.88	681				N.....		1.29
2.....	1.16	12	N.....	2.61	517	6.....		1.22	36		
			1.....	2.42	418	12.....		1.17	28		

SUPPLEMENTAL RECORD.—July 18, 6:40 a. m., gage height 7.07 feet, discharge 6,080 second-feet.

KARR VALLEY CREEK AT ALMOND, N. Y.

LOCATION.—Lat. 42°18'40", long. 77°45'05", 500 feet downstream from McHenry Valley Creek, three-quarters of a mile upstream from mouth, and 1 mile upstream from Almond, Allegany County. Datum of gage is 1,353.68 feet above mean sea level (levels by Corps of Engineers).

DRAINAGE AREA.—27.6 square miles.

GAGE-HEIGHT RECORD.—Water-stage recorder graph except for period 5 a. m. July 18 to 3 a. m. July 20 for which a graph was drawn based on floodmarks and frequent readings of staff gage.

DISCHARGE RECORD.—Artificial control of concrete. Stage-discharge relation defined by current-meter measurements up to 1,600 second-feet and extended to slope-area measurement for crest gage height.

MAXIMA.—1942: Discharge, 5,900 second-feet 5:45 a. m. July 18 (gage height, 8.8 feet from floodmarks).

1937–41: Discharge, 3,800 second-feet March 31, 1940 (gage height, 5.8 feet, from floodmark) from rating curve extended above 1,600 second-feet by logarithmic plotting.

Daily mean discharge, in second-feet, 1942

Day	July	Aug.	Day	July	Aug.	Day	July	Aug.	Day	July	Aug.
1.....	1.1	11	9.....	0.8	3.0	17.....	1.4	4.0	25.....	2.6	1.6
2.....	1.0	6.3	10.....	1.2	6.8	18.....	513	3.2	26.....	2.9	1.5
3.....	.9	4.7	11.....	18	4.4	19.....	51	2.6	27.....	4.0	1.4
4.....	.9	3.4	12.....	5.7	3.4	20.....	28	2.1	28.....	10	1.4
5.....	.9	2.8	13.....	3.0	11	21.....	14	1.8	29.....	15	1.3
6.....	1.0	2.4	14.....	2.0	15	22.....	7.9	1.6	30.....	12	1.2
7.....	1.0	2.1	15.....	1.6	5.8	23.....	5.2	1.7	31.....	16	1.1
8.....	.9	2.5	16.....	1.4	4.2	24.....	3.6	1.6			

	July	Aug.
Monthly mean discharge, in second-feet.....	23.5	3.77
Runoff, in inches.....	.98	.16

Gage height, in feet, and discharge, in second-feet, at indicated time, 1942

Hour	Gage height	Discharge	Hour	Gage height	Discharge	Hour	Gage height	Discharge	Hour	Gage height	Discharge
<i>July 17</i>			<i>July 18— Con.</i>			<i>July 18— Con.</i>			<i>July 19</i>		
N.....	1.73	1.3	6.....	8.13	4,980	4.....	3.20	201	3.....	2.72	70
12.....	1.82	2.8	7.....	5.70	2,120	5.....	3.13	176	6.....	2.68	63
			8.....	4.90	1,350	6.....	3.06	154	9.....	2.63	54
<i>July 18</i>			9.....	4.40	910	7.....	3.01	139	N.....	2.59	48
1.....	1.89	4.7	10.....	4.06	658	8.....	2.96	126	6.....	2.52	38
2.....	1.91	5.4	11.....	3.77	479	9.....	2.92	115	12.....	2.48	33
3.....	1.92	5.7	N.....	3.58	376	10.....	2.88	105			
4.....	1.98	8.1	1.....	3.46	315	11.....	2.84	96			
5.....	3.00	180	2.....	3.36	268	12.....	2.80	87			
			3.....	3.28	233						

SUPPLEMENTAL RECORD.—July 18, 5:45 a. m., gage height 8.80 feet, discharge 5,900 second-feet.

WEST BRANCH SUSQUEHANNA RIVER AT KARTHAUS, PA.

LOCATION.—At mouth of Mosquito Creek at Karthaus, Clearfield County. Gage is at lat. 41°06'55", long. 78°06'40", 900 feet upstream from highway bridge, 1,200 feet upstream from Mosquito Creek, and 3.3 miles downstream from Moshannon Creek. Datum of gage is 830.59 feet above mean sea level, datum of 1929, New York-Pennsylvania supplementary adjustment of 1943.

DRAINAGE AREA.—1,462 square miles, including that of Mosquito Creek.

GAGE-HEIGHT RECORD.—Water-stage recorder graph.

DISCHARGE RECORD.—Includes flow of Mosquito Creek. Stage-discharge relation defined by current-meter measurements up to 52,000 second-feet.

MAXIMA.—July 1942: Discharge during flood period, 1,120 second-feet 2 a. m. July 19 (gage-height, 2.15 feet).

1918-20, 1940 to June 1942: Discharge, 50,900 second-feet Apr. 1, 1940 (gage-height, about 13.9 feet).

1889-1917, 1921-39: Discharge, about 135,000 second-feet March 18, 1936 (gage-height, about 24.5 feet, from floodmark at highway bridge).

Daily mean discharge, in second-feet, 1942

Day	July	Aug.	Day	July	Aug.	Day	July	Aug.	Day	July	Aug.
1.....	633	386	9.....	410	312	17.....	308	527	25.....	348	701
2.....	786	441	10.....	379	431	18.....	550	481	26.....	317	576
3.....	930	416	11.....	384	481	19.....	874	420	27.....	316	453
4.....	722	415	12.....	379	400	20.....	588	359	28.....	311	390
5.....	594	359	13.....	350	359	21.....	457	321	29.....	290	354
6.....	521	326	14.....	335	447	22.....	407	316	30.....	301	326
7.....	510	280	15.....	321	405	23.....	372	552	31.....	348	298
8.....	458	263	16.....	289	359	24.....	346	800			

	July	Aug.
Monthly mean discharge, in second-feet.....	456	418
Runoff, in inches.....	0.36	0.33

Gage height, in feet, and discharge, in second-feet, at indicated time, 1942

Hour	Gage height	Discharge	Hour	Gage height	Discharge	Hour	Gage height	Discharge	Hour	Gage height	Discharge
<i>July 17</i>			<i>July 18—</i>			<i>July 19—</i>			<i>July 20—</i>		
			Con.			Con.			Con.		
6.....	0.97	302	4.....	1.60	652	2.....	1.71	730	N.....	1.48	576
N.....	1.00	316	6.....	1.68	708	4.....	1.76	769	4.....	1.43	545
6.....	1.00	316	8.....	1.82	817	6.....	1.84	834	8.....	1.38	515
12.....	1.00	316	10.....	1.93	912	8.....	1.82	817	12.....	1.34	492
			12.....	2.08	1,050	10.....	1.77	777			
			<i>July 19</i>			12.....	1.72	738	<i>July 21</i>		
<i>July 18</i>						<i>July 20</i>					
2.....	1.00	316	2.....	2.15	1,120	2.....	1.67	701	N.....	1.28	458
4.....	1.07	350	4.....	2.07	1,040	4.....	1.62	666	12.....	1.22	426
6.....	1.08	354	6.....	2.01	984	6.....	1.57	633	<i>July 22</i>		
8.....	1.13	379	8.....	1.93	912	8.....	1.53	607	N.....	1.18	405
10.....	1.17	400	10.....	1.84	834	10.....	1.51	594	12.....	1.15	390
N.....	1.27	453	N.....	1.77	777						
2.....	1.47	570									

WEST BRANCH SUSQUEHANNA RIVER AT RENOVO, PA.

LOCATION.—Lat. 41°18'50'', long. 77°44'45'', at highway bridge at Renovo, Clinton County, 1 mile upstream from Paddy Run. Datum of gage is 633.99 feet above mean sea level, unadjusted.

DRAINAGE AREA.—2,975 square miles.

GAGE-HEIGHT RECORD.—Water-stage recorder graph.

DISCHARGE RECORD.—Stage-discharge relation defined by current-meter measurements up to 90,000 second-feet and extended to slope-area measurement for crest gage height of flood of Mar. 18, 1936.

MAXIMA.—1942: Discharge, 117,000 second-feet 11:55 p. m. July 18 (gage height, 18.92 feet).

1895–1903, 1905–42: Discharge, 236,000 second-feet March 18, 1936 (gage height, 29.39 feet, from floodmark in gage shelter).

1846–94, 1904: Discharge, about 211,000 second-feet June 1, 1889 (gage height, 27.3 feet, from floodmark).

Daily mean discharge, in second-feet, 1942

Day	July	Aug.	Day	July	Aug.	Day	July	Aug.	Day	July	Aug.
1.....	870	1,510	9.....	860	780	17.....	445	2,130	25.....	2,460	1,540
2.....	880	1,370	10.....	683	840	18.....	26,800	2,300	26.....	2,130	1,220
3.....	1,050	1,330	11.....	683	1,100	19.....	64,800	1,910	27.....	1,890	990
4.....	1,040	1,150	12.....	691	957	20.....	15,800	1,540	28.....	1,980	820
5.....	860	990	13.....	651	830	21.....	8,010	1,250	29.....	1,650	744
6.....	806	850	14.....	566	2,180	22.....	5,180	1,080	30.....	1,440	700
7.....	1,620	762	15.....	500	2,380	23.....	3,820	1,070	31.....	1,330	643
8.....	1,280	736	16.....	465	1,910	24.....	2,960	1,500			

										July	Aug.
Monthly mean discharge, in second-feet.....										4,974	1,262
Runoff, in inches.....										1.93	0.49

Gage height, in feet, and discharge, in second-feet, at indicated time, 1942

Hour	Gage height	Dis-charge	Hour	Gage height	Dis-charge	Hour	Gage height	Dis-charge	Hour	Gage height	Dis-charge
<i>July 17</i>			<i>July 19</i>			<i>July 20— Con.</i>			<i>July 22— Con.</i>		
4.....	0.16	438	2.....	18.70	115,000	2.....	5.92	14,200	4.....	2.91	4,800
8.....	.18	452	4.....	17.93	107,000	4.....	5.72	13,400	8.....	2.80	4,560
N.....	.18	452	6.....	16.65	93,900	6.....	5.51	12,600	12.....	2.70	4,320
4.....	.18	452	8.....	15.15	80,700	8.....	5.31	11,800			
8.....	.16	438	10.....	13.75	68,100	10.....	5.10	11,000	<i>July 23</i>		
12.....	.16	438	N.....	15.50	57,400	12.....	4.92	10,400	4.....	2.60	4,080
			2.....	11.38	48,600				8.....	2.52	3,850
<i>July 18</i>			4.....	10.50	41,400	<i>July 21</i>			N.....	2.44	3,740
2.....	.18	452	6.....	9.77	36,300	4.....	4.57	9,500	4.....	2.43	3,740
4.....	.27	515	8.....	9.05	30,700	8.....	4.28	8,600	8.....	2.40	3,630
6.....	.37	588	10.....	8.47	27,700	N.....	4.05	7,700	12.....	2.31	3,420
8.....	.78	968	12.....	7.97	24,700	4.....	3.85	7,250			
10.....	2.17	3,120	<i>July 20</i>			8.....	3.67	6,680	<i>July 24</i>		
N.....	3.06	5,180	2.....	7.56	22,300	12.....	3.50	6,300	N.....	2.06	2,920
2.....	5.64	13,000	4.....	7.19	20,300	<i>July 22</i>			12.....	1.88	2,650
4.....	7.40	21,300	6.....	6.84	18,300	4.....	3.32	5,800			
6.....	10.12	38,400	8.....	6.58	17,300	8.....	3.16	5,420			
8.....	14.10	70,800	10.....	6.35	16,300	N.....	3.03	5,180			
10.....	18.10	109,000	N.....	6.12	15,000						
12.....	18.92	117,000									

SUPPLEMENTAL RECORD.—July 18, 11:55 p. m., gage height 18.92 feet, discharge 117,000 second-feet.

SINNEMAHONING CREEK AT SINNEMAHONING, PA.

LOCATION.—Lat. 41°18'45'', long. 78°05'30'', a quarter of a mile upstream from Grove Run and 3,500 feet upstream from Pennsylvania Railroad bridge at Sinnemahoning, Cameron County. Datum of gage is 769.36 feet above mean sea level, datum of 1929, New York-Pennsylvania supplementary adjustment of 1943.

DRAINAGE AREA.—699 square miles.

GAGE-HEIGHT RECORD.—Water-stage recorder graph.

DISCHARGE RECORD.—Stage-discharge relation defined by current-meter measurements up to 17,000 second-feet and extended to slope-area measurement for crest gage height.

MAXIMA.—1942: Discharge, 59,800 second-feet 10:45 p. m. July 18 (gage height, 21.58 feet).

1938-41: Discharge, 19,100 second-feet March 31, 1940 (gage height, 10.57 feet). Maximum stage known, 21.94 feet March 18, 1936, from flood-mark (discharge, 61,200 second-feet).

Daily mean discharge, in second-feet, 1942

Day	July	Aug.	Day	July	Aug.	Day	July	Aug.	Day	July	Aug.
1.....	155	518	9.....	246	217	17.....	116	502	25.....	951	288
2.....	170	416	10.....	196	362	18.....	18,400	437	26.....	729	221
3.....	149	422	11.....	200	333	19.....	24,400	339	27.....	768	177
4.....	133	316	12.....	233	231	20.....	6,250	277	28.....	721	158
5.....	126	256	13.....	177	212	21.....	3,150	241	29.....	588	150
6.....	236	221	14.....	149	486	22.....	1,920	217	30.....	516	147
7.....	934	190	15.....	133	370	23.....	1,500	284	31.....	481	129
8.....	382	190	16.....	118	437	24.....	1,040	383			

	July	Aug.
Monthly mean discharge, in second-feet.....	2,105	295
Runoff, in inches.....	3.47	0.49

Gage height, in feet, and discharge, in second-feet, at indicated time, 1942

Hour	Gage height	Discharge	Hour	Gage height	Discharge	Hour	Gage height	Discharge	Hour	Gage height	Discharge
<i>July 17</i>			<i>July 18— Con.</i>			<i>July 19— Con.</i>			<i>July 21</i>		
6-----	1.66	113	4-----	9.28	14,800	2-----	10.10	17,200	6-----	4.74	3,520
N-----	1.68	118	6-----	18.22	46,200	4-----	9.19	14,500	N-----	4.53	3,160
6-----	1.68	118	8-----	20.32	54,600	6-----	8.59	12,700	6-----	4.33	2,800
12-----	1.69	120	10-----	21.32	58,600	8-----	8.07	11,300	12-----	4.09	2,370
<i>July 18</i>			<i>July 19</i>			<i>July 20</i>			<i>July 22</i>		
2-----	1.77	145	2-----	19.02	49,400	6-----	6.60	7,410	N-----	3.81	1,900
4-----	2.06	260	4-----	17.22	42,200	N-----	6.05	6,020	12-----	3.63	1,620
6-----	2.47	485	6-----	14.77	33,200	6-----	5.55	5,060			
8-----	3.26	1,210	8-----	12.97	26,900	10-----	5.56	4,080			
10-----	4.26	2,640	10-----	11.75	22,700						
N-----	5.73	5,480	N-----	10.72	19,000						
2-----	6.61	7,410									

SUPPLEMENTAL RECORD.—July 18, 10:45 p. m., gage height 21.58 feet, discharge 59,800 second-feet.

DRIFTWOOD BRANCH SINNEMAHONING CREEK AT STERLING RUN, PA.

LOCATION.—Lat. 41°24'25'', long. 78°11'35'', at highway bridge at village of Sterling Run, Cameron County, 300 feet upstream from Sterling Run. Datum of gages is 894.84 feet above mean sea level, datum of 1929, New York-Pennsylvania supplementary adjustment of 1943.

DRAINAGE AREA.—281 square miles.

GAGE-HEIGHT RECORD.—Water-stage recorder graph from recording gage at highway bridge, July 1–19, and thereafter twice-daily readings on staff gage, 800 feet upstream.

DISCHARGE RECORD.—Stage-discharge relation defined by current-meter measurements up to 800 second-feet and extended to slope-area measurement for crest gage height.

MAXIMA.—1942: Discharge, 47,800 second-feet 8:45 p. m. July 18 (gage height, 14.7 feet at recording-gage site and 15.0 feet at staff gage; both from flood-marks).

1913–41: Discharge, 28,400 second-feet Mar. 17, 1936 (gage height, 12.0 feet from graph based on staff-gage readings).

Daily mean discharge, in second-feet, 1942

Day	July	Aug.	Day	July	Aug.	Day	July	Aug.	Day	July	Aug.
1-----	69	230	9-----	47	115	17-----	31	300	25-----	442	99
2-----	45	208	10-----	26	204	18-----	16,300	219	26-----	313	75
3-----	28	183	11-----	71	132	19-----	15,300	170	27-----	395	64
4-----	19	138	12-----	76	99	20-----	2,870	144	28-----	317	57
5-----	18	118	13-----	30	102	21-----	1,240	121	29-----	278	57
6-----	61	104	14-----	19	270	22-----	725	107	30-----	246	53
7-----	186	89	15-----	13	163	23-----	622	132	31-----	242	46
8-----	98	89	16-----	8.3	170	24-----	410	132			

	July	Aug.
Monthly mean discharge, in second-feet	1,308	135
Runoff, in inches	5.37	0.55

Gage height, in feet, and discharge, in second-feet, at indicated time, 1942

Hour	Gage height	Discharge	Hour	Gage height	Discharge	Hour	Gage height	Discharge	Hour	Gage height	Discharge
<i>July 17</i>			<i>July 18— Con.</i>			<i>July 19— Con.</i>			<i>July 21</i>		
4.....	0.28	7.6	2.....	6.64	9,250	2.....	7.36	11,600	6.....	3.91	1,550
8.....	.33	12	4.....	10.94	26,400	4.....	6.86	10,100	N.....	3.70	1,340
N.....	.33	12	6.....	13.25	38,700	6.....	6.41	8,750	6.....	3.48	1,140
4.....	.39	19	8.....	14.63	47,200	8.....	5.94	7,510	12.....	3.28	960
8.....	.72	84	10.....	13.89	42,900	10.....	5.51	6,570	<i>July 22</i>		
12.....	.77	98	12.....	12.40	34,200	12.....	5.21	5,900	N.....	3.14	840
<i>July 18</i>			<i>July 19</i>			<i>July 20</i>			N.....	3.02	725
2.....	.74	90	2.....	11.34	28,400	6.....	5.44	3,660	6.....	2.92	655
4.....	.87	126	4.....	10.44	23,900	N.....	4.84	2,720	12.....	2.86	622
6.....	1.09	194	6.....	9.66	20,600	6.....	4.42	2,170			
8.....	2.14	750	8.....	9.01	17,600	12.....	4.12	1,790			
10.....	4.54	4,550	10.....	8.42	15,200						
N.....	5.72	7,030	N.....	7.85	13,000						

SUPPLEMENTAL RECORD.—July 18, 8:45 p. m., gage height 14.70 feet, discharge 47,800 second-feet.

KETTLE CREEK AT CROSS FORK, PA.

LOCATION.—Lat. 41°28'15", long. 77°49'50", at bridge on State Highway 144, 0.2 mile downstream from Potter-Clinton County line, and 0.9 mile downstream from Cross Fork, Potter County. Datum of gage is 1,027.12 feet above mean sea level, adjustment of 1912.

DRAINAGE AREA.—136 square miles.

GAGE-HEIGHT RECORD.—Water-stage recorder graph.

DISCHARGE RECORD.—Stage-discharge relation defined by current-meter measurements up to 2,000 second-feet and extended above.

MAXIMA.—July 1942: Discharge, 2,960 second-feet 11:45 a. m. July 18 (gage height, 5.97 feet).

1940 to June 1942: Discharge, 6,100 second-feet May 22, 1942 (gage height, 7.98 feet).

Maximum stage known, about 14.0 feet March 18, 1936, from information by local residents (discharge, about 20,000 second-feet).

Daily mean discharge, in second-feet, 1942

Day	July	Aug.	Day	July	Aug.	Day	July	Aug.	Day	July	Aug.
1.....	37	100	9.....	27	66	17.....	25	410	25.....	140	98
2.....	33	84	10.....	25	62	18.....	1,720	353	26.....	119	82
3.....	29	79	11.....	40	50	19.....	1,400	288	27.....	130	73
4.....	26	66	12.....	30	44	20.....	703	230	28.....	102	65
5.....	25	60	13.....	24	146	21.....	436	184	29.....	106	64
6.....	44	58	14.....	21	320	22.....	302	150	30.....	96	58
7.....	57	52	15.....	18	265	23.....	227	147	31.....	109	56
8.....	35	60	16.....	16	278	24.....	178	124			
										July	Aug.
Monthly mean discharge, in second-feet.....										203	135
Runoff, in inches.....										1.72	1.14

Gage height, in feet, and discharge, in second-feet, at indicated time, 1942

Hour	Gage height	Dis-charge	Hour	Gage height	Dis-charge	Hour	Gage height	Dis-charge	Hour	Gage height	Dis-charge
<i>July 17</i>			<i>July 18— Con.</i>			<i>July 19— Con.</i>			<i>July 21</i>		
N-----	1.17	26	3-----	5.63	2,640	8-----	4.58	1,600	6-----	2.87	486
12-----	1.20	29	4-----	5.62	2,580	10-----	4.47	1,460	N-----	2.77	445
<i>July 18</i>			5-----	5.60	2,580	N-----	4.35	1,380	6-----	2.63	388
			6-----	5.53	2,520	2-----	4.23	1,300	12-----	2.52	342
2-----	1.25	34	7-----	5.48	2,470	4-----	4.11	1,180	<i>July 22</i>		
4-----	1.28	38	8-----	5.38	2,370	6-----	3.99	1,110	6-----	2.47	327
6-----	1.58	82	9-----	5.31	2,270	8-----	3.88	1,040	N-----	2.40	302
7-----	4.50	1,510	10-----	5.24	2,220	10-----	3.79	970	6-----	2.32	275
8-----	4.88	1,870	11-----	5.17	2,120	12-----	3.72	910	6-----	2.25	252
9-----	4.75	1,740	12-----	5.10	2,070	<i>July 20</i>			12-----		
10-----	5.24	2,220	<i>July 19</i>			6-----	3.52	793			
11-----	5.73	2,740	2-----	4.97	1,920	N-----	3.33	703			
N-----	5.95	2,960	4-----	4.83	1,820	6-----	3.12	599			
1-----	5.82	2,800	6-----	4.71	1,690	12-----	2.97	530			
2-----	5.65	2,640									

SUPPLEMENTAL RECORD.—July 18, 11:45 a. m., gage height 5.97 feet, discharge 2,960 second-feet.

PINE CREEK AT CEDAR RUN, PA.

LOCATION.—Lat. 41°31'20'', long. 77°26'55'', at highway bridge at village of Cedar Run, Lycoming County, 2,000 feet downstream from Cedar Run. Datum of gage is 781.96 feet above mean sea level (New York Central Railroad bench mark).

DRAINAGE AREA.—604 square miles.

GAGE-HEIGHT RECORD.—Water-stage recorder graph.

DISCHARGE RECORD.—Stage-discharge relation defined by current-meter measurements up to 10,000 second-feet and extended to slope-area measurement for crest gage height of flood of March 18, 1936.

MAXIMA.—July 1942: Discharge, 10,200 second-feet 7 p. m. July 18 (gage height, 6.50 feet).

1918-41: Discharge, 30,900 second-feet March 18, 1936 (gage height, 11.39 feet).

Daily mean discharge, in second-feet, 1942

Day	July	Aug.	Day	July	Aug.	Day	July	Aug.	Day	July	Aug.
1-----	123	522	9-----	117	209	17-----	69	1,030	25-----	454	263
2-----	119	383	10-----	100	199	18-----	4,940	802	26-----	376	224
3-----	104	330	11-----	104	189	19-----	5,570	637	27-----	412	189
4-----	93	274	12-----	143	156	20-----	2,320	522	28-----	474	170
5-----	90	255	13-----	105	148	21-----	1,310	417	29-----	476	373
6-----	112	209	14-----	80	1,400	22-----	882	356	30-----	454	272
7-----	223	184	15-----	75	792	23-----	654	330	31-----	449	194
8-----	166	189	16-----	69	620	24-----	522	311			
										July	Aug.
Monthly mean discharge, in second-feet-----										683	391
Runoff, in inches-----										1.30	0.75

Gage height, in feet, and discharge, in second-feet, at indicated time, 1942

Hour	Gage height	Dis-charge									
<i>July 17</i>			<i>July 19</i>			<i>July 20</i>			<i>July 23</i>		
N.....	1.12	69	4.....	5.63	7,520	6.....	3.19	1,970	6.....	2.15	690
12.....	1.14	75	6.....	5.43	7,090	8.....	3.13	1,880	N.....	2.11	654
<i>July 18</i>			8.....	5.23	6,530	10.....	3.06	1,770	6.....	2.08	628
2.....	1.47	209	10.....	5.00	5,850	12.....	2.99	1,670	12.....	2.04	595
4.....	1.86	454	N.....	4.82	5,330	<i>July 21</i>			<i>July 24</i>		
6.....	2.02	578	2.....	4.62	4,830	6.....	2.83	1,440	6.....	1.98	545
8.....	2.27	802	4.....	4.46	4,470	N.....	2.72	1,300	N.....	1.93	506
10.....	3.00	1,680	6.....	4.32	4,120	6.....	2.62	1,170	6.....	1.90	483
N.....	4.92	5,590	8.....	4.17	3,780	12.....	2.52	1,060	12.....	1.88	468
2.....	5.78	8,100	10.....	4.03	3,560	<i>July 22</i>			<i>July 25</i>		
4.....	5.97	8,700	12.....	3.89	3,250	6.....	2.42	952	6.....	1.84	439
6.....	6.42	9,900	<i>July 20</i>			N.....	2.34	870	N.....	1.88	468
7.....	6.50	10,200	2.....	3.78	3,050	6.....	2.28	811	6.....	1.84	439
8.....	6.43	9,900	4.....	3.68	2,860	12.....	2.22	754	12.....	1.81	417
10.....	6.23	9,300	6.....	3.58	2,670	<i>July 23</i>			<i>July 26</i>		
12.....	6.05	8,700	8.....	3.52	2,490	6.....	2.15	700	6.....	1.78	385
<i>July 19</i>			10.....	3.44	2,390	<i>July 24</i>			<i>July 27</i>		
2.....	5.83	8,100	N.....	3.38	2,290	6.....	2.08	650	6.....	1.74	360
			2.....	3.32	2,180	12.....	2.02	600	12.....	1.70	335
			4.....	3.24	2,050	<i>July 25</i>			<i>July 28</i>		
						6.....	1.98	550	6.....	1.66	290
						N.....	1.93	506	N.....	1.61	265
						6.....	1.88	468	6.....	1.57	240
						6.....	1.84	439	6.....	1.53	215
						12.....	1.81	417	12.....	1.49	190

ALLEGHENY RIVER DRAINAGE BASIN

ALLEGHENY RIVER AT ELDRED, PA.

LOCATION.—Lat. 41°57'50'', long. 78°23'10'', at site of former highway bridge, 1,000 feet upstream from Knapp Creek, and half a mile north of Eldred, McKean County. Datum of gage is 1,416.20 feet above mean sea level, unadjusted.

DRAINAGE AREA.—550 square miles.

GAGE-HEIGHT RECORD.—Water-stage recorder graph, except for period 10:30 p. m. July 18 to 11:30 a. m. July 26 for which a graph was drawn based on floodmark, twice-daily readings of inside gage July 21-25, information from local residents, and records for station at Red House, N. Y.

DISCHARGE RECORD.—Stage-discharge relation defined by current-meter measurements up to 12,000 second-feet and extended to crest gage height on the basis of slope-area measurement at Bullis Mills, 4.2 miles downstream.

MAXIMA.—1942: Discharge, 55,000 second-feet 9:30 a. m. July 19 (gage height, 27.60 feet, from floodmark).

1915-41: Discharge, 12,900 second-feet Apr. 5, 1940 (gage height, 18.48 feet).

Daily mean discharge, in second-feet, 1942

Day	July	Aug.	Day	July	Aug.	Day	July	Aug.	Day	July	Aug.
1.....	126	1,240	9.....	159	604	17.....	96	493	25.....	1,840	304
2.....	140	930	10.....	130	556	18.....	11,400	524	26.....	1,240	257
3.....	120	842	11.....	439	508	19.....	48,100	406	27.....	1,260	228
4.....	109	652	12.....	349	379	20.....	24,600	353	28.....	1,560	210
5.....	111	540	13.....	208	366	21.....	10,500	316	29.....	1,320	203
6.....	263	463	14.....	151	540	22.....	6,150	304	30.....	1,320	199
7.....	392	406	15.....	125	463	23.....	4,180	392	31.....	1,180	183
8.....	242	406	16.....	105	379	24.....	2,770	392			
										July	Aug.
Monthly mean discharge, in second-feet.....										3,893	453
Runoff, in inches.....										8.16	0.95

Gage height, in feet, and discharge, in second-feet, at indicated time, 1942

Hour	Gage height	Discharge	Hour	Gage height	Discharge	Hour	Gage height	Discharge	Hour	Gage height	Discharge
<i>July 17</i>			<i>July 18— Con.</i>			<i>July 19— Con.</i>			<i>July 20— Con.</i>		
4.....	2.32	96	4.....	18.00	11,900	6.....	26.35	47,000	8.....	20.65	17,600
8.....	2.32	96	6.....	19.80	16,400	8.....	26.00	44,600	10.....	20.30	16,600
N.....	2.32	96	8.....	22.00	24,000	10.....	25.55	42,200	12.....	20.00	15,600
4.....	2.31	95	10.....	24.15	34,200	12.....	25.05	38,600	<i>July 21</i>		
8.....	2.30	94	12.....	25.20	39,800	<i>July 20</i>			6.....	19.10	12,800
12.....	2.39	105	<i>July 19</i>			2.....	24.60	36,200	N.....	18.15	10,600
<i>July 18</i>			2.....	25.90	44,000	4.....	24.15	34,200	6.....	17.20	8,680
2.....	2.50	119	4.....	26.40	47,000	6.....	23.70	31,700	12.....	16.30	7,600
4.....	2.83	169	6.....	26.90	50,100	8.....	23.25	29,200	<i>July 22</i>		
6.....	3.70	353	8.....	27.45	53,600	10.....	22.80	27,200	N.....	14.95	6,240
8.....	4.72	652	10.....	27.60	55,000	N.....	22.30	24,800	12.....	13.65	5,120
10.....	9.65	2,600	N.....	27.50	54,300	2.....	21.85	22,600			
N.....	15.10	7,080	2.....	27.10	51,500	4.....	21.40	20,800			
2.....	17.10	10,200	4.....	26.70	48,800	6.....	20.95	19,200			

SUPPLEMENTAL RECORD.—July 19, 9:30 a. m., gage height 27.6 feet, discharge 55,000 second-feet.

ALLEGHENY RIVER AT RED HOUSE, N. Y.

LOCATION.—Lat. 42°06'50", long. 78°48'15", at site of old highway bridge in Red House, Cattaraugus County, and 0.7 mile upstream from Meetinghouse Run. Datum of gage is 1,327.68 feet above mean sea level, datum of 1929.

DRAINAGE AREA.—1,690 square miles.

GAGE-HEIGHT RECORD.—Water-stage recorder graph.

DISCHARGE RECORD.—Stage-discharge relation defined by current-meter measurements.

MAXIMA.—1942: Discharge, 45,300 second-feet 8:30 a. m. July 20 (gage height, 14.55 feet).

1903–41: Discharge observed, 41,000 second-feet March 2, 1910 (gage height, 13.6 feet); gage height, 13.78 feet March 4, 1934 (ice jam).

Daily mean discharge, in second-feet, 1942

Day	July	Aug.	Day	July	Aug.	Day	July	Aug.	Day	July	Aug.
1.....	392	2,650	9.....	506	1,170	17.....	324	936	25.....	3,720	872
2.....	401	2,180	10.....	392	1,360	18.....	8,210	1,030	26.....	2,970	726
3.....	366	1,840	11.....	866	1,360	19.....	36,800	975	27.....	2,530	636
4.....	348	1,620	12.....	1,140	1,120	20.....	44,200	846	28.....	2,570	576
5.....	324	1,360	13.....	726	962	21.....	34,700	774	29.....	2,610	558
6.....	348	1,170	14.....	515	1,030	22.....	20,000	726	30.....	2,710	548
7.....	589	1,030	15.....	410	1,120	23.....	10,000	910	31.....	2,550	529
8.....	690	949	16.....	357	949	24.....	6,040	1,030			

	July	Aug.
Monthly mean discharge, in second-feet.....	6,074	1,082
Runoff, in inches.....	4.14	0.74

Gage height, in feet, and discharge, in second-feet, at indicated time, 1942

Hour	Gage height	Discharge	Hour	Gage height	Discharge	Hour	Gage height	Discharge	Hour	Gage height	Discharge
<i>July 18</i>			<i>July 19— Con.</i>			<i>July 20— Con.</i>			<i>July 23— Con.</i>		
2.....	3.56	401	10.....	13.13	37,200	6.....	14.33	43,900	6.....	8.23	11,200
4.....	3.75	589	N.....	13.35	38,400	8.....	14.23	43,200	N.....	7.81	9,680
6.....	4.53	1,730	2.....	13.54	39,500	10.....	14.12	42,500	6.....	7.52	8,690
8.....	5.29	3,300	4.....	13.72	40,500	12.....	14.00	41,800	12.....	7.22	7,710
10.....	4.97	2,590	6.....	13.86	41,200	<i>July 21</i>			<i>July 24</i>		
N.....	5.08	2,830	8.....	13.98	41,900	6.....	13.51	38,600	N.....	6.65	6,000
2.....	6.03	5,180	10.....	14.11	42,700	N.....	12.93	34,800	12.....	6.13	4,590
4.....	7.65	10,400	12.....	14.24	43,400	6.....	12.26	30,800	<i>July 25</i>		
6.....	9.17	16,900	<i>July 20</i>			12.....	11.58	26,900	N.....	5.75	3,650
8.....	9.87	20,200	2.....	14.35	44,100	<i>July 22</i>			12.....	5.52	3,120
10.....	10.31	22,400	4.....	14.44	44,600	6.....	10.91	23,200			
12.....	10.73	24,400	6.....	14.49	44,900	N.....	10.24	19,900			
<i>July 19</i>			8.....	14.53	45,200	6.....	9.53	16,700			
2.....	11.26	27,100	10.....	14.54	45,200	N.....	8.80	13,500			
4.....	11.90	30,400	N.....	14.52	45,100						
6.....	12.42	33,300	2.....	14.48	44,900						
8.....	12.81	35,500	4.....	14.42	44,500						

SUPPLEMENTAL RECORD.—July 20, 8:30 a. m., gage height 14.55 feet, discharge 45,300 second-feet.

ALLEGHENY RIVER NEAR KINZUA, PA.

LOCATION.—Lat. 41°50'50'', long. 78°59'30'', at Pennsylvania Railroad bridge, half a mile upstream from Bent Run, 2 miles southwest of Kinzua, Warren County, and 2.3 miles downstream from Kinzua Creek. Datum of gage is 1,200.00 feet above mean sea level (Corps of Engineers, U. S. Army, bench mark).

DRAINAGE AREA.—2,179 square miles.

GAGE-HEIGHT RECORD.—Water-stage recorder graph.

DISCHARGE RECORD.—Stage-discharge relation defined by current-meter measurements. Shifting-control method used July 1-17.

MAXIMA.—1942: Discharge, 46,800 second-feet 5 p. m. July 20 (gage height, 17.70 feet).

1935-41: Discharge, 42,000 second-feet March 28, 1936 (gage height, 16.69 feet).

Daily mean discharge, in second-feet, 1942

Day	July	Aug.	Day	July	Aug.	Day	July	Aug.	Day	July	Aug.
1.....	563	3,290	9.....	775	1,420	17.....	486	1,220	25.....	5,260	1,200
2.....	556	2,960	10.....	612	1,760	18.....	10,500	1,220	26.....	3,980	1,010
3.....	510	2,480	11.....	947	1,700	19.....	34,300	1,250	27.....	3,400	870
4.....	510	2,160	12.....	1,480	1,510	20.....	45,300	1,100	28.....	3,180	784
5.....	486	1,830	13.....	1,140	1,290	21.....	41,500	997	29.....	3,290	748
6.....	849	1,570	14.....	811	1,250	22.....	26,700	1,010	30.....	3,290	714
7.....	802	1,380	15.....	612	1,380	23.....	14,500	1,200	31.....	3,290	681
8.....	900	1,240	16.....	517	1,310	24.....	8,630	1,310			
										July	Aug.
Monthly mean discharge, in second-feet.....										7,086	1,414
Runoff, in inches.....										3.75	0.75

Gage height, in feet, and discharge, in second-feet, at indicated time, 1942

Hour	Gage height	Dis-charge	Hour	Gage height	Dis-charge	Hour	Gage height	Dis-charge	Hour	Gage height	Dis-charge
<i>July 17</i>			<i>July 19— Con.</i>			<i>July 21— Con.</i>			<i>July 24</i>		
4.....	5.31	468	10.....	14.51	31,400	6.....	17.21	44,400	4.....	9.54	9,990
8.....	5.33	480	N.....	14.76	32,900	8.....	17.06	43,900	8.....	9.32	9,300
N.....	5.35	492	2.....	15.10	34,300	10.....	16.90	42,900	N.....	9.11	8,630
4.....	5.35	492	4.....	15.50	36,200	N.....	16.71	42,000	4.....	8.89	7,970
8.....	5.35	492	6.....	15.87	38,100	2.....	16.51	41,000	8.....	8.69	7,330
12.....	5.34	486	8.....	16.17	39,600	4.....	16.28	40,000	12.....	8.49	6,710
<i>July 18</i>			<i>July 20</i>			<i>July 22</i>			<i>July 25</i>		
2.....	5.33	480	2.....	16.85	42,400	10.....	15.52	36,200	N.....	8.00	5,260
4.....	5.42	543	4.....	17.06	43,900	12.....	15.25	34,800	12.....	7.66	4,340
6.....	5.76	840	6.....	17.25	44,400	<i>July 23</i>			<i>July 26</i>		
8.....	6.36	1,670	8.....	17.41	45,300	4.....	14.70	32,400	N.....	7.46	3,860
10.....	9.05	8,300	10.....	17.52	45,800	8.....	14.14	29,600	12.....	7.41	3,740
N.....	10.41	13,800	2.....	17.61	46,300	N.....	13.56	27,300			
2.....	10.89	15,400	4.....	17.67	46,800	4.....	13.01	24,500			
4.....	10.76	15,000	6.....	17.69	46,800	8.....	12.51	22,300			
6.....	10.66	14,600	8.....	17.65	46,300	12.....	12.01	20,100			
8.....	10.81	15,000	10.....	17.59	46,300	<i>July 24</i>					
10.....	11.63	18,400	12.....	17.51	45,800	4.....	11.56	18,400			
12.....	12.65	22,700	<i>July 21</i>			8.....	11.10	16,200			
<i>July 19</i>			2.....	17.42	45,300	N.....	10.67	14,600			
2.....	13.55	27,300	4.....	17.32	44,800	4.....	10.30	13,000			
4.....	14.01	29,100				8.....	10.00	11,800			
6.....	14.22	30,000				12.....	9.75	11,100			
8.....	14.35	31,000									

SUPPLEMENTAL RECORD.—July 20, 5:00 p. m., gage height 17.70 feet, discharge 46,800 second-feet.

CLARION RIVER AT RIDGWAY, PA.

LOCATION.—Lat. 41°25', long. 78°44', at bridge on Main Street in Ridgway, Elk County, 50 feet downstream from Elk Creek. Datum of gage is 1,361.62 feet above mean sea level, unadjusted.

DRAINAGE AREA.—303 square miles.

GAGE-HEIGHT RECORD.—From graph based on floodmark, twice daily readings of chain gage, and local information.

DISCHARGE RECORD.—Stage-discharge relation defined by current-meter measurements up to 12,000 second-feet and extended to slope-area measurement for crest gage height. Shifting-control method used except July 16–25.

MAXIMA.—July 1942: Discharge, 34,000 second-feet 1 a. m. July 19 (gage height, 16.4 feet).

1940 to June 1942: Discharge, 6,940 second-feet March 9, 1942 (gage height, 8.0 feet). The flood of March 1936 reached a stage of 14 feet (discharge, 24,000 second-feet).

Daily mean discharge, in second-feet 1942

Day	July	Aug.	Day	July	Aug.	Day	July	Aug.	Day	July	Aug.
1.....	158	444	9.....	88	326	17.....	86	412	25.....	792	191
2.....	93	326	10.....	73	326	18.....	4,550	276	26.....	545	165
3.....	73	326	11.....	183	235	19.....	12,100	227	27.....	1,210	148
4.....	75	280	12.....	117	187	20.....	3,020	199	28.....	905	135
5.....	77	244	13.....	83	327	21.....	1,620	180	29.....	615	141
6.....	207	218	14.....	73	448	22.....	1,060	176	30.....	545	126
7.....	288	195	15.....	61	252	23.....	1,620	326	31.....	511	120
8.....	117	231	16.....	58	280	24.....	980	257			
										July	Aug.
Monthly mean discharge, in second-feet.....										1,032	249
Runoff, in inches.....										3.93	0.95

Gage height, in feet, and discharge, in second-feet, at indicated time, 1942

Hour	Gage height	Dis-charge	Hour	Gage height	Dis-charge	Hour	Gage height	Dis-charge	Hour	Gage height	Dis-charge
<i>July 17</i>			<i>July 18— Con.</i>			<i>July 19— Con.</i>			<i>July 21— Con.</i>		
4.....	0.80	63	6.....	6.50	4,720	8.....	6.50	4,720	N.....	3.30	1,620
8.....	.82	67	8.....	8.60	8,080	10.....	6.20	4,370	4.....	3.20	1,540
N.....	.88	79	10.....	13.40	21,200	12.....	6.00	4,150	8.....	3.10	1,460
4.....	.98	103	12.....	16.10	32,600	<i>July 20</i>			12.....	3.00	1,380
8.....	1.06	126	<i>July 19</i>			<i>July 21</i>			<i>July 22</i>		
12.....	1.20	172	1.....	16.4	34,000	4.....	5.60	3,710	4.....	2.90	1,300
<i>July 18</i>			2.....	16.10	32,600	8.....	5.30	3,400	8.....	2.70	1,140
2.....	1.32	218	4.....	13.70	22,400	N.....	4.90	3,020	N.....	2.50	980
4.....	1.48	290	6.....	11.30	14,500	4.....	4.60	2,750	4.....	2.50	980
6.....	1.68	412	8.....	10.20	11,600	8.....	4.30	2,480	8.....	2.70	1,140
8.....	1.98	615	10.....	9.30	9,560	12.....	4.00	2,220	12.....	3.00	1,380
10.....	2.45	942	N.....	8.50	7,880	<i>July 21</i>					
N.....	3.00	1,380	2.....	7.80	6,580	4.....	3.80	2,040			
2.....	3.90	2,130	4.....	7.30	5,780	8.....	3.50	1,790			
4.....	5.00	3,110	6.....	6.80	5,080						

GENESEE RIVER DRAINAGE BASIN

GENESEE RIVER AT SCIO, N. Y.

LOCATION.—Lat. 42°09'50'', long. 77°58'50'', at highway bridge, 0.4 mile upstream from Vandermark Creek, and three-quarters of a mile upstream from Scio, Allegany County.

DRAINAGE AREA.—309 square miles.

GAGE-HEIGHT RECORD.—Water-stage recorder graph except for period August 15-21.

DISCHARGE RECORD.—Stage-discharge relation defined by current-meter measurements. Discharge for period August 15-21 computed on basis of records for nearby stations.

MAXIMA.—1942: Discharge, 9,740 second-feet 12:30 p. m. July 18 (gage height, 9.74 feet).

1916-41: Discharge observed, 10,600 second-feet May 22, 1919 (gage height, 10.1 feet, present datum) from rating curve extended above 3,600 second-feet by logarithmic plotting.

Daily mean discharge, in second-feet, 1942

Day	July	Aug.	Day	July	Aug.	Day	July	Aug.	Day	July	Aug.
1	104	539	9	60	194	17	44	161	25	266	161
2	82	363	10	58	378	18	5,830	206	26	233	136
3	67	311	11	166	221	19	3,100	209	27	331	119
4	60	249	12	112	180	20	1,190	180	28	955	112
5	60	212	13	76	185	21	686	161	29	1,050	115
6	73	180	14	64	364	22	500	235	30	634	106
7	107	159	15	52	209	23	420	276	31	616	95
8	74	159	16	44	183	24	318	212			
										July	Aug.
Monthly mean discharge, in second-feet										562	212
Runoff, in inches										2.10	0.79

Gage height, in feet, and discharge, in second-feet, at indicated time, 1942

Hour	Gage height	Dis-charge	Hour	Gage height	Dis-charge	Hour	Gage height	Dis-charge	Hour	Gage height	Dis-charge
<i>July 17</i>			<i>July 18—</i> Con.			<i>July 19—</i> Con.			<i>July 21</i>		
4	1.19	43	11	9.41	9,000	8	5.95	3,340	6	3.07	734
8	1.19	43	N	9.71	9,670	10	5.57	2,900	N	2.98	684
N	1.19	43	1	9.58	9,380	N	5.29	2,590	6	2.89	637
6	1.19	43	2	9.38	8,940	2	5.01	2,300	12	2.74	562
10	1.19	43	3	9.02	8,190	4	4.82	2,110	<i>July 22</i>		
11	1.33	65	4	8.81	7,770	6	4.59	1,890	6	2.64	514
12	1.46	90	5	8.73	7,620	8	4.37	1,690	N	2.60	495
<i>July 18</i>			6	8.72	7,600	10	4.18	1,520	6	2.54	468
1	1.90	201	7	8.75	7,660	12	4.01	1,390	12	2.59	490
2	2.84	588	8	8.77	7,690	<i>July 20</i>			<i>July 23</i>		
3	3.29	856	9	8.73	7,620	2	3.92	1,320	6	2.44	424
4	3.83	1,240	10	8.67	7,500	4	3.91	1,310	12	2.27	356
5	4.64	1,940	11	8.53	7,240	6	3.94	1,330	<i>July 24</i>		
6	5.68	3,020	12	8.33	6,870	8	3.95	1,340	N	2.17	318
7	6.67	4,260	<i>July 19</i>			10	3.94	1,330	N	2.08	286
8	7.30	5,170	2	7.79	5,940	N	3.88	1,280			
9	7.89	6,110	4	7.11	4,880	6	3.57	1,050			
10	8.58	7,330	6	6.46	3,980	12	3.25	838			

SUPPLEMENTAL RECORD.—July 18, 12:30 p. m., gage height 9.74 feet, discharge 9,740 second-feet.

GENESEE RIVER AT ST. HELENA, N. Y.

LOCATION.—Lat. 42°37'20'', long. 77°59'20'', at highway bridge in St. Helena, Wyoming County, 1½ miles downstream from Wolf Creek, and 3 miles east of Castile.

DRAINAGE AREA.—1,017 square miles.

GAGE-HEIGHT RECORD.—Water-stage recorder graph.

DISCHARGE RECORD.—Stage-discharge relation defined by current-meter measurements.

MAXIMA.—July 1942: Discharge, 18,900 second-feet 12:15 a. m. July 19 (gage height, 10.06 feet).

1908-41: Discharge, 44,400 second-feet May 17, 1916 (gage height, 12.8 feet), from rating curve extended above 29,000 second-feet by logarithmic plotting.

Daily mean discharge, in second-feet, 1942

Day	July	Aug.	Day	July	Aug.	Day	July	Aug.	Day	July	Aug.
1.....	221	1,330	9.....	185	400	17.....	167	438	25.....	695	600
2.....	242	1,020	10.....	160	586	18.....	8,820	586	26.....	605	552
3.....	220	775	11.....	310	783	19.....	11,200	446	27.....	636	359
4.....	184	720	12.....	533	604	20.....	4,070	376	28.....	700	242
5.....	158	602	13.....	348	448	21.....	2,030	336	29.....	1,700	354
6.....	156	487	14.....	268	529	22.....	1,260	308	30.....	1,740	446
7.....	184	411	15.....	195	694	23.....	1,010	334	31.....	1,310	440
8.....	184	368	16.....	173	472	24.....	874	439			
										July	Aug.
Monthly mean discharge, in second-feet.....										1,308	532
Runoff, in inches.....										1.49	0.60

Gage height, in feet, and discharge, in second-feet, at indicated time, 1942

Hour	Gage height	Dis-charge	Hour	Gage height	Dis-charge	Hour	Gage height	Dis-charge	Hour	Gage height	Dis-charge
<i>July 17</i>			<i>July 18— Con.</i>			<i>July 19— Con.</i>			<i>July 21</i>		
4.....	2.55	163	7.....	9.61	16,600	8.....	6.75	6,060	6.....	4.80	2,290
8.....	2.54	159	8.....	9.72	17,200	9.....	6.61	5,700	N.....	4.53	1,930
N.....	2.60	182	9.....	9.83	17,800	10.....	6.51	5,460	6.....	4.37	1,730
4.....	2.53	155	10.....	9.92	18,200	11.....	6.40	5,200	12.....	4.19	1,530
8.....	2.48	137	11.....	10.00	18,600	12.....	6.29	4,950	<i>July 22</i>		
10.....	2.50	144	12.....	10.05	18,800	<i>July 20</i>			N.....	3.90	1,210
12.....	2.74	245	<i>July 19</i>			1.....	6.20	4,750	12.....	3.75	1,060
<i>July 18</i>			1.....	10.03	18,800	2.....	6.13	4,600	<i>July 23</i>		
1.....	2.86	307	2.....	9.96	18,400	3.....	6.07	4,480	6.....	3.74	1,050
2.....	2.84	296	3.....	9.84	17,800	4.....	6.04	4,410	N.....	3.65	960
3.....	2.82	285	4.....	9.69	17,000	5.....	6.06	4,460	6.....	3.72	1,030
4.....	2.91	335	5.....	9.55	16,300	6.....	6.09	4,520	12.....	3.68	990
5.....	3.29	614	6.....	9.37	15,500	7.....	6.09	4,520	<i>July 24</i>		
6.....	3.45	762	7.....	9.17	14,600	8.....	6.13	4,600	6.....	3.67	980
7.....	3.49	800	8.....	8.95	13,600	9.....	6.15	4,640	N.....	3.52	830
8.....	3.45	762	9.....	8.73	12,600	10.....	6.12	4,580	6.....	3.53	840
9.....	3.48	791	10.....	8.56	11,900	11.....	6.09	4,520	12.....	3.44	752
10.....	7.38	7,740	11.....	8.35	11,100	N.....	6.04	4,410			
11.....	8.24	10,700	N.....	8.21	10,500	2.....	5.92	4,170			
N.....	8.21	10,500	1.....	8.06	9,950	4.....	5.74	3,830			
1.....	8.30	10,900	2.....	7.87	9,260	6.....	5.53	3,440			
2.....	8.56	11,900	3.....	7.68	8,640	8.....	5.35	3,140			
3.....	8.81	12,900	4.....	7.48	8,030	10.....	5.22	2,920			
4.....	9.05	14,000	5.....	7.26	7,410	12.....	5.12	2,760			
5.....	9.26	15,000	6.....	7.07	6,890						
6.....	9.46	15,900	7.....	6.91	6,470						

SUPPLEMENTAL RECORD.—July 19, 12:15 a. m., gage height 10.06 feet, discharge 18,900 second-feet.

SUMMARY OF FLOOD DISCHARGES

The maximum discharges at gaging stations in and adjacent to the flood area, together with the miscellaneous measurements of peak discharge by slope-area, contracted-opening, and similar methods, are summarized in table 4. The gaging stations are indicated in the proper column by the period of record available, to and including 1942. The places of miscellaneous measurement are indicated in the same column by the method of measuring. When slope-area and similar methods of measuring the peak discharge are used at a gaging station,

there is usually additional information available so that the peak discharge given for the gaging-station record is not based on the measurement alone. There is, therefore, no particular point in indicating, in table 4, slope-area and similar measurements made at gaging stations, especially as they are discussed in the station descriptions in the preceding section.

The index numbers used in table 4 are those used for the same points in tables 1, 3, 4, 1-A, 3-A, and 4-A of Water-Supply Paper 847, Maximum discharges at stream-measurement stations through December 31, 1937, with a supplement including additions and changes through September 30, 1938. For those places not given in Water-Supply Paper 847, new index numbers were assigned using the same decimal system. The places of measurement also are identified on plate 2 by these index numbers.

The maximum flood previously known may include a flood outside the period of record, the general aim being to list the highest flood for which the discharge is known. The gage heights of all floods listed are the gage heights of the maximum discharge unless qualified otherwise. Other information in table 4 is considered self-explanatory.

TABLE 4.—Summary of flood discharges in north-central Pennsylvania for the flood of July 1942

No. on pl. 2	Stream and place of determination	Drainage area (square miles)	Period of record or method of measuring	Maximum flood previously known			Maximum during present flood			
				Date	Gage height (feet)	Discharge	Time	Gage height (feet)	Discharge	Second-foot per square mile
	SUSQUEHANNA RIVER DRAINAGE BASIN									
753	Canacadea Creek near Hornell, N. Y.	58.7	(1924-29)	July 1935	12.3	21,000	6:40 a. m. July 18	7.07	6,080	104
754	Karr Valley Creek at Almond, N. Y.	27.6	1938-42	Mar. 31, 1940	15.8	3,800	5:45 a. m. July 18	18.8	5,900	214
781.5	West Branch Susquehanna River at Karthaus, Pa.	1,462	(1918-20)	Mar. 18, 1936	23.0	3,185,000	2 a. m. July 19	2.15	1,120	8
783	West Branch Susquehanna River at Renovo, Pa.	2,975	(1895-1903)	do	29.39	236,000	11:55 p. m. July 18	18.92	117,000	39
784	West Branch Susquehanna River at Williamsport, Pa.	5,682	(1905-42)	do	33.57	264,000	2:30 p. m. July 19	18.76	110,000	19.4
787.5	Sinnemahoning Creek at Sinnemahoning, Pa.	689	1938-42	do	21.94	61,200	10:45 p. m. July 18	21.58	59,800	86
787.8	Driftwood Branch Sinnemahoning Creek at Emporium, Pa.	91.7	Slope area				6:30 p. m. July 18		28,000	310
789	Driftwood Branch Sinnemahoning Creek at Sterling Run, Pa.	281	1913-42	Mar. 17, 1936	12.0	28,400	8:45 p. m. July 18	15.00	47,800	170
789.1	North Creek near Lockwood, Pa.	20.3	Contracted opening				5 p. m. July 18		4,700	230
789.2	West Creek near Emporium, Pa.	55.9	do				July 18		11,000	200
789.3	Sinnemahoning Portage near Emporium, Pa.	63.8	Slope area				do		18,000	280
789.4	Salt Run near Emporium, Pa.	7.67	Dam				do		1,900	250
789.50	First Fork Sinnemahoning Creek near Costello, Pa.	103	Slope area				1 p. m. July 18		31,000	300
789.52	First Fork Sinnemahoning Creek near mouth, near Sinnemahoning, Pa.	250	2 slope area				4 p. m. July 18		81,000	324
789.54	South Fork of First Fork Sinnemahoning Creek near Costello, Pa.	17.8	Slope area				July 18		7,500	420
789.56	Freeman Run above Austin Dam, near Odin, Pa.	12.1	do				do		9,800	810
789.58	Freeman Run below Austin Dam, near Austin, Pa.	14.3	2 slope area and a dam				11 a. m. July 18		17,000	1,200
789.60	Nelson Run near Wharton, Pa.	10.2	Slope area				July 18		6,600	650
789.62	East Fork of First Fork Sinnemahoning Creek, near Wharton, Pa.	49.1	Contracted opening				do		6,100	120
789.64	Bailey Run near Wharton, Pa.	11.7	Slope area				do		5,300	450
789.66	Rattlesnake Run near First Fork, Pa.	4.43	do				do		1,300	290
789.68	Brooks Run at Lashbaugh, Pa.	4.73	do				do		1,600	340

TABLE 4.—Summary of flood discharges in north-central Pennsylvania for the flood of July 1942—Continued

No. on pl. 2	Stream and place of determination	Drainage area (square miles)	Period of record or method of measuring	Maximum flood previously known			Maximum during present flood		
				Date	Gage height (feet)	Discharge Second- feet Second- feet per square mile	Time	Gage height (feet)	Discharge Second- feet Second- feet per square mile
	ALLEGHENY RIVER DRAINAGE BASIN— Continued								
030.4	Straight Creek near Instantler, Pa.---	21.7	Slope area.						
030.5	South Fork Straight Creek near In- stantler, Pa.	8.41	do.						
030.6	Johnson Run at Kethner Dam, near Johnsontown, Pa.	7.78	Dam.						
030.8	Windfall Run at Halsey Dam, near Mount Jewett, Pa.	1.61	do.						
	GENESEE RIVER DRAINAGE BASIN								
143	Genesee River at Scio, N. Y.---	309	1916-42	May 22, 1919	6 10.1	10,600	12:30 p. m. July 18.	9.74	9,740
144	Genesee River at St. Helena, N. Y.---	1,017	1908-42	May 17, 1916	12.8	44,400	12:15 a. m. July 19.	10.06	18,900
147.5	Angelica Creek at Angelica, N. Y.---	61.0	Slope area.			34.3	July 18.		14,000
						43.7			230

1 From floodmark.

2 About.

3 From floodmark at staff gage.

4 Greater flood occurred Mar. 18, 1936; discharge not determined.

5 Furnished by Corps of Engineers.

6 Observed.

7 One furnished by Corps of Engineers.

The maximum discharges, in second-feet per square mile, for the flood of July 1942 as given in table 4 are plotted with respect to the drainage area, in figure 43. Such a diagram is helpful in comparing the relative size of the flood in streams draining basins of different size. The size of a drainage basin, though important, is only one of many factors that influence the magnitude of the runoff. Some of the other factors are slope and shape of drainage basin, vegetative cover, and underlying rocks. As an aid to those who use a flood formula expressing the discharge in terms of the square root of the drainage area, a guide line representing $5,000/\sqrt{\text{drainage area}}$ has been drawn on figure 43. All measurements of drainage areas of less than 1 square mile and especially those less than 0.1 square mile are subject to errors that may be quite large owing to the small-scale maps on which they were measured.

RAINFALL AND RUNOFF STUDIES

Studies of the volume of flood runoff and the precipitation required to produce it are often made to obtain basic information useful in other flood studies and in estimating floods produced by other storms. To obtain such basic information requires that both the data on rainfall and the data on runoff be complete in themselves and reasonably free of error.

There were only six gaging stations with drainage basins lying within, or mostly within, the 4-inch isohyetal lines on plate 2. At one of the gaging stations there was a staff gage only, and at two others staff-gage readings were used to fill in some of the gaps in the record from water-stage recorders. In preparing flood hydrographs for periods not based on continuous records of gage height, rainfall information was used as a matter of course as an aid in their definition. Thus at three gaging stations the runoff cannot be considered as having been obtained independently of the rainfall. With only three gaging stations at which the runoff was computed independently of the rainfall, and a total of six gaging stations in the storm area, it would seem that little basic rainfall-runoff information could be obtained for this flood. This conclusion is supported by the fact that, as explained on page 116, the interpretations made in drawing the isohyetal map were influenced strongly by the runoff as computed for the stream-gaging stations.

The rainfall-runoff studies made for this report are considered as a test of the reasonableness of the data given. They also show what is believed to be the actual relationship between the two quantities. The data are not considered good enough for use in other flood studies.

The results of the rainfall-runoff studies are given in table 5. The average depth of rainfall on each drainage basin was obtained by planimetry of the isohyetal map (pl. 2). The direct storm runoff and

apparent ground-water recharge were obtained from the discharge hydrograph in the following manner: The recession curve, for flow before the flood, was extended through the flood period. The recession curve for the flood was drawn to exclude runoff from subsequent storms and was extended until it approached the recession curve of antecedent flow closely enough that the volume of runoff between the two curves beyond that point could be neglected without appreciable error. The volume of runoff represented by the flood hydrograph and recession curve, in excess of the recession curve of antecedent flow, was taken as the total storm runoff—the sum of the volume of direct storm runoff and the volume of apparent ground-water recharge. The separation between these last two quantities was obtained by arbitrarily drawing a straight line connecting the antecedent-flow recession curve, at about the time of the flood-peak discharge, with the point on the flood recession curve representing the end of direct runoff. This point was taken at the break in slope of the hydrograph when plotted semilogarithmically. The principles involved in this type of analysis are discussed in modern texts on hydrology and in previous flood reports of the Geological Survey (Langbein and others, 1947).

TABLE 5.—*Rainfall and associated direct runoff in selected drainage basins*

No. on pl. 2	Stream and point of measurement	Drainage area (square miles)	Rain-fall (inches)	Direct storm runoff (inches)	Apparent ground-water recharge (inches)	Basin retention (inches)	Infiltration index
789	Driftwood Branch Sinnemahoning Creek at Sterling Run, Pa.-----	281	9.5	4.2	0.7	4.6	0.8
789.8	Kettle Creek at Cross Fork, Pa.-----	136	4.0	1.0	.5	2.5	.6
001	Allegheny River at Eldred, Pa.-----	550	11.5	5.6	2.0	3.9	.9
002	Allegheny River at Red House, N. Y.-----	1,690	7.8	2.8	.8	4.2	.9
028.4	Clarion River at Ridgway, Pa.-----	303	6.7	2.3	.4	4.0	.8
143	Genesee River at Scio, N. Y.-----	309	4.7	1.1	.2	3.4	.7

The ground-water recharge could be measured either as the volume of water in the ground represented by a rise in the water table, or it can be computed as the volume reaching the streams as ground-water seepage in excess of the ground-water seepage that would have reached them had there been no recharge. The latter method was used in this report. The overly simplified way in which it was obtained, however, makes it necessary to label it apparent ground-water recharge. As computed it is sufficiently accurate for the purpose of this study but for use in studies of ground-water supplies it may be appreciably in error.

The basin retention is the rainfall minus direct runoff and apparent ground-water recharge. It represents initial basin losses, replenishment of soil moisture, water stored in perched water tables and other

forms of temporary storage. For the most part the basin retention probably will become part of the water loss from the basin. Some of the retention may be expected eventually to reach the zone of saturation. As used in this study, however, it is significant only as the difference between rainfall and runoff.

The infiltration index was computed in the usual manner (Langbein and others, 1947). It is given in table 5 to show the general agreement among the several drainage basins.

FLOOD CRESTS

Records of available flood-crest elevations and the time of their occurrence are given in table 6. These data are basic for the study of time of travel of flood crests, amount of valley and channel storage, and of the limit of future development along a river.

The elevations given are referred to mean sea level, Sandy Hook datum, using the latest adjustment to the precise level net that was available at the time of the surveys in 1942. In a few instances the elevations given for gaging stations are slightly different from the ones that would be computed from information given in the station description, because the gage datum is referred to a later adjustment in the description.

Flood-crest elevations are never precise observations. The water surface of a stream during flood is wavy and frequently is higher on one bank than on the other. Crest elevations for points far back from the main channel must always be used with caution.

TABLE 6.—*Flood-crest elevations*

Stream and location	Miles above month	Day and hour (July)	Elevation (feet)
SUSQUEHANNA RIVER DRAINAGE BASIN			
Bennett Branch Sinnemahoning Creek:			
Caledonia, Pa.-----	23.2		1,108.4
Medix Run, mouth of-----	19.5		1,043.2
Benezette Run, mouth of-----	16.1		989.1
Dents Run, mouth of-----	9.0		898.2
Hicks Run, mouth of-----	7.8		891.7
Pennsylvania R. R. bridge-----	1.2		810.6
Sinnemahoning Creek:			
Driftwood, Pa., confluence of Bennett Branch and Driftwood Branch-----	15.5		
Sinnemahoning, Pa., USGS gage-----	12.9	18, 10:45 p. m.	790.6
Sinnemahoning, Pa., Baltimore & Ohio R. R. bridge—upstream, right bank-----	12.6		787.2
R. R. bridge—upstream, left bank-----	12.5		786.8
R. R. bridge—downstream, left bank-----	12.5		786.8
First Fork Sinnemahoning Creek, mouth of-----	11.8		786.6
Mouth, Keating, Pa.-----	0	18, 11:30 p. m.	711.3
Driftwood Branch Sinnemahoning Creek:			
Lockwood, Pa., mouth of North Creek-----	22.0	18, 5 p. m.	1,076.7
Emporium, Pa.-----	20.3	18, 6:30 p. m.	1,034.3
Emporium Junction, Pa., mouth of Sinnemahoning Portage-----	19.3		1,017.2
Cameron, Pa., highway bridge-----	14.0		957.4
Sterling Run, Pa., USGS staff gage-----	10.1		909.6
Sterling Run, Pa., highway bridge, USGS recording gage-----	9.9	18, 8:45 p. m.	909.3
Driftwood, Pa., Pennsylvania R. R. bridge-----	.05		808.0

TABLE 6.—*Flood-crest elevations*—Continued

Stream and location	Miles above month	Day and hour (July)	Elevation (feet)
SUSQUEHANNA RIVER DRAINAGE BASIN—Continued			
First Fork Sinnemahoning Creek:			
Costello, Pa., mouth of Freeman Run	25.2	18, 1 p. m.	1, 195.4
Nelson Run, 0.6 mile below mouth of	21.4		1, 132.7
Wharton, Pa., mouth of East Fork	19.7	18, 2 p. m.	1, 094.5
Bailey Run, mouth of	17.2		1, 063.5
First Fork, Pa.	12.0	18, 3 p. m.	
Lushbaugh, Pa., 1.1 miles downstream at mouth of Short Bend Run	8.1		907.4
Lick Island	4.3	18, 4 p. m.	844.7
Mouth	0	18, 5 p. m.	
Freeman Run:			
Austin, Pa., above, at dam that failed	5.6	18, 11 a. m.	
Austin, Pa., Ford garage	3.3	18, 11:30 a. m.	1, 353.5
Highway bridge	.6		1, 223.4
East Fork Sinnemahoning Creek: Highway bridge	2.5		1, 172.8
ALLEGHENY RIVER DRAINAGE BASIN			
Allegheny River:¹			
Seven Bridges, Pa., Dunn farm	317.9	18, 7:30 a. m.	1, 930.9
Coudersport, Pa., highway bridge	308.6	18, 1:30 p. m.	1, 646.7
Roulette, Pa., highway bridge	298.3	18, 2 p. m.	1, 535.1
Burtville, Pa., highway bridge	295.1		1, 505.4
Port Allegany, Pa., State Highway 155, bridge	289.6		1, 482.1
Port Allegany, Pa., U. S. Highway 6, old bridge	288.9	18, 3:30 p. m.	1, 479.0
Port Allegany, Pa., U. S. Highway 6, new bridge (destroyed by flood)	288.0		1, 477.4
Turtlepoint, Pa., highway bridge	281.7		1, 453.7
Larabee, Pa., highway bridge, USGS gage (discontinued)	276.4	19, 1 a. m.	1, 447.4
Eldred, Pa., highway bridge, USGS gage	269.0	19, 9:30 a. m.	1, 443.8
Mill Grove, N. Y., highway bridge	262.9		1, 434.5
Portville, N. Y., fire department building	261.4	19, 3 p. m.	1, 434.5
Olean, N. Y., highway bridge	255.5	19, 6:30 p. m.	1, 423.3
North Allegany, N. Y., highway bridge	250.7		1, 413.9
Vandalia, N. Y., highway bridge	246.4	19, 11:30 p. m.	1, 404.7
Riverside Junction, N. Y., Erie R. R. bridge	242.1		1, 393.5
South Carrollton, N. Y., railroad bridge	240.4		1, 388.6
Salamanca, N. Y., highway bridge	233.7	20, 5 a. m.	1, 374.2
Red House, N. Y., highway bridge, USGS gage	226.0	20, 8:30 a. m.	1, 342.2
Quaker Bridge, N. Y., highway bridge	220.4		1, 320.2
Onoville, N. Y., highway bridge	214.0		1, 290.6
Kinzua, Pa., railroad bridge, USGS gage	200.0	20, 5 p. m.	1, 217.7
Clarion River:			
Instanter	109.4	18, 3:30 p. m.	
Johnsonburg, Pa., lower highway bridge	94.0	18, 9 p. m.	1, 439.8
Ridgway, Pa., West Penn power station	88.4		1, 390.0
Ridgway, Pa., Main Street bridge, USGS gage	87.4	19, 1 a. m.	1, 378.0
Carman, Pa., highway bridge	80.1		1, 330.4
Bell Town, Pa., highway bridge	62.2		1, 225.2
Cooksburg, Pa., dam site gage	49.3		1, 170.6
Cooksburg, Pa., highway bridge, USGS gage	47.6	19, 9:30 a. m.	1, 161.4
Clarion, Pa., Piney Dam, upper pool	25.1		1, 093.1
St. Petersburg, Pa., highway bridge, USGS gage	4.5	19, 2 p. m.	891.0

¹ Data other than for gaging stations furnished by Corps of Engineers.**RECORDS OF PREVIOUS FLOODS**

The floods of July 18, 1942, were unprecedented in the area of heaviest rainfall. That such extreme floods have occurred in the past in other parts of Pennsylvania and adjoining States is shown by the following descriptions of earlier floods. Perhaps the earliest flood from the cloudburst type of storm in Pennsylvania of which there is accurate record is that of August 5, 1843, in Delaware County. In view of the description of the storm given in an earlier section of this report it seems appropriate to include a description of the flood flows also. Following that description are short descriptions of recent

floods caused by intense local rains in Pennsylvania and New Jersey. The section closes with a few notes on previous floods in Allegheny River at Salamanca, N. Y.

FLOOD OF AUGUST 5, 1843, IN DELAWARE COUNTY, PA.

Although confined to a smaller area, the flood of August 5, 1843, in Delaware County was every bit as large as the flood of July 1942 in the north-central part of the State. The following description of that flood is included in this report so that comparisons between the two can be made and to emphasize the fact that previous floods of this magnitude have occurred. Descriptive details of the storm have been given in a previous section by Mr. Stewart. The measured amounts of rainfall given in the original report (Delaware County Inst. Sci., 1910) on that flood have been summarized in table 7.

TABLE 7.—*Rainfall records for storm of August 5, 1843, in Delaware County*

Place	Amount (inches)	Remarks
Haverford School.....	5.82	Measured in a rain gage. 0.5 inch before 12 m., heavy rain 3 to 7 p. m., 1 inch in 15 minutes just before 7 p. m.
Upper Darby.....	3.75	
Newtown Township.....	Between 11 and 13	2 to 5 p. m.
Newtown Square.....	5.5	4:20 to 5 p. m.
Concord Township.....	16	2:45 to 5:45 p. m.
Brandywine Hundred, Del.....	10	2 to 4 p. m.

NOTE.—“The amount of rain which fell on that part of the County which borders on the Delaware River and embraces the mouths and lower parts of the inundated creeks was not sufficient to produce even an ordinary rise in the stream” (Delaware County Inst. Sci., 1910, p. 9).

The heavy rainfall apparently did not last more than about 3 hours, but the time of its occurrence varied throughout the county. The area covered by the cloudburst was less than the area of Delaware County although small areas of adjacent counties were affected. It followed on the heels of a general storm that extended much beyond the limits of Delaware County in every direction. The total rain prior to the cloudburst probably did not exceed three-quarters of an inch and little rise was noted in the streams. As a result of the general rain, however, the ground was soaked, thus reducing the infiltration and increasing the direct runoff from the cloudburst storm that followed. The heavy rain occurred near the headwaters of the streams unknown to residents of the lower valleys. The floods burst on them with devastating suddenness, causing the loss of 12 lives. The rapid rise of water is attested to by the many accounts of an almost instantaneous rise in the water from 5 to 8 or 10 feet. At one point on Crum Creek the water rose 7 or 8 feet in 10 minutes. Farther downstream at Avondale it was observed that (Delaware County Inst. Sci., 1910, p. 28):

* * * the water at this place rose 19 feet—6 feet higher than the great ice freshet of 1839. The rise in the creek commenced at about half past seven o'clock. The water rose very suddenly as well as unexpectedly, and was at its highest point a little after eight o'clock.

The fall of the flood waters was equally rapid; in Chester Creek at Flower's Mill the waters fell 10 feet in 50 minutes.

The maximum stages above low water reached by the major streams in the County were: Darby Creek, 17.5 feet; Crum Creek, 20 feet; Ridley Creek, 21 feet; and Chester Creek, 33 feet. There is little quantitative information available on peak discharges in these streams other than the description of a cross section on each:

Darby Creek, a short distance below the West Chester Road (Delaware County Inst. Sci., 1910, p. 22):

The flood attained a height of 15 feet at this place, with a cross section of 80 yards, which with proper allowances, would give an area of 2,800 feet.

Crum Creek (Delaware County Inst. Sci., 1910, p. 26):

Immediately below the point where the road from Newtown Square crosses the creek, the flood reached the height of 9 feet upon a cross section at the surface of 330 feet.

Ridley Creek (Delaware County Inst. Sci., 1910, p. 29):

* * * on the farm of George Howard, in Edgmont. At this place the water attained a height of 12 feet 6 inches, which was 6 feet 6 inches higher than the great freshet of 1839, and 6 feet 4 inches higher than that of 1795; this last being the highest which had previously occurred at the same place during a period of at least 90 years. * * * A cross section of the flood of 1795 gives but 900 square feet, while that of 1843 gives upwards of 2,500 square feet.

Chester Creek. A detailed description of a cross section on this stream is given by John F. Frazer of Philadelphia, who was visiting at the farm of Samuel West (Delaware County Inst. Sci., 1910, p. 43).

The height of the flood I measured with as much accuracy as my means would permit, and am confident that my measures are correct within one or two inches, at the same time I must observe that the elevations are taken above the level of the creek, a day or two after the flood, when from the continuance of wet weather, the creek was still above its ordinary level; how much it is impossible for me to say. At the position where I first measured it (upon Mr. West's upper meadow) the creek was sixty feet wide, and averaged about six feet in depth (it is the upper end of the backwater from Flower's dam). The vertical height of the flood was 20.58 feet above the water line, or 26.5 feet above the bottom of the channel of the creek. The breadth of the water line at the highest point of the flood (measured at right angles to the direction of the creek) was 534.8 feet (say 535 feet). The meadows overflowed, on either side, are quite flat, and appear to have been at some former time, the banks of the stream or current, so that I think we may assume the area of the cross section as at least two-thirds of the rectangular area given by multiplying the breadth by the height. Assuming then the number 535 and 21 are representing these (neglecting the channel actually occupied by the creek) we shall have the area of the cross section 7,490 square feet. Assuming the creek to be sixty feet wide and six feet deep, and its cross section a rectangle

(as it is very nearly) we have an area of 360 square feet for the creek at its ordinary high water, by which we see that the cross section was increased twenty times. The increase of velocity I had no means of ascertaining, as the greater breadth at this point give rise to the formation of extensive eddies. * * * Professor Frazer is of the opinion that at Mr. West's (judging from the motion of the cotton bales) the velocity of the middle of the stream was not less than from fifteen to twenty miles per hour.

The cross section described by Professor Frazer is about a mile below the present Geological Survey gaging station at Dutton Mill Bridge.

For the four cross sections just described, I. E. Houk (1922) estimated velocities after a personal field inspection of the locations and a study of the channel slopes. The results of his calculations are as follows:

TABLE 8.—*Summary of flood discharges in Delaware County, Aug. 5, 1843*

Stream	Drainage area (square miles)	Runoff	
		Cubic feet per second per square mile	Inches per hour
Darby Creek.....	32	880	1.36
Crum Creek.....	22	410	.64
Ridley Creek.....	20	750	1.16
Chester Creek.....	62	1,000	1.55

¹ Careful investigation indicates that 32 square miles is a better figure for this location than 48 square miles as given by Houk. Other figures have been revised based on the same peak discharge.

The destruction caused by the storm and flood was calamitous, the losses in Delaware County being estimated at nearly a quarter of a million dollars. This amount of damage is not surprising when one reads of the dams and bridges destroyed and stone buildings being knocked down "stone by stone." The force of the flood is shown in the accounts of a rock weighing 2½ tons that was carried 200 yards, a meadow that was excavated to depths of 4 and 5 feet, and a rock weighing between 10 and 12 tons that was moved about 75 feet.

FLOOD OF SEPTEMBER 1, 1940, IN SOUTHERN NEW JERSEY

Peak flood discharges in southern New Jersey streams Sept. 1, 1940, were as great as many of those in the flood of July 1942 in Pennsylvania from drainage basins of comparable size. The greatest discharge, both in actual amount and in relation to size of drainage area, was the flow of 26,100 second-feet in Salem Creek below Woodstown. The floods resulted primarily from extraordinary rainfalls associated with the passing of a hurricane off the New Jersey coast. Contributing factors were the breaching of many small dams and the week of antecedent rainfall that soaked the ground and weakened earth dams.

The rainfalls that produced these floods included the maximum 12-hour rainfall in the United States—24 inches in 9 hours at Ewan. The average rainfall on 2,000 square miles was 7.8 inches according to the Corps of Engineers. The flood caused the death of four persons and damage amounting to \$1,000,000 according to the Weather Bureau. In addition many thousands of persons were seriously inconvenienced by washed out railroad and highway bridges and flooded highways during Labor Day week-end. Some communities were virtually isolated. A large part of the damage was caused by the breaking of many small dams.

Miscellaneous measurements of peak discharge were made by the Geological Survey and the New Jersey State Water Policy Commission. The results of these measurements are given in table 9 together with peak discharges at stream-gaging stations. Daily discharges at the stream-gaging stations are published in Water-Supply Paper 891, Surface-water supply of the United States, part 1, north Atlantic slope basins.

TABLE 9.—Summary of flood discharges in southern New Jersey for the flood of September 1, 1940

Stream and place of determination	Drainage area (square miles)	Period of record or method of measuring	Maximum flood previously known			Maximum during present flood				
			Date	Gage height (feet)	Discharge		Time	Gage height (feet)	Discharge	
					Second-foot	Second-foot per square mile			Second-foot	Second-foot per square mile
GREAT EGG HARBOR RIVER DRAINAGE BASIN										
Great Egg Harbor River at Folsom.....	56.3	1925-40.....	Sept. 23, 1938	6.59	718	12.8	Sept. 3.....	9.09	1,440	25.6
MAURICE RIVER BASIN										
Maurice River at Norma.....	113	1932-40.....	Sept. 8, 1935	5.30	1,060	9.6	Sept. 2, 1 p. m.....	8.72	7,360	65.1
Maurice River at Millville.....	218	Dam.....	Aug. 20, 1939	6.21	1,050	47.1	Sept. 2, 2 p. m.....	3.88	6,500	29.8
Manantico Creek, near Millville.....	22.3	1931-40.....							363	16.3
COHANSEY CREEK BASIN										
Cohansey Creek at Bostwick Dam, at Beals Mill.....	8.3	Dam.....					Sept. 1.....		2,100	253
Barrett Run at Mary Elmer Dam at, Bridgeton.....	7.5	do.....					do.....		1,850	247
DELAWARE RIVER DRAINAGE BASIN										
Assunpink Creek at Trenton.....	89.4	1923-40.....	Sept. 22, 1938	10.74	3,320	37.1	Sept. 3, 3-4 a. m.....	6.00	1,230	13.8
Crosswicks Creek at Extonville.....	83.6	1940.....					Sept. 1.....	12.05	3,360	40.2
North Branch Rancocas Creek at Pemberton.....	111	1921-40.....	Aug. 21, 1939	10.77	1,730	15.6	Sept. 1, about 3 p. m.....	9.65	1,480	13.3
Haynes Creek at Medford Lakes.....	3.3	Orifice.....					Sept. 1.....		223	67.6
Kettle Run at Trenton.....	7.3	do.....					do.....		500	68.5
Sharps Run at Pa. R. R. culvert at Modford.....	4.9	do.....					do.....		700	143
Big Timber Creek at Clenont Park Dam, at Clenont.....	2.8	Critical depth.....					do.....		800	286
Clamata Creek at Pitman.....	6.75	Dam.....					do.....		4,200	622
Panola Creek at State Highway 45, at Mullica Hill.....	14.0	Critical depth.....					do.....		2,900	207
Oklmans Creek at Jessups Mills.....	4.15	Contracted opening.....	June 27, 1938	9.08	1,190	61.6	do.....	20.3	2,950	711
Oklmans Creek near Woodstown.....	19.3	Dam.....					do.....		8,100	420
Salem Creek at Woods Mill.....	4.3	Slope area.....					do.....		7,000	1,630
Salem Creek below Woodstown.....	17.5	do.....					do.....		96,100	1,400
Branch of Salem Creek 3 miles east of Woodstown.....	3.2	do.....					do.....		3,880	1,210
Branch of Alloway Creek 1½ miles northeast of Alloway.....	8.7	do.....					do.....		10,800	1,240
Branch of Alloway Creek east of Alloway.....	5.5	Contracted opening.....					do.....		3,300	600

FLOODS OF JUNE 4-5, 1941, IN SOUTHWESTERN PENNSYLVANIA

Outstanding floods occurred in many small streams in Monongahela River Basin as a result of heavy rains June 3 and 4. The flood discharge of Castile Run at Riggle Farm, Pa., was comparable with the higher discharges in the 1942 flood in the north-central part of the State.

The flood-producing rains were preceded by showers that began on May 30. The heavy rains occurred in the afternoon and night of June 3 and again in the afternoon of June 4. They were described by the official in charge, Weather Bureau office, Pittsburgh (Monthly Weather Review, June 1941):

The rains were unprecedented for several of the southwestern counties in Pennsylvania, and adjacent counties in West Virginia, being in excess of 6 inches for the 24 hours ending at 7 a. m. of the 5th. At Brownsville, Pa., Government lock No. 5, the precipitation on the morning of the 5th measured 6.27 inches.

The Geological Survey made an extensive investigation of the flood area to obtain the peak discharges of the small streams. The results of that investigation are given in table 10. Also given in table 10 are the peak discharges at the stream-gaging stations in the area; daily discharges at these stations are published in Water-Supply Paper 923, Surface-water supply of the United States, 1941, part 3, Ohio River Basin.

TABLE 10.—Summary of flood discharges in southwestern Pennsylvania for the floods of June 4-5, 1941

Stream and place of determination	Drainage area (square miles)	Period of record or method of measuring	Maximum flood previously known			Maximum during present flood			
			Date	Gage height (feet)	Discharge Second-foot square mile	Time	Gage height (feet)	Discharge Second-foot square mile	
Ohio River at Sewickley	19,500	1932-41	Mar. 18, 1936	34.75	29	June 5	16.68	294,000	12
Monongahela River at Greensboro	4,467	1939-41	Mar. 18, 1936	28.4	33	June 4	24.28	117,000	—
Monongahela River at Charleston	5,213	1898-1905, 1933-41	July 11, 1888	28.1	30	June 5	20.85	133,000	—
Monongahela River at Bradlock	7,337	1939-41	Mar. 18, 1936	38.8	(3)	June 5	31.20	201,000	—
Dunkard Creek at Shannon	7,229	1941	Nov. 4, 1936	13.8	45	June 4, 12 p. m.	14.02	16,800	75
South Fork Tenmile Creek at Jefferson	180	1931-41	Nov. 4, 1936	13.8	45	June 4	18.45	13,800	77
Tenmile Creek in West Bethlehem Township	99.3	Dam				June 4 or 5	7,800	7,827	79
Wisecarver Run near Waynesburg	3.91	do				do	8,900	422	—
Ruff Creek at Grimes School	21.1	Slope area				do	9,490	399	—
Castile Run at Old Castle School	23.8	do				do	1,320	1,320	—
Castile Run at Riggle Farm	1.76	do				do	10,000	2,350	—
Tenmile Run near Brownsville	4.28	do				do	3,650	1,390	—
Tributary to Monongahela River in East Pike Run Township	2.68	Dam				do	286	1,680	—
Run Township	.17					do			—
Dunkard Creek at Dunlap	36.0	Slope area				do	3,560	99	—
Branch of Dunlap Creek in Redstone Township	.10	Dam				do	45	237	—
Koumle Run in Redstone Township	.24	do				do	58	242	—
Koumle Creek at Brazzetta	105	Slope area				do	12,800	122	—
Allen Run, near Stock	3.17	Dam				do	3,070	968	—
Crabapple Run at Spillway Lake	1.33	do				do	1,860	—	—
Washwater Run in Jefferson Township	1.38	do				do	1,320	835	—
Colvin Run at Grindstone	2.68	do				do	2,000	746	—
Pike Run at Delsytown	20.1	Contracted opening				do	7,710	384	—
Pike Run near Coal Center	20.7	Slope area				do	7,860	380	—
North Branch Little Redstone Creek in Jefferson Township	.28	Dam				do	194	550	—
Youghiogheny River below Confluence	1,029	1940-41	Mar. 18, 1936	20.28	—	June 4	16.42	43,800	43
Youghiogheny River at Connelville	1,328	1908-41	do	30.65	—	June 5	17.02	63,600	45
Youghiogheny River at Sutersville	1,715	1913-29, 1931-36, 1938-41	Mar. 17, 1936	16.4	58	June 5	27.34	78,000	49
Casselman River at Marklefon	121	do	do	10.30	85	June 5, 12:30 a. m.	10.24	18,700	49
Laurel Hill Creek at Urshna	110	Dam	do	10.28	85	June 4, 10:30 p. m.	7.98	9,400	78
Indian Creek at Indian Run Dam	.65	do	do	do	do	June 4 or 5	6,430	58	—
Branch of Washington Run at Upper Dam, at Star Junction	1.02	do	do	do	do	do	730	1,120	—
Branch of Washington Run at Lower Dam, at Star Junction	31.8	do	do	do	do	do	620	5,197	—
Jacobs Creek at Upper Dam, at Bridgeport		do	do	do	do	do			—

1. 1941-42. 2. 1942-43. 3. Discharge not determined. 4. Furnished by Pennsylvania Department of Forests and Waters. 5. Affected by storage. 6. Furnished by Corps of Engineers.

PREVIOUS FLOODS AT SALAMANCA, N. Y.

In the course of his investigation of the flood of July 1942, Hollister Johnson obtained interesting information concerning previous floods in Allegheny River at Salamanca, N. Y. Although the information may not be very accurate owing to the lapse of time and the changes that have taken place in the river channel, they are reported for possible future use.

Floods of June 1889 and June 1892.—Although there is some doubt as to which of these two floods was the higher, there is little doubt that both were higher than the flood of 1942. Mr. A. R. Eaton remembered that the flood of 1889 reached the bottom of the floor boards in the basement of the Eaton Department Store on Broad St. (now the S and S store) in which he was living. (Flood elevation was 1,374.3 feet compared with 1,373.7 feet in 1942.) Mr. Fred F. Nies has photographs presumably taken during the flood of 1892. From one of these photographs, the flood elevation at the Shultz Bros. tailoring shop, about 200 feet above the highway bridge, was found to be 1,375.7 feet. Mr. Nies also stated that the flood of 1892 just covered plumbing benches in the basement of Andrews Hardware Store, just downstream from the highway bridge, at elevation 1,374.9 feet. The 1942 flood at these locations was 1,374.9 and 1,374.0 feet, respectively. The significance of these old flood elevations is largely dissipated by the statement of Mr. Eaton that there have been important changes in the river channel since then. There used to be a mill race on the left bank, about three-quarters of a mile long, with a dam for diverting water into it.

Flood of March 1913.—Mr. Fred F. Nies built his garage and plumbing shop on the right bank of the river about 100 feet below the highway bridge. Although the flood of 1913 was about 18 inches below the floor of this garage, according to Mr. Nies, no flood prior to 1942 ever reached the floor. (Flood elevations, 1,371.2 and 1,373.9 feet, respectively.)

REFERENCES CITED

- Corbett, D. M., and others, 1943, Stream-gaging procedure, a manual describing methods and practices of the Geological Survey: U. S. Geol. Survey Water-Supply Paper 888.
- Delaware County Institute of Science, 1910, The flood of 1843 [report of a special committee, reprinted]: Delaware Co. Inst. Sci. Proc., vol. 6, no. 1, 1910, pp. 1-46; vol. 6, no. 2, 1911, pp. 54-86.
- Dwight, B. W., 1822, An account of a remarkable storm which occurred at Catskill, July 26, 1819; Am. Jour. Sci. [1st series, Benjamin Silliman, editor], vol. 4, pp. 125-142.
- Houk, I. E., 1922, An unusual flood of eighty years ago: Eng. News-Record, vol. 89, pp. 402-3.

- King, H. W., 1932, Handbook of hydraulics, 3d ed., McGraw-Hill Book Co.
- Langbein, W. B., and others, 1947, Major winter and nonwinter floods in selected basins in New York and Pennsylvania: U. S. Geol. Survey Water-Supply Paper 915, pp. 4-13.
- Monthly Weather Review, June 1941, vol. 69, no. 6, p. 188.
- Pennsylvania Dept. of Forests and Waters, 1943, The flood of July 1942 in the upper Allegheny River and Sinnemahoning Creek basins, Harrisburg.
- Schrader, F. F., 1945, Notable local floods of 1939, part 2, Flood of July 5, 1939, in eastern Kentucky: U. S. Geol. Survey Water-Supply Paper 967-B, pp. 44-45.
- Showalter, A. K., 1941, Meteorology of the storm, in Youngquist, C. V., and Langbein, W. B., Flood of August 1935 in the Muskingum River basin, Ohio: U. S. Geol. Survey Water-Supply Paper 869, pp. 29-31.
- U. S. Weather Bureau [1944], Storm of July 17-18, 1942, New York-Pennsylvania, supplement to Daily and Hourly Precipitation, compiled by Hydrologic Unit, Albany, N. Y.

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