

# A Galerkin Finite-Element Flow Model to Predict the Transient Response of a Radially Symmetric Aquifer

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Public Works,  
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Department of  
Health Services,  
Suffolk County Water  
Authority, and  
New York State  
Department of  
Environmental  
Conservation



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By Thomas E. Reilly

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# CONTENTS

Abstract	1
Introduction	1
Model assumptions and capabilities	1
Theory	2
Solution of radial flow equation by Galerkin finite-element method	2
Boundary conditions	5
Matrix equations	6
Computer program	7
Main	7
Subroutine CHECK	8
Subroutine MLTBM	8
Subroutine SOLVE	8
Evaluation of the model	8
Comparison with analytical solutions	8
Comparison with model solutions	9
Observations on model behavior	10
Possible program modifications	10
Design considerations for the finite-element grid	11
Sample simulation	11
Conclusions	13
References cited	13
Supplemental Data:	
I. Fortran IV program listing	16
II. Definition of selected program variables	24
III. Data input formats	25
IV. Array dimensioning	27
V. Input data for test problem	28
VI. Selected output for test problem	32

## FIGURES

1. Diagram showing conceptual model of aquifer flow system	1
2. Diagram of modeled aquifer section showing configuration of triangular elements	3
3. Diagram showing typical linear triangular basis function	4
4-7. Graphs showing:	
4. Comparison of model results with Theis' dimensionless analytical solution	9
5. Comparison of model results with Hantush's analytical solution for head buildup in aquifer directly beneath center of a circular recharge basin and at 500 ft from basin center	10
6. Comparison of water-table drawdowns as predicted by Stallman's analog model and the Galerkin finite-element model	11
7. Comparison of drawdowns at the base of an aquifer as predicted by Stallman's analog model and the Galerkin finite-element model	12
8. Diagram showing radial cross section of aquifer for sample simulation	13

## GLOSSARY

$A$	Cross sectional area of an element ( $L^2$ ).	$i$	Node number.
$[A]$	Coefficient matrix.	$j$	Node number.
$[B]$	Time-dependent coefficient matrix.	$k$	Node number.
$I$	General surface integral for a linear triangular element.	$k$	Time level.
$\{F\}$	Vector containing the flux terms.	$l$	Length long the side of an element ( $L$ ).
$K_r$	Hydraulic conductivity in the radial ( $r$ ) direction ( $LT^{-1}$ ).	$m$	Number of nodes.
$K_z$	Hydraulic conductivity in the vertical ( $z$ ) direction ( $LT^{-1}$ ).	$n_r$	Outward pointing directional cosine.
$[LHS]$	Coefficient matrix containing all terms on the left hand side of an equation.	$n_z$	Outward pointing directional cosine.
$M$	Number of nodes.	$q$	Specific discharge ( $LT^{-1}$ ).
$N$	Number of elements.	$q_r$	Flux through an element in the radial direction ( $LT^{-1}$ ).
$NE$	Number of elements with boundaries at the well.	$q_z$	Flux through an element in the vertical direction ( $LT^{-1}$ ).
$Q$	Total well discharge ( $L^3T^{-1}$ ).	$r$	Radial direction or distance from the center line of the well ( $L$ ).
$Q_e$	Well discharge through element $e$ ( $L^3T^{-1}$ ).	$\bar{r}$	Average radius of an element ( $L$ ).
$R$	Radius of basin ( $L$ ).	$r^\circ$	Radial distance of a free surface node ( $L$ ).
$R$	Residual.	$r_w$	radius of the well ( $L$ ).
$\{RHS\}$	Vector comprising all terms on the right hand side of an equation.	$s$	drawdown ( $L$ )
$S$	Storage coefficient.	$s_j(t)$	time dependent coefficient associated with node $j$ .
$S_s$	Specific storage coefficient ( $L^{-1}$ ).	$\hat{s}$	approximation of $s$ ( $L$ ).
$S_y$	Specific yield.	$t$	time ( $T$ ).
$T$	Transmissivity ( $L^2T^{-1}$ ).	$u$	$\frac{r^2 S}{4Tt}$
$W$	Recharge rate ( $LT^{-1}$ ).	$z$	vertical direction or distance ( $L$ ).
$W(u)$	Well function of $u$ , an exponential integral.	$\Sigma$	summation.
$a_i$	A constant in the basis function.	$\alpha^\circ$	coefficient associated with the unconfined boundary.
$a_{ij}$	A member of matrix $[A]$ .	$\Delta$	area of triangular element.
$b$	Thickness of aquifer ( $L$ ).	$\partial$	partial differential.
$b_i$	A coefficient in the basis function.	$\int_r ds$	surface integral over the outer boundary.
$b_{ij}$	A member of matrix $[B]$ .	$\mu$	a general dependent variable used as an example.
$c_i$	A coefficient in the basis function.	$\pi$	3.1416.
$e$	Element number.	$\phi_j^e(r, z)$	linear basis function associated with node $j$ in element $e$ .
$f_i$	Flux term.	$\Psi$	$\frac{r}{b} \sqrt{\frac{K_z}{K_r}}$

# A Galerkin Finite-Element Flow Model to Predict the Transient Response of a Radially Symmetric Aquifer

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## Abstract

A computer program developed to evaluate radial flow of ground water, such as at a pumping well, recharge basin, or injection well, is capable of simulating anisotropic, inhomogeneous, confined, or pseudo-unconfined (constant saturated thickness) conditions. Results compare well with those calculated from published analytical and model solutions. The program is based on the Galerkin finite-element technique.

A sample model run is presented to illustrate the use of the program; supplementary material provides the program listing as well as a sample problem data set and output. From the text and other material presented, one can use the program to predict drawdowns from pumping and ground-water buildups from recharge in a radially symmetric ground-water system.

## INTRODUCTION

Several aspects of ground-water hydrology that concern radial flow address such questions as the distribution of drawdowns near a pumping well or ground-water buildup beneath a circular recharge basin or at a recharge well. To study and evaluate the performance of wells or basins and determine short- and long-term effects of their operation on the ground-water system, it is necessary to represent the system's physical properties mathematically and to calculate the response to given rates of pumping or withdrawal.

Questions concerning simple hydrologic situations can be solved through published analytical solutions, which generally provide accurate predictions. However, problems concerning more complex flow systems such as multi-aquifer systems require analysis by numerical methods. The purpose of this report is to introduce a computer program capable of solving many of the more complex radial-flow problems. The program was developed in cooperation with the Nassau County Department of Public Works, the Suffolk County Department of Health Services, the Suffolk County Water Authority, and the New York State Department of Environmental Conservation.

## MODEL ASSUMPTIONS AND CAPABILITIES

The numerical model of ground-water flow described in this report simulates transient radial flow of ground water in which the flow field is two dimensional and symmetric around a central axis. Figure 1 illustrates the conceptual flow system to be simulated by the model. The numerical tech-

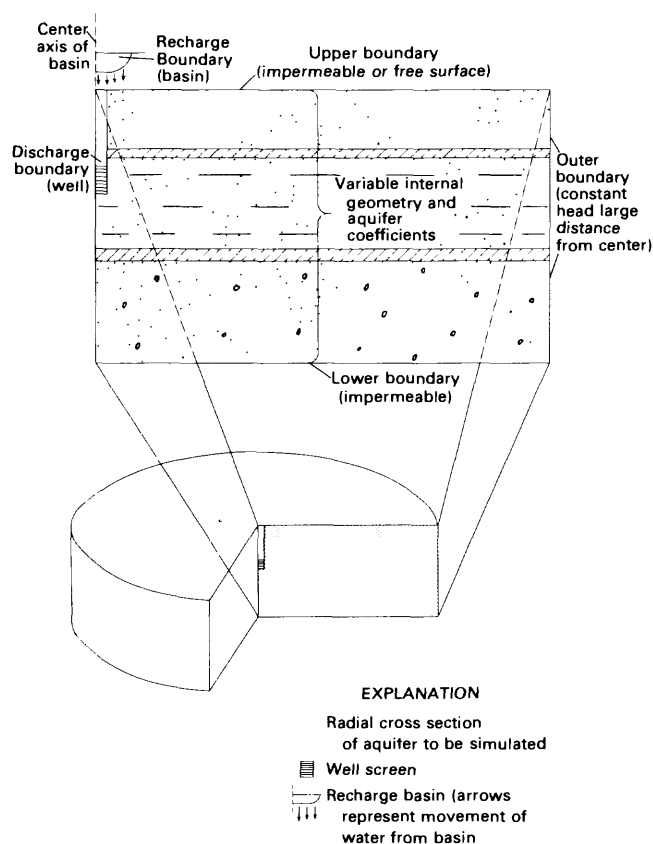


Figure 1. Conceptual model of aquifer flow system.

nique used to solve the transient radial flow equations is called the Galerkin finite-element method.

Use of the model requires certain assumptions, or simplifications, which must be evaluated before the program can be used successfully. These assumptions are

1. The flow field is radially symmetric.
2. No seepage face occurs in the well.
3. The saturated thickness of an unconfined aquifer does not change significantly during pumping or recharge.
4. The aquifer is finite in extent. (A constant-potential boundary far from the well or basin is generally established in the model.)

5. The well is pumped at either a constant rate or at stepped rates.
6. The specific yield of the aquifer is a constant.
7. The coefficient of specific storage ( $S_s$ ) is a constant over the entire model grid.

Even with these assumptions, few of which are entirely true in the real system, the computer program can be used to predict drawdowns or buildups resulting from a wide range of conditions. The program is capable of simulating anisotropic, inhomogeneous, confined, and pseudo-unconfined (constant saturated thickness) conditions. (The pseudo-unconfined aquifer is so called because the element configuration in the model is constant, which means that, even though the model nodes representing the free surface have a storage coefficient representing unconfined or water-table conditions, the saturated thickness does not decrease in response to drawdowns. Thus, caution or "engineering judgment" must be used when the predicted drawdown at the free surface represents a significant percentage of the aquifer thickness.) In addition, the well-bore geometry can be simulated, and the well screen can be partially penetrating and screened in zones of differing hydraulic conductivity.

Linear triangular elements are used to represent the flow field. The radial section to be simulated is represented as a net, or grid, of connected elements, as depicted in figure 2. The model program allows each element to be assigned a value of the hydraulic conductivity in the radial ( $K_r$ ) and vertical ( $K_z$ ) directions. In contrast, the coefficient of specific storage ( $S_s$ ) and the specific yield ( $S_y$ ) are treated as constants throughout the grid. The treatment of the storage coefficients as constants is not a restriction of the solution technique but only the manner in which it was programmed; this is discussed later in the section "Possible Program Modifications."

## THEORY

### Solution of Radial Flow Equation by Galerkin Finite-Element Method

The equation that describes two-dimensional radial flow of ground water in cross section (Cooley, 1974, p. 20) is

$$\frac{\partial}{\partial r} \left( K_r r \frac{\partial s}{\partial r} \right) + \frac{\partial}{\partial z} \left( K_z r \frac{\partial s}{\partial z} \right) = S_s r \frac{\partial s}{\partial t}, \quad (1)$$

where

- $s$  = drawdown ( $L$ ),
- $r$  = radial distance ( $L$ ),
- $z$  = vertical distance ( $L$ ),
- $t$  = time ( $T$ ),
- $K_r$  = radial hydraulic conductivity ( $LT^{-1}$ ),
- $K_z$  = vertical hydraulic conductivity ( $LT^{-1}$ ),
- $S_s$  = coefficient of specific storage ( $L^{-1}$ ).

To simulate the continuous system, equation 1 is approximated by a series of linear algebraic equations. These linear equations are derived from the finite-element method by the Galerkin method of weighted residuals (Pinder and Gray, 1977, p. 54).

The linear equations are generated by discretizing, or dividing, the entire flow field, or aquifer cross section (fig. 1), into separate elements (linear triangular elements in this model). The drawdown in the flow field can be approximated through a linear basis function  $\phi(r, z)$ , which is defined separately for each element. The drawdown is approximated as

$$\hat{s} = \sum_{e=1}^N \sum_{j=1}^M s_j(t) \phi_j^e(r, z) \quad (2)$$

where

- $\hat{s}$  = approximation of  $s$ ,
- $N$  = number of elements,
- $M$  = number of nodes,
- $\phi_j^e(r, z)$  = linear basis function associated with node  $j$  in element  $e$ ,
- $s_j(t)$  = time dependent coefficient associated with node  $j$ ,

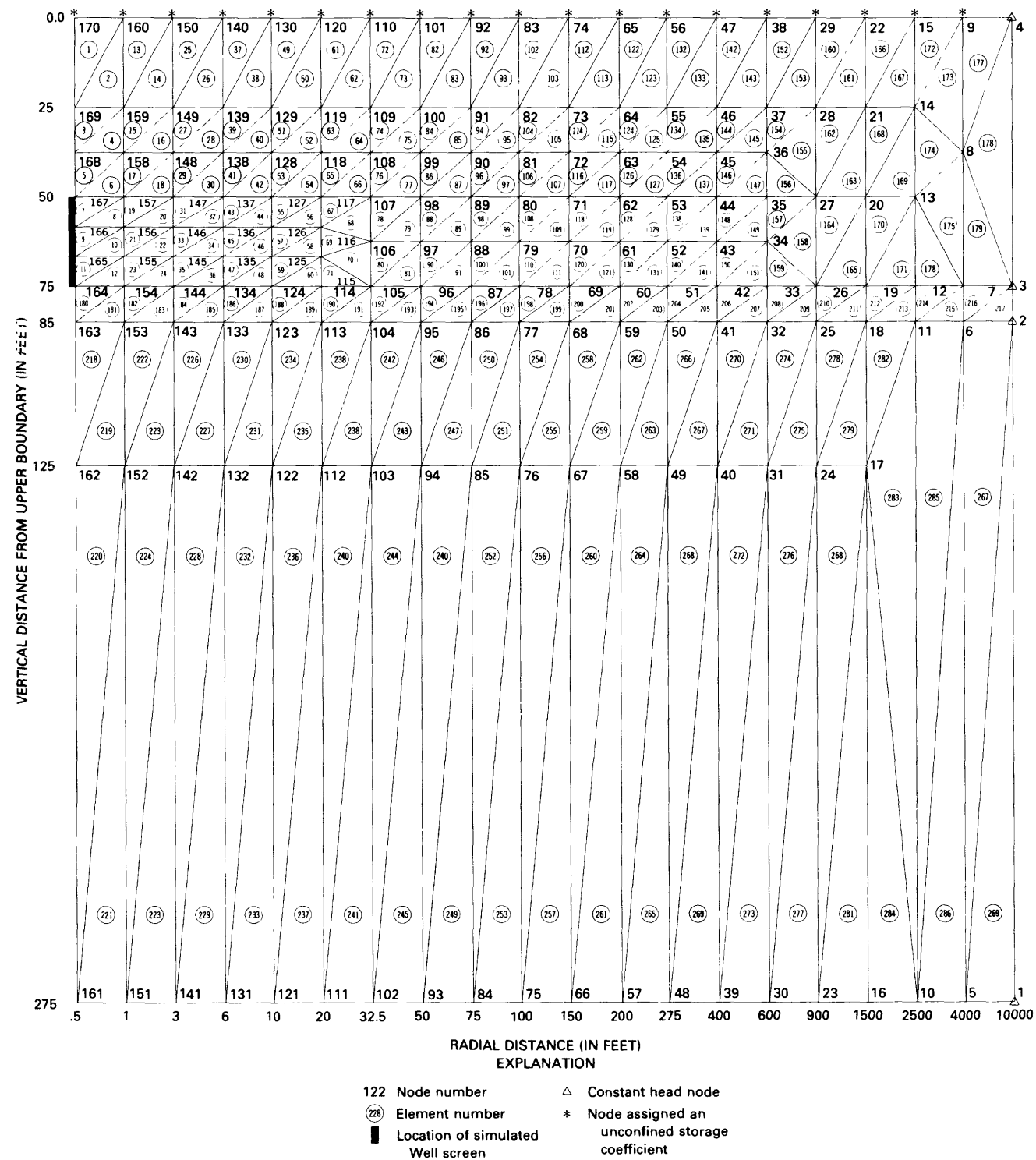
Substitution of  $\hat{s}$  for  $s$  into equation 1 and rearrangement of terms gives

$$\frac{\partial}{\partial r} \left( K_r r \frac{\partial \hat{s}}{\partial r} \right) + \frac{\partial}{\partial z} \left( K_z r \frac{\partial \hat{s}}{\partial z} \right) - S_s r \frac{\partial \hat{s}}{\partial t} \approx 0. \quad (3)$$

The residual is the amount by which the equation with  $\hat{s}$  varies from the actual solution of zero. Therefore, the residual  $R$  can be defined as:

$$R = \frac{\partial}{\partial r} \left( K_r r \frac{\partial \hat{s}}{\partial r} \right) + \frac{\partial}{\partial z} \left( K_z r \frac{\partial \hat{s}}{\partial z} \right) - S_s r \frac{\partial \hat{s}}{\partial t}. \quad (4)$$

If  $\hat{s}$  were exact, the residual would be zero. Galerkin's



**Figure 2.** Modeled aquifer section showing configuration of triangular elements.



method attempts to force the residual toward zero over the entire domain by weighting the residual by the basis function. The weighted residual equation is

$$\int_r \int_z R \phi_i dz dr = 0 \quad (i=1,2,3,\dots,m). \quad (5)$$

Each node has an approximate solution  $\hat{s}$  and therefore a weighted residual equation. Because the number of nodes equals the number of equations, with use of appropriate boundary and initial conditions, this set of simultaneous ordinary differential equations in time can be solved for  $\hat{s}$ .

The set of equations is derived by substituting for  $R$  into our weighted residual equation (eq. 5), which gives

$$\begin{aligned} \int_r \int_z \left\{ \frac{\partial}{\partial r} \left[ K_r r \frac{\partial}{\partial r} \left( \sum_{n=1}^N \sum_{j=1}^M s_j(t) \phi_j^e(r,z) \right) \right] \phi_i^e \right. \\ \left. + \frac{\partial}{\partial z} \left[ K_z r \frac{\partial}{\partial z} \left( \sum_{n=1}^N \sum_{j=1}^M s_j(t) \phi_j^e(r,z) \right) \right] \phi_i^e \right. \\ \left. - S_r \frac{\partial}{\partial t} \left( \sum_{n=1}^N \sum_{j=1}^M s_j(t) \phi_j^e(r,z) \right) \phi_i^e \right\} dz dr = 0 \quad (6) \\ \text{(for } i=1,2,3,\dots,m). \end{aligned}$$

Before equation 6 can be simplified, the basis function must be defined. For this model, the linear triangular element was selected. The basis function ( $\phi$ ) is in the form of a plane and is expressed as

$$\phi_i(r,z) = a_i + b_i r + c_i z \quad (7)$$

where the coefficients are

$$a_i = (r_j z_k - r_k z_j) / 2\Delta,$$

$$b_i = (z_j - z_k) / 2\Delta,$$

$$c_i = (r_k - r_j) / 2\Delta,$$

$$\Delta = \text{area of triangular element,}$$

and

$$i, j, k = \text{element nodes, in counterclockwise order.}$$

Thus, a basis function is defined for each node in each element. A property of the basis function defined is

$$\phi_i^e + \phi_j^e + \phi_k^e = 1 \quad (8)$$

where

$i, j$ , and  $k$  are the three nodes in element  $e$ .

The basis function has a value only over the element for which it is defined and is zero over all other elements. Its value is unity at the node for which it is defined and decreases linearly to zero at the other two nodes associated with the element. Figure 3 depicts the basis function  $\phi_i^e$ .

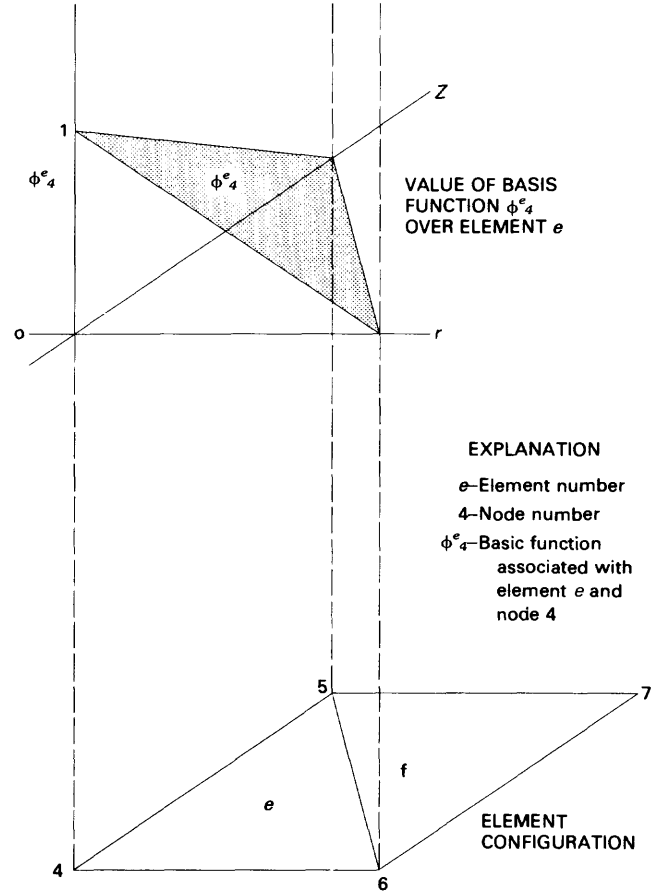


Figure 3. Typical linear triangular basis function.

Two additional assumptions that will allow for simplification of equation 6 are (a) both radial and vertical hydraulic conductivity are defined as a constant in each element, and (b) an average radius ( $\bar{r}$ ) is defined for each element. This average radius ( $\bar{r}$ ) is defined as

$$\bar{r} = (r_i + r_j + r_k) / 3 \quad (9)$$

This average radius ( $\bar{r}$ ) was defined similarly by Pinder and Gray (1977, p. 139).

Substituting and using the notation for the inner product as

$$\int_r \int_z \phi_j^e \phi_i^e dz dr = \langle \phi_j^e, \phi_i^e \rangle \quad (10)$$

equation 6 can be written as

$$\begin{aligned} \sum_{j=1}^M \sum_{n=1}^N \left[ s_j(t) K_r^e \bar{r}^e \left\langle \frac{\partial^2 \phi_j^e}{\partial r^2}, \phi_i^e \right\rangle + s_j(t) K_z^e \bar{r}^e \left\langle \frac{\partial^2 \phi_j^e}{\partial z^2}, \phi_i^e \right\rangle \right. \\ \left. - S_r r_j \left\langle \phi_j^e, \phi_i^e \right\rangle \frac{\partial s_j(t)}{\partial t} \right] = 0 \quad (11) \\ (i = 1, 2, 3, \dots, m) \end{aligned}$$

Because the basis function ( $\phi$ ) is a linear function, the second derivative is trivial (zero on the interior of an element). To circumvent this problem, Green's Theorem or Integration by Parts (Pinder and Gray, 1977, p. 83) is used. This changes the inner products to

$$\left\langle \frac{\partial^2 \phi_j^e}{\partial r^2}, \phi_i^e \right\rangle = - \left\langle \frac{\partial \phi_j^e}{\partial r}, \frac{\partial \phi_i^e}{\partial r} \right\rangle + \int_r \phi_i^e \frac{\partial \phi_j^e}{\partial r} n_r ds \quad (12a)$$

and

$$\left\langle \frac{\partial^2 \phi_j^e}{\partial z^2}, \phi_i^e \right\rangle = - \left\langle \frac{\partial \phi_j^e}{\partial z}, \frac{\partial \phi_i^e}{\partial z} \right\rangle + \int_r \phi_i^e \frac{\partial \phi_j^e}{\partial z} n_z ds \quad (12b)$$

where

$\int_r ds$  represents a surface integral over the outer boundary, and  $n_r$  and  $n_z$  are outward-pointing directional cosines. These surface integrals (flux terms) become the forcing functions of the problem for all flux boundaries. The surface integrals become zero for impermeable boundaries and are readily defined for discharge and recharge boundaries.

Substituting the result of Green's Theorem into equation 11 results in the final form, which is a set of ordinary differential equations:

$$\begin{aligned} \sum_{j=1}^M \sum_{e=1}^N \left[ s_j(t) K_r^e \bar{r}^e \left\langle \frac{\partial \phi_j^e}{\partial r}, \frac{\partial \phi_i^e}{\partial r} \right\rangle + s_j(t) K_z^e \bar{r}^e \left\langle \frac{\partial \phi_j^e}{\partial z}, \frac{\partial \phi_i^e}{\partial z} \right\rangle \right. \\ \left. + S_s r_j \left\langle \phi_j^e, \phi_i^e \right\rangle \frac{\partial s_j(t)}{\partial t} - s_j(t) K_r^e \bar{r}^e \int_r \phi_i^e \frac{\partial \phi_j^e}{\partial r} n_r ds \right. \\ \left. - s_j(t) K_z^e \bar{r}^e \int_r \phi_i^e \frac{\partial \phi_j^e}{\partial z} n_z ds \right] = 0 \quad (13) \end{aligned}$$

( $i=1,2,3, \dots, m$ )

The inner products are non-zero only when the node indices  $i$  and  $j$  are part of the same element. Thus, the inner products can be defined over an element. The integration formulae for a linear triangular element are

$$\left\langle \frac{\partial \phi_j}{\partial r}, \frac{\partial \phi_i}{\partial r} \right\rangle = b_i b_j \triangle \quad (14a)$$

$$\left\langle \frac{\partial \phi_j}{\partial z}, \frac{\partial \phi_i}{\partial z} \right\rangle = c_i c_j \triangle \quad (14b)$$

$$\langle \phi_j, \phi_i \rangle = \triangle/6 \quad (j = i) \quad (14c)$$

$$\langle \phi_j, \phi_i \rangle = \triangle/12 \quad (j \neq i) \quad (14d)$$

When the integration formulae are substituted into equation 13 as indicated by the summation indices, the result is a set of  $M$  (the number of unknowns) simultaneous ordinary differential equations. Thus, application of the Galerkin finite-element technique has changed the continuous partial differential equation into a set of simultaneous ordinary differential equations.

## Boundary Conditions

Constant-head (zero drawdown) boundaries are readily handled in the finite-element method. The nodes that are set at a constant zero drawdown are not unknowns, and the equations associated with those nodes drop out of the set of simultaneous equations.

Flux boundaries (Neumann boundary conditions) enter into the finite element representation of the radial-flow equation by means of the surface integrals generated by using Green's Theorem (eq. 12). For linear triangular elements, the surface integral, which can be represented in general terms as

$$I = \int_r \left( \frac{\partial \mu}{\partial n} \right) \phi_i ds, \quad (15)$$

can be integrated to become the following (Pinder and Gray, 1977, p. 124):

$$I = \frac{q l}{2} \quad (16)$$

where

$q$  = average value of  $\frac{\partial \mu}{\partial n}$  applied along the side, and

$l$  = length along the side.

In examining the flux boundaries that occur in the radial-flow problem, the evaluation of the surface integrals is identical to equation 16.

Ground-water discharge to the well is treated as a flux across the boundary of the elements at the well-screen location. The surface integral is reduced as

$$- \int_r s_j(t) K_r^e \bar{r}^e \phi_i^e \frac{\partial \phi_j^e}{\partial r} n_r ds = - \bar{r} \int_r \phi_i K_r \frac{\partial s}{\partial r} ds, \quad (17)$$

from Darcy's law,  $K_r \frac{\partial s}{\partial r} = q_r$ , which is the flux across the well boundary.

Then

$$- \bar{r} \int_r \phi_i q_r ds = - \bar{r} q_r^e \frac{(z_k - z_i)}{2} \quad (18)$$

where

$z_k - z_i$  = length of the element along the well screen,

and

$q_r^e$  = flux through element  $e$ .

At the well, the boundary of the element is at  $r_w$  (well radius), and the sum of all flows from the elements must be equal to total discharge ( $Q$ ).

Examining  $q_r^e$  more closely and again using Darcy's law, it can be shown that

$$q_r^e = \frac{Q}{A} = \frac{Q_e}{2\pi r_w (z_k^e - z_i^e)}, \quad (19)$$

where

$Q_e$  = flow through element  $e$ .

Substituting this result into the right-hand side of equation 18 gives

$$-\bar{r}q_r \frac{(z_k - z_i)}{2} = \frac{\bar{r} Q_e (z_k - z_i)}{4\pi r_w (z_k - z_i)}. \quad (20)$$

If  $\bar{r} \approx r_w$ , then

$$-\bar{r} \int_r \phi_i q_r^e ds = Q_e / 4\pi. \quad (21)$$

Therefore, the surface integral along the well simplifies down to equation 21.

The discharge from an individual element ( $Q_e$ ) is calculated in a manner similar to that used for a model prepared for the U.S. Geological Survey by Intercomp Resource Development and Engineering, Inc. (1976, p. B.2)<sup>1</sup>. The discharge of an element ( $Q_e$ ) in relation to the entire discharge ( $Q$ ) is proportional to the transmissivity of the element divided by the transmissivity of the entire screen length. Thus,

$$Q_e = \frac{Q K_r (z_k - z_i)}{\sum_{n=1}^{NE} K_r (z_k - z_i)}, \quad (22)$$

where

$Q_e$  is the discharge from the  $e$ th element,

$NE$  is the number of elements with boundaries at the well,

$Q$  is the total discharge, and

$(z_k - z_i)$  is the length of the element boundary at the well.

Therefore, the model allocates discharge rates along the well bore according to equation 22 for wells screened in aquifers of varying hydraulic conductivity. This boundary condition is not exact; it only approximates the theoretical withdrawal distribution of the well. The correct boundary condition is an equal drawdown along the well bore. Therefore, the model results will be somewhat in error if this condition is not realized.

Recharge is treated as a flux across the top surface of the aquifer. The surface integral in this case reduces as

$$-\int_r K_z \bar{r} \frac{\partial \hat{s}}{\partial z} \phi_i n_z ds = \frac{\bar{r} q_z (r_j - r_k)}{2} = -2\pi \bar{r} (r_j - r_k) q_z / 4\pi, \quad (23)$$

where

$q_z$  = recharge in flux units (ft/day).

The term  $2\pi \bar{r} (r_j - r_k)$  represents the surficial area of the top element, where the average circumference is  $2\pi \bar{r}$ , and  $(r_j - r_k)$  is the width of the element at the surface. Therefore,  $2\pi \bar{r} (r_j - r_k)$  is simply an approximation of  $\pi(r_j^2 - r_k^2)$ , which is the

actual surface area. The recharge at the surface then simplifies as

$$-\int_r K_z \bar{r} \frac{\partial \hat{s}}{\partial z} \phi_i n_z ds = - (r_j^2 - r_k^2) q_z / 4. \quad (24)$$

This is actually the same expression as the well discharge (or recharge) because  $\pi(r_j^2 - r_k^2) q_z$  is the amount of water being added into the element.

The water in storage released by the movement of the free surface (water table) can be simulated, although the actual movement of the free surface is not simulated. Thus, the solution for unconfined aquifers is valid only when the drawdown equals a small percentage of the total aquifer thickness.

For the unconfined boundary condition, the surface integral for the top boundary is reduced by a few assumptions. These were first described by Boulton (1954) and later by Stallman (1963), who show that

$$S_y \frac{\partial s}{\partial t} = K_z \left( \frac{\partial s}{\partial r} \right)^2 + K_z \left[ \left( \frac{\partial s}{\partial z} \right)^2 - \frac{\partial s}{\partial z} \right], \quad (25)$$

where

$S_y$  is called the specific yield and represents the volume of water which the rock or soil, after being saturated, will yield by gravity divided by the volume of the rock or soil (Lohman, 1972). This assumes that once a particle of water is on the free surface, it never leaves that surface. The additional assumption that the squared derivatives are much smaller than the first-order derivatives gives

$$S_y \frac{\partial s}{\partial t} \approx -K_z \frac{\partial s}{\partial z}. \quad (26)$$

The surface integral for the top horizontal boundary can therefore be evaluated as

$$-\int_r K_z \bar{r} \frac{\partial \hat{s}}{\partial z} \phi_i n_z ds \approx \int_r S_y \bar{r} \frac{\partial \hat{s}}{\partial t} \phi_i n_z ds \approx \frac{\bar{r} (r_j^2 - r_k^2) S_y}{2} \frac{\partial \hat{s}}{\partial t}, \quad (27)$$

where

$r^o$  indicates the radial distance of a free surface node.

By the same analysis for the surface area as in the recharge term, the boundary can be approximately evaluated as

$$\approx \frac{(r_j^2 - r_k^2) S_y}{4} \frac{\partial \hat{s}}{\partial t} \quad (28)$$

## Matrix Equations

The preceding section describes the finite-element equations that are used to generate a system of simultaneous ordinary differential equations from the general equation for radial flow of ground water in cross section. The equations are produced by calculating an element matrix (a set of coef-

<sup>1</sup> Use of trade names is for identification purposes only and does not imply endorsement by the U.S. Geological Survey.

ficients for that element). Because all elements contain three nodes, each element is associated with a three-by-three matrix. The coefficients for each element are then assembled into one "global" coefficient matrix, which represents the entire radial section of aquifer.

The global matrix equation to be solved (Pinder and Gray, 1977, p. 81) can be represented by

$$[A]\{s(t)\} + [B] \left\{ \frac{ds(t)}{dt} \right\} = \{F\} \quad (29)$$

where

$[A] = a_{ij}$ , which is made up of terms of the form

$$\sum_{e=1}^N \left[ \left\langle \frac{\partial \phi_i}{\partial r}, \frac{\partial \phi_j}{\partial r} \right\rangle + \left\langle \frac{\partial \phi_i}{\partial z}, \frac{\partial \phi_j}{\partial z} \right\rangle \right],$$

$[B] = b_{ij}$ , which is made up of terms of the form

$$\sum_{e=1}^N \left[ \langle \phi_i, \phi_j \rangle + \alpha^o S_j \right],$$

$\{F\} = f_i$ , which is made up of terms of the form

$$\sum_{e=1}^N \left[ \int_r f_i ds \right],$$

for  $i = 1, 2, \dots, M$

and  $j = 1, 2, \dots, M$

$\alpha^o$  = coefficient associated with the unconfined boundary condition,

$f_i$  = flux term.

The time derivative is then approximated using a backward difference formulation. When the finite difference approximation is used for the time derivative, the set of ordinary differential equations becomes a set of simultaneous algebraic equations. The backward difference approximation is unconditionally stable and is represented simply as

$$\frac{ds_j(t)}{dt} = \frac{s_j^{k+1} - s_j^k}{t_{k+1} - t_k} \quad (30)$$

where

$k$  is the time level.

The matrix equation can then be written:

$$[A]\{s^{k+1}\} + [B] \left\{ \frac{s^{k+1} - s^k}{\Delta t} \right\} = \{F\} \quad (31)$$

or simply,

$$\left( [A] + \frac{1}{\Delta t} [B] \right) \{s^{k+1}\} = \{F\} + \frac{1}{\Delta t} [B] \{s^k\}. \quad (32)$$

Let:

$$\left( [A] + \frac{1}{\Delta t} [B] \right) = [\text{LHS}] \quad (33a)$$

$$\left( \{F\} + \frac{1}{\Delta t} [B] \{s^k\} \right) = \{\text{RHS}\}. \quad (33b)$$

The final matrix equation is

$$[\text{LHS}] \{s^{k+1}\} = \{\text{RHS}\} \quad (34)$$

from which the drawdown at the new time step can then be solved. The technique used for this model is a direct solution technique.

The method that is used to generate the global matrices is actually less complicated than in equation 13. Because the inner products are zero when  $i$  and (or)  $j$  are not nodes of element  $e$ , the global matrix can be calculated more easily by using a modified form:

$$\begin{aligned} \sum_{e=1}^N \sum_{j=1}^3 \left[ s_j(t) K_r^e \bar{r}^e \left\langle \frac{\partial \phi_i^e}{\partial r}, \frac{\partial \phi_j^e}{\partial r} \right\rangle + s_j(t) K_z^e \bar{r}^e \left\langle \frac{\partial \phi_i^e}{\partial z}, \frac{\partial \phi_j^e}{\partial z} \right\rangle \right. \\ \left. + S_s r_j \langle \phi_j^e, \phi_i^e \rangle \frac{\partial s_j(t)}{\partial t} - s_j(t) K_r^e \bar{r}^e \int_r \phi_i^e \frac{\partial \phi_j^e}{\partial r} n_r ds \right. \\ \left. - s_j(t) K_z^e \bar{r}^e \int_r \phi_i^e \frac{\partial \phi_j^e}{\partial z} n_z ds \right] = 0 \end{aligned} \quad (i=1,2,3) \quad (13 \text{ modified})$$

where

$i=1, 2, 3$  and  $j=1, 2, 3$  are the three nodes in element  $e$ , and

$N$  = number of elements.

Thus, for each element, a three-by-three element matrix is defined. These matrices are then summed over all elements to obtain the global matrix to be used in the final matrix equations.

## COMPUTER PROGRAM

The input and output of the program data are not complex, and changes in the basic computer code to improve input-output or to add additional information should be straightforward to implement. The computer code consists of four routines—the main, subroutine CHECK, subroutine MLTBM, and subroutine SOLVE, which are listed in supplemental data I. A brief summary of each follows.

### Main

The main program runs the input, output, and equation setup. The equation setup is the calculation of the coefficients for the boundary conditions and the global coefficient matrices.

To save computer storage, the program uses banded matrix storage. The global coefficient matrix has a half band width equal to the largest difference between two adjacent node numbers. To run the program, the global array must be dimensioned properly (dimensions are given in sup-

plemental data IV), but the program calculates the band width, and the operator should note the size required.

### Subroutine CHECK

This subroutine checks the input data to see if they meet the requirements as stated in supplemental data III (Data Input Formats) and in the section "Design Considerations for the Finite-Element Grid." Data are checked for three criteria: (1) consistent constant head information, (2) proper vertical coordinate system, and (3) proper ordering of nodes in an element. If the input data fail to meet one of these criteria the reason is printed, and the program is terminated.

### Subroutine MLTBM

This small subroutine multiplies an array stored in a transformed banded manner with a vector.

### Subroutine SOLVE

This subroutine solves the final matrix equation and returns the drawdown solution. This routine was originally programmed by James O. Duguid of the Oak Ridge National Laboratory, (R. B. Wells, J. W. Mercer, and C. R. Faust, written commun., 1976). SOLVE accepts a fully banded non-symmetric matrix. During the solution process, the routine is called twice. The first call upper triangularizes the matrix using the Gauss-Doolittle method, and the second call solves the triangularized form by back substitution.

## EVALUATION OF THE MODEL

To insure that the model theory and programming are correct, comparisons were made between results from this model and those obtained from published problem solutions. Although the comparisons with analytical solutions by Theis (Lohman, 1972), and Hantush (Walton, 1970, p. 370) were done for simple examples, the close fit indicates that the model is sound. The comparison of model results to the simple analytical solutions is also intended to give some insight into the discretization errors of the finite-element model.

In a more complex test, results of Stallman's (Lohman, 1972) analog-model type curves were compared with results obtained by the model described in this report, and again, the similarity of results indicates that the model is valid. The comparison with Stallman's model results is primarily intended to check the trends in the response of the finite-element model to a complex problem. Although there may be better solutions available to compare the results of the finite-element model against, the purpose of this comparison is only to further substantiate that the finite-element model can simulate complex two-dimensional radial solutions. A discussion of both tests follows.

## Comparison with Analytical Solutions

In comparing the model results to Theis' analytical solution, the radial cross section was set to resemble as closely as possible the conditions of the analytical solution. The only condition that could not be met was that of an infinite aquifer, because the model must have a radial boundary at a finite distance. For comparison with the analytical solutions, a zero-drawdown boundary 10,000 ft from the center of the radial section was simulated.

To verify the model response for a well pumping under transient artesian conditions, model results were compared with the Theis type curve (from Lohman, 1972). The Theis solution is for a fully penetrating well in an infinite confined aquifer with no vertical movement of water. The model results were transformed into Theis' dimensionless coefficients  $u$  and  $W(u)$  from their relationship to drawdown and time:

$$u = \frac{r^2 S}{4T} \frac{1}{t} \quad (35a)$$

$$W(u) = \frac{4\pi T}{Q} (s) \quad (35b)$$

where

- $r$  = radial distance from well,
- $S$  = storage coefficient,
- $T$  = transmissivity,
- $Q$  = well discharge,
- $t$  = time,
- $s$  = drawdown.

Transformed model results for two different simulations are plotted against the Theis type curve in figure 4. The first simulation is plotted with circles, and the first point plotted is the worst comparison point. The second simulation used the same aquifer conditions as the first, except that a smaller initial time step (DELTA) was used. This second simulation is plotted with squares and indicates that part of the error in the first simulation was probably due to the truncation error associated with the time derivative. Thus, the smaller initial time step gives a better match with the Theis type curve.

The next check on the model response was to evaluate its accuracy in predicting water-table mounding beneath a circular recharge basin. In this check, the assumption of no recharge, which was used in formulating the upper unconfined boundary, was violated in that both the recharge flux and free-surface boundary conditions were used simultaneously.

The simplified form of Boulton's free-surface equation (eq. 26) is

$$S_r \frac{\partial s}{\partial t} \approx -K_r \frac{\partial s}{\partial z},$$

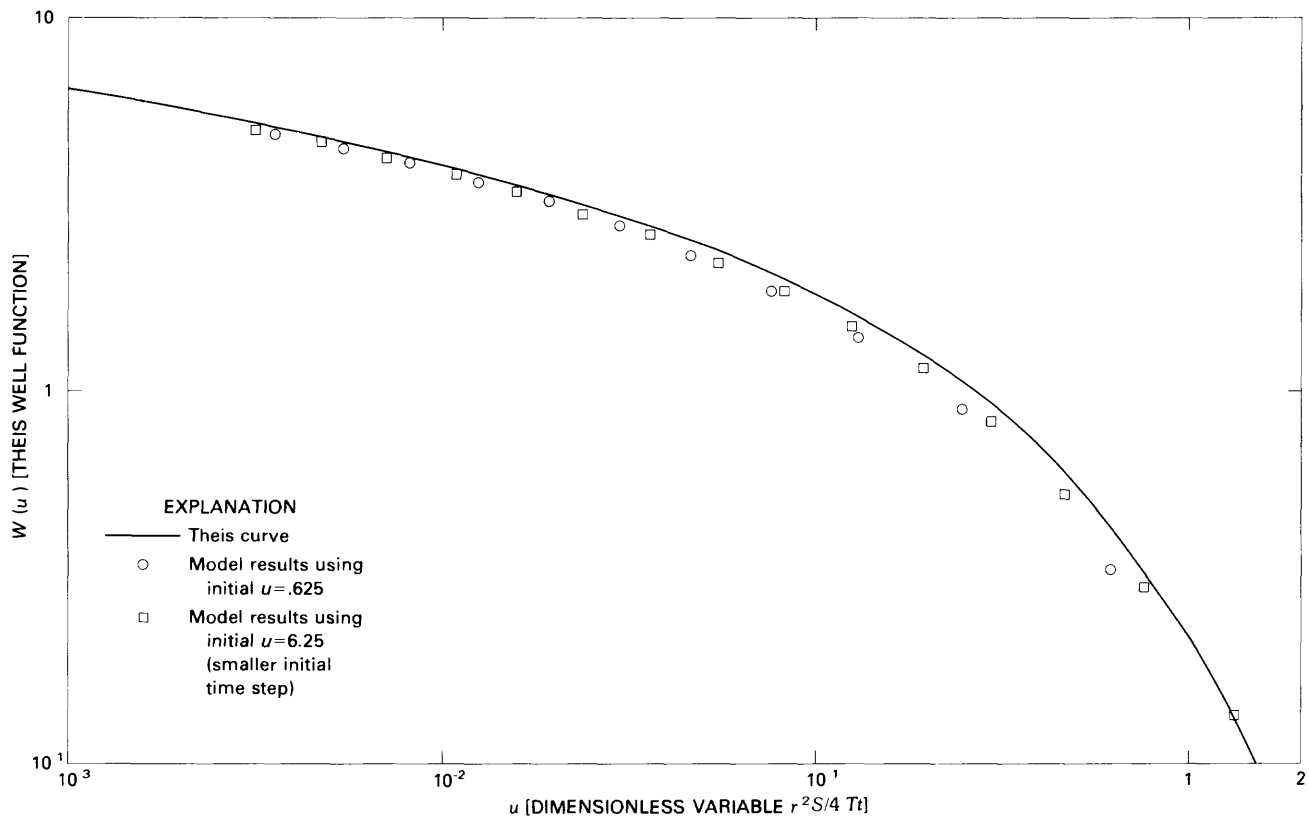


Figure 4. Comparison of model results with Theis' dimensionless analytical solution.

which relates two expressions for the rate of downward movement at the surface (Bennett and others, 1967). Boulton evaluated the substantial time derivative at the free surface, which made it necessary to assume that a particle of water always stays on the free surface. However, an equation analogous to the simplified equation (eq. 26) can be derived using a mass balance on an elemental volume at the free surface. This analogous equation allows recharge at the free surface and relates the boundary flux to the storage term and the recharge rate, which can be written:

$$-K_z \frac{\partial s}{\partial z} = S_y \frac{\partial s}{\partial t} - W(r) \quad (36)$$

where

$W(r)$  is the recharge rate.

This equation relates three expressions for the rate of downward movement at the free surface. Thus, comparison of the model results to the analytical solution for a recharge mound derived by Hantush (Walton, 1970) provides a check on the appropriateness of the boundary condition as expressed in equation 36.

To reproduce Hantush's solution, a simulation was

made for a one-dimensional homogenous aquifer with the following properties:

$b$  = thickness of aquifer = 700 ft,

$R$  = radius of basin = 100 ft,

$W$  = recharge rate = 10 ft/d,

$K_r$  = radial hydraulic conductivity = 100 ft/d,

$K_z$  = vertical hydraulic conductivity = 1,000,000 ft/d  
(The reason for the high vertical hydraulic conductivity is to approximate no vertical head gradient—the Dupuit assumption [Lohman, 1972, p. 11]),

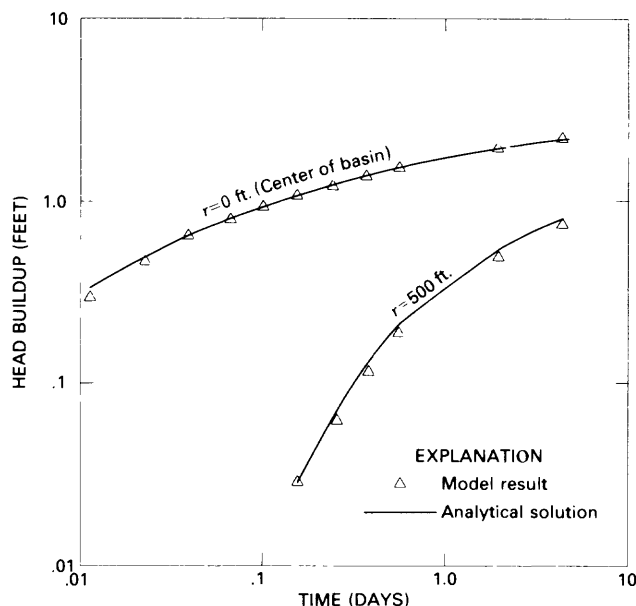
$S_s$  = specific storage = 0,

$S_y$  = specific yield = 0.3.

Model results are compared with Hantush's solution in figure 5. The results show a reasonably close match between the analytical solution and the model results.

## Comparison with Model Solutions

To fully evaluate the accuracy of the model results, it was necessary to simulate a pumping well in an unconfined, anisotropic aquifer. Stallman (Lohman, pl. 6, 1972) developed dimensionless type curves for such a case, with five families of curves, each for a different vertical screen setting.



**Figure 5.** Comparison of model results with Hantush's analytical solution for head buildup in aquifer directly beneath center of circular recharge basin and 500 ft from basin center.

Figures 6 and 7 compare results from the model described in this paper to results generated by Stallman's electric-analog model. Figure 6 indicates drawdowns at the water table, and figure 7 indicates drawdowns at the bottom of the aquifer. Model results are for various psi ( $\Psi$ ) values:

$$\Psi = \frac{r}{b} \sqrt{\frac{K_z}{K_r}} \quad (37)$$

where

- $b$  = thickness of aquifer,
- $r$  = radial distance of observation point from center,
- $K_r$  = radial hydraulic conductivity,
- $K_z$  = vertical hydraulic conductivity.

As can be seen in figures 6 and 7, the predicted response of the aquifer has the same characteristics for both solutions. The response at the water table is very different from the predicted response at the base of the aquifer and both solutions produce the same trends. Since Stallman's analog model was subject to errors due to space discretization and inaccuracies associated with the electrical components (resistors and capacitors usually have accuracies in the range of  $\pm 10$  percent) and the finite-element method presented is subject to errors due to space and time discretization, the results were expected to be slightly different. Taking this into account, the results appear to compare well even though some differences exist.

## Observations on Model Behavior

Although the model worked well in all tests, some noteworthy features emerged during some of the tests.

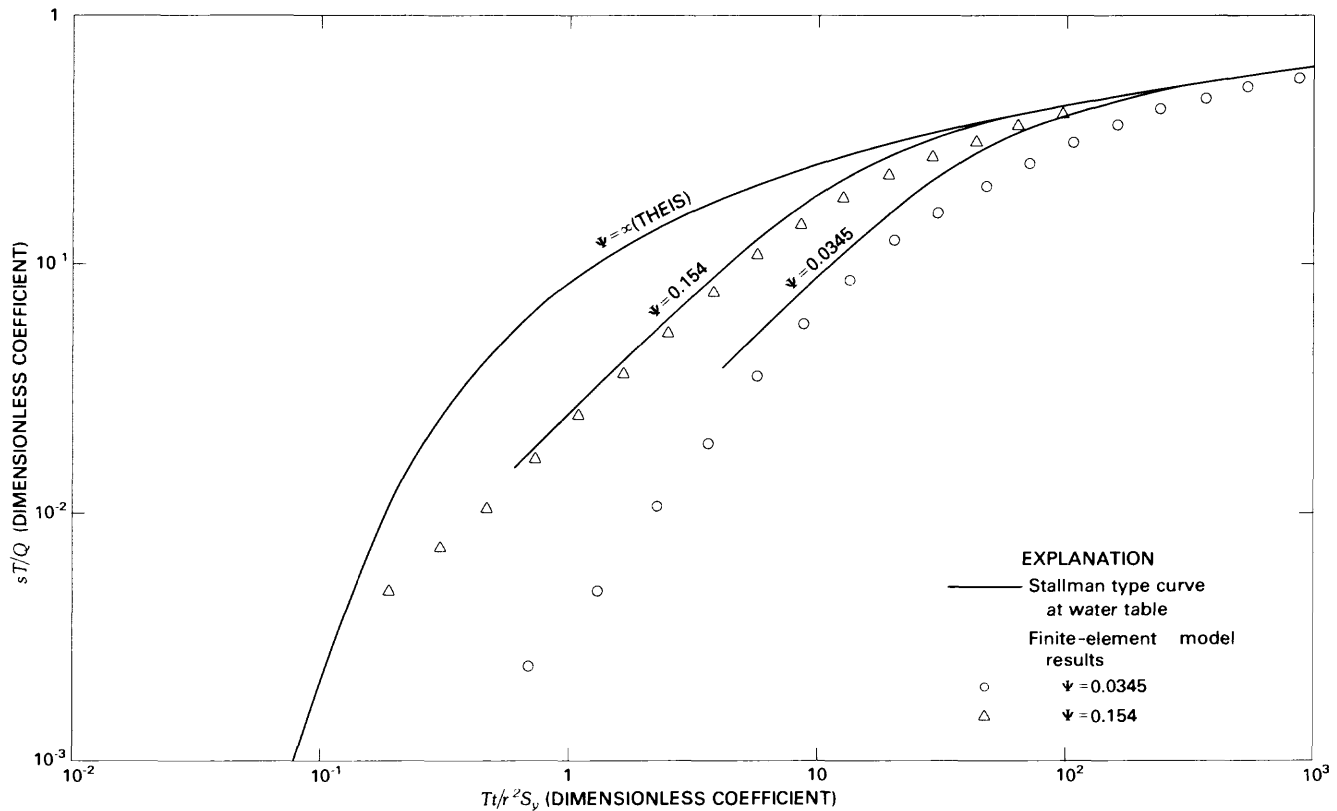
1. The average radius in the integration formula (eq. 9) is an approximation. Zienkiewicz (1971, p. 79-80) observed that because a fairly fine subdivision is required with linear triangular elements, this "one-point" integration is satisfactory. With a coarse element mesh, however, errors about the true solution become apparent, and in some cases the predicted drawdown will be slightly smaller or larger than in the actual system, depending on the three radius values of each element. In the tests involving comparison with the analytical solutions, most errors were insignificant compared to the magnitude of the drawdown.
2. The FORTRAN computer code was written for the CDC 7600 computer<sup>2</sup> at Brookhaven National Laboratories. In most test cases, it ran for the minimum charge. The computer code should be capable of running on most computers with no necessary modifications, but the cost may vary considerably among computer centers.

## Possible Program Modifications

The computer program presented contains assumptions that satisfied the author's needs for local application but may not satisfy the user's needs; for example, the assumption of a constant coefficient of specific storage ( $S_s$ ) over the entire grid. However, this constant in the program could easily be changed into an array that could assign a different specific storage to each element, and this can be accomplished by simply adding a read statement to read a specific storage for each element and changing the specific-storage constant in the calculation of the element matrices to an array associated with each element.

Two major modifications are being evaluated for future implementation. Although they would require extensive reprogramming, they could offer major advantages. The first would involve use of a logarithmic transformation on the radial coordinate system so that the flow system could be evaluated without use of the average-radius (eq. 9) approximation. As discussed in the previous section, this approximation introduces some error, and the logarithmic coordinate transformation should eliminate it. The second modification would simulate the vertical movement of the free surface (the

<sup>2</sup>Use of brand names is for identification purposes only and does not imply endorsement by the U.S. Geological Survey.



**Figure 6.** Comparison of water-table drawdowns as predicted by Stallman's analog model and the Galerkin finite-element model.

solution of the nonlinear problem), which may probably be accomplished by an iterative technique in which the element configuration heaves (or distorts) in the vertical dimension.

## DESIGN CONSIDERATIONS FOR THE FINITE-ELEMENT GRID

In designing a finite-element grid, two types of design constraints must be met—those imposed by the mathematical procedures and those imposed by the computer-programming procedure. Both should be understood before the user attempts to make use of the program.

Three design considerations are associated with the mathematical methods used. First, because the solution presented involves linear triangular elements and requires use of an average radius for each element, the flow field should be represented in as much detail as possible (a fine grid). Second, the solution technique assumes that the nodal order for each element is specified in counterclockwise direction (also noted in supplemental data III). Third, as mentioned previously, the global coefficient matrices are sparse-banded, and the smaller the band width, the more efficient the solution technique for both time and storage requirements.

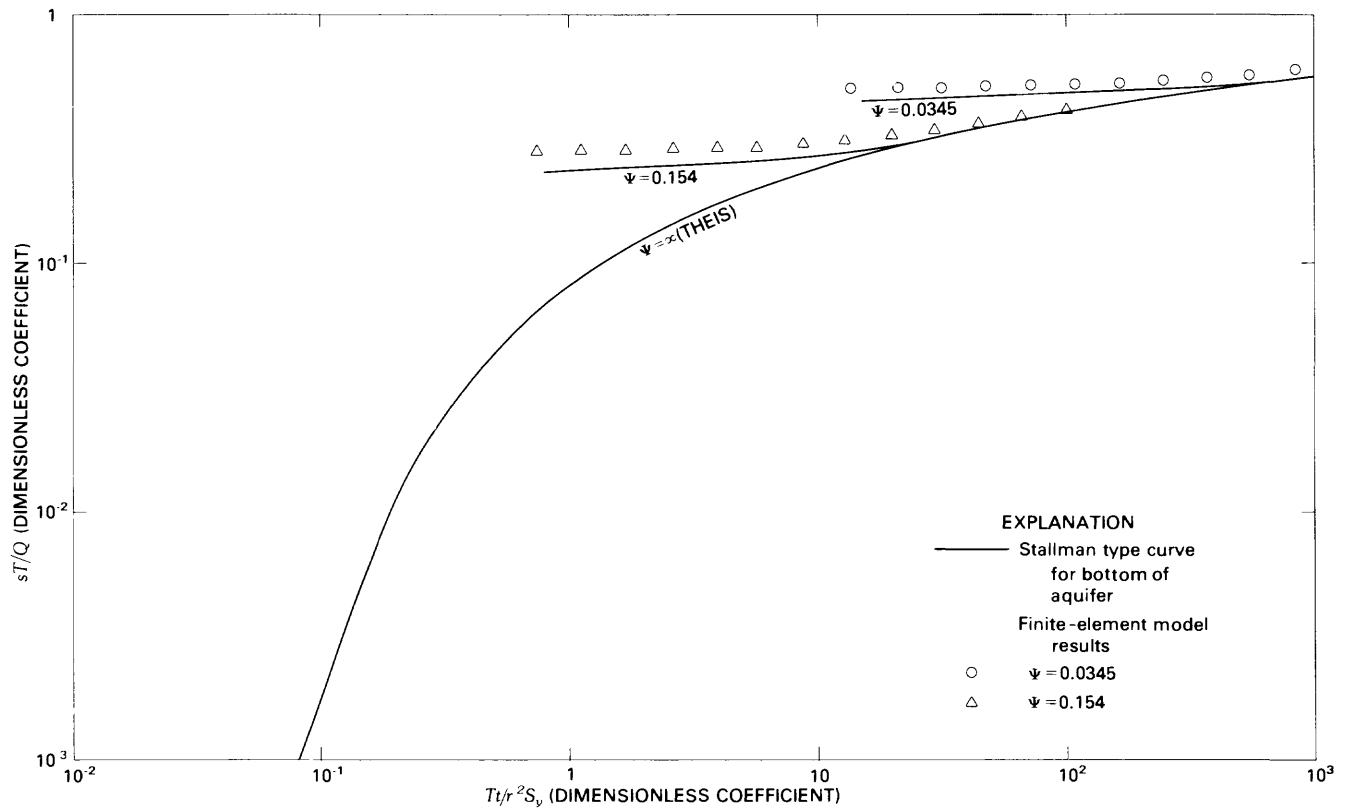
The band width is determined by the largest difference between two node numbers in an element. Thus, an efficient nodal order improves the efficiency of the solution. The program also outputs the band width for the user to check the array dimensions.

Two additional design considerations are necessary because of the way in which the mathematical procedure was programmed, and both must be met for the program to work. The first is that the constant zero-drawdown nodes must be numbered first, which means that if constant drawdown is to be represented by three nodes, they must be nodes 1, 2 and 3. The second consideration is that the vertical coordinate system must start with zero at the free surface and increase in a positive manner to indicate the depth of the node below the top boundary. This second restriction is due solely to the manner in which the surface area of the top boundary is calculated.

## SAMPLE SIMULATION

To illustrate the use of the model, the radial cross section shown in figure 8 was simulated. The cross section shown is typical of Long Island, New York, and is composed





**Figure 7.** Comparison of drawdowns at the base of an aquifer as predicted by Stallman's analog model and the Galerkin finite-element model.

of two aquifers separated by a confining unit. The element configuration used to simulate this section is shown in figure 2.

Supplemental data III and V give the input formats and data used for this model run. The well screen is in the bottom 25 feet of the upper aquifer. The coefficients used for the simulation are

$K_z$  (vertical hydraulic conductivity of upper glacial aquifer) = 27.0 ft/d,

$K_r$  (horizontal hydraulic conductivity of upper aquifer) = 270.0 ft/d

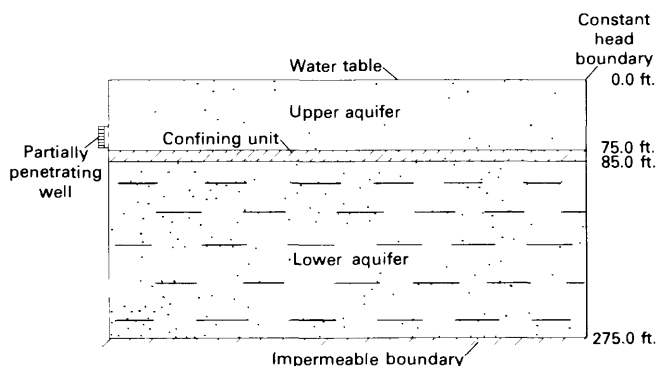
$K_z$  (vertical hydraulic conductivity of confining unit) = 0.001 ft/d,

$K_r$  (horizontal hydraulic conductivity of confining unit)  
 = 0.01 ft/d,  
 $K_z$  (vertical hydraulic conductivity of lower aquifer)  
 = 1.4 ft/d,  
 $K_r$  (horizontal hydraulic conductivity of lower aquifer)  
 = 50.0 ft/d,  
 $Q$  (pumping rate) = 1.0 ft<sup>3</sup>/s,  
 $S_y$  (specific yield) = 0.25,  
 $S_s$  (specific storage) =  $1.0 \times 10^{-6}$  ft<sup>-1</sup>.

The results for one time step are presented in supplemental data VI. The results simply present the total time into the simulation and the drawdown for each node in the finite-element configuration.

## CONCLUSIONS

The Galerkin finite-element radial-flow ground-water model described can simulate anisotropic, inhomogeneous, confined, and pseudo-unconfined aquifer conditions. Com-



**Figure 8.** Radial cross section of aquifer for sample simulation.

parisons between radial-flow model results and published solutions are satisfactory. The program described allows more freedom in representing the field conditions than previously published methods.

The program, names of variables, data input formats, and array dimensioning are described in the supplemental data. From the text and supplemental data, one can use the program and modify the input and output of data as necessary to obtain accurate predictions.

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## SUPPLEMENTAL DATA

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# SUPPLEMENTAL DATA I

## FORTTRAN IV Program Listings

### A. Main Program

```

PROGRAM RAD(INPUT, OUTPUT, TAPE5=INPUT, TAPE6=OUTPUT, TAPE7=OUTPUT)  MAN 10
  DIMENSION NG(288,3), PR(288), PZ(288), RE(170), ZE(170), IFLUX(170)MAN 20
  1), IQ(170), IRCH(170), NSCON(170), F(170), S1(170), S3(166), S5(16MAN 30
  26), F2(166), WG1(166,21), WTRA1(166,21), R(3), Z(3), ZI(3), RI(3),MAN 40
  3 L(3), W(3,3), WR(3,3), WZ(3,3), WW(3,3), WST(3,3), NND(50), TITLEMAN 50
  4(20) MAN 60
C***** MAN 70
C MAN 80
C A GALERKIN FINITE-ELEMENT FLOW MODEL FOR THE TRANSIENT MAN 90
C RESPONSE OF A RADIALY SYMMETRIC AQUIFER MAN 100
C MAN 110
C PR, PZ = FT/DAY ; ZE, RE=FT ; Q=CFS ; TIME =DAYS MAN 120
C QRCH=FT/DAY (+ MEANS RECHARGE ; - MEANS DISCHARGE) MAN 130
C MAN 140
C CONSTANT DRAWDOWN NODES MUST BE NUMBERED FIRST MAN 150
C (I.E. 1,2,3,.....) MAN 160
C PPR=PRIMARY RADIAL HYDRAULIC CONDUCTIVITY MAN 170
C PPZ=PRIMARY VERTICAL HYDRAULIC CONDUCTIVITY MAN 180
C NDIF=# OF DIFFERING HYDRAULIC CONDUCTIVITY ELEMENTS MAN 190
C NE=# OF ELEMENTS; NN=# OF NODES; NCH=# OF CONSTANT DRAWDOWN NODES MAN 200
C NQ=# OF NODES DISCHARGING; NND=NODES DISCH. MAN 210
C IF NODE HAS AN 'UNCONFINED' BOUNDARY PUT A '1' IN COL. 35 MAN 220
C IF NODE HAS A RECHARGE TOP BOUNDARY PUT A '1' IN COL. 40 MAN 230
C IF NODE HAS A CONSTANT ZERO DRAWDOWN PUT A '1' IN COL. 45 MAN 240
C MAN 250
C***** MAN 260
C MAN 270
C CALCULATE CONSTANTS MAN 280
C MAN 290
C TPI=3.1416*2. MAN 300
C CONV1=1440.*60. MAN 310
C MAN 320
C READ (5,11) TITLE MAN 330
11 FORMAT (20A4) MAN 340
WRITE (6,12) TITLE MAN 350
12 FORMAT (1H1,10X,20A4) MAN 360
READ (5,13) PPR,PPZ,IPP MAN 370
13 FORMAT (2F10.0,I10) MAN 380
READ (5,14) S,SY MAN 390
14 FORMAT (2F10.0) MAN 400
WRITE (6,15) S MAN 410
15 FORMAT (1H0,36H COEFFICIENT OF SPECIFIC STORAGE=,E12.5,5H 1/FT)MAN 420
WRITE (6,16) SY MAN 430
16 FORMAT (1H0,17H SPECIFIC YIELD =,F10.4) MAN 440
WRITE (6,17) PPR,PPZ MAN 450
17 FORMAT (1H0,38H THE PRIMARY HYDRAULIC CONDUCTIVITY =,F10.3,12H RMAN 460
1ADIALLY &,F10.3,21H VERTICALLY (FT/DAY)) MAN 470
READ (5,18) NE,NN,NCH,NQ,NDIF MAN 480
18 FORMAT (5I5) MAN 490
WRITE (6,19) NE,NN,NCH,NQ,NDIF MAN 500
19 FORMAT (1H0,I5,13H ELEMENTS ,I5,9H NODES ,I5,22H CONSTANT VAMAN 510
1LUE NODES,I5,18H NODES DISCHARGING,I5,23H ELEMENTS OF DIFF. H.C.) MAN 520
IF (NQ.EQ.0) GO TO 24 MAN 530
WRITE (6,20) MAN 540
20 FORMAT (1H0,17HDISCHARGING NODES) MAN 550
DO 23 I=1,NQ MAN 560
READ (5,21) NND(I) MAN 570

```

# FORTRAN IV Program Listings—Continued.

## A. Main Program—Continued.

21	FORMAT (I5)	MAN 580
	WRITE (6,22) NND(I)	MAN 590
22	FORMAT (1X, I9)	MAN 600
23	CONTINUE	MAN 610
24	DO 25 I=1, NE	MAN 620
	PR(I)=PPR	MAN 630
	PZ(I)=PPZ	MAN 640
25	CONTINUE	MAN 650
C		MAN 660
C	DEFINE ELEMENTS OF DIFFERENT CONDUCTIVITIES	MAN 670
C		MAN 680
	IF (NDIF.EQ.0) GO TO 31	MAN 690
	WRITE (6,26)	MAN 700
26	FORMAT (1H0, 1X, 39H ELEMENTS WITH DIFFERENT CONDUCTIVITIES)	MAN 710
	WRITE (6,27)	MAN 720
27	FORMAT (1H0, 1X, 7HELEMENT, 5X, 11HRADIAL H. C., 5X, 13HVERTICAL H. C.)	MAN 730
	DO 30 K=1, NDIF	MAN 740
	READ (5,28) IE, PR(IE), PZ(IE)	MAN 750
28	FORMAT (I10, 2F10.0)	MAN 760
	WRITE (6,29) IE, PR(IE), PZ(IE)	MAN 770
29	FORMAT (1X, I5, 10X, F10.3, 10X, F10.3)	MAN 780
30	CONTINUE	MAN 790
C		MAN 800
C	READ NODAL ORDER OF EACH ELEMENT	MAN 810
C		MAN 820
31	WRITE (6,32)	MAN 830
32	FORMAT (1H0, 7HELEMENT, 10X, 11HNODAL ORDER)	MAN 840
	DO 35 I=1, NE	MAN 850
	READ (5,33) IE, NG(IE,1), NG(IE,2), NG(IE,3)	MAN 860
33	FORMAT (4I5)	MAN 870
	WRITE (6,34) IE, NG(IE,1), NG(IE,2), NG(IE,3)	MAN 880
34	FORMAT (5X, I5, 5X, 3(2X, I5))	MAN 890
35	CONTINUE	MAN 900
C		MAN 910
C	READ NODAL COORDINATES FOR EACH NODE	MAN 920
C		MAN 930
	WRITE (6,36)	MAN 940
36	FORMAT (1H0, 17HNODAL INFORMATION)	MAN 950
	WRITE (6,37)	MAN 960
37	FORMAT (1X, 4HNODE, 10X, 1HR, 10X, 1HZ, 10X, 10HUNCONFINED, 10X, 16HSURFACE	MAN 970
	1 RECHARGE, 10X, 13HCONSTANT HEAD)	MAN 980
	DO 40 J=1, NN	MAN 990
	IQ(J)=0	MAN1000
	READ (5,38) IND, RE(IND), ZE(IND), IFLUX(IND), IRCH(IND), NSCON(IND)	MAN1010
38	FORMAT (I10, 2F10.0, 3I5)	MAN1020
	WRITE (6,39) IND, RE(IND), ZE(IND), IFLUX(IND), IRCH(IND), NSCON(IND)	MAN1030
39	FORMAT (1X, I5, 2F10.2, 3(15X, I5))	MAN1040
40	CONTINUE	MAN1050
C		MAN1060
C	CHECK INPUT DATA FOR CONSISTANCY	MAN1070
C		MAN1080
	CALL CHECK(NE, NN, NCH, NG, RE, ZE, NSCON, IERR)	MAN1090
	IF (IERR.EQ.1) GO TO 91	MAN1100
C		MAN1110
C	DEFINE DISCHARGING NODES	MAN1120
C		MAN1130
	IF (NQ.EQ.0) GO TO 46	MAN1140
	DO 41 K=1, NQ	MAN1150
	IQ(NND(K))=1	MAN1160

# FORTRAN IV Program Listings—Continued.

## A. Main Program—Continued.

41	CONTINUE	MAN1170
C		MAN1180
C	DEFINE TOTAL TRANSMISSIVITY OF SCREEN LENGTH	MAN1190
C		MAN1200
	TRTOT=0.	MAN1210
	DO 45 J=1, NE	MAN1220
	DO 44 I=1, 3	MAN1230
	IF (IQ(NG(J, I)).NE. 1) GO TO 44	MAN1240
	IF (I.EQ. 1) GO TO 42	MAN1250
	IF (IQ(NG(J, I-1)).NE. 1) GO TO 44	MAN1260
	TI=PR(J)*ABS(ZE(NG(J, I))-ZE(NG(J, I-1)))	MAN1270
	GO TO 43	MAN1280
42	IF (IQ(NG(J, 3)).NE. 1) GO TO 44	MAN1290
	TI=PR(J)*ABS(ZE(NG(J, 1))-ZE(NG(J, 3)))	MAN1300
43	TRTOT=TRTOT+TI	MAN1310
44	CONTINUE	MAN1320
45	CONTINUE	MAN1330
C		MAN1340
C	DETERMINE HALF BAND WIDTH AND NEEDED MATRIX WIDTH	MAN1350
C		MAN1360
46	IHBW1=0	MAN1370
	DO 50 I=1, NE	MAN1380
	DO 49 J=1, 3	MAN1390
	IF (J.EQ. 1) GO TO 47	MAN1400
	IF (NSCON(NG(I, J-1)).EQ. 1) GO TO 49	MAN1410
	IF (NSCON(NG(I, J)).EQ. 1) GO TO 49	MAN1420
	IHBW=IABS(NG(I, J)-NG(I, J-1))	MAN1430
	GO TO 48	MAN1440
47	IF (NSCON(NG(I, 1)).EQ. 1) GO TO 49	MAN1450
	IF (NSCON(NG(I, 3)).EQ. 1) GO TO 49	MAN1460
	IHBW=IABS(NG(I, 1)-NG(I, 3))	MAN1470
48	IF (IHBW.LT. IHBW1) GO TO 49	MAN1480
	IHBW1=IHBW	MAN1490
49	CONTINUE	MAN1500
50	CONTINUE	MAN1510
	M=2*IHBW1+1	MAN1520
	WRITE (6, 51) IHBW1, M	MAN1530
51	FORMAT (1H0, 22HTHE HALF BAND WIDTH IS, I5, 32H AND THE WIDTH OF THE	MAN1540
	1 MATRIX IS, I5)	MAN1550
	DO 52 II=1, NN	MAN1560
	S1(II)=0. 0	MAN1570
52	CONTINUE	MAN1580
	MM=NN-NCH	MAN1590
C		MAN1600
C	SET UP PUMPING PERIOD	MAN1610
C		MAN1620
	TITO=0. 0	MAN1630
	DO 90 NPP=1, IPP	MAN1640
	DO 53 II=1, NN	MAN1650
	F(II)=0. 0	MAN1660

# **FORTRAN IV Program Listings—Continued.**

## **A. Main Program—Continued.**

53	CONTINUE	MAN1670
	READ (5, 54) Q, QRCH, DELT, TMAXF, TSM, NTS	MAN1680
54	FORMAT (5F10. 0, I10)	MAN1690
	DT=DELT	MAN1700
	TM=0. 0	MAN1710
	DO 55 I=1, NTS	MAN1720
	TM=TM+DT	MAN1730
	IF (TM. GE. TMAXF) GO TO 56	MAN1740
	DT=TSM*DT	MAN1750
55	CONTINUE	MAN1760
	GO TO 57	MAN1770
56	DELT=TMAXF/TM*DELT	MAN1780
	NTS=I	MAN1790
57	WRITE (6, 58) NPP, DELT, NTS	MAN1800
58	FORMAT (1H1, 14HPUMPING PERIOD, I10, /, 20H INITIAL TIME STEP =, F10. 5,	MAN1810
	15H DAYS, /, 46H NUMBER OF TIME STEPS IN THIS PUMPING PERIOD =, I10)	MAN1820
	WRITE (6, 59) Q	MAN1830
59	FORMAT (1H0, 11HDISCHARGE= , E12. 5, 6H CFS)	MAN1840
	WRITE (6, 60) QRCH	MAN1850
60	FORMAT (1H0, 10HRECHARGE= , F7. 2, 8H FT/DAY)	MAN1860
	Q=Q*CONV1	MAN1870
C		MAN1880
C	VALUE OF SINK MATRIX	MAN1890
C		MAN1900
	QTOT=0. 0	MAN1910
	DO 65 K=1, NE	MAN1920
	DO 61 J=1, 3	MAN1930
	L(J)=NG(K, J)	MAN1940
	R(J)=RE(L(J))	MAN1950
	Z(J)=ZE(L(J))	MAN1960
61	CONTINUE	MAN1970
	DO 64 I=1, 3	MAN1980
	DO 63 J=1, 3	MAN1990
	IF (IQ(L(J)). NE. 1) GO TO 62	MAN2000
	IF (IQ(L(I)). NE. 1) GO TO 62	MAN2010
	BLEN=SQRT((ABS(R(I)-R(J)))**2+(ABS(Z(I)-Z(J)))**2)	MAN2020
	F(L(I))=Q*PR(K)*BLEN/(2. *TRTOT)+F(L(I))	MAN2030
	QTOT=Q*PR(K)*BLEN/(2. *TRTOT)+QTOT	MAN2040
62	IF (IRCH(L(J)). NE. 1) GO TO 63	MAN2050
	IF (IRCH(L(I)). NE. 1) GO TO 63	MAN2060
	AREA=3. 1416*ABS(R(J)**2-R(I)**2)	MAN2070
	F(L(I))=QRCH*AREA/2. +F(L(I))	MAN2080
63	CONTINUE	MAN2090
64	CONTINUE	MAN2100
65	CONTINUE	MAN2110
	DO 66 I=1, MM	MAN2120
	F2(I)=F(I+NCH)	MAN2130
66	CONTINUE	MAN2140
C		MAN2150
C	CHECK DISCHARGE	MAN2160

# FORTRAN IV Program Listings—Continued.

## A. Main Program—Continued.

C	QTOT=QTOT/CONV1	MAN2170
	WRITE (6,67) QTOT	MAN2180
	67 FORMAT (1H0,28H CALCULATED WELL DISCHARGE =,F10.2,4H CFS)	MAN2190
C		MAN2200
C	START TIME LOOP	MAN2210
C		MAN2220
	TIME=0.0	MAN2230
	DO 89 IT=1,NTS	MAN2240
C		MAN2250
C	INITIALIZE VARIABLES	MAN2260
C		MAN2270
	DO 69 I=1,MM	MAN2280
	DO 68 J=1,M	MAN2290
	WTRA1(I,J)=0.0	MAN2300
	WG1(I,J)=0.0	MAN2310
	68 CONTINUE	MAN2320
	69 CONTINUE	MAN2330
	DO 79 K=1,NE	MAN2340
	DO 70 J=1,3	MAN2350
	L(J)=NG(K,J)	MAN2360
	R(J)=RE(L(J))	MAN2370
	Z(J)=ZE(L(J))	MAN2380
	70 CONTINUE	MAN2390
C		MAN2400
C	AVERAGE DISTANCE R OF ELEMENT	MAN2410
C		MAN2420
	RBAR=(R(1)+R(2)+R(3))/3.	MAN2430
C		MAN2440
	DO 72 I=1,3	MAN2450
	DO 71 J=1,3	MAN2460
	W(I,J)=0.0	MAN2470
	71 CONTINUE	MAN2480
	72 CONTINUE	MAN2490
	ZI(1)=Z(2)-Z(3)	MAN2500
	ZI(2)=Z(3)-Z(1)	MAN2510
	ZI(3)=Z(1)-Z(2)	MAN2520
	RI(1)=R(3)-R(2)	MAN2530
	RI(2)=R(1)-R(3)	MAN2540
	RI(3)=R(2)-R(1)	MAN2550
	DEL=(ZI(1)*R(1)+ZI(2)*R(2)+ZI(3)*R(3))/2.	MAN2560
C		MAN2570
C	CALCULATE ELEMENT MATRICIES	MAN2580
C		MAN2590
	DO 78 I=1,3	MAN2600
	DO 77 J=1,3	MAN2610
	WR(I,J)=TPI*RBAR*PR(K)*ZI(I)*ZI(J)/(4.*DEL)	MAN2620
	WZ(I,J)=TPI*RBAR*PZ(K)*RI(I)*RI(J)/(4.*DEL)	MAN2630
	IF (I.EQ.J) GO TO 73	MAN2640
	WST(I,J)=S*TPI*R(I)*DEL/(DELT*12.)	MAN2650
	GO TO 74	MAN2660
	73 WST(I,J)=S*TPI*R(I)*DEL/(DELT*6.)	MAN2670
	74 WW(I,J)=WR(I,J)+WZ(I,J)	MAN2680
	IF (I.NE.J) GO TO 76	MAN2690
	IF (IFLUX(L(I)).NE.1) GO TO 76	MAN2700
	AREA=0.0	MAN2710
	DO 75 KK=1,3	MAN2720
	IF (KK.EQ.I) GO TO 75	MAN2730
	IF (IFLUX(L(KK)).NE.1) GO TO 75	MAN2740
	AREA=3.1416*ABS(R(I)*R(I)-R(KK)*R(KK))	MAN2750
		MAN2760



# **FORTRAN IV Program Listings—Continued.**

## **A. Main Program—Continued.**

75	CONTINUE	MAN2770
	W(I, J)=-SY*AREA/(2. *DELT)	MAN2780
C		MAN2790
C	ASSEMBLE ELEMENT MATRICIES INTO GLOBAL	MAN2800
C		MAN2810
76	IF (NSCON(L(J)). EQ. 1) GO TO 77	MAN2820
	IF (NSCON(L(I)). EQ. 1) GO TO 77	MAN2830
	II=NG(K, I)-NCH	MAN2840
	JJ=NG(K, J)-NCH	MAN2850
	MTRAN=JJ-II+(M+1)/2	MAN2860
C		MAN2870
C	GLOBAL TRANSIENT MATRIX	MAN2880
C		MAN2890
	WTRA1(II, MTRAN)=WST(I, J)+W(I, J)+WTRA1(II, MTRAN)	MAN2900
C		MAN2910
C	GLOBAL MATRIX	MAN2920
C		MAN2930
	WG1(II, MTRAN)=WW(I, J)+WG1(II, MTRAN)	MAN2940
77	CONTINUE	MAN2950
78	CONTINUE	MAN2960
79	CONTINUE	MAN2970
C		MAN2980
C		MAN2990
C	SUM OF GLOBAL AND TRANSIENT COEFFICIENT MATRICIES	MAN3000
C	ON LEFT HAND SIDE	MAN3010
C		MAN3020
	DO 81 I=1, MM	MAN3030
	DO 80 J=1, M	MAN3040
	WG1(I, J)=WTRA1(I, J)+WG1(I, J)	MAN3050
80	CONTINUE	MAN3060
81	CONTINUE	MAN3070
C		MAN3080
C	MULT. OF MATRICIES ON RIGHT HAND SIDE	MAN3090
C		MAN3100
	DO 82 I=1, MM	MAN3110
	S3(I)=S1(I+NCH)	MAN3120
	S5(I)=0. 0	MAN3130
82	CONTINUE	MAN3140
	CALL MLTBM(WTRA1, S3, S5, MM, M)	MAN3150
	DO 83 I=1, MM	MAN3160
	S5(I)=S5(I)+F2(I)	MAN3170
83	CONTINUE	MAN3180
C		MAN3190
C	SOLUTION	MAN3200
C		MAN3210
	CALL SOLVE(1, WG1, S5, MM, IHBW1, MM, M)	MAN3220
	CALL SOLVE(2, WG1, S5, MM, IHBW1, MM, M)	MAN3230
	NNN=NCH+1	MAN3240
	DO 84 I=NNN, NN	MAN3250
	S1(I)=S5(I-NCH)	MAN3260
84	CONTINUE	MAN3270
	TIME=TIME+DELT	MAN3280
	TIMM=TIME*1440.	MAN3290
	ACTT=TIME+T110	MAN3300
C		MAN3310
C	OUTPUT	MAN3320
C		MAN3330
	WRITE (6, 85) ACTT	MAN3340
85	FORMAT (1H1, 30HTOTAL TIME IN THE SIMULATION =, F10. 3, 5H DAYS)	MAN3350
	WRITE (6, 86) TIME, TIMM	MAN3360

## FORTRAN IV Program Listings—Continued.

### A. Main Program—Continued.

86	FORMAT (1H0, 18H THE DRAWDOWN AFTER, E10. 3, 9H DAYS OR, E10. 3, 26H MIN.	MAN3370
	1 IN THE PUMPING PERIOD)	MAN3380
	WRITE (6, 87)	MAN3390
87	FORMAT (1H0, 10X, 12H NODE NUMBER, 5X, 8H DRAWDOWN, 10X, 12H NODE NUMBER,	MAN3400
	15X, 8H DRAWDOWN, 10X, 12H NODE NUMBER, 5X, 8H DRAWDOWN)	MAN3410
	WRITE (6, 88) ((I, S1(I)), I=1, NN)	MAN3420
88	FORMAT (10X, I5, 10X, F10. 3, 10X, I5, 10X, F10. 3, 10X, I5, 10X, F10. 3)	MAN3430
	DELT=DELT*TSM	MAN3440
89	CONTINUE	MAN3450
	TITO=TIME+TITO	MAN3460
90	CONTINUE	MAN3470
	GO TO 93	MAN3480
91	WRITE (6, 92)	MAN3490
92	FORMAT (1H1, 49H **TERMINATION OF PROGRAM DUE TO INPUT DATA ERRORS)	MAN3500
93	STOP	MAN3510
	END	MAN3520-

### B. Subroutine CHECK

	SUBROUTINE CHECK(NE, NN, NCH, NG, RE, ZE, NSCON, IERR)	CHK 010
	DIMENSION NG(NE, 3), RE(NN), ZE(NN), NSCON(NN)	CHK 020
C		CHK 030
C	THIS SUBROUTINE CHECKS THE ELEMENT INPUT DATA FOR CONSISTANCY	CHK 040
C		CHK 050
	IERR=0	CHK 060
	NCK=0	CHK 070
C		CHK 080
C	CHECK NUMBER AND ORDER OF CONSTANT HEAD NODES	CHK 090
C		CHK 100
C	FIRST CHECK IF CONSTANT HEAD NODES ARE THE FIRST NODES NUMBERED	CHK 110
C		CHK 120
	DO 10 I=1, NCH	CHK 130
	NCK=NSCON(I)+NCK	CHK 140
10	CONTINUE	CHK 150
	IF(NCK.EQ.NCH) GO TO 30	CHK 160
	IERR=1	CHK 170
	WRITE(6, 20) NCH, NCK	CHK 180
20	FORMAT(1H1, 20H *** PROGRAM EXPECTED, I5, 39H CONSTANT HEAD NODES BUT	CHK 190
	1 ONLY THE FIRST, I5, 28H NODES WERE FLAGGED AS SUCH)	CHK 200
C		CHK 210
C	THEN CHECK THE TOTAL NUMBER OF CONSTANT HEAD FLAGS	CHK 220
C		CHK 230
30	NCK=0	CHK 240
	DO 40 I=1, NN	CHK 250
	NCK=NSCON(I)+NCK	CHK 260
40	CONTINUE	CHK 270
	IF(NCK.EQ.NCH) GO TO 60	CHK 280
	IERR=1	CHK 290
	WRITE(6, 50)	CHK 300
50	FORMAT(1H0, 71H *** TOTAL NUMBER OF CONSTANT HEAD FLAGS DOES NOT AGREE	CHK 310
	1E WITH NCH(# CODED))	CHK 320
C		CHK 330
C	NEXT CHECK IS TO INSURE THAT ALL ELEMENTS ARE NUMBERED	CHK 340
C	COUNTERCLOCKWISE	CHK 350
C		CHK 360
60	DO 100 K=1, NE	CHK 370
	L=NG(K, 1)	CHK 380
	M=NG(K, 2)	CHK 390
	N=NG(K, 3)	CHK 400

## FORTRAN IV Program Listings—Continued.

### B. Subroutine CHECK—Continued.

A=(RE(L)-RE(N))*ZE(M)+(RE(M)-RE(L))*ZE(N)+(RE(N)-RE(M))*ZE(L)	CHK 410
IF(A.LT.0.) GO TO 100	CHK 420
IERR=1	CHK 430
WRITE(6,70) K	CHK 440
70 FORMAT(1H0,17H***EITHER ELEMENT,15,23H IS NUMBERED CLOCKWISE,/,11CHK	450
1X,92HOR THE VERTICAL COORDINATES ARE NOT POSITIVE DOWNWARD STARTINCHK	460
2G WITH ZERO AT THE TOP BOUNDARY)	CHK 470
100 CONTINUE	CHK 480
RETURN	CHK 490
END	CHK 500-

### C. Subroutine MLTBM

SUBROUTINE MLTBM(A,B,R,MM,M)	MLT 010
DIMENSION A(MM,M),B(MM),R(MM)	MLT 020
C	MLT 030
C MULT. OF A BANDED MATRIX(ORIGINALLY MM*MM)	MLT 040
C WITH A VECTOR MATRIX	MLT 050
C ( COMPACTED BANDED MATRIX OF M*MM AND VECTOR OF MM*1 )	MLT 060
C	MLT 070
DO 1 I=1,MM	MLT 080
R(I)=0.	MLT 090
DO 2 J=1,M	MLT 100
K=J+I-(M+1)/2	MLT 110
IF(K.LT.1) GO TO 2	MLT 120
IF(K.GT.MM) GO TO 2	MLT 130
R(I)=A(I,J)*B(K)+R(I)	MLT 140
2 CONTINUE	MLT 150
1 CONTINUE	MLT 160
RETURN	MLT 170
END	MLT 180-

### D. Subroutine SOLVE

SUBROUTINE SOLVE(KKK,B,R,NEQ,IHALFB,NDIM,MDIM)	SOL 10
C *****	SOL 20
C	SOL 30
C ASYMMETRIC BAND MATRIX EQUATION SOLVER	SOL 40
C ORIGINALLY PROGRAMED BY JAMES O. DUGUID	SOL 50
C	SOL 60
C KKK=1 TRIANGULARIZES THE BAND MATRIX B	SOL 70
C KKK=2 SOLVES FOR RIGHT SIDE R, SOLUTION RETURNS IN R	SOL 80
C	SOL 90
DIMENSION B(NDIM,MDIM), R(NDIM)	SOL 100
NRS=NEQ-1	SOL 110
IHBP=IHALFB+1	SOL 120
IF (KKK.EQ.2) GO TO 30	SOL 130
C	SOL 140
C TRIANGULARIZE MATRIX A USING DOOLITTLE METHOD	SOL 150
C	SOL 160
DO 20 K=1,NRS	SOL 170
PIVOT=B(K, IHBP)	SOL 180
KK=K+1	SOL 190
KC=IHBP	SOL 200
DO 10 I=KK,NEQ	SOL 210
KC=KC-1	SOL 220
IF (KC.LE.0) GO TO 20	SOL 230
C=-B(I,KC)/PIVOT	SOL 240
B(I,KC)=C	SOL 250
KI=KC+1	SOL 260
LIM=KC+IHALFB	SOL 270
DO 10 J=KI,LIM	SOL 280
JC=IHBP+J-KC	SOL 290

## FORTRAN IV Program Listings—Continued.

### D. Subroutine SOLVE—Continued.

10	B(I, J)=B(I, J)+C*B(K, JC)	SOL 300
20	CONTINUE	SOL 310
	GO TO 100	SOL 320
C		SOL 330
C	MODIFY LOAD VECTOR R	SOL 340
C		SOL 350
30	NN=NEQ+1	SOL 360
	IBAND=2*IHALFB+1	SOL 370
	DO 70 I=2, NEQ	SOL 380
	JC=IHBP-I+1	SOL 390
	JI=1	SOL 400
	IF (JC. LE. 0) GO TO 40	SOL 410
	GO TO 50	SOL 420
40	JC=1	SOL 430
	JI=I-IHBP+1	SOL 440
50	SUM=0. 0	SOL 450
	DO 60 J=JC, IHALFB	SOL 460
	SUM=SUM+B(I, J)*R(JI)	SOL 470
60	JI=JI+1	SOL 480
70	R(I)=R(I)+SUM	SOL 490
C		SOL 500
C	BACK SOLUTION	SOL 510
C		SOL 520
	R(NEQ)=R(NEQ)/B(NEQ, IHBP)	SOL 530
	DO 90 IBACK=2, NEQ	SOL 540
	I=NN-IBACK	SOL 550
	JP=I	SOL 560
	KR=IHBP+1	SOL 570
	MR=MIN0(IBAND, IHALFB+IBACK)	SOL 580
	SUM=0. 0	SOL 590
	DO 80 J=KR, MR	SOL 600
	JP=JP+1	SOL 610
80	SUM=SUM+B(I, J)*R(JP)	SOL 620
90	R(I)=(R(I)-SUM)/B(I, IHBP)	SOL 630
100	RETURN	SOL 640
	END	SOL 650-

## SUPPLEMENTAL DATA II

### Definition of Selected Program Variables

A - area of element used to check nodal order	IHBW1 - half band width
ACTT - total time in simulation	IND - node number
AREA - surface area of an element along the top boundary	IPP - number of pumping periods
BLEN - length of screen between two nodes	IQ - discharge boundary flag
CONV1 - conversion factor	IRCH - recharge boundary flag
DEL - area of triangular element	L - global node number in an element
DELT - time step (days)	M - band width of global matrices
DT - dummy time step used in setting up pumping period	MM - number of unknowns
F - vector containing the flux terms in all nodes on the right hand side of equation	MTRAN - transformed column location for the compacted banded matrix
F2 - vector containing the flux terms for all unknown nodes on the right hand side of equation	NCH - number of constant head nodes
E1 - element number	NDIF - number of elements with a different hydraulic conductivity than the primary hydraulic conductivity
IERR - error flag for input data	NE - number of elements
IFLUX - unconfined boundary flag	NG - array storing nodal order for each element
	NN - number of nodes

NND - nodes along the well screen	S5 - vector representing right hand side of final matrix equation
NPP - pumping period number	T1 - increment of transmissivity along well screen for an element
NQ - number of nodes along the well screen	TIME - total time of pumping period in days
NSCON - flag for constant head nodes	TIMM - total time of pumping period in minutes
NTS - number of time steps in pumping period	TITLE - title to be printed at start of computer output
PPR - the primary hydraulic conductivity in the radial direction	TMAXF - maximum time of simulation (days)
PPZ - the primary hydraulic conductivity in the vertical direction	TPI - constant
PR - the hydraulic conductivity in the radial direction	TRTOT - total transmissivity along the well screen
PZ - the hydraulic conductivity in the vertical direction	TSM - time-step multiplier
Q - discharge of the well (cfs)	W - element matrix for unconfined storage coefficients
QRCH - recharge rate of the basin (ft/d)	WGI - global matrix for non-time dependent coefficients and also total matrix on the left hand side of final matrix equation
QTOT - calculated discharge of well after distribution along the well bore	WR - element matrix for radial direction coefficients
R - radius of the nodes for an element	WST - element matrix for storage coefficients
RBAR - average radius for an element	WTRAI - global matrix for time dependent coefficients
RE - radial distance of node	WW - sum of WR and WZ
RI - coefficient for calculation of element arrays	WZ - element matrix for vertical direction coefficients
S - Specific storage $S_s$ (ft <sup>-1</sup> )	Z - vertical location of the nodes for an element
SY - specific yield $S_y$ (unitless)	ZE - vertical location of a node
S1 - drawdowns	ZI - coefficient for calculation of element arrays
S3 - drawdowns at unknown nodes at previous time step	

### SUPPLEMENTAL DATA III

#### Data Input Formats

##### Group 1: Title and problem setup

Card	Columns	Format	Variable	Definition
1	1-80	20A4	TITLE	Any title the user wishes to print on one line at start of output.
2	1-10	F10.0	PPR	Primary radial hydraulic conductivity (ft/d)
	11-20	F10.0	PPZ	Primary vertical hydraulic conductivity (ft/d)
	21-30	I10	IPP	number of pumping periods

Note: PPR and PPZ are the hydraulic conductivities assigned to all elements unless redefined in data Group 3.

3	1-10	F10.0	S	Coefficient of compressive storage ( $S_s$ ) in ft <sup>-1</sup>
	11-20	F10.0	SY	Specific yield (unitless)
4	1-5	I5	NE	Number of elements in finite element mesh
	6-10	I5	NN	Number of nodes in finite element mesh
	11-15	I5	NCH	Number of constant-head nodes in finite element mesh
	16-20	I5	NQ	Number of nodes associated with well screen
	21-25	I5	NDIF	Number of elements having different hydraulic conductivities than the primary ones on card 2

## Data Input Formats—Continued.

### Group 2: Nodes along the well screen

NQ number of cards

<u>Card</u>	<u>Columns</u>	<u>Format</u>	<u>Variable</u>	<u>Definition</u>
--	1-5	I5	NND(I)	Node that is along well screen. All nodes along screen must be identified because water is withdrawn only from elements with two nodes along screen.

### Group 3: Elements of different hydraulic conductivities

DIF number of cards

<u>Card</u>	<u>Columns</u>	<u>Format</u>	<u>Variable</u>	<u>Definition</u>
--	1-10	I10	IE	Element number
	11-20	F10.0	PR(IE)	Radial hydraulic conductivity of element (ft/d)
	21-30	F10.0	PZ(IE)	Vertical hydraulic conductivity of element (ft/d)

### Group 4: Nodal configuration of each element

NE number of cards

<u>Card</u>	<u>Columns</u>	<u>Format</u>	<u>Variable</u>	<u>Definition</u>
--	1-5	I5	IE	Element number
	6-10	I5	NG(IE,1)	First node of triangular element.
	11-15	I5	NG(IE,2)	Second node in counter-clockwise direction of triangular element.
	16-20	I5	NG(IE,3)	Third node in counter-clockwise direction of triangular element.

See note on node numbers in Data Group 5.

### Group 5: Location of each node and flags for constant head, unconfined boundary and recharge boundary.

NN number of cards

<u>Card</u>	<u>Columns</u>	<u>Format</u>	<u>Variable</u>	<u>Definition</u>
--	1-10	I10	IND	Node number
	11-20	F10.0	RE(IND)	Radial location of node (ft)
	21-30	F10.0	ZE(IND)	Vertical location of node (ft)
	35	I5	IFLUX(IND)	If set to 1, node is treated as unconfined and specific-yield boundary condition is applied.
	40	I5	IRCH(IND)	If set to 1, node is part of a recharge boundary. This is for ground-water mounding problems and is only the top boundary. Recharge occurs only to elements with two nodes on top boundary.
	45	I5	NSCON(IND)	If set to 1, node is considered to be a constant zero drawdown (constant head).

Important Note: For this program, constant-drawdown nodes (constant head) must be the first nodes numbered and must be sequential. The program assumes that if there are four constant-head nodes (NCH = 4), they are nodes 1, 2, 3, and 4.

## Data Input Formats—Continued.

### Group 6: Pumping period information

IPP number of cards

Card	Columns	Format	Variable	Definition
--	1-10	F10.0	Q	Pumping rate (ft <sup>3</sup> /s)
	11-20	F10.0	QRCH	Recharge rate (ft/d)
	21-30	F10.0	DELT	Initial time step (days)
	31-40	F10.0	TMAXF	Maximum length of pumping period (days)
	41-50	F10.0	TSM	Time-step multiplier (each time step after DELT is multiplied by TSM).
	51-60	I10	NTS	Number of time steps in pumping period.

The program has two options for the pumping period:

1. To simulate a given number of time steps, set TMAXF to a value larger than the expected simulation period. The program will use NTS, TSM, and DELT as coded.
2. To simulate a given pumping period, set NTS larger than the number required for the simulation period (for example, 100). The program will compute the exact DELT (which will be  $\leq$  DELT coded) and NTS to arrive exactly at TMAXF on the last time step.

## SUPPLEMENTAL DATA IV

### Array Dimensioning

Several of the arrays must be dimensioned for each specific grid setup.

The arrays can be divided into six different groups:

- (1) Arrays associated with the number of elements (NE)  
 ARRAY NAME (Size)  
 a) NG (NE, 3)  
 b) PR (NE)  
 c) PZ (NE)
- (2) Arrays associated with the total number of nodes (NN).  
 ARRAY NAME (Size)  
 a) RE (NN)  
 b) ZE (NN)  
 c) IFLUX (NN)  
 d) IQ (NN)  
 e) IRCH (NN)  
 f) NSCON (NN)  
 g) F (NN)  
 h) S1 (NN)
- (3) Arrays associated with the number of unknown nodes (MM). The number of unknown nodes (MM) equals the number of nodes (NN) minus the number of constant head nodes (NCH). Therefore,  $MM = NN - NCH$ .  
 ARRAY NAME (Size)  
 a) S3 (MM)  
 b) S5 (MM)  
 c) F2 (MM)
- (4) The compacted global arrays associated with the number of unknown nodes (MM) and the band width of the global coefficient matrix (M). The band width (M) is equal to twice the largest difference between two adjacent node numbers plus one.  
 ARRAY NAME (Size)  
 a) WG1 (MM,M)  
 b) WTR1 (MM,M)

## Array Dimensioning—Continued.

- (5) Array associated with the number of nodes along the well screen (NQ).  
 This array is dimensioned at 50 in the program, which should be sufficient for most problems.  
 ARRAY NAME (Size)  
 a) NND (NQ)

- (6) Arrays that are a constant size regardless of problem.  
 ARRAY NAME (Size)  
 a) R (3)  
 b) Z (3)  
 c) ZI (3)  
 d) RI (3)  
 e) L (3)  
 f) W (3,3)  
 g) WR (3,3)  
 h) WZ (3,3)  
 i) WW (3,3)  
 j) WST (3,3)  
 k) TITLE (20)

## SUPPLEMENTAL DATA V

### Input Data for Test Problem

SAMPLE MODEL SIMULATION OF A LONG ISLAND, NEW YORK PUMP TEST						
270	27		3	232	50	1 4
000001	25			233	50	1 4
288 170	4	4	109	234	50	1 4
167				235	50	1 4
166				236	50	1 4
165				237	50	1 4
164				238	50	1 4
180	01		001	239	50	1 4
181	01		001	240	50	1 4
182	01		001	241	50	1 4
183	01		001	242	50	1 4
184	01		001	243	50	1 4
185	01		001	244	50	1 4
186	01		001	245	50	1 4
187	01		001	246	50	1 4
188	01		001	247	50	1 4
189	01		001	248	50	1 4
190	01		001	249	50	1 4
191	01		001	250	50	1 4
192	01		001	251	50	1 4
193	01		001	252	50	1 4
194	01		001	253	50	1 4
195	01		001	254	50	1 4
196	01		001	255	50	1 4
197	01		001	256	50	1 4
198	01		001	257	50	1 4
199	01		001	258	50	1 4
200	01		001	259	50	1 4
201	01		001	260	50	1 4
202	01		001	261	50	1 4
203	01		001	262	50	1 4
204	01		001	263	50	1 4
205	01		001	264	50	1 4
206	01		001	265	50	1 4
207	01		001	266	50	1 4
208	01		001	267	50	1 4
209	01		001	268	50	1 4
210	01		001	269	50	1 4
211	01		001	270	50	1 4
212	01		001	271	50	1 4
213	01		001	272	50	1 4
214	01		001	273	50	1 4
215	01		001	274	50	1 4
216	01		001	275	50	1 4
217	01		001	276	50	1 4
218	50	1 4		277	50	1 4
219	50	1 4		278	50	1 4
220	50	1 4		279	50	1 4
221	50	1 4		280	50	1 4
222	50	1 4		281	50	1 4
223	50	1 4		282	50	1 4
224	50	1 4		283	50	1 4
225	50	1 4		284	50	1 4
226	50	1 4		285	50	1 4
227	50	1 4		286	50	1 4
228	50	1 4		287	50	1 4
229	50	1 4		288	50	1 4
230	50	1 4		1	170 169 160	
231	50	1 4		2	160 169 159	
				3	159 169 168	



# Input Data for Test Problem—Continued.

4	168	158	159	96	90	89	81
5	158	168	167	97	81	89	80
6	167	157	158	98	80	89	88
7	167	166	157	99	88	79	80
8	157	166	156	100	88	87	79
9	156	166	165	101	79	87	78
10	165	155	156	102	83	82	74
11	155	165	164	103	74	82	73
12	164	154	155	104	73	82	81
13	160	159	150	105	81	72	73
14	150	159	149	106	81	80	72
15	149	159	158	107	72	80	71
16	158	148	149	108	71	80	79
17	158	157	148	109	79	70	71
18	148	157	147	110	79	78	70
19	147	157	156	111	70	78	69
20	156	146	147	112	74	73	65
21	156	155	146	113	65	73	64
22	146	155	145	114	64	73	72
23	145	155	154	115	72	63	64
24	154	144	145	116	72	71	63
25	140	150	149	117	63	71	62
26	149	139	140	118	62	71	70
27	139	149	148	119	70	61	62
28	148	138	139	120	70	69	61
29	138	148	147	121	61	69	60
30	147	137	138	122	65	64	56
31	137	147	146	123	56	64	55
32	146	136	137	124	55	64	63
33	136	146	145	125	63	54	55
34	145	135	136	126	63	62	54
35	145	144	135	127	54	62	53
36	135	144	134	128	53	62	61
37	140	139	130	129	61	52	53
38	130	139	129	130	61	60	52
39	139	138	129	131	52	60	51
40	129	138	128	132	56	55	47
41	128	138	137	133	47	55	46
42	137	127	128	134	46	55	54
43	137	136	127	135	54	45	46
44	127	136	126	136	54	53	45
45	126	136	135	137	45	53	44
46	135	125	126	138	44	53	52
47	125	135	134	139	52	43	44
48	134	124	125	140	52	51	43
49	130	129	120	141	43	51	42
50	120	129	119	142	47	46	38
51	119	129	128	143	38	46	37
52	128	118	119	144	37	46	45
53	128	127	118	145	45	36	37
54	118	127	117	146	45	44	36
55	117	127	126	147	36	44	35
56	126	116	117	148	35	44	43
57	126	125	116	149	43	34	35
58	116	125	115	150	43	42	34
59	115	125	124	151	34	42	33
60	124	114	115	152	38	37	29
61	120	119	110	153	29	37	28
62	110	119	109	154	28	37	36
63	109	119	118	155	36	27	28
64	118	108	109	156	36	35	27
65	118	117	108	157	27	35	34
66	108	117	107	158	34	26	27
67	107	117	116	159	34	33	26
68	116	106	107	160	29	28	22
69	116	115	106	161	22	28	21
70	106	115	105	162	21	28	27
71	105	115	114	163	27	20	21
72	110	109	101	164	27	26	20
73	101	109	100	165	20	26	19
74	100	109	108	166	22	21	15
75	108	99	100	167	15	21	14
76	108	107	99	168	14	21	20
77	99	107	98	169	20	13	14
78	98	107	106	170	20	19	13
79	106	97	98	171	13	19	12
80	106	105	97	172	15	14	9
81	97	105	96	173	9	14	8
82	101	100	92	174	8	14	13
83	92	100	91	175	13	7	8
84	91	100	99	176	13	12	7
85	99	90	91	177	9	8	4
86	99	98	90	178	4	8	3
87	90	98	89	179	3	8	7
88	89	98	97	180	164	163	154
89	97	88	89	181	154	163	153
90	97	96	88	182	154	153	144
91	88	96	87	183	144	153	143
92	92	91	83	184	144	143	134
93	83	91	82	185	134	143	133
94	82	91	90	186	134	133	124
95	90	81	82	187	124	133	123
				188	124	123	114

# Input Data for Test Problem—Continued.

189	114	123	113
190	114	113	105
191	105	113	104
192	105	104	96
193	96	104	95
194	96	95	87
195	87	95	86
196	87	86	78
197	78	86	77
198	78	77	69
199	69	77	68
200	69	68	60
201	60	68	59
202	60	59	51
203	51	59	50
204	51	50	42
205	42	50	41
206	42	41	33
207	33	41	32
208	33	32	26
209	26	32	25
210	26	25	19
211	19	25	18
212	19	18	12
213	12	18	11
214	12	11	7
215	7	11	6
216	7	6	3
217	3	6	2
218	163	162	153
219	153	162	152
220	152	162	161
221	161	151	152
222	153	152	143
223	143	152	142
224	142	152	151
225	151	141	142
226	143	142	133
227	133	142	132
228	132	142	141
229	141	131	132
230	133	132	123
231	123	132	122
232	122	132	131
233	131	121	122
234	123	122	113
235	113	122	112
236	112	122	121
237	121	111	112
238	113	112	104
239	104	112	103
240	103	112	111
241	111	102	103
242	104	103	95
243	95	103	94
244	94	103	102
245	102	93	94
246	95	94	86
247	86	94	95
248	85	94	73
249	93	84	85
250	86	85	77
251	77	85	76
252	76	85	84
253	84	75	76
254	77	76	68
255	68	76	67
256	67	76	75
257	75	66	67
258	68	67	59
259	59	67	58
260	58	67	66
261	66	57	59
262	59	58	50
263	50	58	49
264	49	58	57
265	57	48	49
266	50	49	41
267	41	49	40
268	40	49	48
269	48	39	40
270	41	40	32
271	32	40	31
272	31	40	39
273	39	30	31
274	32	31	25
275	25	31	24
276	24	31	30
277	30	23	24
278	25	24	18
279	18	24	17
280	17	24	23

281	23	16	17			
282	18	17	11			
283	11	17	10			
284	10	17	16			
285	11	10	6			
286	6	10	5			
287	6	5	2			
288	2	5	1			
	1	10000		275		1
	2	10000		85		1
	3	10000		75		1
	4	10000		0	1	1
	5	4000		275		
	6	4000		85		
	7	4000		75		
	8	4000		37 5		
	9	4000		0		
	10	2500		275		
	11	2500		85		
	12	2500		75		
	13	2500		50		
	14	2500		25		
	15	2500		0	1	
	16	1500		275		
	17	1500		125		
	18	1500		85		
	19	1500		75		
	20	1500		50		
	21	1500		25		
	22	1500		0	1	
	23	900		275		
	24	900		125		
	25	900		85		
	26	900		75		
	27	900		50		
	28	900		25		
	29	900		0	1	
	30	600		275		
	31	600		125		
	32	600		85		
	33	600		75		
	34	600		62 5		
	35	600		50		
	36	600		37 5		
	37	600		25		
	38	600		0	1	
	39	400		275		
	40	400		125		
	41	400		85		
	42	400		75		
	43	400		62 5		
	44	400		50		
	45	400		37 5		
	46	400		25		
	47	400		0	1	
	48	275		275		
	49	275		125		
	50	275		85		
	51	275		75		
	52	275		62 5		
	53	275		50		
	54	275		37 5		
	55	275		25		
	56	275		0	1	
	57	200		275		
	58	200		125		
	59	200		85		
	60	200		75		
	61	200		62 5		
	62	200		50		
	63	200		37 5		
	64	200		25		
	65	200		0	1	
	66	150		275		
	67	150		125		
	68	150		85		
	69	150		75		
	70	150		62 5		
	71	150		50		
	72	150		37 5		
	73	150		25		
	74	150		0	1	
	75	100		275		
	76	100		125		
	77	100		85		
	78	100		75		
	79	100		62 5		
	80	100		50		
	81	100		37 5		
	82	100		25		
	83	100		0	1	
	84	75		275		
	85	75		125		

# Input Data for Test Problem—Continued.

86	75	85		131	6	275			
87	75	75		132	6	125			
88	75	62 5		133	6	85			
89	75	50		134	6	75			
90	75	37 5		135	6	66 67			
91	75	25		136	6	58 33			
92	75	0	1	137	6	50			
93	50	275		138	6	37 5			
94	50	125		139	6	25			
95	50	85		140	6	0	1		
96	50	75		141	3	275			
97	50	62 5		142	3	125			
98	50	50		143	3	85			
99	50	37 5		144	3	75			
100	50	25		145	3	66 67			
101	50	0	1	146	3	58 33			
102	32 5	275		147	3	50			
103	32 5	125		148	3	37 5			
104	32 5	85		149	3	25			
105	32 5	75		150	3	0	1		
106	32 5	62 5		151	1	275			
107	32 5	50		152	1	125			
108	32 5	37 5		153	1	85			
109	32 5	25		154	1	75			
110	32 5	0	1	155	1	66 67			
111	20	275		156	1	58 33			
112	20	125		157	1	50			
113	20	85		158	1	37 5			
114	20	75		159	1	25			
115	20	66 67		160	1	0	1		
116	20	58 33		161	5	275			
117	20	50		162	5	125			
118	20	37 5		163	5	85			
119	20	25		164	5	75			
120	20	0	1	165	5	66 67			
121	10	275		166	5	58 33			
122	10	125		167	5	50			
123	10	85		168	5	37 5			
124	10	75		169	5	25			
125	10	66 67		170	5	0	1		
126	10	58 33							
127	10	50		-1		01	2	1 5	50
128	10	37 5		0		01	2	1 5	50
129	10	25		-1		01	2	1 5	50
130	10	0	1						

## SUPPLEMENTAL DATA VI

### Selected Output for Test Problem

SAMPLE MODEL SIMULATION OF A LONG ISLAND, NEW YORK PUMP TEST

COEFFICIENT OF SPECIFIC STORAGE= 10000E-05 1/FT

SPECIFIC YIELD = 2500

THE PRIMARY HYDRAULIC CONDUCTIVITY = 270 000 RADIALY & 27 000 VERTICALLY (FT/DAY)

288 ELEMENTS 170 NODES 4 CONSTANT VALUE NODES 4 NODES DISCHARGING 109 ELEMENTS OF DIFF H C

DISCHARGING NODES

167  
166  
165  
164

ELEMENTS WITH DIFFERENT CONDUCTIVITIES

ELEMENT	RADIAL H C	VERTICAL H C
180	010	001
181	010	001
182	010	001
183	010	001
184	010	001
185	010	001
186	010	001
187	010	001
188	010	001
189	010	001
190	010	001
191	010	001
192	010	001
193	010	001
194	010	001
195	010	001
196	010	001
197	010	001
198	010	001
199	010	001
200	010	001
201	010	001
202	010	001
203	010	001
204	010	001
205	010	001
206	010	001
207	010	001
208	010	001
209	010	001
210	010	001
211	010	001
212	010	001
213	010	001
214	010	001
215	010	001
216	010	001
217	010	001
218	50 000	1 400
219	50 000	1 400
220	50 000	1 400
221	50 000	1 400
222	50 000	1 400
223	50 000	1 400
224	50 000	1 400

THE HALF BAND WIDTH IS 10 AND THE WIDTH OF THE MATRIX IS 21

PUMPING PERIOD 1

INITIAL TIME STEP = 00777 DAYS

NUMBER OF TIME STEPS IN THIS PUMPING PERIOD = 12

DISCHARGE= - 10000E+01 CFS

RECHARGE= -0 00 FT/DAY

CALCULATED WELL DISCHARGE = -1 00 CFS

# Selected Output for Test Program—Continued.

TOTAL TIME IN THE SIMULATION = 037 DAYS

THE DRAWDOWN AFTER .369E-01 DAYS OR 531E+02MIN IN THE PUMPING PERIOD

NODE NUMBER	DRAWDOWN	NODE NUMBER	DRAWDOWN	NODE NUMBER	DRAWDOWN
1	0.000	2	0.000	3	0.000
4	0.000	5	.000	6	.000
7	.000	8	.000	9	.000
10	.000	11	.000	12	.000
13	.000	14	.000	15	.000
16	.000	17	.000	18	.000
19	.000	20	.000	21	.000
22	.000	23	.000	24	.000
25	.000	26	.002	27	.002
28	.001	29	.000	30	.000
31	.000	32	.000	33	.016
34	.015	35	.014	36	.011
37	.008	38	.001	39	.000
40	.001	41	.001	42	.067
43	.065	44	.059	45	.049
46	.036	47	.005	48	.000
49	.001	50	.001	51	.183
52	.176	53	.159	54	.131
55	.096	56	.013	57	.000
58	.001	59	.001	60	.354
61	.340	62	.300	63	.243
64	.175	65	.024	66	.000
67	.001	68	.002	69	.575
70	.546	71	.472	72	.370
73	.257	74	.035	75	.000
76	.001	77	.002	78	.981
79	.923	80	.768	81	.565
82	.370	83	.050	84	.000
85	.001	86	.002	87	1.359
88	1.265	89	1.014	90	.700
91	.434	92	.059	93	.000
94	.001	95	.002	96	1.962
97	1.820	98	1.388	99	.866
100	.499	101	.066	102	.000
103	.001	104	.002	105	2.680
106	2.496	107	1.811	108	.995
109	.538	110	.070	111	.000
112	.001	113	.003	114	3.654
115	3.456	116	3.049	117	2.231
118	1.084	119	.559	120	.073
121	.000	122	.001	123	.003
124	4.863	125	4.765	126	4.271
127	2.806	128	1.146	129	.569
130	.073	131	.000	132	.001
133	.003	134	5.804	135	5.770
136	5.247	137	3.222	138	1.164
139	.572	140	.073	141	.000
142	.001	143	.003	144	7.029
145	7.121	146	6.580	147	3.763
148	1.173	149	.573	150	.073
151	.000	152	.001	153	.003
154	9.777	155	9.156	156	8.607
157	4.555	158	1.177	159	.573
160	.073	161	.000	162	.001
163	.003	164	10.000	165	10.513
166	9.965	167	5.087	168	1.177
169	.573	170	.073		