

Digital Models of Ground-Water Flow in the Cape Cod Aquifer System, Massachusetts

United States
Geological
Survey
Water-Supply
Paper 2209

Prepared in cooperation
with the Commonwealth
of Massachusetts, Water
Resources Commission;
Barnstable County; and
the National Park Service



Digital Models of Ground-Water Flow in the Cape Cod Aquifer System, Massachusetts

By JOHN H. GUSWA and DENIS R. LeBLANC

Prepared in cooperation with the
Commonwealth of Massachusetts,
Water Resources Commission;
Barnstable County; and the
National Park Service

DEPARTMENT OF THE INTERIOR
DONALD PAUL HODEL, Secretary

U.S. GEOLOGICAL SURVEY
Dallas L. Peck, Director



UNITED STATES GOVERNMENT PRINTING OFFICE: 1985

For sale by the Distribution Branch, U.S. Geological Survey,
604 South Pickett Street, Alexandria, VA 22304

Library of Congress Cataloging in Publication Data

Guswa, John H.
Digital models of ground-water flow in the Cape Cod
aquifer system, Massachusetts.
(Water-supply paper ; 2209)
Bibliography: p.
Supt. of Docs. no. : I 19.13:2209
1. Groundwater flow—Massachusetts—Cape Cod—
Mathematical models. 2. Groundwater flow—Mas-
sachusetts—Cape Cod—Data processing. I. Guswa,
John H. II. LeBlanc, Denis R. III. Title. IV. Series:
U.S. Geological Survey water-supply paper ; 2209.
GB1197.7.G87 551.49'09744'92 82-600101

CONTENTS

| | |
|--|-----|
| Abstract | 1 |
| Introduction | 1 |
| Background | 1 |
| Purpose and scope | 1 |
| Hydrogeology of Cape Cod | 2 |
| Geologic framework and water-bearing characteristics | 2 |
| Hydraulic properties of sediments | 4 |
| Analysis of specific-capacity data | 4 |
| Aquifer test analyses | 6 |
| Hydrologic system | 6 |
| Digital-simulation model | 7 |
| Flow equation | 7 |
| Numerical method | 7 |
| Application of simulation model | 9 |
| Model specifications | 9 |
| Water-transmitting properties of the aquifer | 9 |
| Boundaries and hydraulic stresses | 12 |
| Well discharge | 12 |
| Boundary between fresh and saline water | 12 |
| Streams and marshes | 13 |
| Aquifer recharge | 14 |
| Calibration of the steady-state flow models | 15 |
| Purpose and procedure | 15 |
| Results | 15 |
| WCAPE model | 15 |
| ECAPE model | 18 |
| ESTHM model | 18 |
| WLFLT model | 18 |
| TRURO model | 18 |
| Summary and conclusions | 18 |
| References | 27 |
| Supplemental data—model data, input documentation, and source code | 31 |
| Metric conversion factors | 112 |

FIGURES

1. Index map showing physical features of Cape Cod **2**
2. Water-table map of Cape Cod showing the freshwater-flow systems and the approximate boundaries of the modeled areas **3**
- 3-7. Plan view of finite-difference grid for:
 3. WCAPE model **8**
 4. ECAPE model **9**
 5. ESTHM model **11**
 6. WLFLT model **11**
 7. TRURO model **12**
8. Schematic representation of equivalent vertical hydraulic conductivity **13**
9. Schematic hydrologic section of the seaward boundary of fresh ground-water flow **14**
10. Calculated and observed average water table, WCAPE model **16**
11. Calculated water table and interface between fresh and saline ground water for cross section *A-A'* **17**
12. Calculated water table and interface between fresh and saline ground water for cross section *B-B'* **17**
13. Calculated water table and interface between fresh and saline ground water for cross section *C-C'* **18**
14. Calculated and observed average water table, ECAPE model **21**
15. Calculated water table and interface between fresh and saline ground water for cross section *D-D'* **22**
- 16-18. Calculated and observed average water table:
 16. ESTHM model **23**
 17. WLFLT model **23**
 18. TRURO model **24**
19. Calculated water table and interface between fresh and saline ground water for cross section *E-E'* **25**
- 20-23. Transmissivity, WCAPE model:
 20. Layer 1 **32**
 21. Layer 2 **33**
 22. Layer 3 **34**
 23. Layer 4 **35**
24. Hydraulic conductivity, WCAPE model, layer 5 **36**
- 25-28. Ratio of lateral to vertical hydraulic conductivity, WCAPE model:
 25. Layer 2 **37**
 26. Layer 3 **38**
 27. Layer 4 **39**
 28. Layer 5 **40**
- 29-32. Transmissivity, ECAPE model:
 29. Layer 1 **41**
 30. Layer 2 **42**
 31. Layer 3 **43**
 32. Layer 4 **44**
33. Hydraulic conductivity, ECAPE model, layer 5 **45**
- 34-37. Ratio of lateral to vertical hydraulic conductivity, ECAPE model:
 34. Layer 2 **46**
 35. Layer 3 **47**
 36. Layer 4 **48**
 37. Layer 5 **49**

| | |
|--|-----------|
| 38-39. Transmissivity, ESTHM model: | |
| 38. Layer 5 | 50 |
| 39. Layer 6 | 50 |
| 40. Hydraulic conductivity, ESTHM model, layer 7 | 51 |
| 41-43. Ratio of lateral to vertical hydraulic conductivity, ESTHM model: | |
| 41. Layer 5 | 51 |
| 42. Layer 6 | 52 |
| 43. Layer 7 | 52 |
| 44-46. Transmissivity, WLFLT model: | |
| 44. Layer 4 | 53 |
| 45. Layer 5 | 53 |
| 46. Layer 6 | 54 |
| 47. Hydraulic conductivity, WLFLT model, layer 7 | 54 |
| 48-50. Ratio of lateral to vertical hydraulic conductivity, WLFLT model: | |
| 48. Layer 5 | 55 |
| 49. Layer 6 | 55 |
| 50. Layer 7 | 56 |
| 51-52. Transmissivity, TRURO model: | |
| 51. Layer 5 | 56 |
| 52. Layer 6 | 57 |
| 53. Hydraulic conductivity, TRURO model, layer 7 | 57 |
| 54-55. Ratio of lateral to vertical hydraulic conductivity, TRURO model: | |
| 54. Layer 5 | 58 |
| 55. Layer 6 | 58 |
| 56-60. Steady-state recharge: | |
| 56. WCAPE model | 59 |
| 57. ECAPE model | 62 |
| 58. ESTHM model | 63 |
| 59. WLFLT model | 63 |
| 60. TRURO model | 64 |

TABLES

| | |
|---|-----------|
| 1. Estimates of average hydraulic conductivity for lithologic types calculated from specific-capacity data | 5 |
| 2. Average hydraulic conductivity values used to estimate transmissivity | 5 |
| 3. Descriptive information for finite-difference grids of modeled areas | 10 |
| 4-8. Comparison of calculated nodal head values and observed average water levels for selected wells, 1963-1976: | |
| 4. WCAPE model | 19 |
| 5. ECAPE model | 20 |
| 6. ESTHM model | 26 |
| 7. WLFLT model | 26 |
| 8. TRURO model | 27 |
| 9. Values for transmissivity, average hydraulic conductivity, and ratio of lateral to vertical hydraulic conductivity used in calibrated models | 31 |
| 10. Summary of well discharges represented in models | 60 |
| 11. Input documentation | 65 |
| 12. Computer source code | 72 |

Digital Models of Ground-Water Flow in the Cape Cod Aquifer System, Massachusetts

By JOHN H. GUSWA *and* DENIS R. LEBLANC

Abstract

The Cape Cod aquifer system was simulated with three-dimensional finite-difference ground-water-flow models. Five areas were modeled to provide tools that can be used to evaluate the hydrologic impacts of regional water development and waste disposal.

The model boundaries were selected to represent the natural hydrologic boundaries of the aquifer. The boundary between fresh and saline ground water was treated as an interface along which there is no dispersion. The saline-water zone was treated as static (nonflowing).

Comparisons of calculated and observed values of head, position of the boundary between fresh and saline water, and ground-water discharge (at locations where data were available) indicate that the simulated ground-water reservoirs generally agree with field conditions.

Model analyses indicate that the total steady-state freshwater-flow rate through the five modeled areas is approximately 412 cubic feet per second.

INTRODUCTION

Background

Ground water is the principal source of freshwater for Cape Cod, Massachusetts. The Cape is composed of unconsolidated glacial moraines and outwash plains that form a hook-shaped peninsula extending 40 mi into the ocean (fig. 1). It is separated from the mainland by a sea-level canal connecting Buzzards Bay and Cape Cod Bay. A lens-shaped reservoir of fresh ground water is maintained in dynamic equilibrium beneath the Cape by recharge from precipitation and discharge to the sea.

Demand for water has increased as the number of year-round residents and summer vacationers on Cape Cod has grown. State and local governments are concerned that increased pumpage to meet the increased demand may cause undesirable changes in water-table and pond levels and may reduce discharge of freshwater to coastal brackish-water bodies. Also, there is great concern that land

disposal of solid and liquid waste may deteriorate water quality, and contaminants may move through the aquifer to wells, ponds, and streams.

Several problems are generally encountered in the management of ground-water resources. Hydrologic properties of aquifers are dependent on geology, and determination of these properties is usually complex and expensive. In addition, local stresses can have regional hydraulic effects on an aquifer. Since ground-water movement generally is slow and cannot be observed directly, it must be inferred from indirect measurements and abstract mathematical reasoning. Therefore, the consequences of management decisions may not be noticed for decades and the results may then be, for all practical purposes, irreversible.

Flow in an aquifer can be described analytically by differential equations. These equations will yield direct solutions, which may not be useful for some problems because of the complexity of most aquifer systems. Approximate solutions of the flow equations describing aquifers with spatial variation of hydrologic properties and stresses and irregular geometries can be obtained with numerical methods. The reliability of these numerical solutions is dependent upon the accuracy of the input data and the size of the aquifer element which the flow equations describe.

Purpose and Scope

The purpose of this report is to describe digital simulation models prepared as part of a 4-year study of the ground-water resources of Cape Cod, in cooperation with the Massachusetts Water Resources Commission, Divisions of Water Resources and Water Pollution Control; Barnstable County; and the National Park Service. Included in this report are the sources and types of data used in constructing five models, model calibration, and possible sources of inaccuracies in the results. The five modeled areas and their names are shown in figure 2.

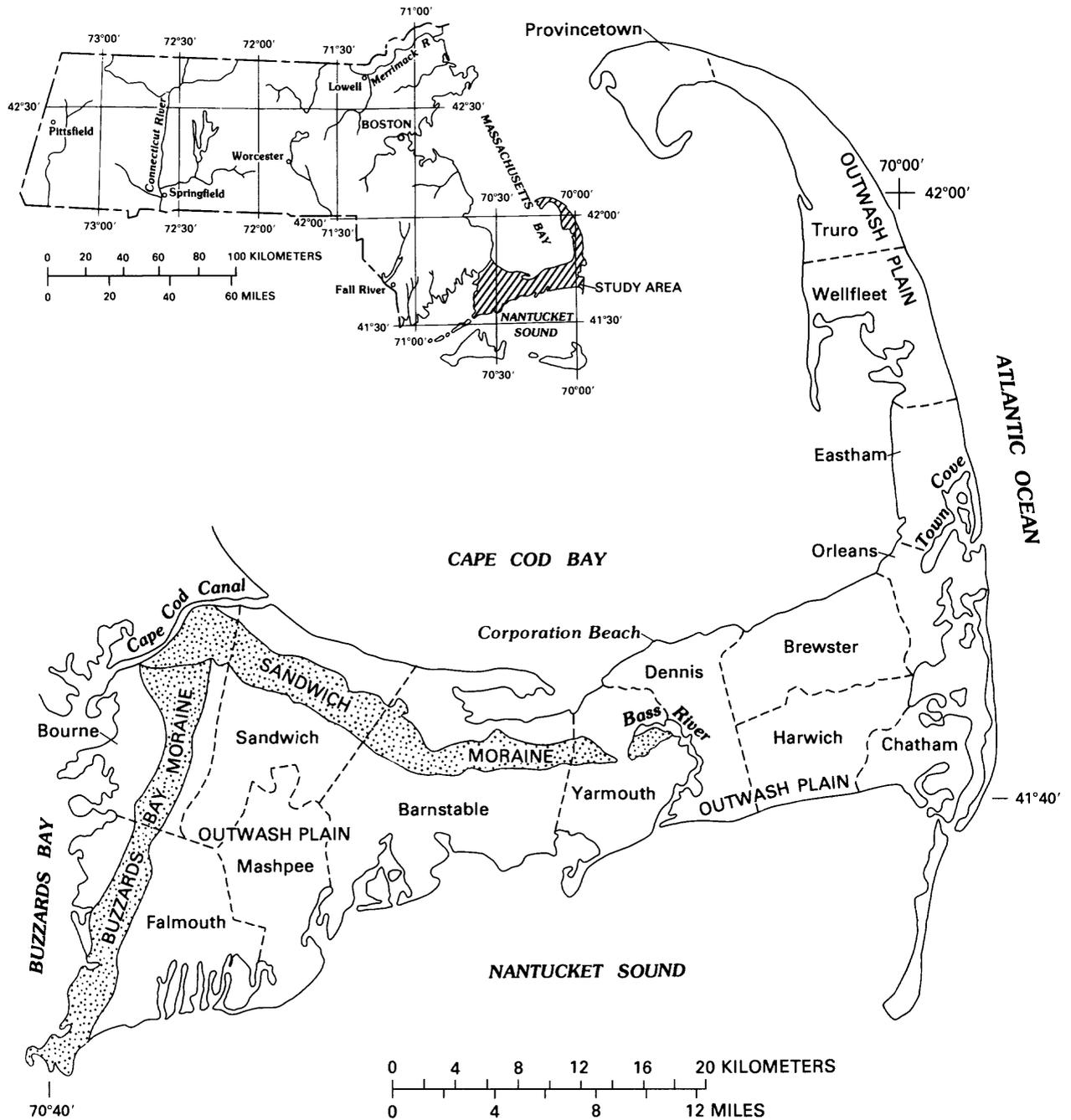


Figure 1. Physical features of Cape Cod (from Oldale, 1976).

The models provide information regarding the regional behavior of the aquifer system. Although detailed analyses of local hydrologic conditions are beyond the scope of this study, the principles used to construct these models can be used to construct more detailed models of smaller areas if sufficient data are available.

HYDROGEOLOGY OF CAPE COD

Geologic Framework and Water-Bearing Characteristics

Bedrock underlies Cape Cod, but is not exposed at land surface. The altitude of the irregular bedrock surface

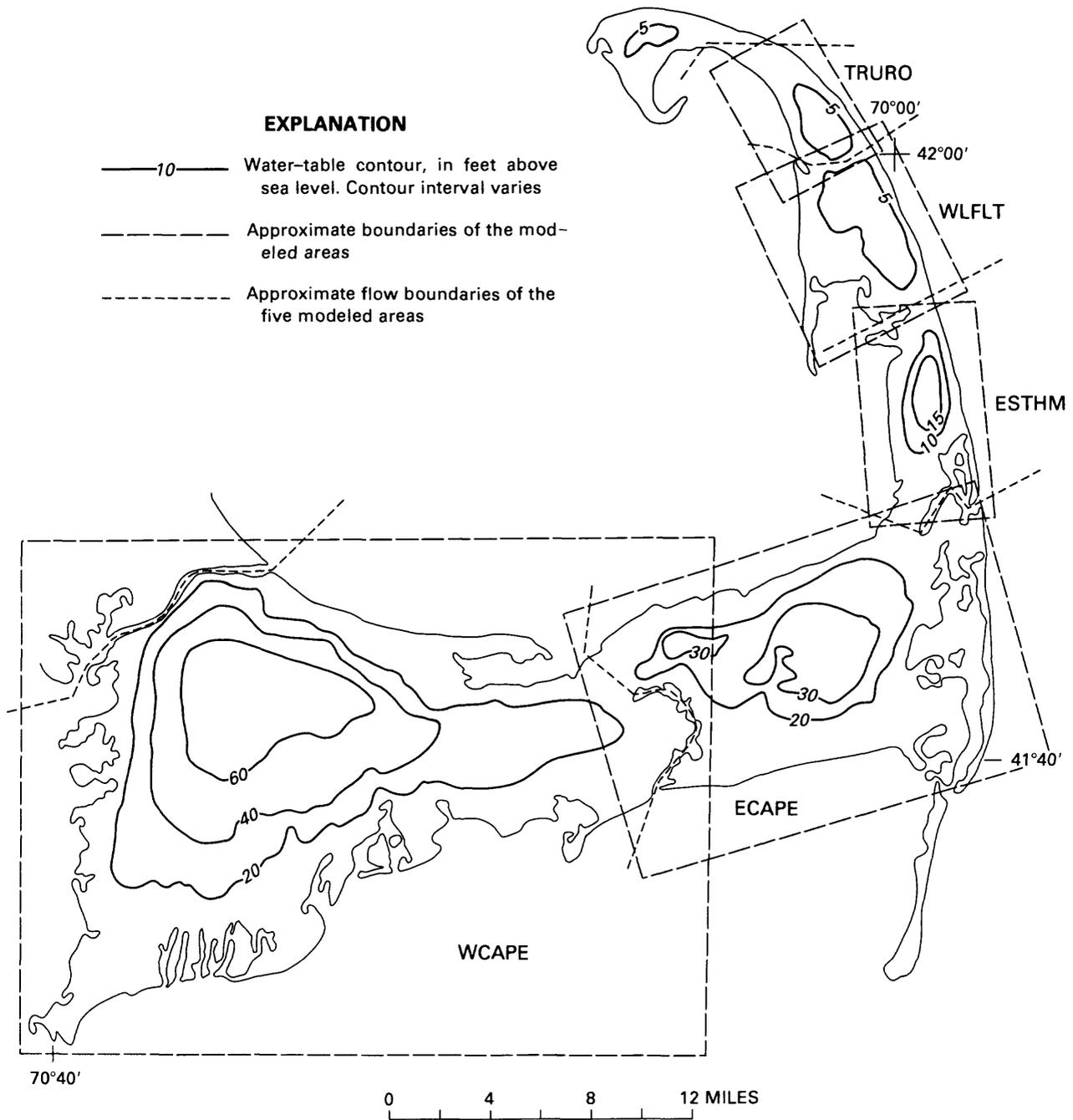


Figure 2. Freshwater-flow systems and the approximate boundaries of the modeled areas, Cape Cod, May 25–27, 1976.

ranges from 80 feet below sea level near the Cape Cod Canal to more than 900 feet below sea level near Provincetown (Oldale, 1969). The bedrock is overlain by unconsolidated sediments deposited by ice sheets during the Pleistocene Epoch as a series of end moraines and outwash plains that characterize coastal New York (Long Island), Rhode Island, and Massachusetts (Oldale, 1976). The glacial sediments consist of sand, gravel, silt, clay, and till. Along the coast, these deposits have been reworked since the Pleistocene by ocean currents and wind.

Sandy till mixed with stratified sand, gravel, and silt forms the Buzzards Bay and Sandwich moraines (fig. 1). These moraines are low, broad north- and east-trending ridges of moderately rugged topography. East of the Bass River, the east-trending Sandwich moraine deposits are buried by outwash-plain deposits.

Extreme lithologic variation over short distances and depths to water generally greater than 30 ft have discouraged exploration for water supply in the moraines. Projected yields of 4.5 ft³/s (2,000 gal/min) have been re-

ported (Thomas Mullen, water superintendent, Barnstable Fire District, oral commun., 1977) for 24-inch diameter gravel-packed wells screened in sand and gravel in the moraine area of Barnstable. Wells drilled in similar nearby areas of the moraine, however, have penetrated silt and clay layers several hundred feet thick and have been reported to be dry holes. The lack of test-hole information and the extreme lithologic variation in the moraines do not permit stratigraphic correlation.

The western part of the outwash-plain deposits in Bourne, Sandwich, Falmouth, Mashpee, and western Barnstable is composed primarily of stratified sand and gravel but has local silt and clay layers. These deposits generally become finer-grained with depth, especially to the south, where very fine sand, silt, and clay predominate below 100 ft in many places.

The eastern part of the outwash plain, in eastern Barnstable, Yarmouth, Dennis, Brewster, Harwich, Chatham, and Orleans, is composed of stratified sand and gravel which, in places, is mixed with till and ice-contact deposits, silt, and clay. The northern section of this part of the outwash plain is generally coarser-grained than the southern section and is mixed with or overlies very coarse-grained ice-contact deposits and till, especially in Yarmouth, Dennis, and Chatham. To the south, clay and silt layers are commonly interbedded with sand and gravel. Along Nantucket Sound, the eastern part of the outwash plain is underlain by a silt and clay deposit which is thicker than 150 ft in places. The silt and clay are reported to have been deposited in a lake that occupied the present site of eastern Nantucket Sound (Oldale, 1976).

The lithologic variation within the outwash plain deposits is not as extreme as within the moraine areas, and well yields range from 0.45 to 1.56 ft³/s (200 to 700 gal/min), or 0.05 to 0.16 ft³/s (20 to 70 gal/min) per foot of 24-inch diameter gravel-packed screen.

North of the Sandwich moraine are deposits of sand and gravel which, in some areas, grade northward into finer-grained deposits (silt and clay). Results of test drilling along the southern shore of Cape Cod Bay (from the Cape Cod Canal to Brewster) indicate that silt and clay layers, more than 100 ft thick in places, are commonly interbedded with the sand and gravel beds.

The silt and clay deposits confine sand and gravel in many areas along the bay. Because of the confining conditions, the interface between fresh and saline ground water is displaced offshore. For example, at Corporation Beach in Dennis (fig. 1) a well located 100 ft inland from the shoreline was drilled through unconfined and confined sediments to bedrock at a depth of 316 ft and penetrated only freshwater (chloride concentration less than 250 mg/L). Hydraulic head was 5 ft greater in the deep aquifer, confined by 35 ft of silt and clay, than in the shallow water-table aquifer.

From Orleans to Truro, the Cape is underlain by glacial outwash deposits consisting primarily of sand and gravel interbedded with silt and clay. Exposures of these deposits along the eastern shore of the outer Cape indicate that the silt and clay layers are contorted and discontinuous. Lack of well-log data precludes determination of the areal extent of the silt and clay layers. The only large-scale water-supply system developed in these deposits is in the town of Truro.

Holocene deposits include the salt marshes, beaches, spits, and dunes. They are composed of sand, silt, and clay. Few wells have been drilled in these deposits.

Hydraulic Properties of Sediments

The transmissivity of an aquifer may be estimated by multiplying a calculated or estimated hydraulic conductivity value by aquifer thickness. A more general method of calculating the transmissivity of an aquifer consisting of several hydrogeologic units is given by equation 1.

$$T = \sum_{i=1}^n K_i m_i, \quad (1)$$

in which

T is the transmissivity (L^2/T);
 K_i is the hydraulic conductivity of the i th hydrogeologic unit (L/T);
 m_i is the thickness of the i th hydrogeologic unit (L);

and

n is the number of hydrogeologic units within the specified aquifer thickness.

Analysis of Specific-Capacity Data

The hydraulic conductivities of different lithologies on Cape Cod were estimated by analyzing specific-capacity data (well pumping rate/observed drawdown) from 265 well-performance tests. The tested wells generally had short screens that tapped a small part of the total thickness of the aquifer. Within the screened interval, the lithology was generally consistent and could be identified.

The specific capacity of each well was converted to transmissivity for the screened interval using the non-steady-flow equation for unconfined aquifers given by Theis (1963) and shown below:

$$T' = \frac{Q}{s} (K - 264 \log_{10} 5S + 264 \log_{10} t), \quad (2)$$

in which

Q is the well discharge, in gallons per minute;
 s is the drawdown in the well, in feet;
 K is a factor equal to $-66 - 264 \log_{10} (3.74 r^2 \cdot 10^{-6})$;

Table 1. Estimates of average hydraulic conductivity for lithologic types calculated from specific-capacity data

| Lithology | Number of samples | Hydraulic conductivity (ft/d) | | |
|------------------------|-------------------|-------------------------------|--------------------|--------------|
| | | Mean value | Standard deviation | Median value |
| Fine sand | 34 | 450 | 310 | 440 |
| Fine to medium sand | 51 | 450 | 280 | 400 |
| Medium sand | 14 | 500 | 450 | 310 |
| Medium to coarse sand | 22 | 600 | 380 | 480 |
| Coarse sand | 11 | 430 | 220 | 400 |
| Fine to coarse sand | <u>42</u> | <u>580</u> | <u>290</u> | <u>520</u> |
| All sand | 174 | 500 | 310 | 400 |
| | | | | |
| Sand and gravel | 16 | 630 | 430 | 420 |
| Fine sand and gravel | 48 | 530 | 370 | 420 |
| Medium sand and gravel | 9 | 740 | 460 | 730 |
| Coarse sand and gravel | <u>18</u> | <u>630</u> | <u>430</u> | <u>420</u> |
| All sand and gravel | 91 | 580 | 400 | 400 |

- r is the radial distance to the well where the drawdown is observed, in feet;
 t is the length of the pumping period, in days;
 S is the aquifer specific yield (dimensionless);
and
 T' is a value related to transmissivity by means of an equation given by Theis (1963, p. 332, eq. 2), in gallons per day per foot.

The value of T' was converted to transmissivity, T , for the screened interval of the well by solving the relationship between T and T' presented by Theis:

$$T' = T - (264Q \log_{10}(T \cdot 10^{-5}))/s.$$

The value of transmissivity for the screened interval was divided by the screened length of the well to obtain the average hydraulic conductivity of the screened interval.

The well-performance data and the calculated hydraulic conductivities were divided into groups according to the lithologic types reported for the screened intervals of the wells. The values within each group were averaged to obtain the estimated hydraulic conductivity for each type of material (table 1).

Because of the large deviation of values from the calculated means and because most of the available lithologic information did not have the same level of detail as table 1, the sediment-type categories were grouped into larger classes, and average values were applied to each class (table 2).

Table 2. Average hydraulic conductivity values used to estimate transmissivity.

| Lithology | Hydraulic conductivity (ft/d) |
|--------------------|-------------------------------|
| Silt and (or) clay | 1 |
| Sand | 450 |
| Sand and gravel | 500 |
| Gravel | 600 |

None of the wells were screened in silt, clay, or gravel only. The hydraulic conductivity of silt and clay is low relative to the other materials, and was therefore assigned a value of 1 ft/d. Gravel was assigned a hydraulic conductivity of 600 ft/d.

During the process of estimating hydraulic conductivity from specific-capacity data, it was assumed that the storage coefficient was 0.2; well-entrance losses were negligible; flow into the well was horizontal, radial, and uniform along the well screen; and the discharge was sustained by withdrawal from aquifer storage within the screened interval. Flow rates in these tests were generally low, and well-entrance losses were probably minimal. Errors generated by deviation from the remaining assumptions and by incorrect estimation of storage coefficient were assumed to be random and, therefore, would be minimized in the averaging process. These estimates of hydraulic conductivity may contain a small systematic error. The vertical span of the flow pattern toward the well screen would always exceed the screened interval of the well. This would cause the calculated values to be high.

The specific-capacity data and the lithologic information used in the hydraulic conductivity calculation were obtained from drillers' records. The method of specific-capacity testing varied from one driller to another, and the identification of lithologic type is somewhat subjective. The data, therefore, were of variable quality. The error due to the variable quality of the data probably is random and would be minimized in the averaging process.

The values of hydraulic conductivity in table 2 were applied by means of equation 1 to numerous locations on Cape Cod for which a lithologic log was available to estimate transmissivities of selected zones of the aquifer. These zones correspond to different layers of the flow models and will be discussed in a later section. The initial estimates of transmissivity, calculated by this method, were adjusted during model calibration.

Aquifer Test Analyses

Lateral hydraulic conductivity, the ratio of lateral to vertical hydraulic conductivity, and the storage properties of the aquifer were estimated from analysis of three aquifer tests. A 72-hour test in Truro, Mass., on a well with a 70-ft screened interval (Guswa and Londquist, 1976) was used to estimate that the average lateral hydraulic conductivity for a predominantly very fine to coarse sand is 220 ft/d. Analysis of 5-day tests in Orleans and Yarmouth, Mass., indicated a lateral hydraulic conductivity of 300 ft/d for a coarse to very coarse sand and very fine gravel with some medium sand, and 200 ft/d for a fine to medium sand.

Analysis of the Truro and Orleans aquifer-test data using a radial-flow model yielded a ratio of lateral to vertical hydraulic conductivity of less than 10:1. Because other field data were not available and other studies (Getzen, 1977, p. 10; Meyer and others, 1975, p. 19) have indicated that this ratio is reasonable for glacial outwash, the 10:1 ratio was applied to each lithologic type.

There are no reliable data available for the vertical hydraulic conductivity of the silt and clay lenses which are scattered throughout the Cape Cod aquifer system. A value of 0.1 ft/d, which is similar to that used in the analog-model analyses of ground-water flow on Long Island (Getzen, 1977), was used for these beds.

Field data for storage coefficients of the Cape Cod aquifer system are also meager. Palmer (1977, p. 45) reports a range of specific yield of 0.13 to 0.26. His values were based on analysis of aquifer tests in the Falmouth area. Analysis of the Truro (Guswa and Londquist, 1976) and Orleans aquifer-test data indicate a specific yield of between 0.10 and 0.15.

Hydrologic System

The Cape Cod aquifer system comprises the fresh-water-saturated sediments through which water moves. The boundaries of this system are the water table, saline surface water bodies, and either the bedrock surface (or other material of low hydraulic conductivity) or a boundary between fresh ground water and saline ground water.

The saline-surface-water boundaries include the ocean, bays, streams, and marshes, which are generally at a fixed elevation and serve as discharge boundaries for the ground-water-flow system. At several locations, these discharge boundaries extend sufficiently far inland to subdivide the aquifer system into several nearly independent aquifers (fig. 2). Under existing hydrologic conditions, there is no flow between adjacent aquifers across the boundaries. Conditions of severe hydrologic stress may change the nature of these boundaries and cause water to flow between the individual aquifers.

Bedrock has a low hydraulic conductivity compared to the sand and gravel deposits that form the Cape and can be considered to be a no-flow boundary at the bottom of the aquifer system. In some areas, particularly in Harwich and the southern portions of Dennis and Yarmouth, thick accumulations of silt and clay underlie the sand and gravel aquifer and overlie the bedrock. These sediments also have a low hydraulic conductivity and, where they directly overlie bedrock, their upper surface is considered to be the bottom of the aquifer system.

Fresh ground water is bounded by saline surface water at the shores of the ocean, Nantucket Sound, Buzzards Bay, Cape Cod Canal, and Cape Cod Bay (fig. 1) and by saline ground water at depth. Fresh ground water is slightly less dense than saline water and, therefore, "floats" as a lens-shaped body upon underlying saline water in the ground-water reservoir. The boundary between freshwater and saline water is a zone of mixing (or diffusion) of fresh and saline water. This mixing zone is the lower boundary of the aquifer system in those areas where it is above the bedrock surface or the thick silt and clay layers.

DIGITAL-SIMULATION MODEL

The purpose of the simulation model is to calculate the hydraulic head in an aquifer at specified locations under steady-flow conditions. This is achieved by solving the steady-state differential equation of ground-water flow, which requires that the hydraulic properties, boundaries, and inflow and outflow be defined for the modeled area.

A three-dimensional steady-state-flow model was chosen for this study for the following reasons:

1. Existing hydrogeologic information and a previous study (Burns and others, 1975) indicated that the three-dimensional variability of the aquifer system had to be included in the analyses.
2. There were few long-term records of head and stress changes which could be used to calibrate a three-dimensional transient model.
3. The approximation used to represent the boundary between fresh and saline ground water is valid only for equilibrium conditions.

Flow Equation

The differential equation describing three-dimensional steady-state flow in a porous medium is

$$\frac{\partial}{\partial x} (K_{xx} \frac{\partial h}{\partial x}) + \frac{\partial}{\partial y} (K_{yy} \frac{\partial h}{\partial y}) + \frac{\partial}{\partial z} (K_{zz} \frac{\partial h}{\partial z}) = W(x,y,z), \quad (3)$$

in which

K_{xx}, K_{yy}, K_{zz} are the principal components of the hydraulic-conductivity tensor aligned with the principal Cartesian coordinate axes (L/T);

h is the hydraulic head in the aquifer (L),

and

$W(x,y,z)$ is a volumetric flux per unit volume ($1/T$) and is referred to as a source term.

It is sometimes convenient to represent one or more hydrogeologic units as a single layer. If this is done, then equation 3 is multiplied by b , the saturated thickness of the layer, giving approximately,

$$\frac{\partial}{\partial x} (T_{xx} \frac{\partial h}{\partial x}) + \frac{\partial}{\partial y} (T_{yy} \frac{\partial h}{\partial y}) + b \frac{\partial}{\partial z} (K_{zz} \frac{\partial h}{\partial z}) = bW(x,y,z), \quad (4)$$

in which

T_{xx}, T_{yy} are the principal components of the transmissivity tensor (L^2/T).

The source term $W(x,y,z)$ can include well discharge; recharge from precipitation or other sources such as septic tanks, sewage-treatment plants, and irrigation; and steady leakage into or out of the aquifer system through a stream, pond, marsh, or ocean bottom.

In the model, this source term is computed as

$$bW = \frac{Q_w}{\Delta x \Delta y} - Q_{re} - \frac{K_z}{m} (h_s - h), \quad (5)$$

in which

Q_w is well discharge from a "block" or element of the aquifer (L^3/T);

Δx is the length of the "block" in the x direction (L);

Δy is the length of the "block" in the y direction (L);

Q_{re} is aquifer recharge and is the volumetric flux per unit area of the uppermost hydrogeologic unit (L/T);

K_z is the vertical hydraulic conductivity of the streambed or ocean bottom (L/T);

m is the thickness of the streambed or ocean bottom (L);

h_s is the hydraulic head in the streambed or ocean bottom (L);

and

h is the head in aquifer (L).

Direct solution of equation 4 is generally impossible, but a numerical solution of high accuracy can be obtained using a digital computer.

Numerical Method

To obtain a numerical solution, the partial differential equation is replaced by an approximating finite-difference equation (Trescott, 1975; Trescott and Larson, 1976) and the aquifer is subdivided into discrete blocks. Each aquifer block has one finite-difference equation describing flow within it. This yields a set of finite-difference equations which must be solved simultaneously.

There are many procedures available for solving a large number of simultaneous equations. The one used in this study is the strongly implicit procedure (SIP) developed by Stone (1968). For a complete discussion of how the finite-difference equations are solved, see Trescott (1975) and Trescott and Larson (1976).

The computer code used in this study is a modification of the code developed by Trescott (1975); input documentation and a source-code listing are included in the Supplemental Data section of this report.

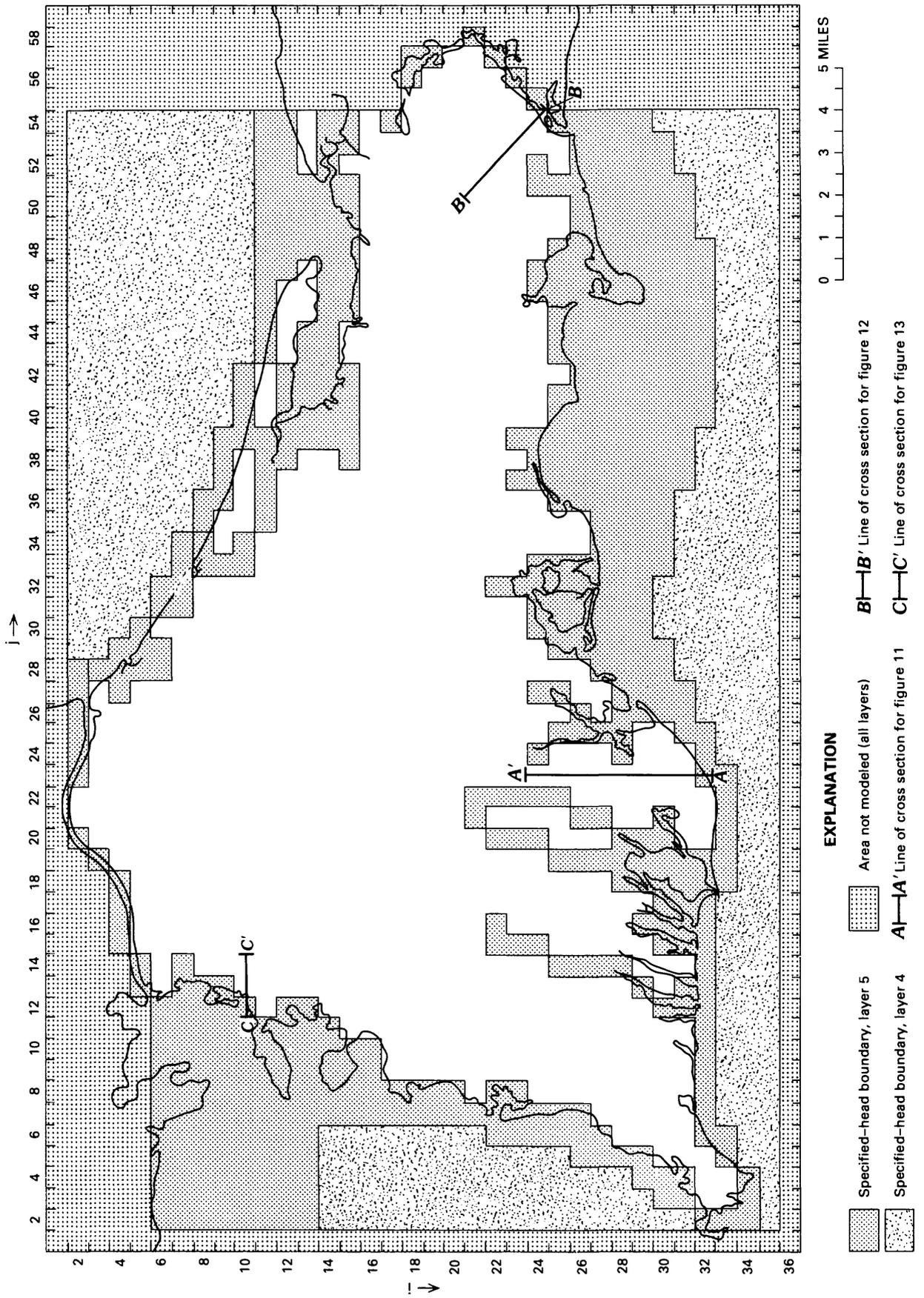


Figure 3. Plan view of finite-difference grid for WCAPE model.

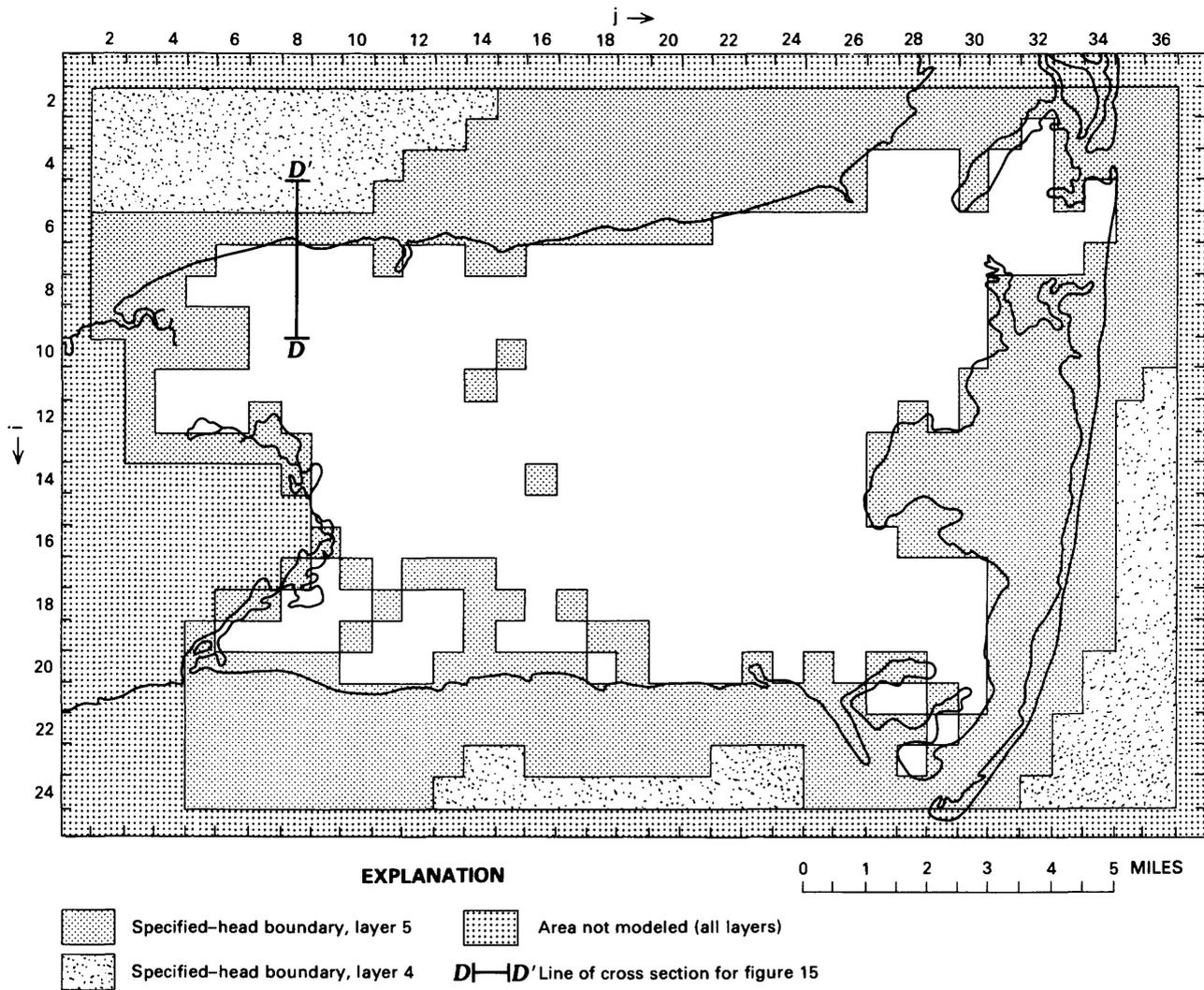


Figure 4. Plan view of finite-difference grid for ECAPE model.

APPLICATION OF SIMULATION MODEL

Model Specifications

Ground-water-flow models were developed for five areas of Cape Cod. The present flow pattern allowed construction of separate models for five of the six individual aquifers (fig. 2). Limits of the modeled areas were selected to include or nearly coincide with the natural flow boundaries of the system. These lateral boundaries are discharge boundaries, such as the seabed, streams, and marshes. They are modeled as specified-head seepage boundaries.

The modeled areas were subdivided into rectangular finite-difference grids with uniform horizontal spacing (figs. 3–7) and uneven vertical spacing. The descriptive information for the individual finite-difference grids can be seen in table 3.

The layers of each model represent elevation horizons within the aquifer system. These horizons contain one or more hydrogeologic units. Aquifer properties and stresses are assumed to be uniform within any given block and must be defined at all nodes of the grid.

By convention, nodes are located at the centers of the blocks of the grid. Any specific node or block may be referenced by citing its row (i), column (j) and layer (k) location.

Water-Transmitting Properties of the Aquifer

At about 320 selected locations in the aquifer system, lithologic logs and the average values of hydraulic conductivity for the lithologic types in table 2 were used in equation 1 to calculate transmissivity values for layers of the model. An average, or equivalent, lateral hydraulic

Table 3. Descriptive information for finite-difference grids of modeled areas

| Modeled area | Number of | | Horizontal grid spacing (feet) | Elevation of bottom of layer (feet below sea level) | | | | | | | |
|--------------|-----------|----------|--------------------------------|---|--------------|-----|-----|-----|-----|----|----|
| | rows | col-umns | | layers | Layer number | | | | | | |
| | | | | 1 | 2 | 3 | 4 | 5 | 6 | 7 | |
| WCAPE | 36 | 59 | 5 | 2640 | 400 | 240 | 140 | 70 | 20 | -- | -- |
| ECAPE | 25 | 37 | 5 | 2640 | 400 | 240 | 140 | 70 | 20 | -- | -- |
| ESTHM | 22 | 36 | 7 | 1320 | 600 | 450 | 300 | 200 | 110 | 50 | 10 |
| WLFLT | 26 | 32 | 7 | 1320 | 400 | 280 | 200 | 140 | 80 | 40 | 10 |
| TRURO | 20 | 26 | 7 | 1320 | 400 | 280 | 200 | 140 | 80 | 40 | 10 |

conductivity for each layer was also calculated using the relationship

$$\bar{K} = \frac{\sum_{i=1}^n K_i m_i}{\sum_{i=1}^n m_i}, \quad (6)$$

in which

- \bar{K} is the average lateral hydraulic conductivity (L/T);
- K_i is the average lateral hydraulic conductivity of the i th hydrogeologic unit (L/T);
- m_i is the thickness of the i th hydrogeologic unit (L);

and

- n is the number of hydrogeologic units that occur within a layer; the top of the uppermost layer coincides with the estimated position of the water table.

Maps of the spatial variation in transmissivity and lateral hydraulic conductivity were made for all layers and served as initial values of the water-transmitting properties of the aquifer.

Equivalent vertical hydraulic conductivities were also calculated from lithologic logs at the 320 locations. Darcy's Law and the analogy between flow in an aquifer and flow of electric current were used to calculate these values. One-dimensional steady-state flow through an aquifer prism can be defined by

$$Q = KA \frac{\Delta h}{\Delta z}, \quad (7)$$

in which

- Q is the rate of flow through the prism (L^3/T);
- K is the hydraulic conductivity of the aquifer in the direction of flow (L/T);
- A is the cross-sectional area of the prism (L^2);
- Δh is the head difference between two measuring points along the flow direction (L);

and

- Δz is the length between the two measuring points (L).

The term $\frac{KA}{\Delta z}$ is analogous to electrical conductance (Prickett, 1975) and will be defined herein as hydraulic conductance, C . In a manner similar to calculating the equivalent electrical conductance when two or more conductors are connected in series, the equivalent vertical hydraulic conductance of a layered aquifer can be calculated using the relationship

$$\frac{1}{C_{veq}} = \sum_{i=1}^n \frac{1}{C_{vi}}, \quad (8)$$

in which

- C_{veq} is the equivalent vertical hydraulic conductance of an aquifer (L^2/T);
- C_{vi} is the vertical hydraulic conductance of the i th layer (L^2/T);

and

- n is the number of layers of the aquifer.

As an example, an aquifer that contains two layers can be seen in figure 8.

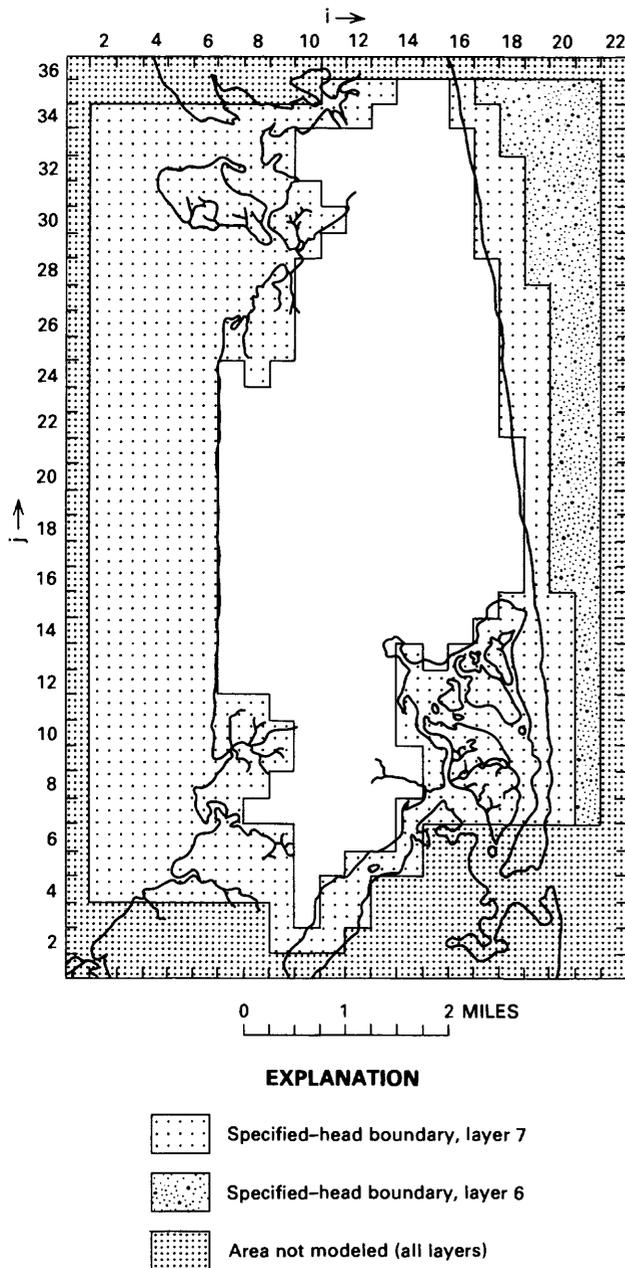


Figure 5. Plan view of finite-difference grid for ESTHM model.

With the appropriate substitutions, equation 8 can be rewritten as

$$\frac{1}{K_{veq} \cdot A} = \sum_{i=1}^n \frac{1}{K_{vi} \cdot A} \cdot \frac{\Delta z_i}{\Delta z_T} \quad (9)$$

in which

K_{veq} is the equivalent vertical hydraulic conductivity (L/T);

A is the cross-sectional area of flow (L^2);

Δz_T is the total thickness of the aquifer and is equal

to $\sum_{i=1}^n \Delta z_i$ (L);

K_{vi} is the vertical hydraulic conductivity of the i th layer (L/T);

and

Δz_i is the thickness of the i th layer (L).

Multiplying both sides of equation 9 by the constant A and rearranging terms yields

$$K_{veq} = \Delta z_T \frac{1}{\sum_{i=1}^n \Delta z_i / K_{vi}} \quad (10)$$

which also can be written as

$$K_{veq} = \frac{\sum_{i=1}^n \Delta z_i}{\sum_{i=1}^n \Delta z_i / K_{vi}} \quad (11)$$

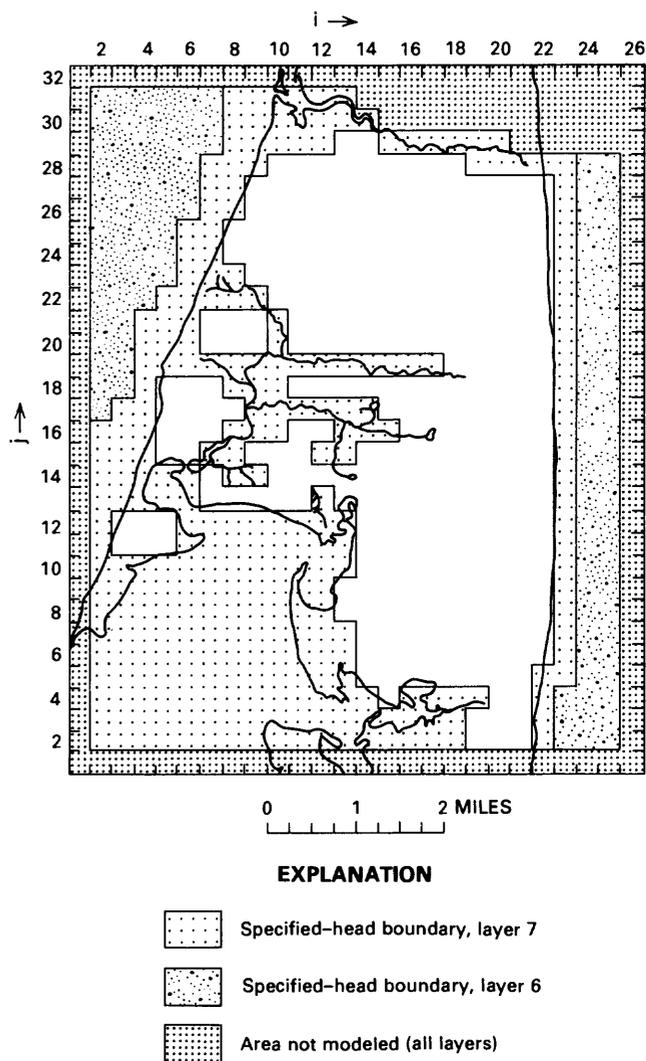


Figure 6. Plan view of finite-difference grid for WLFLT model.

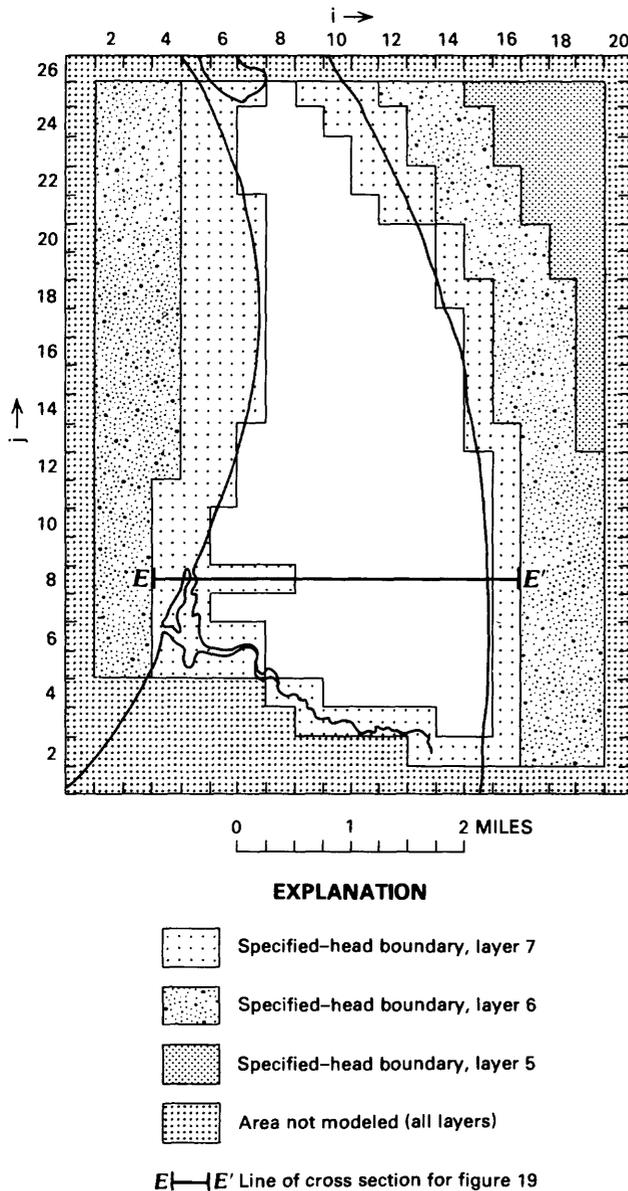


Figure 7. Plan view of finite-difference grid for TRURO model.

Equivalent vertical hydraulic conductivity values for selected nodes were compared with the estimated lateral hydraulic conductivity values to determine a ratio of lateral to vertical hydraulic conductivity. Using values for lateral transmissivity, hydraulic conductivity, the estimated ratio of lateral to vertical hydraulic conductivity, and the thickness of each block, vertical-flow coefficients (Trescott and Larson, 1976, p. X) were calculated.

During model calibration, it was necessary to adjust the initial estimates of transmissivity and lateral and vertical hydraulic conductivity. The final values used in the models are presented in the Supplemental Data section of this report.

Boundaries and Hydraulic Stresses

Well Discharge

Sites of known and significant well discharge within the study area were represented in the model by specifying a constant flux rate at the nodes representing the blocks containing the wells (well discharges are summarized in Supplemental Data, table 10). These average withdrawal rates are based on actual pumpage for 1975 and 1976. The pumpage data were provided by the local water-service districts.

Boundary Between Fresh and Saline Water

The saline surface water surrounding Cape Cod forms a surface boundary to the ground-water reservoir. The seabed is a specified-head boundary to the ground-water-flow system. The distribution of freshwater head along this boundary is a function of the saline-water depth and the density difference between freshwater and saline water. At the bottoms of the bays and the ocean, freshwater heads at the seabed-water interface must be equal to the equivalent freshwater head resulting from the saline surface-water column extending from the seabed to sea level. If the surface-water body is seawater, the resulting equivalent freshwater head above sea level is about 2.5 percent of the saline surface-water depth. It is proportionately less if the density of the surface-water body is less than the density of seawater. This equality of freshwater and saline-water head at the seabed must also exist at the boundary between fresh ground water and saline ground water if (1) the boundary is a sharp interface; (2) the saline ground water is static (nonflowing); (3) the flow system is in a state of dynamic equilibrium; and (4) the only forces acting on the ground water are head gradients arising from gravitational forces (Hubbert, 1940). Figure 9 illustrates this relationship between fresh ground water and saline surface and ground water.

The seabed was modeled as a specified-head seepage boundary. The layers for which this boundary condition was specified were determined by comparing model-layer elevations to the seabed elevation determined from bathymetric and topographic maps. The seabed elevation was specified as the bottom elevation of the source bed. The equivalent freshwater head resulting from the density difference between freshwater and seawater was specified as the source-bed head. A leakance coefficient of 20 (ft/d)/ft was used. Sensitivity analyses indicate that changing this value by an order of magnitude has no significant effect on the calculated heads or flux rates. Provided sufficient precision is used in mass-balance calculations, this is an acceptable way of approximating a constant-head condition (Trescott and others, 1976). Data on the

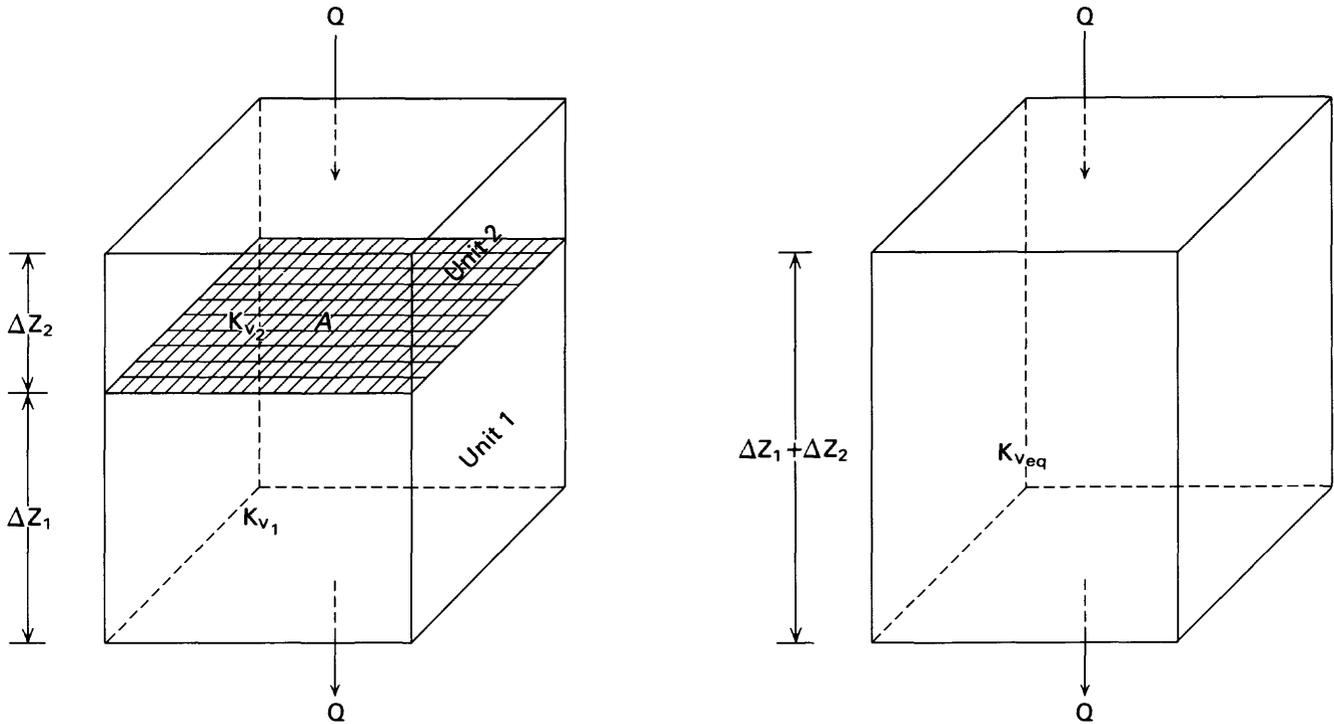


Figure 8. Schematic representation of equivalent vertical hydraulic conductivity.

spatial variability of vertical permeability and thickness of the seabed sediments, if they become available, can be easily incorporated into the models.

The boundary between fresh and saline ground water was modeled as an interface, and the saline-water zone was assumed to be static. This boundary is actually a zone of diffusion or mixing, but data collected concerning the thickness of this zone suggest that it is narrow, and for the purposes of this study it can be treated as a sharp interface.

At specified iterations, the position of this interface is calculated in accordance with the hydrodynamic model of Hubbert (1940). Starting with the bottom layer and proceeding through the grid in a systematic fashion, block transmissivity values are adjusted to reflect the percentage of the block thickness which is occupied by freshwater. For example, the transmissivity of a block in which only the upper 40 percent of the thickness contains freshwater is reduced to 40 percent of its original value. A block calculated to be wholly in the saline-water zone is assigned a transmissivity value of zero for the rest of the simulation. If a block contains freshwater partially or totally, then all blocks directly above it are assumed to contain only freshwater. The vertical-flow coefficients remain unchanged during a simulation unless the freshwater-saturated thickness of a block is determined to be zero. When this occurs, the coefficient describing flow between the block with zero freshwater thickness and the overlying

block is set to zero. An identical adjustment for water-table conditions is described in Trescott and Larson (1976, p. XII).

The computational scheme requires that the starting head values be sufficiently large to define an interface position that is seaward of and deeper than the real interface position. During the iteration sequence, the calculated interface moves landward and upward until it is in balance with the freshwater-flow system.

Streams and Marshes

Many of the streams and marshes are significant boundaries to the freshwater-flow system. The effect of these boundaries varies as a function of the head within the aquifer; they generally derive some or all of their flow from ground-water discharge or seepage, and at certain locations or times of the year may be a source of aquifer recharge. These boundaries were modeled as areas of specified head, separated from the aquifer by a streambed layer. The bottom elevation of the source bed was defined to coincide with the altitude of the stream or marsh bottom. The source-bed head was set equal to the altitude of the stream or marsh surface. A leakage coefficient of 20 (ft/d)/ft was used. This value might represent a 2-ft-thick sand layer.

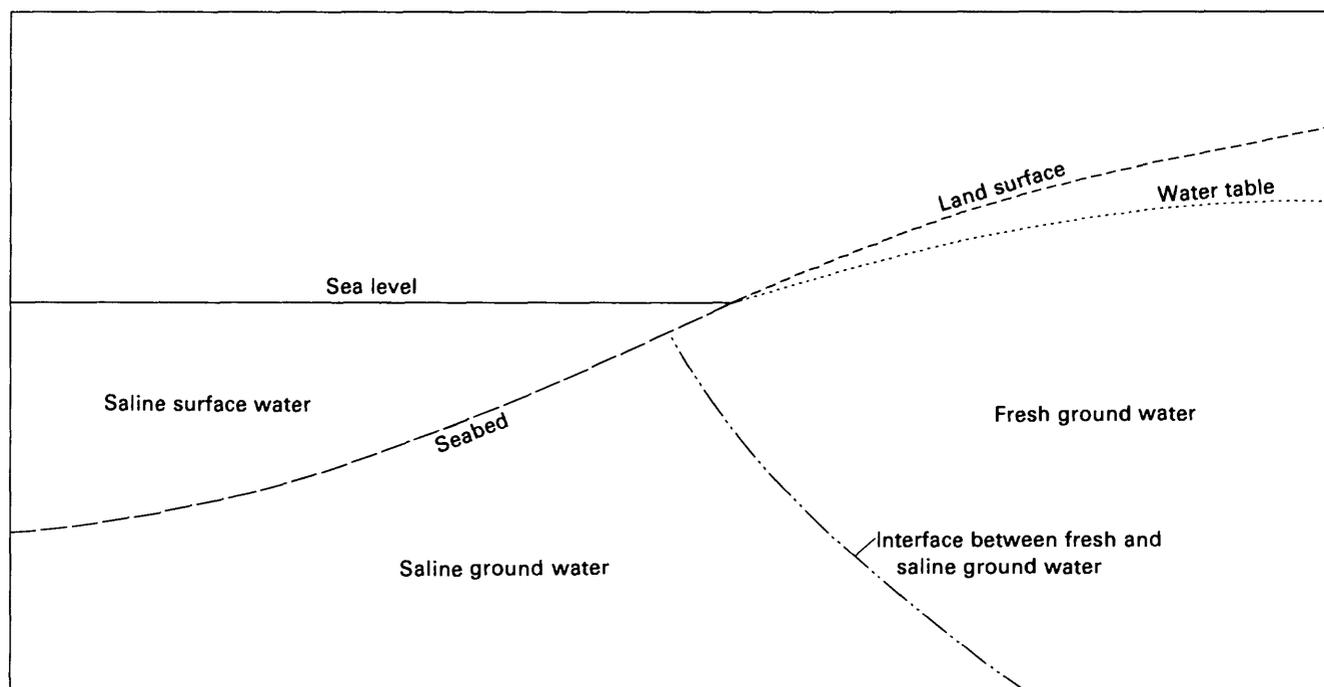


Figure 9. Schematic hydrologic section of the seaward boundary of fresh ground-water flow. (Along the seabed, fresh-water head must be equal to the equivalent freshwater head resulting from the saline surface-water column extending from sea level to the seabed. Along the interface, freshwater head balances saline-water head in the aquifer.)

Aquifer Recharge

Aquifer recharge is a combination of natural recharge from precipitation and artificial recharge from sources such as septic tanks and wastewater-treatment plants. The evapotranspiration rate was estimated for several locations on Cape Cod by the Thornthwaite method (Thornthwaite and Mather, 1957). This empirical determination is based on analysis of climatological data. U.S. Weather Bureau data for 1947–1976 were used in the analysis. The calculated average evapotranspiration rate and the average precipitation rate for the same period were used to estimate an average rate of natural recharge. The rates obtained compare favorably with estimates of average natural recharge on Cape Cod made by Strahler (1972) and Palmer (1977). For each node in the top layer, this value was added to the estimated artificial recharge rate, where applicable, to obtain an average total recharge rate.

Estimated rates of artificial recharge were based on the assumption that artificial recharge occurs in those areas where the water disposal site is not in the same model block as the source of water supply. For example, the disposal lagoons of a wastewater-treatment plant and residential areas serviced by both public water supply and onsite disposal systems were treated as areas of artificial recharge. No net artificial recharge or discharge was simu-

lated in areas serviced by both shallow domestic wells and onsite disposal systems.

The estimated artificial recharge rate was calculated on a town-by-town basis by making these assumptions:

1. Those areas identified in the draft Cape Cod 208 Water Quality Management Plan (U.S. Environmental Protection Agency, 1978) as wastewater-management problem areas (U.S. Environmental Protection Agency, 1978, map 4.1) are areas of artificial recharge because they represent areas of high concentration of septic tanks and are serviced by public water supply.
2. The rate of artificial recharge is uniform within these areas and proportional to the difference between the rate of withdrawal by the public water-supply system and any wastewater-treatment plant flow rates.

Another factor which influences the rates of aquifer recharge is the thickness of the unsaturated zone. Where the water table is close to land surface, as in low-lying swampy areas, proportionally more of the water that infiltrates the soil may be transpired by plants or evaporated, and consequently, less rainfall becomes a net addition to the ground-water reservoir than in areas with a thicker unsaturated zone. To approximate this effect for the WCAPE and ECAPE models, the calculated aquifer recharge was reduced proportionally (by approximately 10 percent) in low-lying areas near the coast. The finer grid spacing of

the ESTHM, WLFLT, and TRURO models allowed a closer approximation of the positions of streams and wetlands where the water table is close to the land surface. These natural discharge areas were modeled as constant head zones. Consequently, net recharge rates did not have to be reduced.

The final recharge distributions used are presented in the Supplemental Data section of this report.

CALIBRATION OF THE STEADY-STATE FLOW MODELS

Purpose and Procedure

An important objective of calibrating the steady-state model is to improve the conceptual model of the aquifer. Developing the conceptual model requires an understanding of the physical and functional nature of the aquifer. This includes identifying sources of recharge and discharge, rates and direction of flow, variation of aquifer properties and hydraulic head, and the relation of the aquifer to surface water. The simulation model numerically integrates the effects of these factors and the computed results are therefore internally consistent with all input data, allowing one to determine if any element of the conceptual model must be revised. After initial best estimates of the input data are made, model development is an evolutionary process in which results of previous simulations are interpreted to make modifications and adjustments to the model. The testing process of adjusting input data and comparing the calculated results with field observations (calibration) allows for a better understanding of the flow system and an improvement of the conceptual model.

To demonstrate that the flow models are reasonable, field observations must be closely correlated with model results. Field observations available for the Cape Cod aquifer system include observed water levels, position of the interface between fresh and saline ground water, and discharge measurements for selected stream segments. A map was prepared from records of water levels at selected long- and short-term observation sites that show the estimated average water table from 1963 to 1976 and the interface position at several locations.

The calibration procedure minimizes differences between observed and computed values by adjusting the input data (aquifer properties, boundary conditions, and hydraulic stresses). Because of the large number of interrelated factors affecting ground-water flow, this is a subjective procedure. The degree of allowable adjustment, however, of any parameter generally is directly related to the uncertainty of its value or specification. For example, withdrawal rates are well defined, and their values were

not adjusted. Hydraulic conductivity and transmissivity, however, are generally imprecisely known because lithologic variation is usually not well defined and because the methods by which they are determined are subject to many limitations. During the calibration, these values were adjusted by as much as 20 percent for those areas where the variation of lithology was believed to be gradual. These areas usually, but not always, coincided with the outwash plains. For areas where lithologic variation is known to be extreme, particularly the moraine, transmissivity and hydraulic conductivity values were adjusted by as much as a factor of 10.

Because the calibrated model is based on an interpretation of field observations, the accuracy of the model results is restricted by the accuracy of the interpretation. The water-table configuration and the position of the interface between fresh and saline ground water are estimates based on limited data. The control points for calibration of the models were approximately 150 sites for observation of water-table elevation; 27 sites for observation of head variation, chloride concentration, and specific conductance of ground water with depth; and 2 stream-segment discharge measurements. If any major corrections, revisions, or additions are made to the data base, the models should be recalibrated to reflect this better knowledge of the aquifer system.

Results

WCAPE Model

For the WCAPE model the agreement between observed average water levels and calculated water levels under steady-state conditions can be seen in table 4 and figure 10. Some of the disagreement, particularly in the higher elevations of the water table, is due to a lack of information on water levels and transmissivity in the moraine. Another reason for disagreement is that the observed average water level refers to the water level at the observation well itself, but the calculated water level refers to the water level at the center of the block which contains the well. Therefore, the discrepancy between calculated and observed water levels is greatest where the hydraulic gradient is steepest, a fact common to all five models.

Figures 11–13 show calculated altitudes of the interface between fresh and saline ground water for selected cross sections (see fig. 3). Measured chloride concentrations and a resistivity log of a test hole drilled in Mashpee are also included in figure 11. The resistivity measurement corresponding to the depth from which the lowermost sample in figure 11A was taken is approximately 750 ohm-meters, interpreted as being within the transition zone between fresh and saline ground water. On figure 11B, the nearly vertical resistivity line from approximately -310 ft to -400 ft represents a

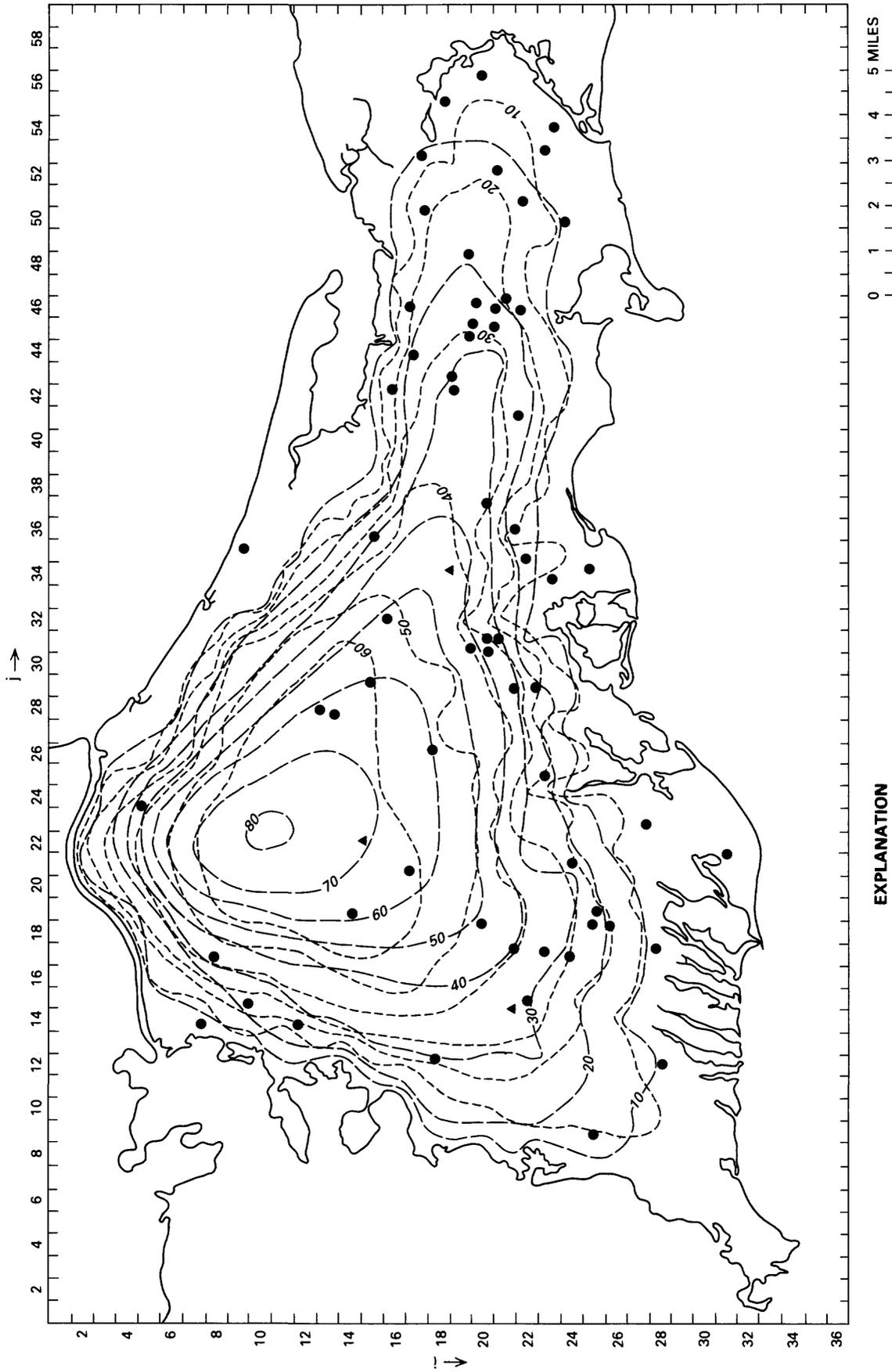


Figure 10. Calculated and observed average water table, 1963-1976, WCAPE model.

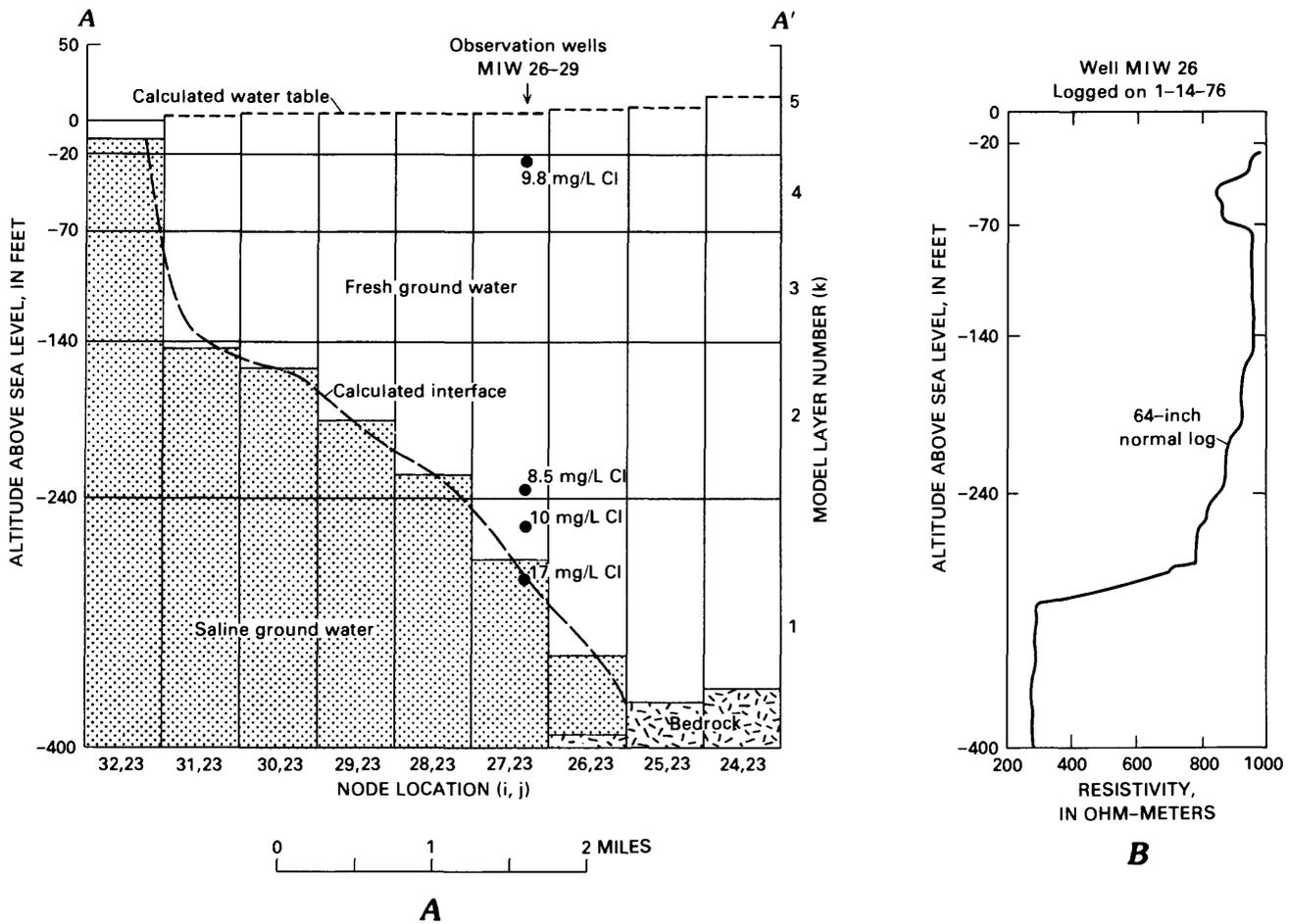
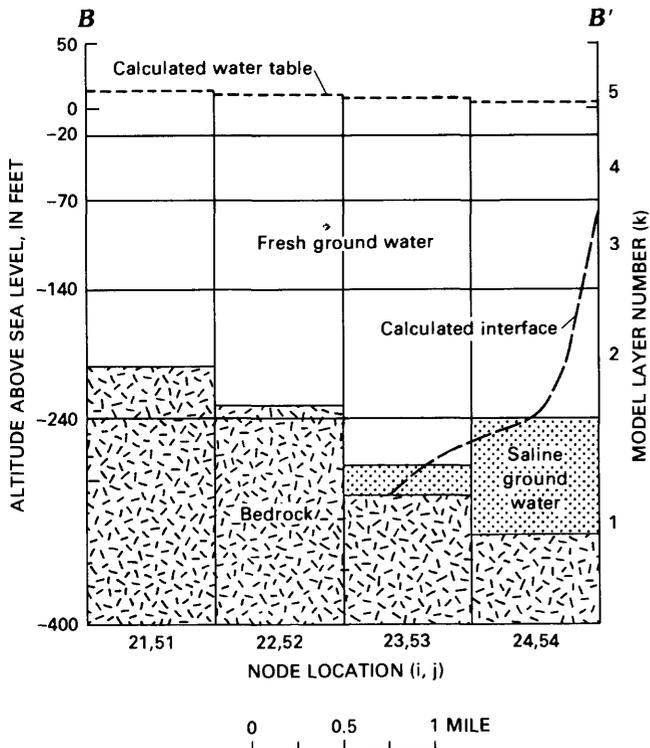


Figure 11. A, Calculated water table and interface between fresh and saline ground water, and chloride (Cl) content, in milligrams per liter (mg/L), of water samples collected from wells on June 20, 1979. B, Resistivity log for well MIW 26. Line of cross section A-A' is shown in figure 3.



chloride concentration equal to or greater than 15,000 mg/L, interpreted as representing the saline ground-water zone.

The lack of streamflow data hindered evaluation of the model's accuracy in simulating discharge to surface-water bodies. Stream discharges for two river segments in Falmouth were measured when streamflow on the Cape was about average and represented ground-water discharge. The measured discharges were 13 and 19 ft³/s, and the calculated discharges to the nodes chosen to represent these stream segments were 9 and 20 ft³/s. Collection of additional streamflow data will help improve the model and provide a better data base for future calibration.

A mass balance was calculated during each simulation. As part of these calculations, a hydrologic budget for the aquifer is computed and tabulated, providing a measure of the importance of each element of the budget. For the WCAPE model, the steady-state-recharge rate is 267 ft³/s; the rate of withdrawal by wells is 17 ft³/s; and discharge to the streams, marshes, and the ocean is 250 ft³/s.

Figure 12. Calculated water table and interface between fresh and saline ground water. Line of cross section B-B' is shown in figure 3.

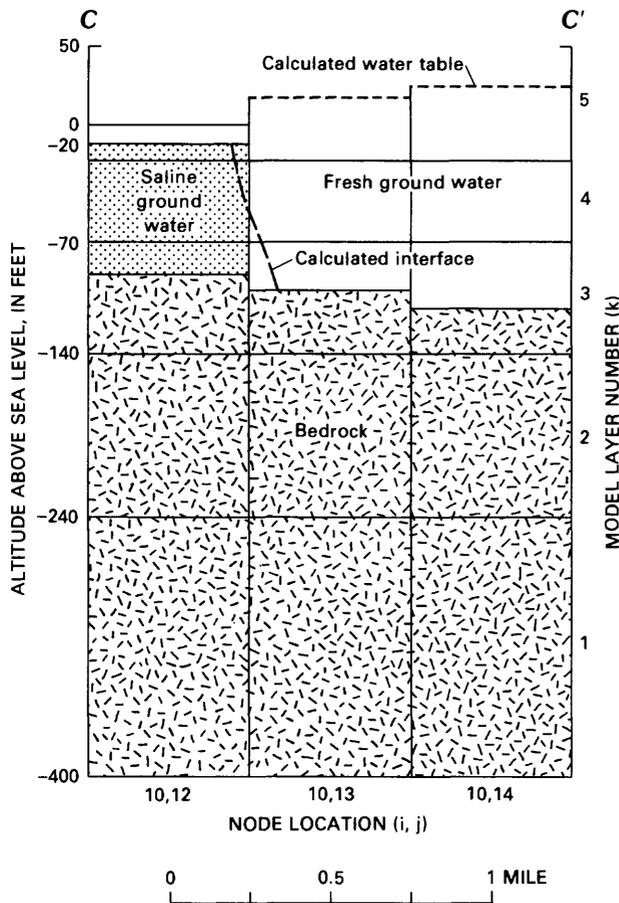


Figure 13. Calculated water table and interface between fresh and saline ground water. Line of cross section C-C' is shown in figure 3.

ECAPE Model

For the ECAPE model, agreement between the observed average and the calculated water levels can be seen in table 5 and figure 14. Disagreement is generally in the area where outwash plains overlie moraine deposits. The model was not able to simulate the 40-foot water-level contour in Dennis (fig. 14), nor the water levels observed deep in the aquifer in the northern part of Dennis. The model probably does not adequately approximate the complex stratigraphy associated with moraine deposits; improvement of this aspect of the model will require additional hydrogeologic information.

The calculated position of the interface between fresh and saline ground water for a selected cross section (D-D' in fig. 4) is shown in figure 15.

The mass balance calculated for the ECAPE model indicates that the steady-state recharge to the aquifer is 91 ft³/s; withdrawal by pumpage is 7 ft³/s; and discharge to streams, marshes, and the ocean is 84 ft³/s.

ESTHM Model

Table 6 and figure 16 show the agreement between the calculated and observed average water levels for the ESTHM model.

The mass-balance calculations indicate that the steady-state rate of aquifer recharge is 19 ft³/s. This rate is balanced by an equal rate of discharge to streams, marshes, and the ocean.

WLFLT Model

The agreement between calculated and observed average water levels for the WLFLT model is shown by table 7 and figure 17.

Mass-balance calculations indicate that the steady-state recharge rate, 23 ft³/s, is balanced by an equal rate of discharge to streams, marshes, and the ocean.

TRURO Model

The agreement between calculated and observed average water levels for the TRURO model is shown in table 8 and figure 18. Mass-balance calculations for the TRURO model indicate a steady-state recharge rate of 12.3 ft³/s; a withdrawal by pumping of 1.4 ft³/s; and a discharge rate to streams, marshes, and the ocean of 10.9 ft³/s.

In figure 19 the calculated position of the interface between fresh and saline ground water along cross section E-E' (in fig. 7) can be seen. Measured chloride concentrations in water samples from a group of wells near that line of section are included for comparison.

SUMMARY AND CONCLUSIONS

Cape Cod satisfies its water-supply demands almost entirely from the freshwater contained within the unconsolidated sediments of Pleistocene and Holocene age. Management of this ground-water resource requires an understanding of the behavior of this large complex aquifer system. Three-dimensional finite-difference ground-water-flow models were prepared for five areas of Cape Cod to provide information on the regional behavior of this system.

The models were developed using these premises: The boundary between fresh and saline ground water can be treated as an interface; the saline-water zone is static; and the natural discharge boundaries subdivide the aquifer system into individual small aquifers which can be modeled separately.

The models were calibrated for steady-state conditions by comparing calculated and observed water levels, position of the interface between fresh and saline ground water, and ground-water discharge rates, at approximately 150, 27, and 2 control points, respectively.

Table 4. Comparison of calculated nodal head values and observed average water levels for selected wells, 1963-1976, WCAPE model

| Node (i,j,k) | U.S. Geological Survey well No. | Water level, in feet above sea level | | Node (i,j,k) | U.S. Geological Survey well No. | Water level, in feet above sea level | |
|-----------------|---|---|---|-----------------|---|---|---|
| | | observed average | calculated at center of grid block | | | observed average | calculated at center of grid block |
| 5,24,5 | SDW 263 | 36.7 | 39.5 | 20,56,5 | YAW 111 | 7.5 | 5.0 |
| 7,14,5 | BHW 62 | 7.2 | 3.1 | 21,14,5 | FSP 4 | 34.9 | 33.3 |
| 8,17,5 | BHW 215 | 46.0 | 36.2 | 21,15,5 | FSP 4 | 34.9 | 35.6 |
| 9,15,5 | BHW 211 | 27.3 | 23.8 | 21,17,5 | FSW 167 | 41.3 | 40.8 |
| 9,35,5 | SDW 252 | 6.3 | 5.3 | 21,29,5 | AIW 60 | 35.3 | 34.2 |
| 12,14,5 | BHW 198 | 22.0 | 29.0 | 21,31,5 | AIW 264 | 26.4 | 27.7 |
| 13,28,5 | SDW 33 | 64.0 | 63.0 | 21,36,5 | AIW 183 | 13.2 | 19.2 |
| 13,28,5 | SDW 256 | 64.3 | 63.0 | 21,45,5 | AIW 301 | 26.6 | 22.5 |
| 14,19,5 | BHW 27 | 66.0 | 64.5 | 21,46,5 | AIW 302 | 19.7 | 19.1 |
| 14,22,5 | SDP 4 | 69.0 | 72.1 | 21,46,5 | AIW 300 | 24.3 | 19.1 |
| 15,22,5 | SDP 4 | 69.0 | 69.8 | 21,52,5 | YAW 89 | 18.0 | 11.3 |
| 15,29,5 | SDW 253 | 61.4 | 61.2 | 22,15,5 | FSW 169 | 32.6 | 32.0 |
| 15,36,5 | AIW 315 | 32.5 | 29.5 | 22,29,5 | AIW 298 | 24.3 | 26.8 |
| 16,32,5 | AIW 291 | 50.4 | 52.7 | 22,35,5 | AIW 255 | 17.0 | 14.9 |
| 16,42,5 | AIW 294 | 18.1 | 16.4 | 22,41,5 | AIW 306 | 27.2 | 19.0 |
| 17,21,5 | SDW 154 | 62.6 | 63.1 | 22,46,5 | AIW 230 | 18.4 | 13.0 |
| 17,44,5 | AIW 295 | 26.7 | 22.2 | 22,51,5 | YAW 104 | 16.6 | 11.4 |
| 17,46,5 | AIW 247 | 20.3 | 19.0 | 23,17,5 | FSW 168 | 35.7 | 32.4 |
| 17,50,5 | YAW 98 | 15.5 | 14.5 | 23,25,5 | MIW 21 | 23.2 | 20.4 |
| 17,53,5 | YAW 93 | 9.1 | 10.2 | 23,34,5 | AIW 260 | 6.8 | 9.1 |
| 18,12,5 | FSW 179 | 25.3 | 29.4 | 23,53,5 | YAW 117 | 7.4 | 7.4 |
| 18,26,5 | SDW 258 | 53.7 | 58.9 | 23,54,5 | YAW 96 | 5.2 | 6.2 |
| 18,34,5 | AIP 1 | 44.2 | 47.3 | 24,17,5 | FSW 173 | 29.8 | 27.8 |
| 18,55,5 | YAW 108 | 6.3 | 6.6 | 24,21,5 | MIW 19 | 26.9 | 23.8 |
| 19,31,5 | AIW 263 | 38.2 | 43.3 | 24,50,5 | YAW 94 | 7.7 | 7.2 |
| 19,42,5 | AIW 293 | 34.0 | 33.2 | 25, 9,5 | FSW 172 | 7.9 | 13.2 |
| 19,43,5 | AIW 292 | 34.1 | 31.6 | 25,18,5 | FSW 175 | 25.7 | 21.6 |
| 19,45,5 | AIW 287 | 30.1 | 25.0 | 25,19,5 | FSW 176 | 22.4 | 21.0 |
| 19,48,5 | YAW 85 | 22.7 | 19.5 | 25,34,5 | AIW 307 | 5.0 | 5.4 |
| 20,18,5 | SDW 262 | 47.6 | 47.8 | 26,18,5 | FSW 185 | 19.7 | 16.3 |
| 20,31,5 | AIW 267 | 33.7 | 36.8 | 27,23,5 | MIW 29 | 6.4 | 7.0 |
| 20,31,5 | AIW 313 | 27.5 | 36.8 | 28,12,5 | FSW 181 | 5.7 | 8.9 |
| 20,37,5 | AIW 254 | 34.9 | 31.6 | 28,17,5 | FSW 5 | 6.2 | 7.6 |
| 20,45,5 | AIW 289 | 29.6 | 25.3 | 31,22,5 | MIW 18 | 3.0 | 3.4 |
| 20,46,5 | AIW 297 | 24.8 | 20.6 | | | | |

Table 5. Comparison of calculated nodal head values and observed average water levels for selected wells, 1963-1976, ECAPE model

| Node (i,j,k) | U.S. Geological Survey well No. | Water level, in feet above sea level | | Node (i,j,k) | U.S. Geological Survey well No. | Water level, in feet above sea level | |
|-----------------|---|---|---|-----------------|---|---|---|
| | | observed average | calculated at center of grid block | | | observed average | calculated at center of grid block |
| 5,31,5 | OSW 22 | 4.2 | 5.2 | 11,25,5 | BMW 46 | 27.5 | 26.2 |
| 6,24,5 | BMW 22 | 19.8 | 12.9 | 12,10,5 | DGW 107 | 20.6 | 25.0 |
| 6,27,5 | OSW 25 | 15.1 | 14.0 | 12,12,5 | DGW 157 | 25.9 | 30.6 |
| 8,27,5 | OSP 1 | 21.7 | 21.0 | 12,18,5 | HJP 3 | 31.6 | 31.1 |
| 8,29,5 | OSP 3 | 7.5 | 15.1 | 12,19,5 | HJP 3 | 31.6 | 31.0 |
| 9,6,5 | DGW 170 | 6.4 | 6.5 | 12,20,5 | HJP 3 | 31.6 | 31.0 |
| 9,16,5 | BMW 44 | 26.4 | 25.3 | 13,12,5 | DGW 88 | 25.9 | 27.7 |
| 9,24,5 | BMP 3 | 28.0 | 27.2 | 13,18,5 | HJP 3 | 31.6 | 31.2 |
| 9,24,5 | BMW 21 | 27.2 | 27.2 | 13,20,5 | HJP 3 | 31.6 | 31.1 |
| 9,25,5 | BMP 5 | 25.8 | 26.1 | 13,21,5 | HJP 3 | 31.6 | 30.8 |
| 9,26,5 | BMP 5 | 25.8 | 24.6 | 14,10,5 | DGW 158 | 19.2 | 17.7 |
| 9,29,5 | OSP 3 | 7.5 | 14.8 | 14,12,5 | DGP 7 | 27.0 | 26.1 |
| 10,8,5 | DGW 100 | 44.7 | 25.8 | 14,15,5 | HJW 145 | 31.7 | 29.0 |
| 10,8,5 | DGW 114 | 28.3 | 25.8 | 14,25,5 | HJW 141 | 19.2 | 20.5 |
| 10,11,5 | DGW 143 | 30.5 | 33.7 | 15,22,5 | HJW 150 | 30.3 | 28.3 |
| 10,12,5 | DGW 123 | 32.8 | 33.6 | 15,25,5 | HJW 151 | 11.4 | 19.6 |
| 10,12,5 | DGW 144 | 30.6 | 33.6 | 16,12,5 | DGW 160 | 8.2 | 14.2 |
| 10,28,5 | OSW 24 | 18.5 | 17.8 | 17,18,5 | HJW 148 | 20.8 | 22.4 |
| 11,8,5 | DGW 124 | 16.9 | 21.9 | 17,26,5 | CGP 4 | 10.4 | 16.3 |
| 11,8,5 | DGP 4 | 24.5 | 21.9 | 18,25,5 | CGW 176 | 12.5 | 16.8 |
| 11,9,5 | DGW 135 | 16.7 | 25.6 | 18,25,5 | CGP 5 | 12.8 | 16.8 |
| 11,10,5 | DGP 2 | 20.3 | 29.3 | 18,26,5 | CGP 3 | 12.7 | 15.6 |
| 11,10,5 | DGP 3 | 20.9 | 29.3 | 18,28,5 | CGW 177 | 11.9 | 11.9 |
| 11,11,5 | DGW 146 | 25.7 | 31.9 | 19,23,5 | CGW 138 | 11.3 | 12.9 |
| 11,11,5 | DGP 6 | 27.6 | 31.9 | 19,27,5 | CGP 1 | 14.8 | 9.7 |
| 11,18,5 | BMW 45 | 31.4 | 30.6 | | | | |

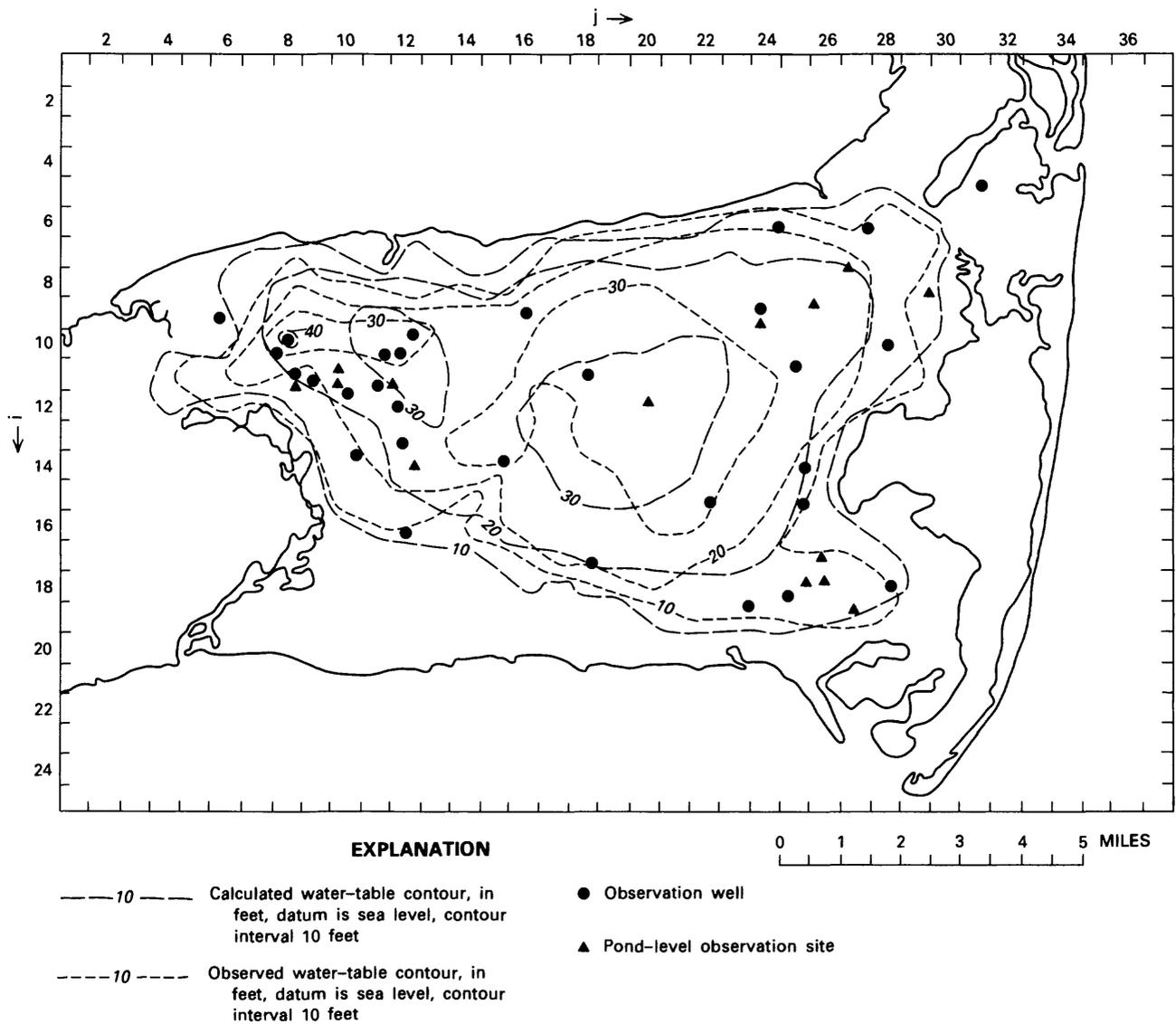


Figure 14. Calculated and observed average water table, 1963–1976, ECAPE model.

The accuracy of the model results is limited by the accuracy of the input data that describe aquifer properties, boundary conditions, and recharge rates. As additional data become available, the models should be recalibrated to improve model accuracy. Continued monitoring of water-level changes, streamflow, and movement of the boundary between fresh and saline ground water in response to natural and man-caused changes in hydrologic stresses will help improve the aquifer models and make them more useful.

The large scale of the models precludes detailed analyses of hydrologic conditions for local areas. More detailed, smaller scale models of local areas can be constructed using similar principles if sufficient data are available.

Mass-balance calculations for the five modeled areas indicate a total steady-state natural and artificial recharge rate of 412 ft³/s; a total rate of withdrawal by wells of 25 ft³/s; and a total discharge rate to streams, marshes, and the ocean of 387 ft³/s.

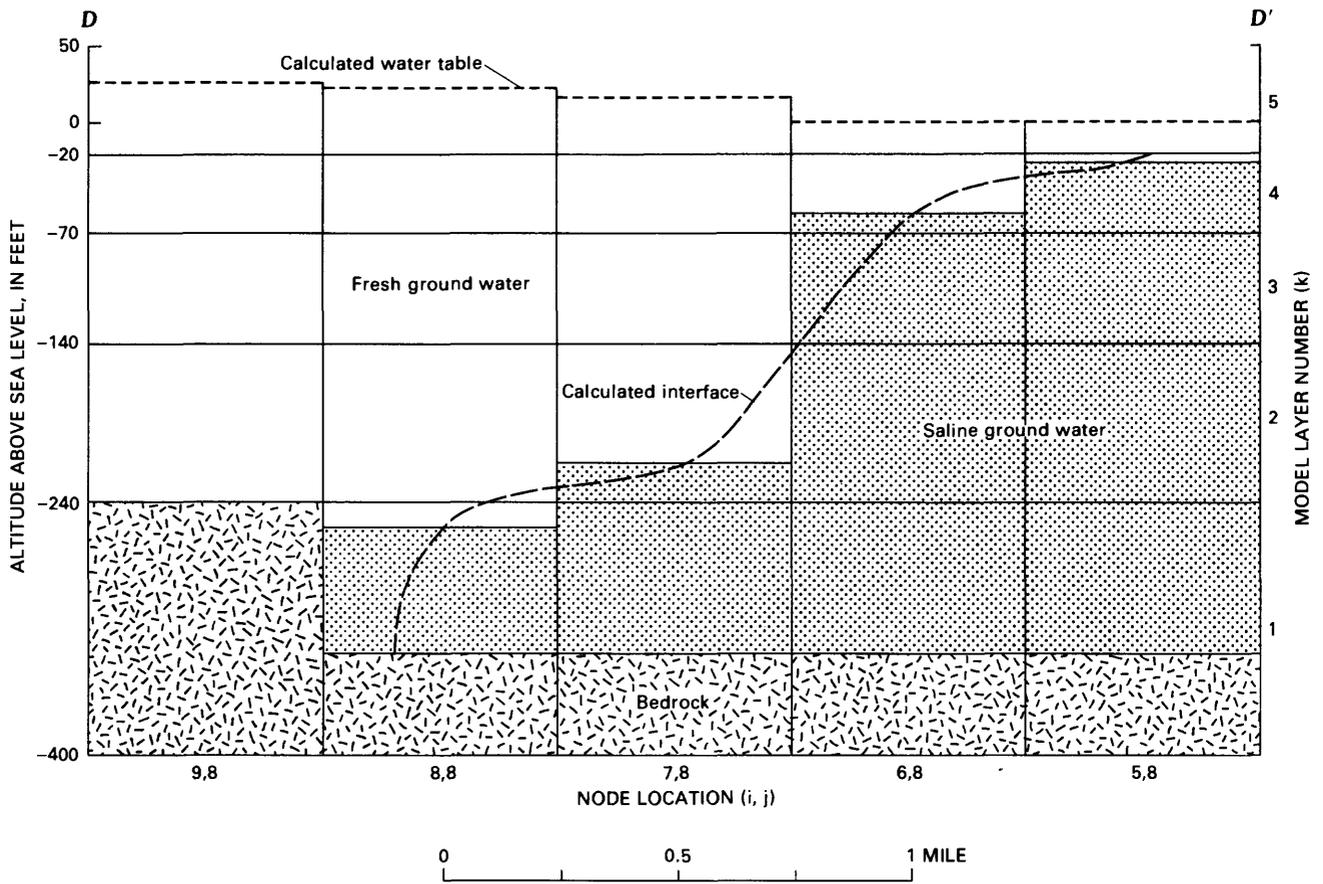
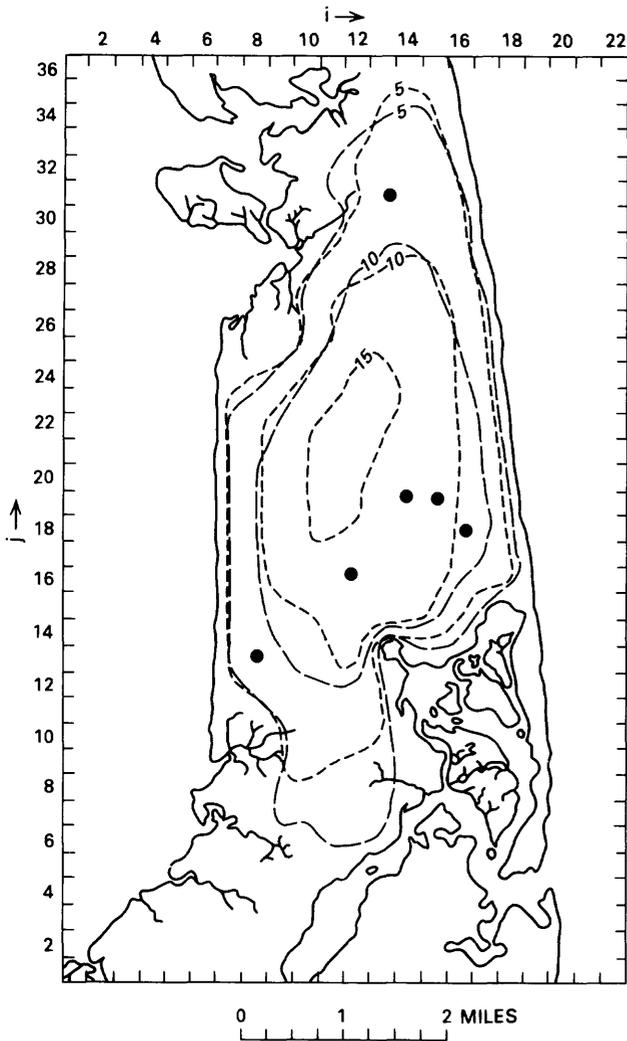


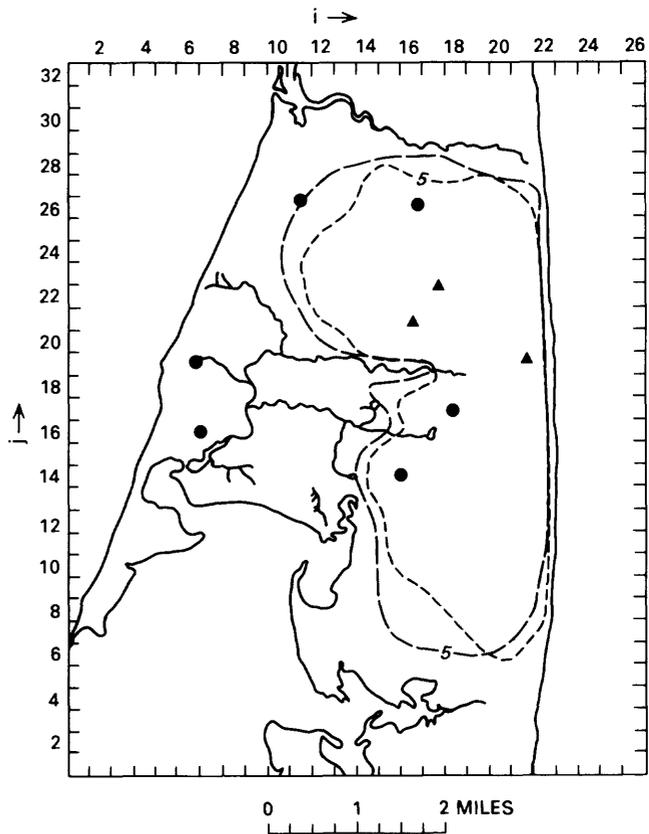
Figure 15. Calculated water table and interface between fresh and saline ground water. Line of cross section *D-D'* is shown in figure 4.



EXPLANATION

- 5 — Calculated water-table contour, in feet, datum is sea level, contour interval 5 feet
- - - 5 - - - Observed average water-table contour, in feet, datum is sea level, contour interval 5 feet
- Observation well

Figure 16. Calculated and observed average water table, 1963–1976, ESTHM model.



EXPLANATION

- 5 — Calculated water-table contour, in feet, datum is sea level, contour interval 5 feet
- - - 5 - - - Observed average water-table contour, in feet, datum is sea level, contour interval 5 feet
- Observation well
- ▲ Pond-level observation site

Figure 17. Calculated and observed average water table, 1963–1976, WLFLT model.

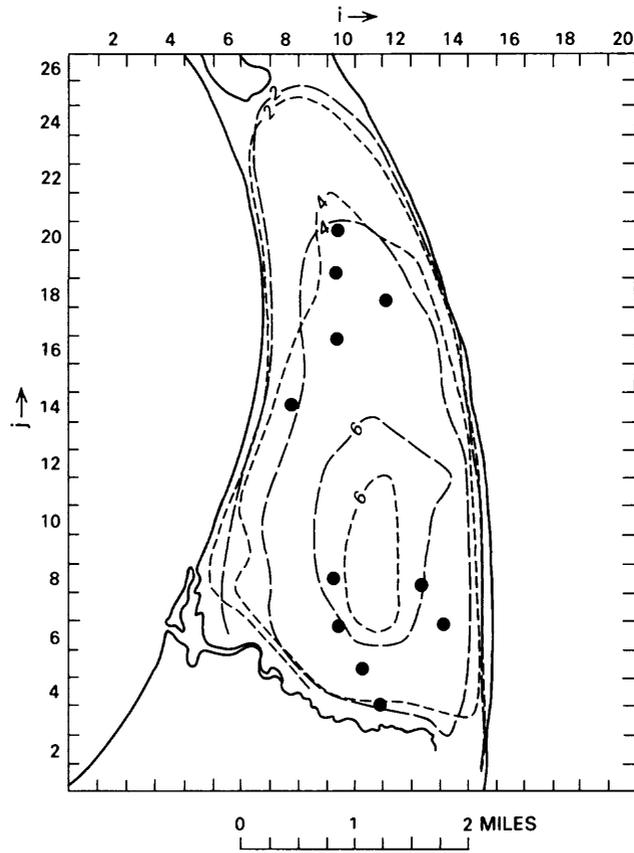


Figure 18. Calculated and observed average water table, 1963-1976, TRURO model.

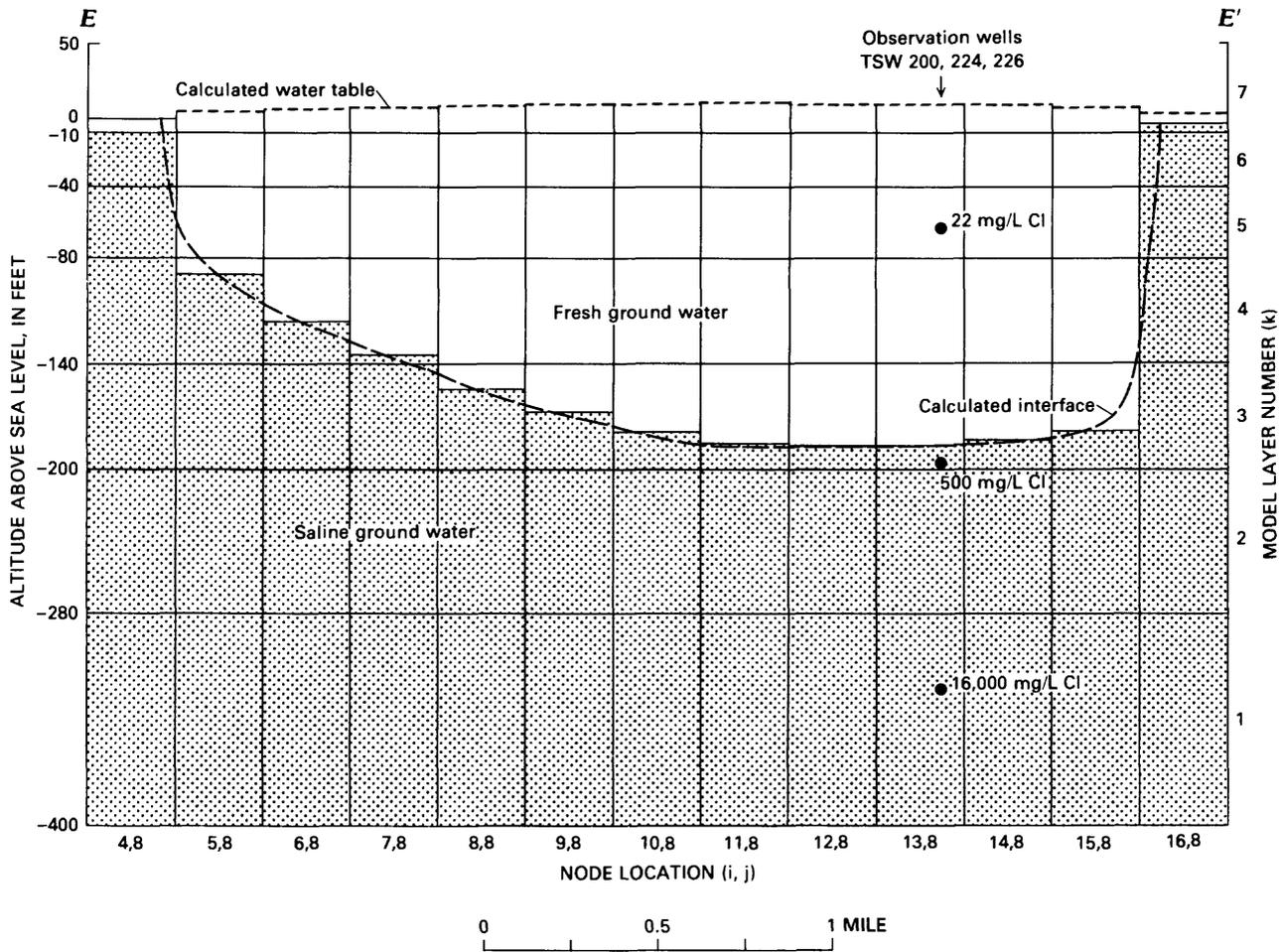


Figure 19. Calculated water table and interface between fresh and saline ground water, and chloride (Cl) content, in milligrams per liter (mg/L), of water samples collected from wells on December 5, 1978. Line of cross section E-E' is shown in figure 7.

Table 6. Comparison of calculated nodal head values and observed average water levels for selected wells, 1963-1976, ESTHM model

| Node (i,j,k) | U.S. Geological Survey well No. | Water level, in feet above sea level | | Node (i,j,k) | U.S. Geological Survey well No. | Water level, in feet above sea level | |
|-----------------|---|---|---|-----------------|---|---|---|
| | | observed average | calculated at center of grid block | | | observed average | calculated at center of grid block |
| 8,13,7 | EGW 37 | 8.0 | 8.2 | 14,19,7 | EGW 36 | 13.5 | 13.9 |
| 12,16,7 | EGW 39 | 13.8 | 13.5 | 15,19,7 | EGW 32 | 12.8 | 13.1 |
| 13,31,7 | WNW 17 | 8.7 | 8.3 | 16,18,7 | EGW 40 | 8.6 | 11.7 |

Table 7. Comparison of calculated nodal head values and observed average water levels for selected wells, 1963-1976, WLFLT model.

| Node (i,j,k) | U.S. Geological Survey well No. | Water level, in feet above sea level | | Node (i,j,k) | U.S. Geological Survey well No. | Water level, in feet above sea level | |
|-----------------|---|---|---|-----------------|---|---|---|
| | | observed average | calculated at center of grid block | | | observed average | calculated at center of grid block |
| 6,16,7 | WNW 78 | 2.7 | 3.4 | 16,26,7 | TSW 198 | 7.6 | 7.1 |
| 11,26,7 | TSW 216 | 4.1 | 5.3 | 17,22,7 | TSP 16 | 7.6 | 7.9 |
| 15,14,7 | WNW 30 | 6.6 | 6.7 | 18,17,7 | WNW 34 | 8.0 | 8.0 |
| 16,14,7 | WNW 30 | 6.6 | 7.4 | 21,19,7 | TSP 18 | 6.4 | 6.5 |
| 16,21,7 | TSP 17 | 6.8 | 7.2 | | | | |

Table 8. Comparison of calculated nodal head values and observed average water levels for selected wells, 1963-1976, TRURO model

| Node (i,j,k) | U.S. Geological Survey well No. | Water level, in feet above sea level | | Node (i,j,k) | U.S. Geological Survey well No. | Water level, in feet above sea level | |
|-----------------|---|---|---|-----------------|---|---|---|
| | | observed average | calculated at center of grid block | | | observed average | calculated at center of grid block |
| 8,14,7 | TSW 157 | 4.4 | 3.8 | 11,5,7 | TSW 218 | 5.2 | 5.4 |
| 10,6,7 | TSW 176 | 5.6 | 5.6 | 12,3,7 | TSW 181 | 4.7 | 3.6 |
| 10,8,7 | TSW 170 | 5.9 | 6.1 | 12,4,7 | TSW 181 | 4.7 | 4.9 |
| 10,16,7 | TSW 89 | 4.4 | 5.2 | 12,18,7 | TSW 134 | 4.9 | 4.5 |
| 10,17,7 | TSW 89 | 4.4 | 5.0 | 13,8,7 | TSW 203 | 5.7 | 6.0 |
| 10,19,7 | TSW 136 | 4.3 | 4.6 | 14,6,7 | TSW 174 | 5.6 | 5.0 |
| 10,20,7 | TSW 126 | 4.2 | 4.3 | | | | |

REFERENCES

- Burns, A. W., Frimpter, M. H., and Willey, R. E., 1975, Evaluation of data availability and examples of modeling for ground-water management on Cape Cod, Massachusetts: U.S. Geological Survey Water Resources Investigation 16-75, 22 p.
- Getzen, R. T., 1977, Analog-model analysis of regional three-dimensional flow in the ground-water reservoir of Long Island, New York: U.S. Geological Survey Professional Paper 982, 49 p.
- Guswa, J. H., and Londquist, C. J., 1976, Potential for development of ground water at a test site near Truro, Massachusetts: U.S. Geological Survey Open-File Report 76-614, 22 p.
- Hubbert, M. K., 1940, The theory of ground-water motion: *Journal of Geology*, v. 48, no. 8, p. 785-944.
- Meyer, William, Reussow, J. P., Gillies, D. C., and Shampine, W. J., 1975, Availability of ground water in Marion County, Indiana: U.S. Geological Survey Open-File Report 75-312, 87 p.
- Oldale, R. N., 1969, Seismic investigations on Cape Cod, Martha's Vineyard, and Nantucket, Massachusetts, and a topographic map of the basement surface from Cape Cod Bay to the Islands: U.S. Geological Survey Professional Paper 650-B, p. B122-B127.
- , 1976, Notes on the generalized geologic map of Cape Cod: U.S. Geological Survey Open-File Report 76-765, 23 p.
- Palmer, C. D., 1977, Hydrogeological implications of various wastewater management proposals for the Falmouth area of Cape Cod, Massachusetts: Pennsylvania State University, unpublished M.S. thesis, 142 p.
- Prickett, T. A., 1975, Modeling techniques for groundwater evaluation, in Chow, V. T., ed., *Advances in hydrosciences*, v. 10: New York, Academic Press, p. 1-143.
- Stone, H. L., 1968, Iterative solution of implicit approximations of multidimensional partial differential equations: *Society of Industrial and Applied Mathematics, Journal of Numerical Analysis*, v. 5, no. 3, p. 530-558.
- Strahler, A. N., 1972, The environmental impact of ground water use on Cape Cod: Impact Study III, Association for the Preservation of Cape Cod: Orleans, Mass., 68 p.
- Theis, C. V., 1963, Estimating the transmissibility of a water-table aquifer from the specific capacity of a well, in Bentall, Ray, *Methods of determining permeability, transmissibility, and drawdown*: U.S. Geological Survey Water-Supply Paper 1536-I, p. 332-336.
- Thornthwaite, C. W., and Mather, J. R., 1957, Instructions and tables for computing potential evapotranspiration and the water balance: *Drexel Institute of Technology Publications in Climatology*, v. 10, no. 3, p. 185-311.
- Trescott, P. C., 1975, Documentation of finite-difference model for simulation of three-dimensional ground-water flow: U.S. Geological Survey Open-File Report 75-438, 30 p.
- Trescott, P. C., and Larson, S. P., 1976, Supplement to Open-File Report 75-438, Documentation of finite-difference model for simulation of three-dimensional ground-water flow: U.S. Geological Survey Open-File Report 76-591, 17 p.
- Trescott, P. C., Pinder, G. F., and Larson, S. P., 1976, Finite-difference model for aquifer simulation in two dimensions with results of numerical experiments: U.S. Geological Survey Techniques of Water Resources Investigations, Chapter C1, Book 7, p. 42.
- U.S. Environmental Protection Agency, 1978, Water Quality Management Plan/EIS for Cape Cod: Draft plan/Environmental Impact Statement, 397 p.

SUPPLEMENTAL DATA-MODEL DATA, INPUT DOCUMENTATION, AND SOURCE CODE

Table 9. Values for transmissivity, average hydraulic conductivity, and ratio of lateral to vertical hydraulic conductivity used in calibrated models.

(*Values apply uniformly within the modeled area identified in figs. 3-7 of the report.)

| Model | Layer | Transmissivity (T) for each node in layer (ft ² /d) | Average lateral hydraulic conductivity for each node in layer (ft/d) | Ratio of lateral to vertical hydraulic conductivity |
|-------|-------|---|---|---|
| WCAPE | 1 | figure 20 | T for node/160 | *10 |
| | 2 | figure 21 | T for node/100 | figure 25 |
| | 3 | figure 22 | T for node/70 | figure 26 |
| | 4 | figure 23 | T for node/50 | figure 27 |
| | 5 | NA | figure 24 | figure 28 |
| ECAPE | 1 | figure 29 | T for node/160 | *10 |
| | 2 | figure 30 | T for node/100 | figure 34 |
| | 3 | figure 31 | T for node/70 | figure 35 |
| | 4 | figure 32 | T for node/50 | figure 36 |
| | 5 | NA | figure 33 | figure 37 |
| ESTHM | 1 | *20,250 | *135 | *10 |
| | 2 | *20,250 | *135 | *10 |
| | 3 | *13,500 | *135 | *10 |
| | 4 | *12,150 | *135 | *10 |
| | 5 | figure 38 | T for node/60 | figure 41 |
| | 6 | figure 39 | T for node/40 | figure 42 |
| | 7 | NA | figure 40 | figure 43 |
| WLFLT | 1 | *9,600 | *80 | *10 |
| | 2 | *6,400 | *80 | *10 |
| | 3 | *4,800 | *80 | *10 |
| | 4 | figure 44 | T for node/60 | *10 |
| | 5 | figure 45 | T for node/40 | figure 48 |
| | 6 | figure 46 | T for node/30 | figure 49 |
| | 7 | NA | figure 47 | figure 50 |
| TRURO | 1 | *10,800 | *90 | *10 |
| | 2 | *7,200 | *90 | *10 |
| | 3 | *5,400 | *90 | *10 |
| | 4 | *7,200 | *120 | *10 |
| | 5 | figure 51 | T for node/40 | figure 54 |
| | 6 | figure 52 | T for node/30 | figure 55 |
| | 7 | NA | figure 53 | *10 |

TK arrays, ESTHM, layers 1-2, 2-3, and 3-4 were multiplied by 0.45.
TK array, WLFLT, layers 3-4 was multiplied by 0.89.
TK arrays, TRURO, layers 1-2, 2-3, and 3-4 were multiplied by 0.9.

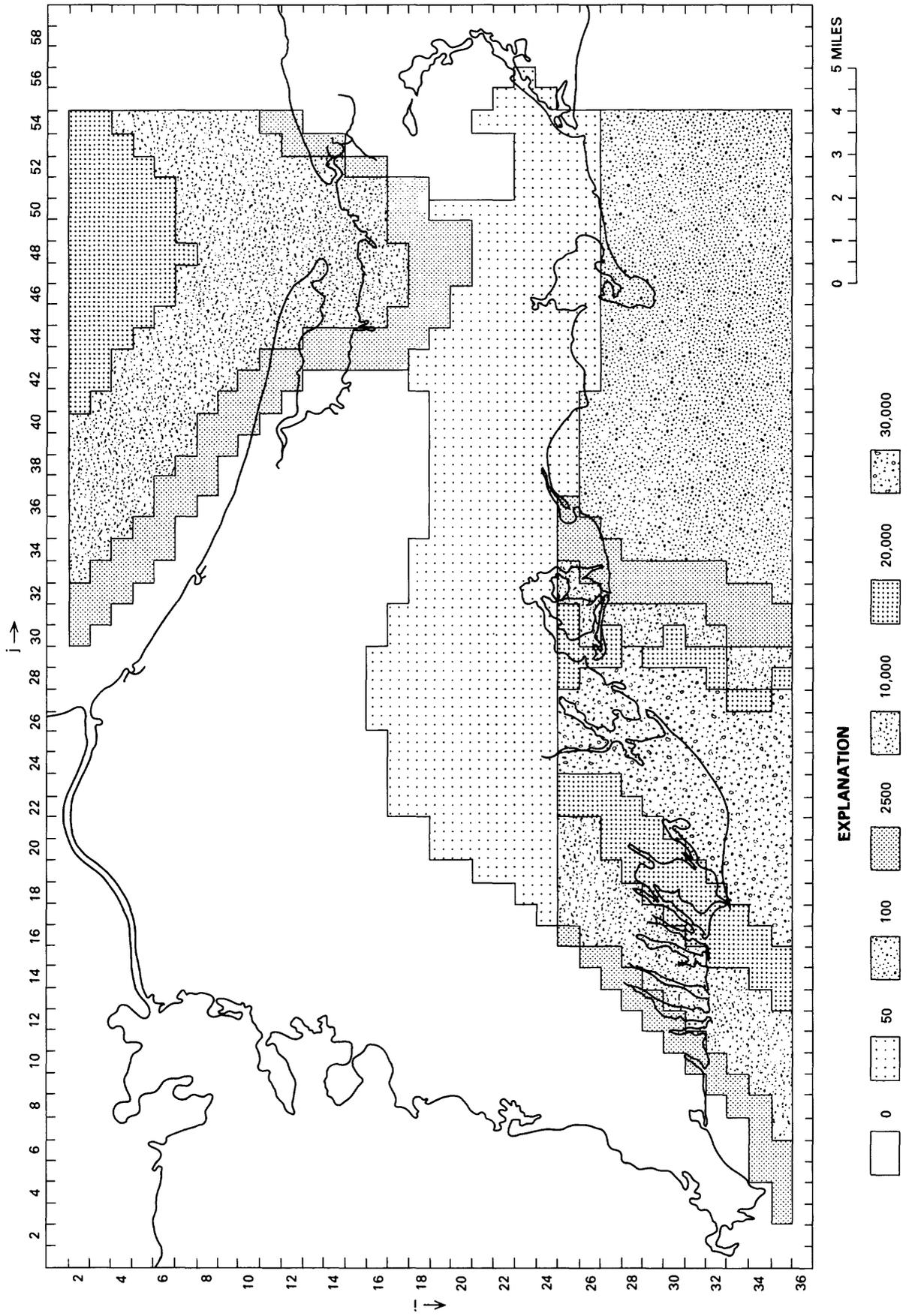


Figure 20. Transmissivity, in feet squared per day, WCAPE model, layer 1.

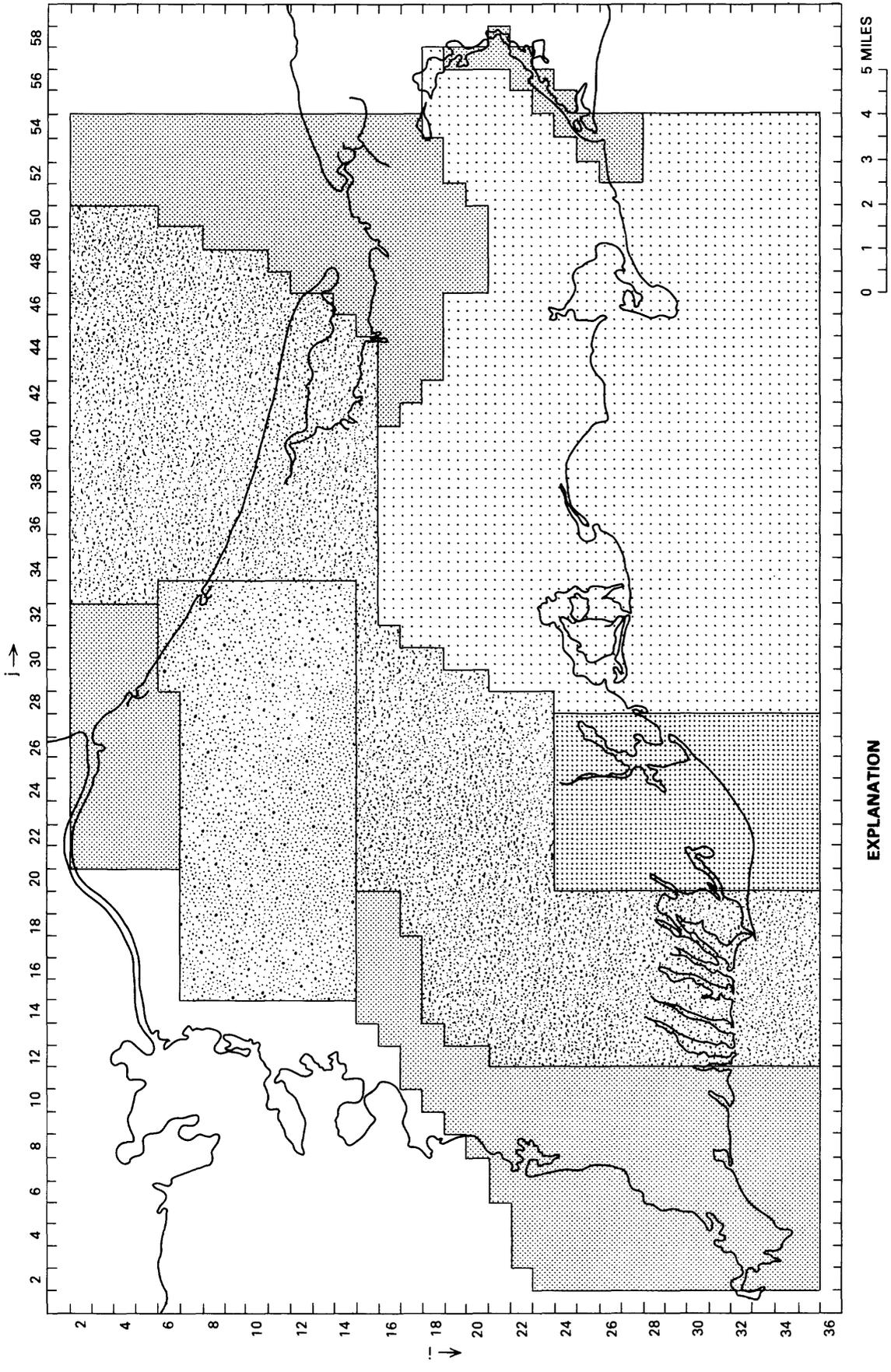


Figure 21. Transmissivity, in feet squared per day, WCAPE model, layer 2.

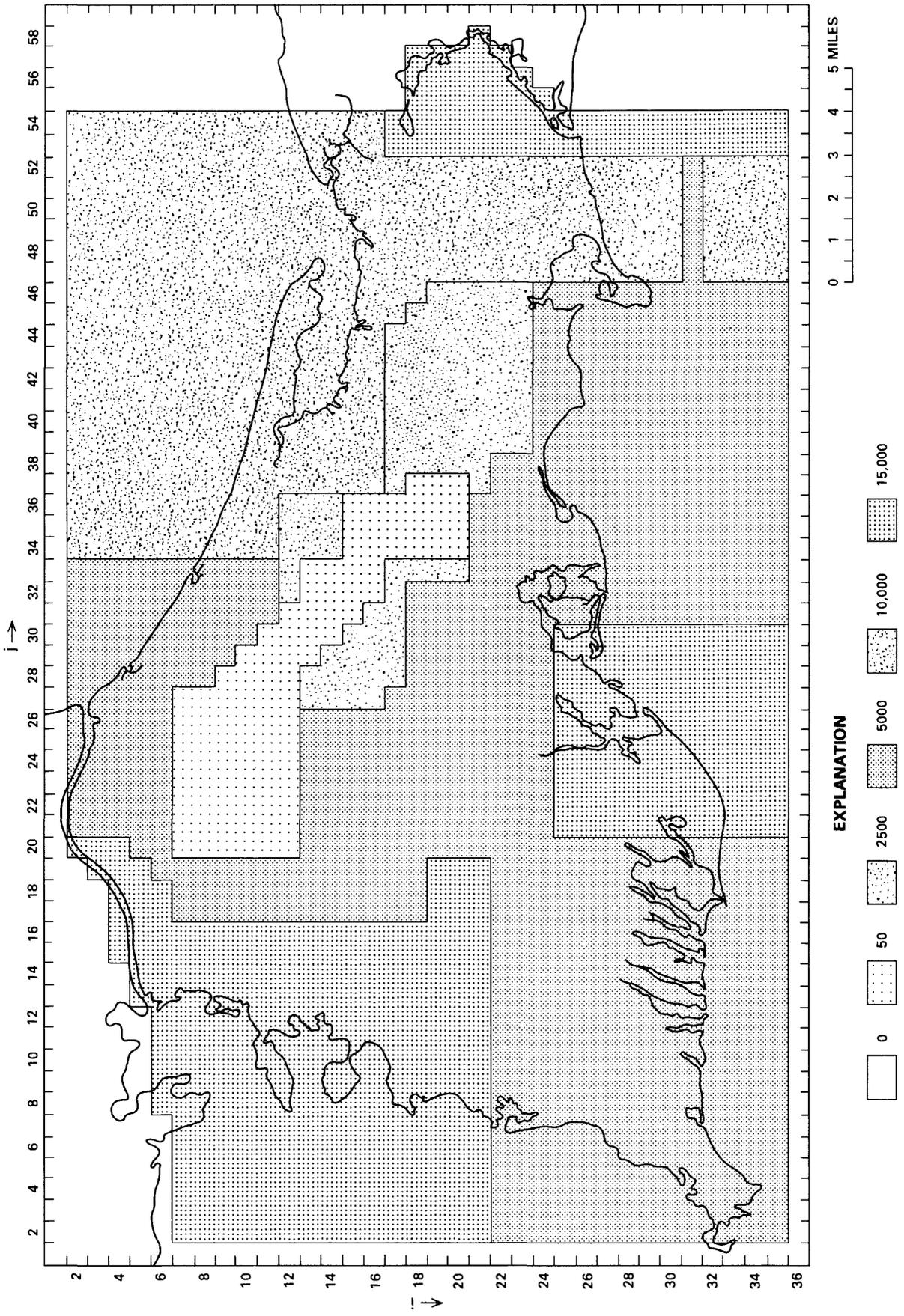


Figure 22. Transmissivity, in feet squared per day, WCAPE model, layer 3.

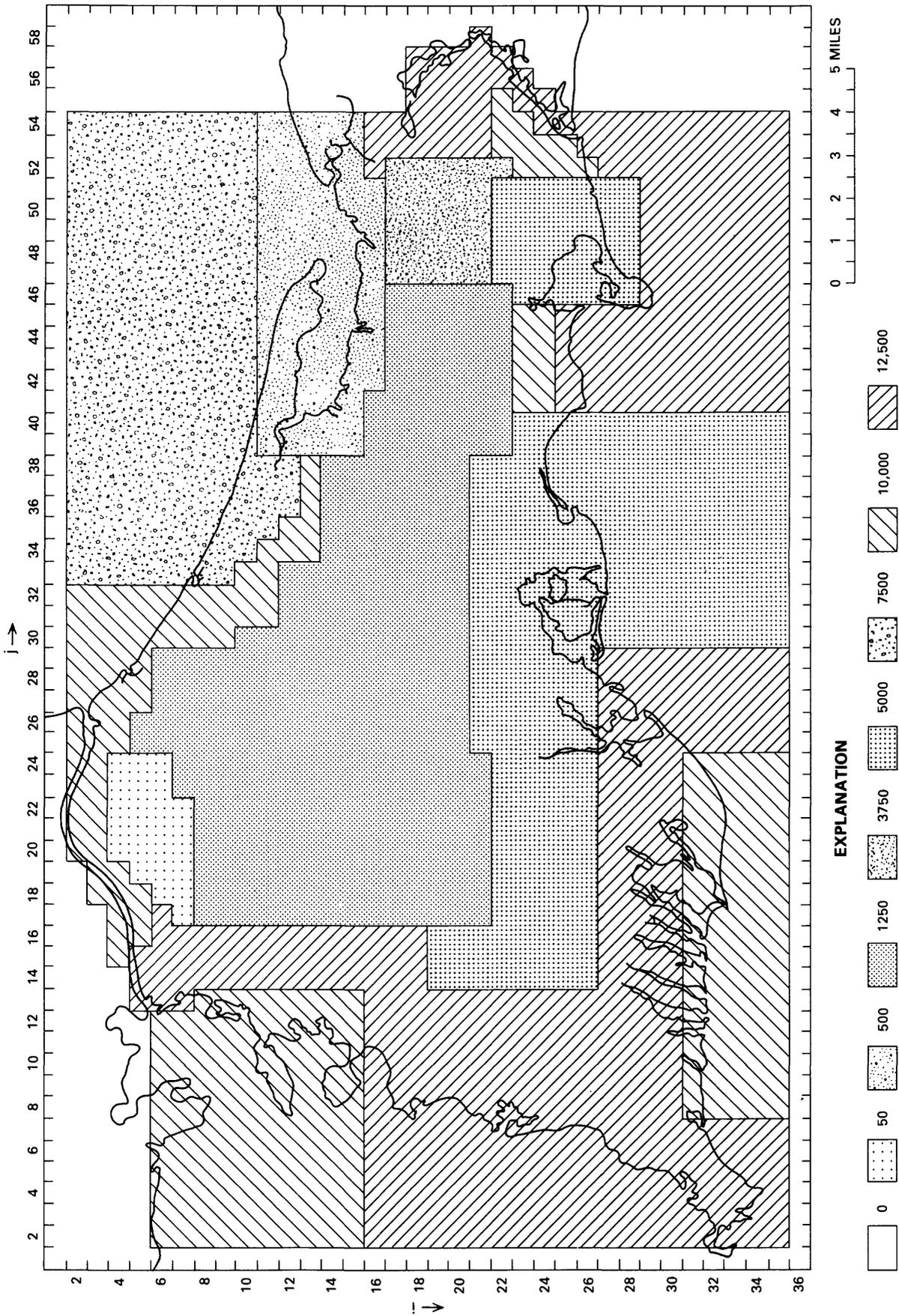


Figure 23. Transmissivity, in feet squared per day, WCAPE model, layer 4.

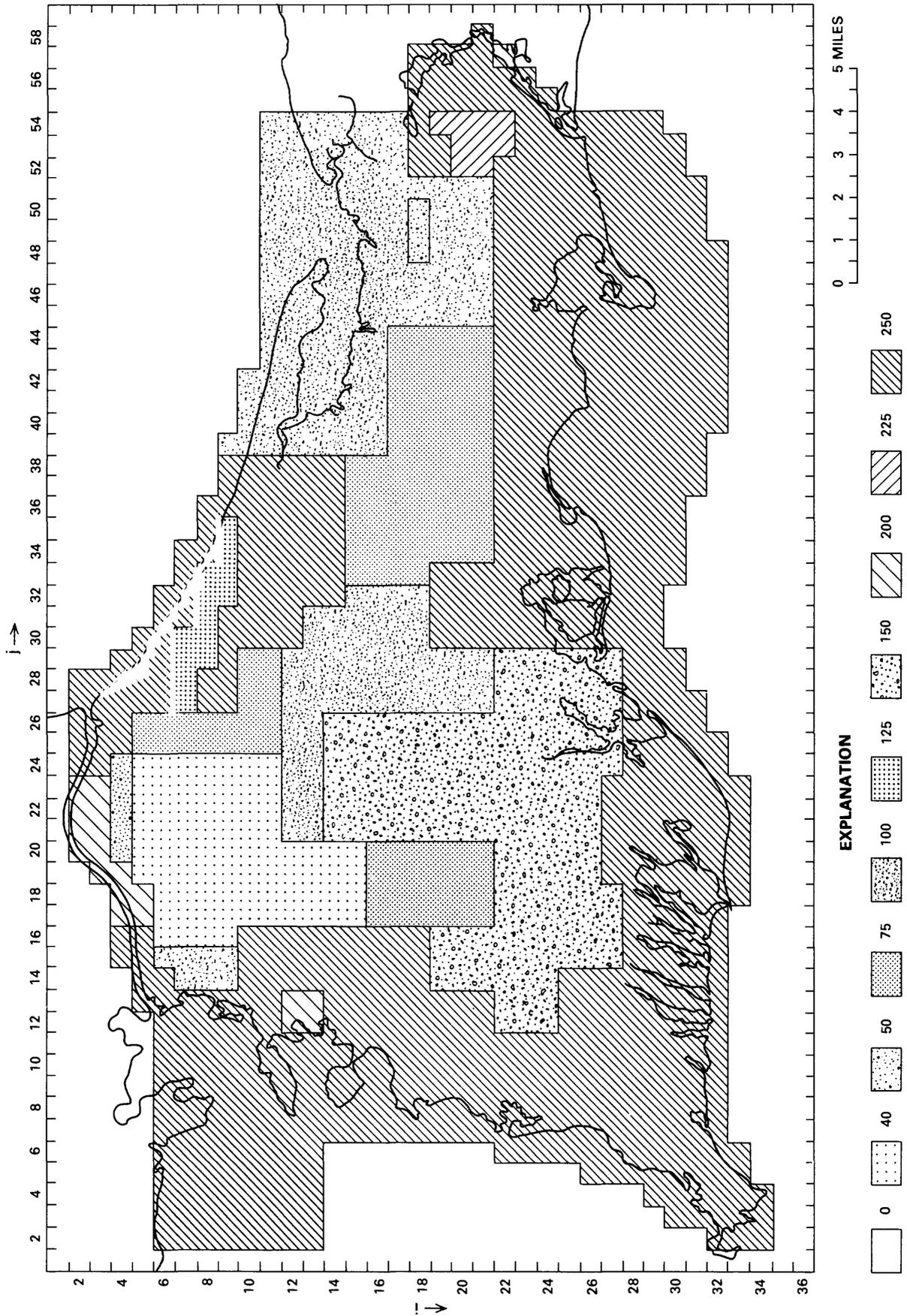


Figure 24. Hydraulic conductivity, in feet per day, WCAPE model, layer 5.

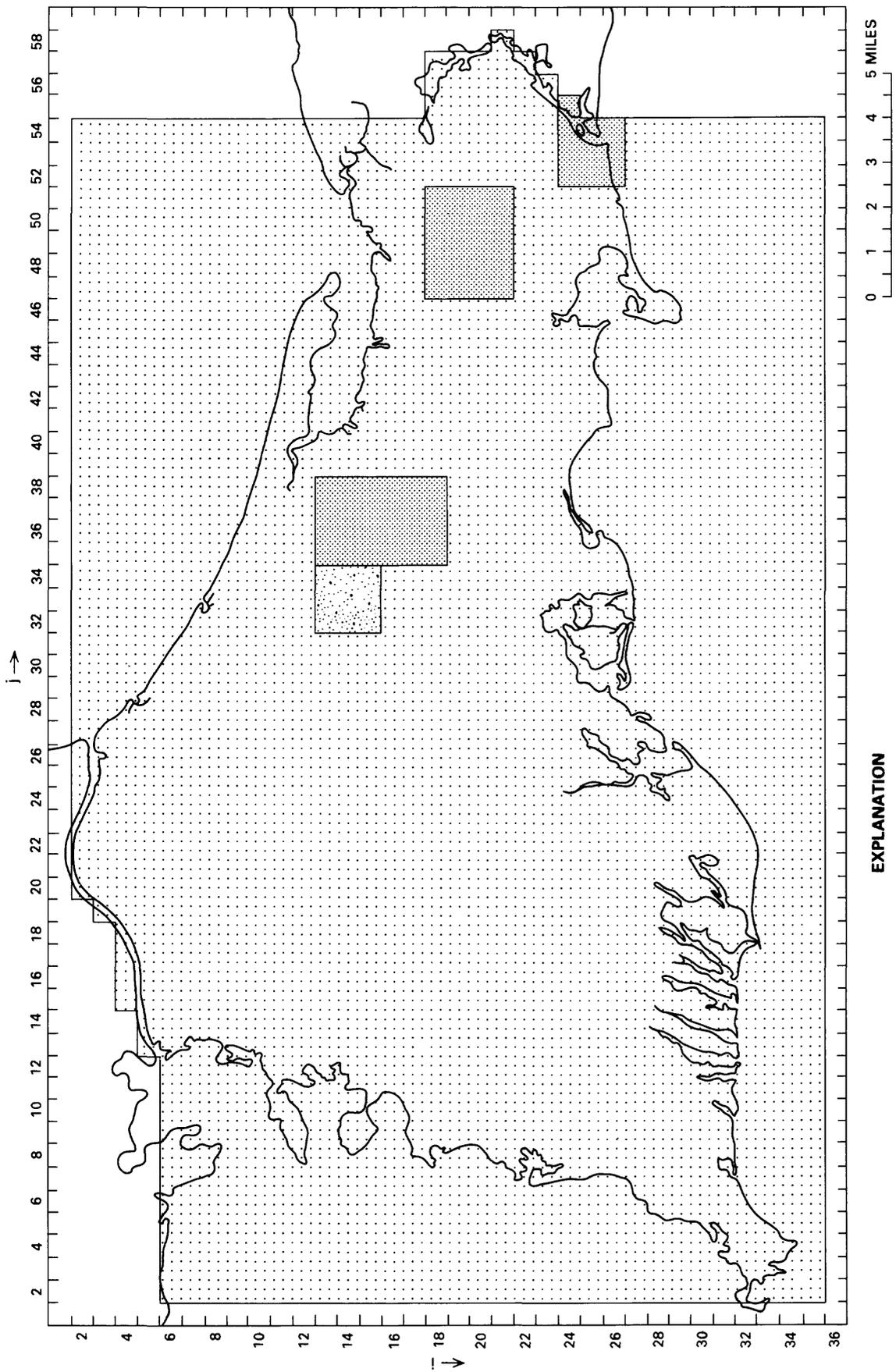


Figure 25. Ratio of lateral to vertical hydraulic conductivity, WCAPE model, layer 2.

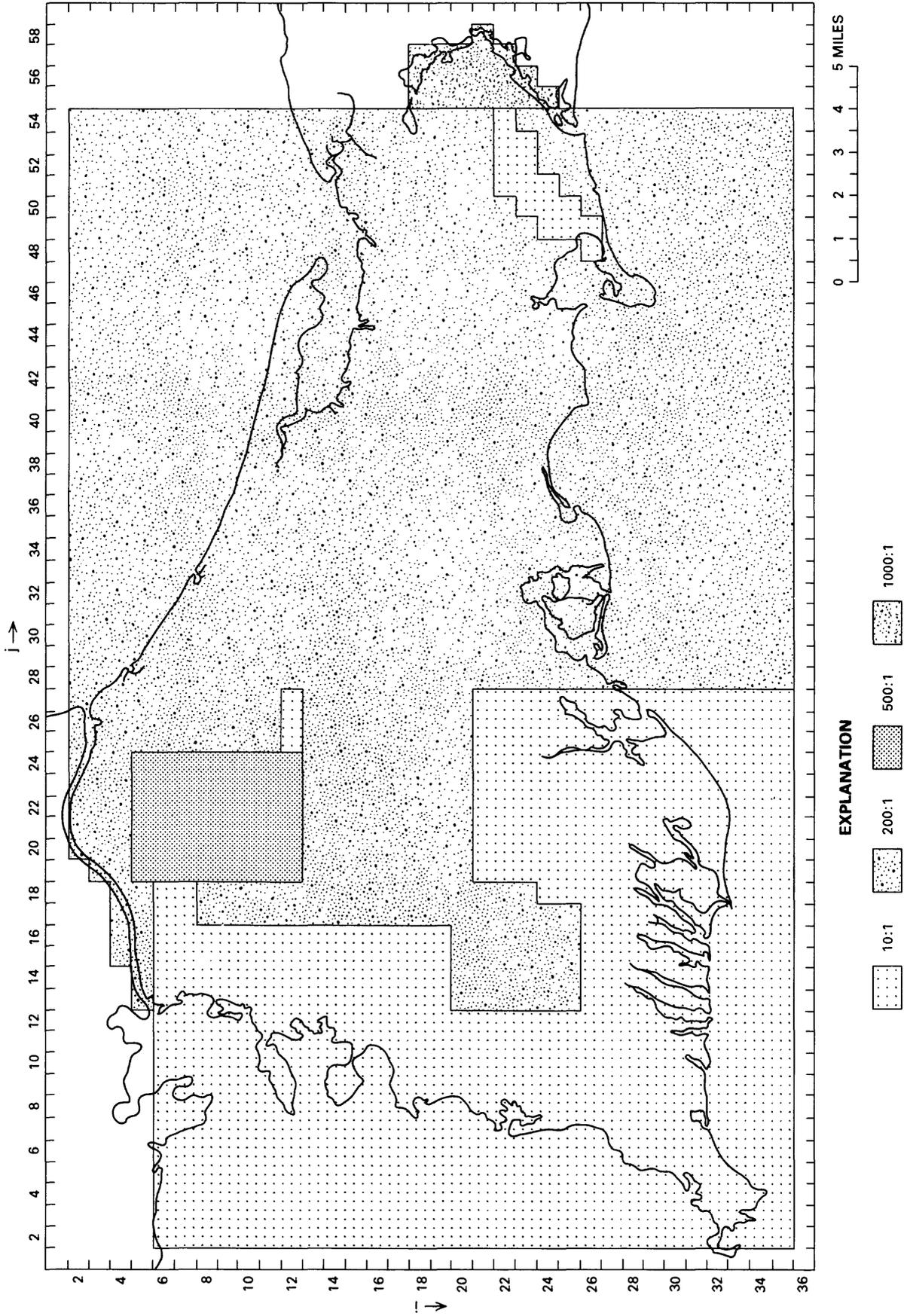


Figure 26. Ratio of lateral to vertical hydraulic conductivity, WCAPE model, layer 3.

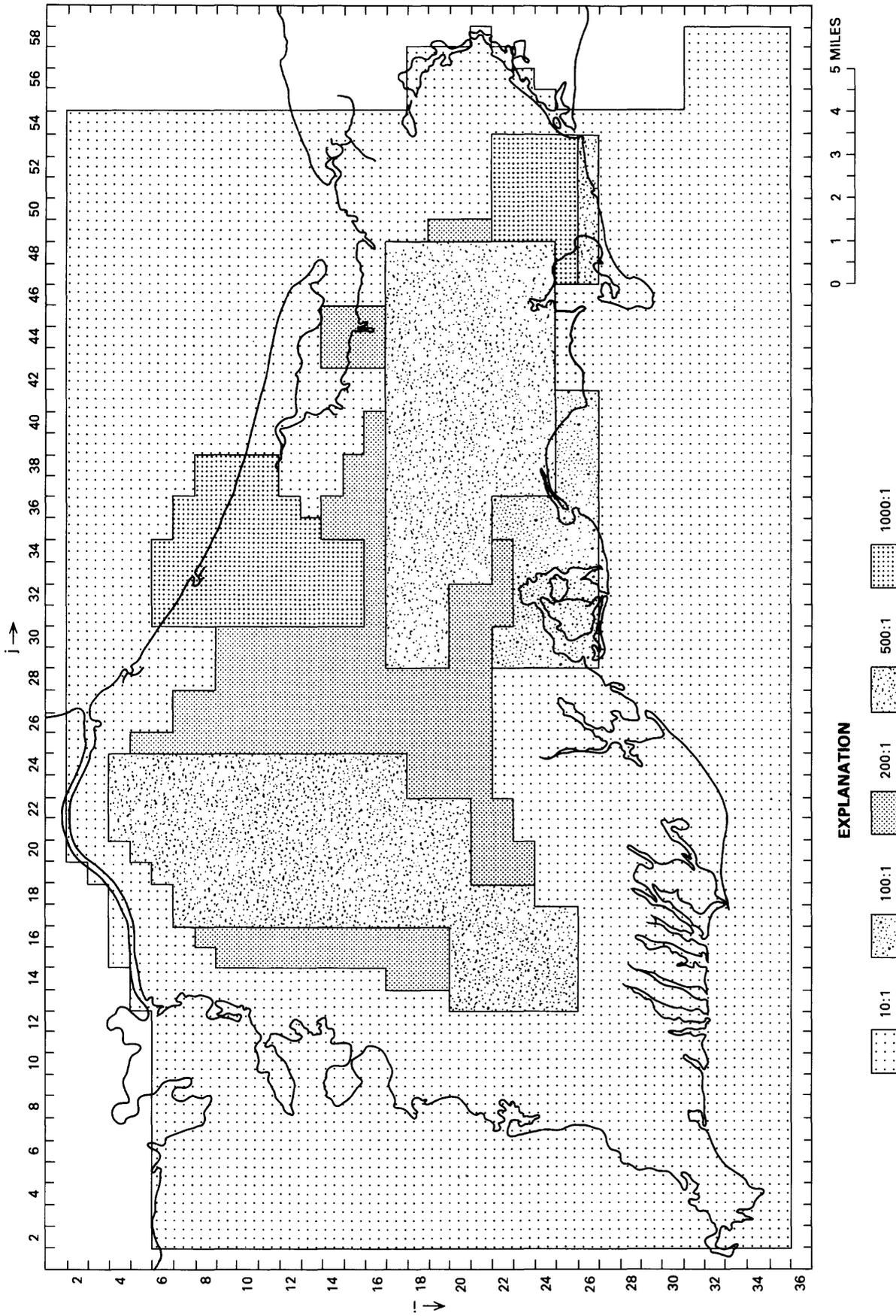


Figure 27. Ratio of lateral to vertical hydraulic conductivity, WCAPE model, layer 4.

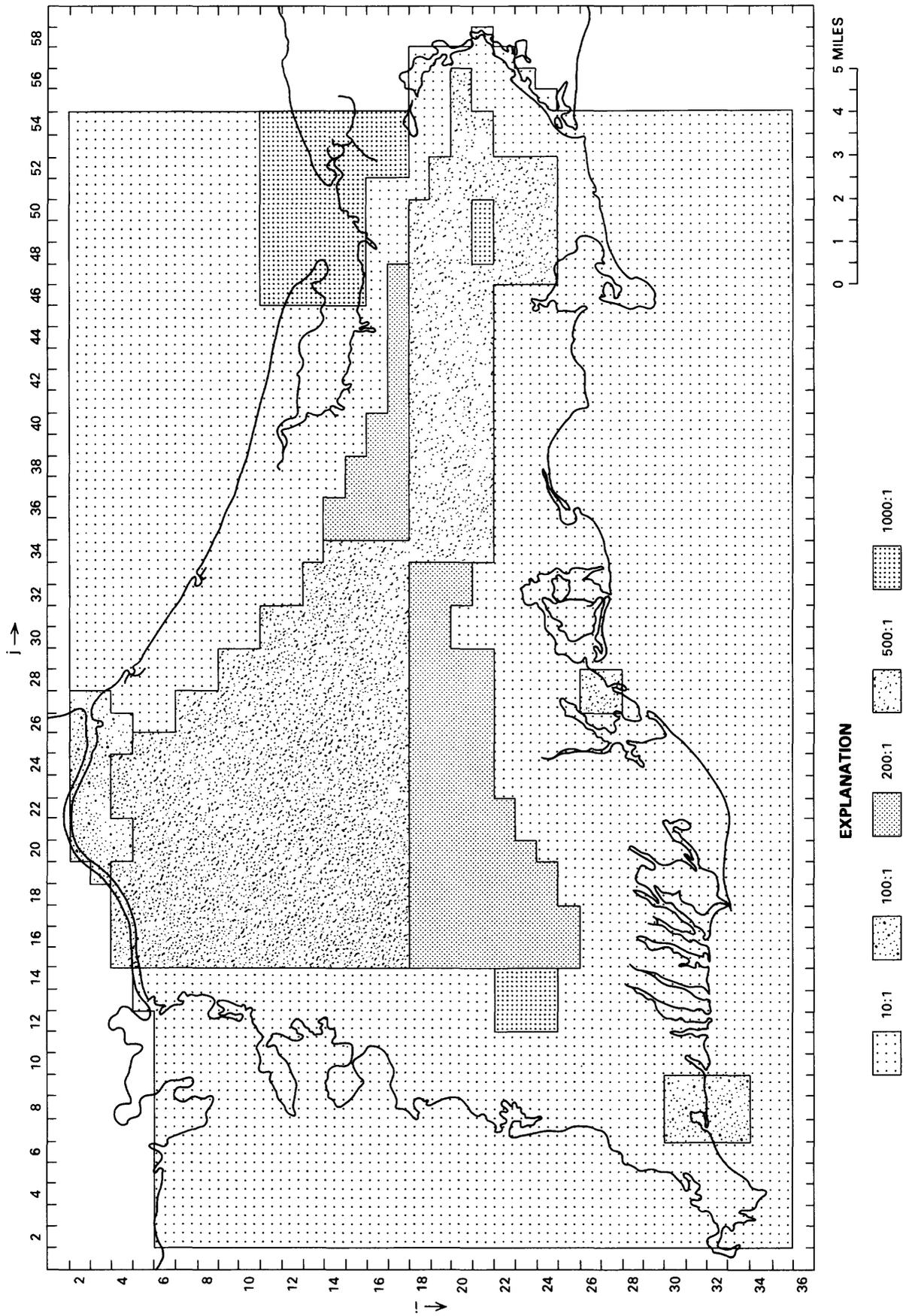


Figure 28. Ratio of lateral to vertical hydraulic conductivity, WCAPE model, layer 5.

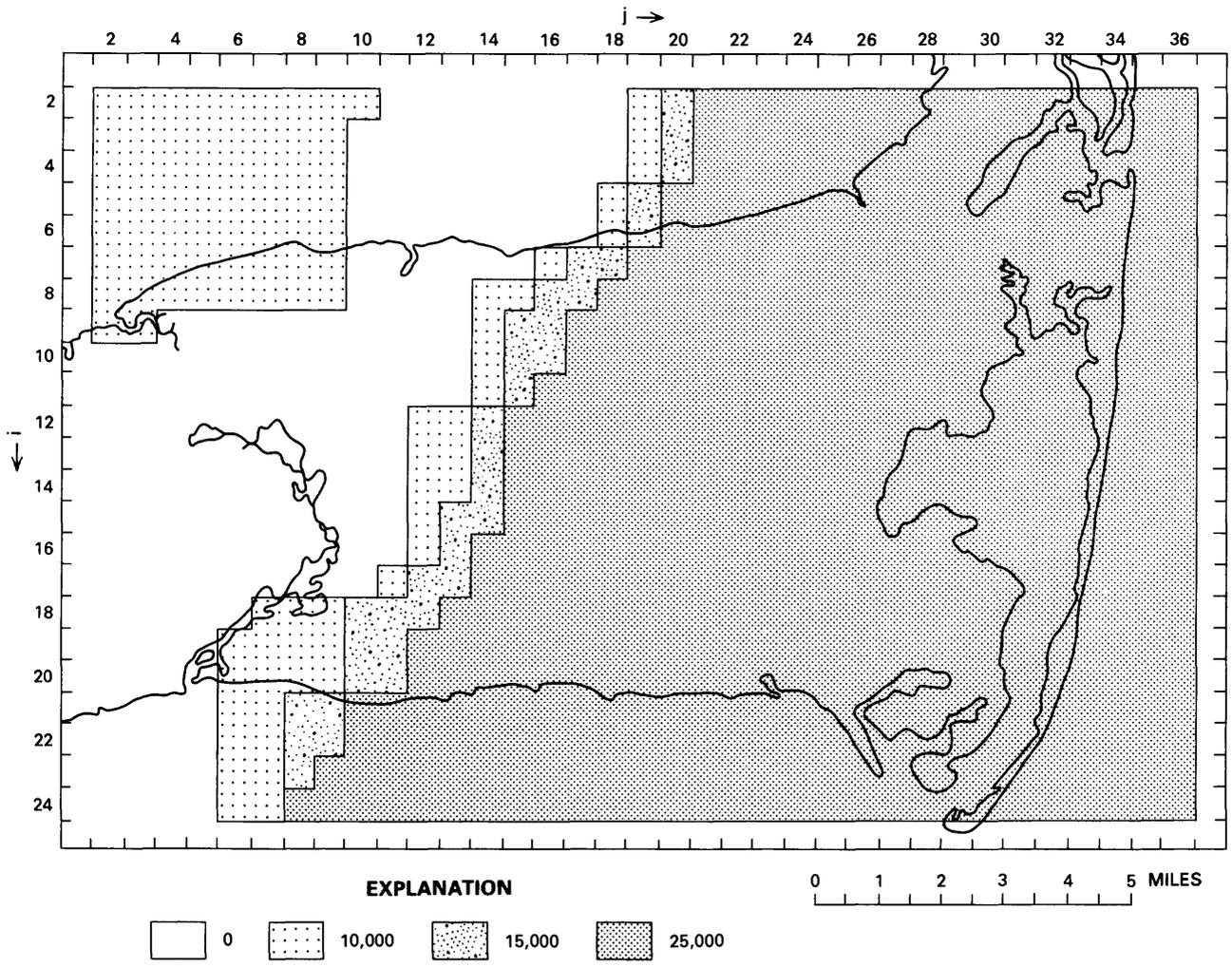


Figure 29. Transmissivity, in feet squared per day, ECAPE model, layer 1.

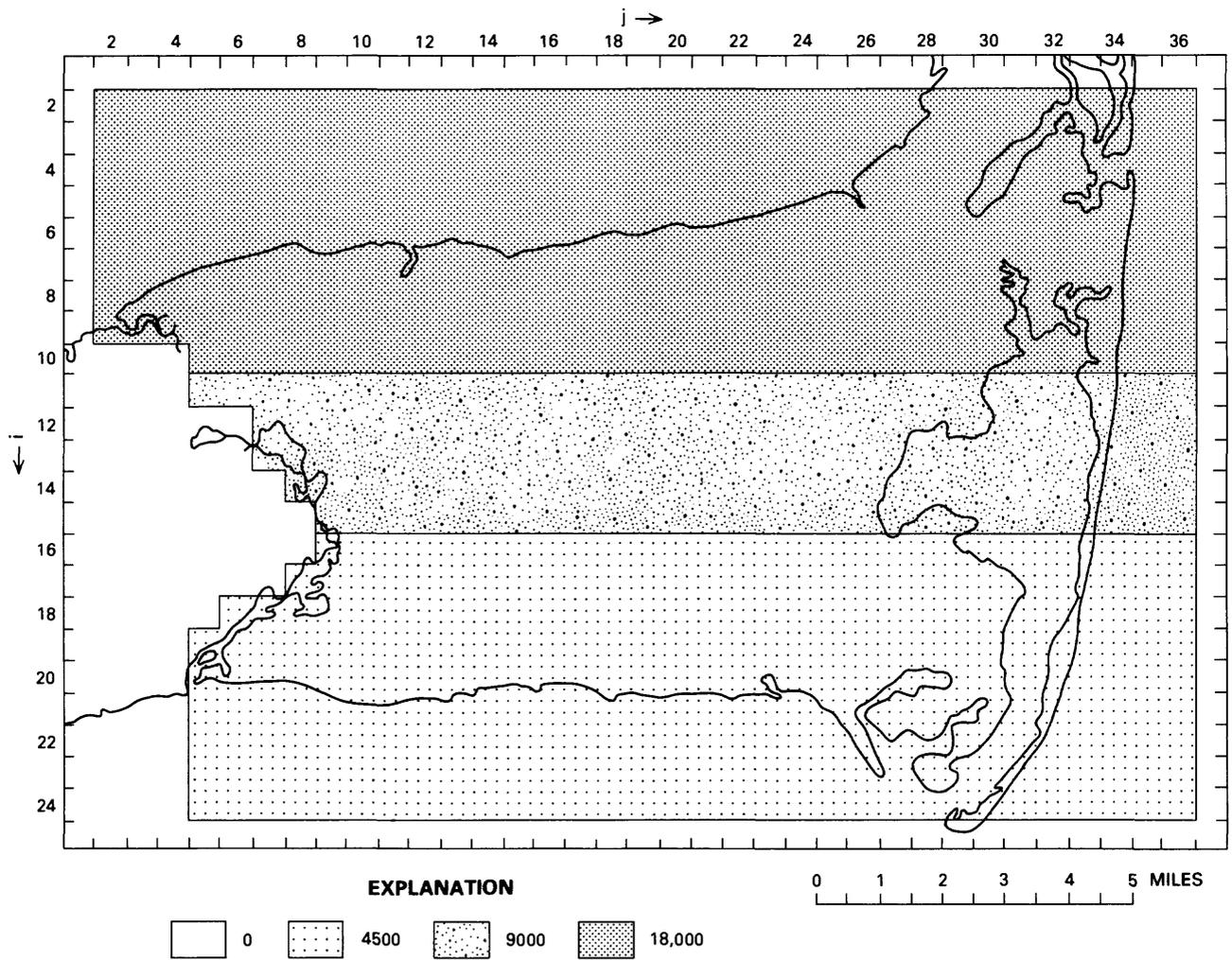


Figure 30. Transmissivity, in feet squared per day, ECAPE model, layer 2.

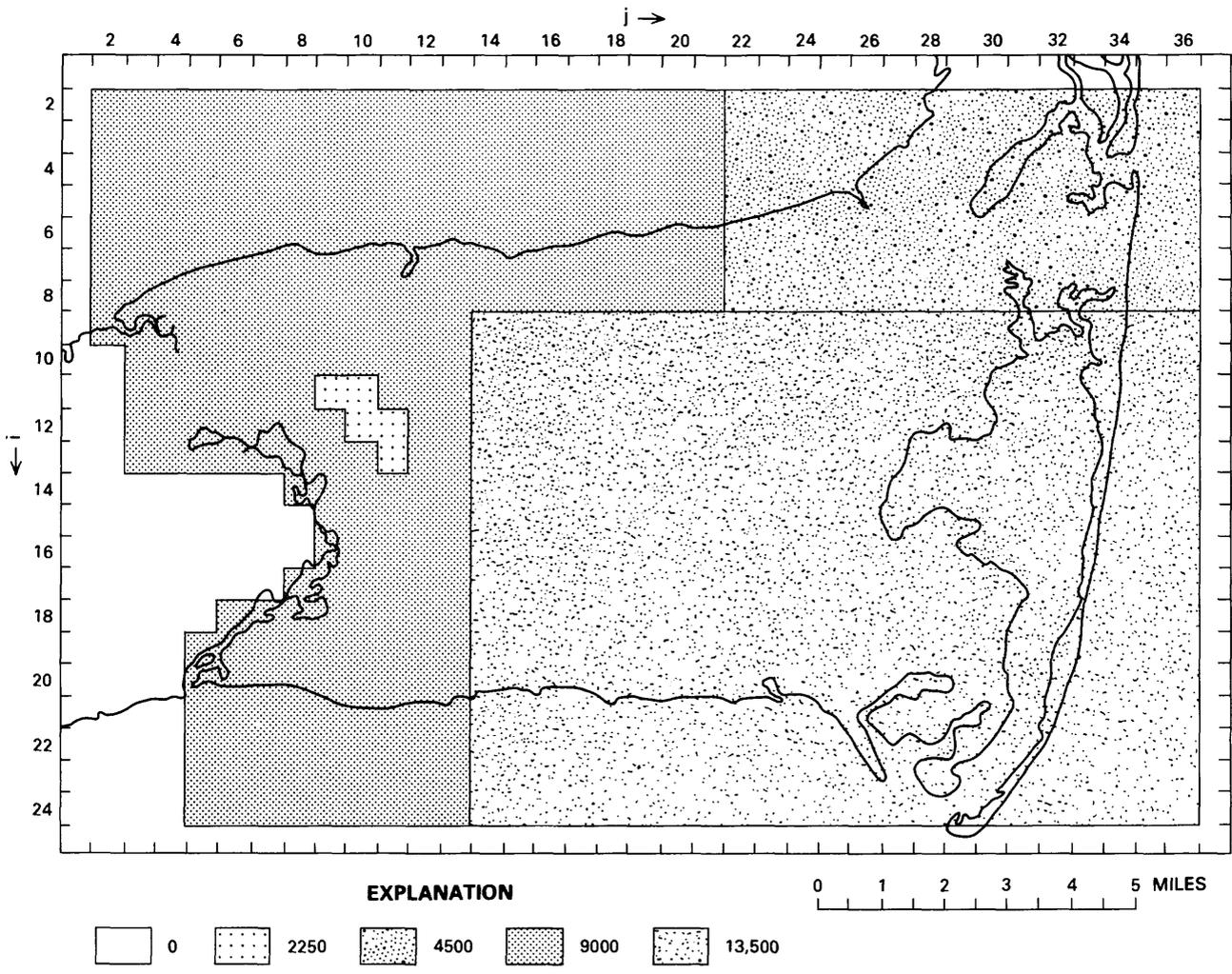


Figure 31. Transmissivity, in feet squared per day, ECAPE model, layer 3.

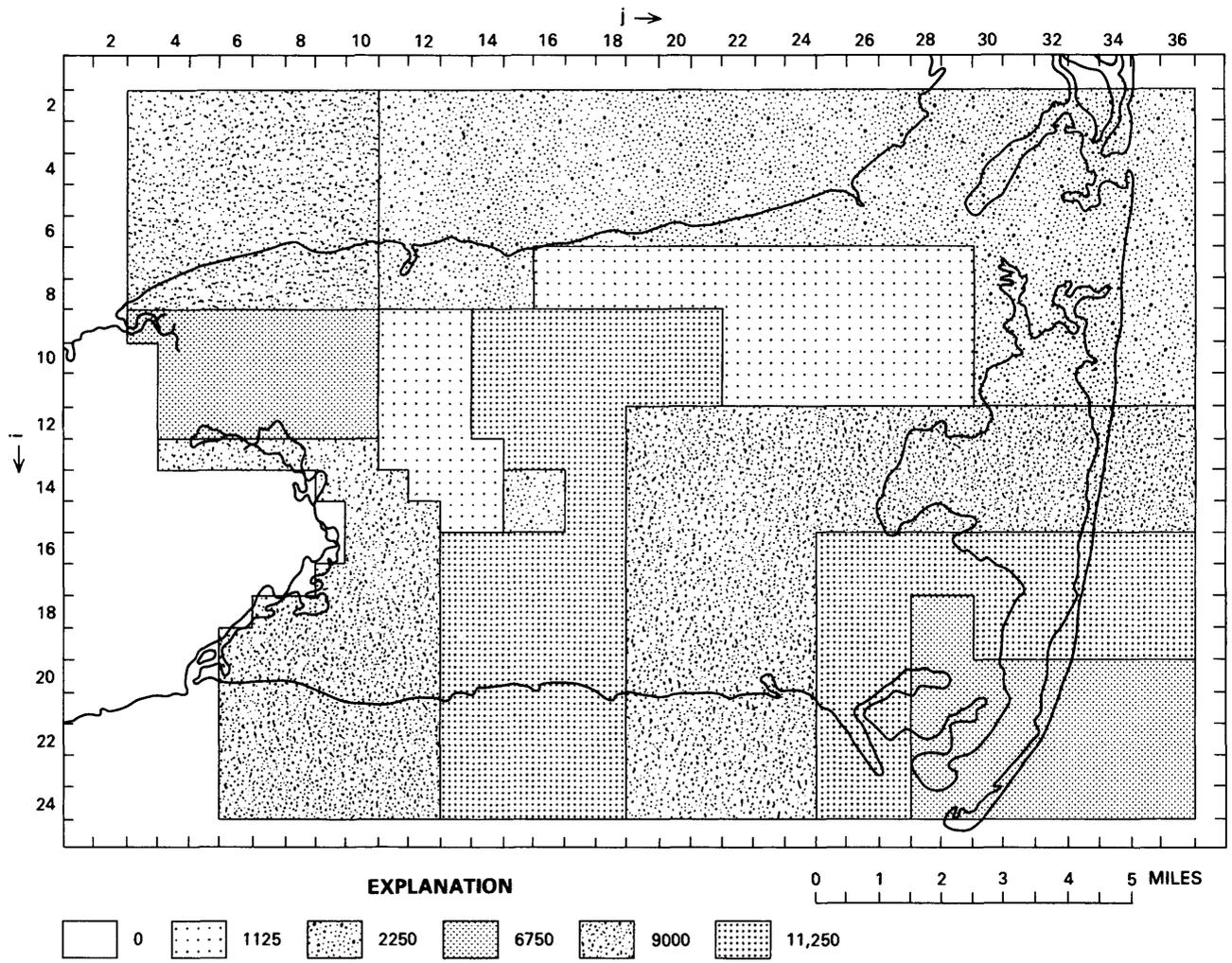


Figure 32. Transmissivity, in feet squared per day, ECAPE model, layer 4.

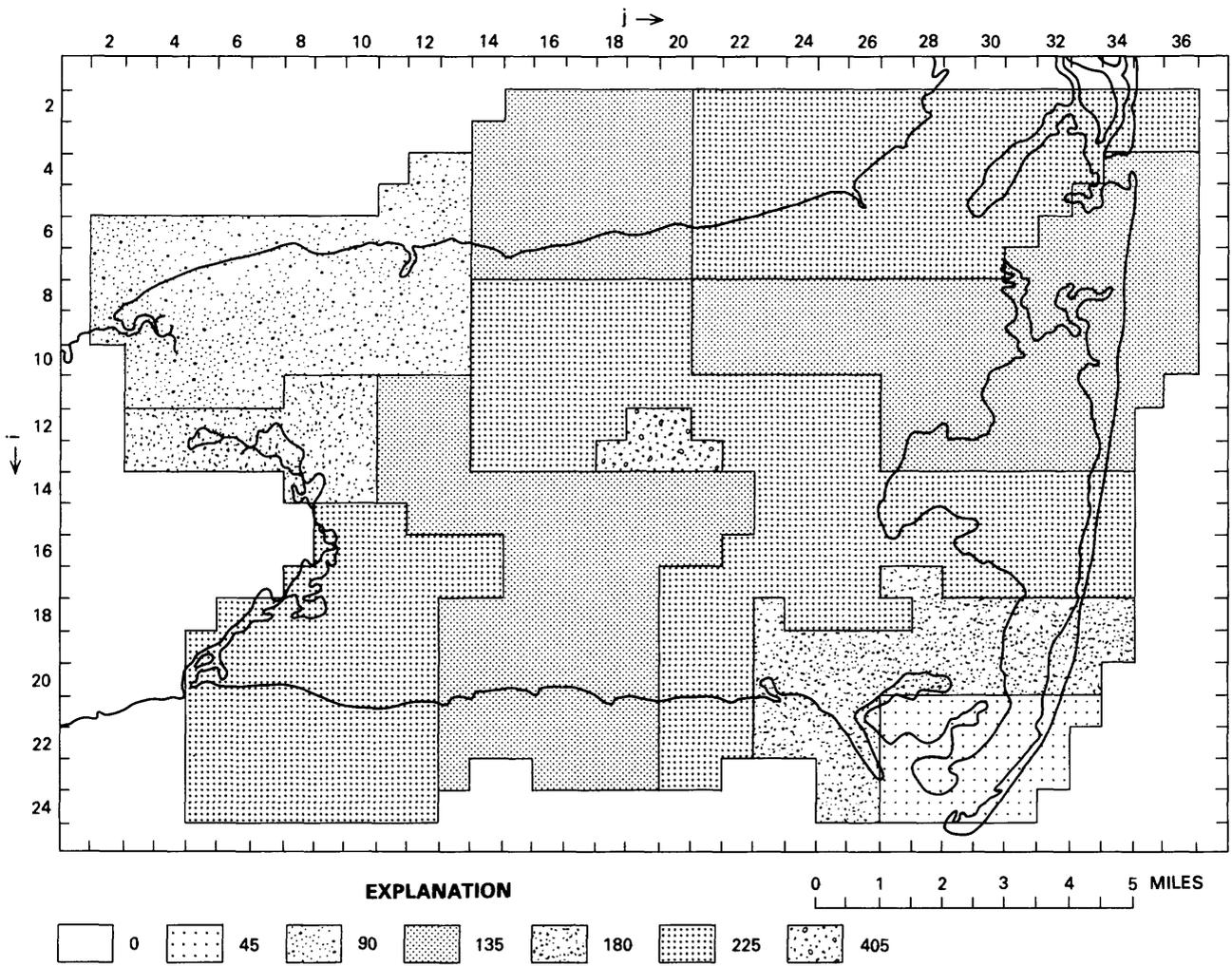


Figure 33. Hydraulic conductivity, in feet per day, ECAPE model, layer 5.

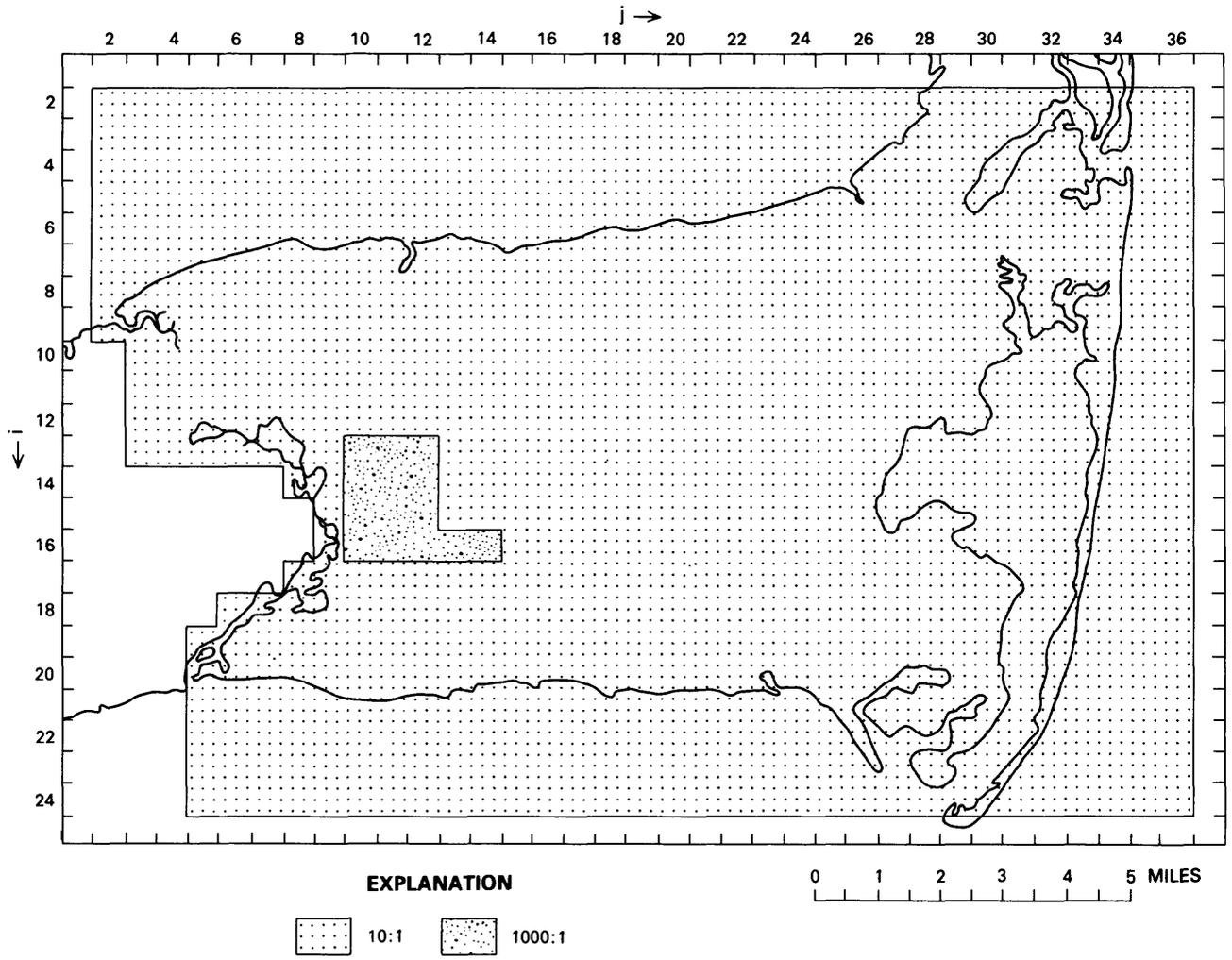


Figure 34. Ratio of lateral to vertical hydraulic conductivity, ECAPE model, layer 2.

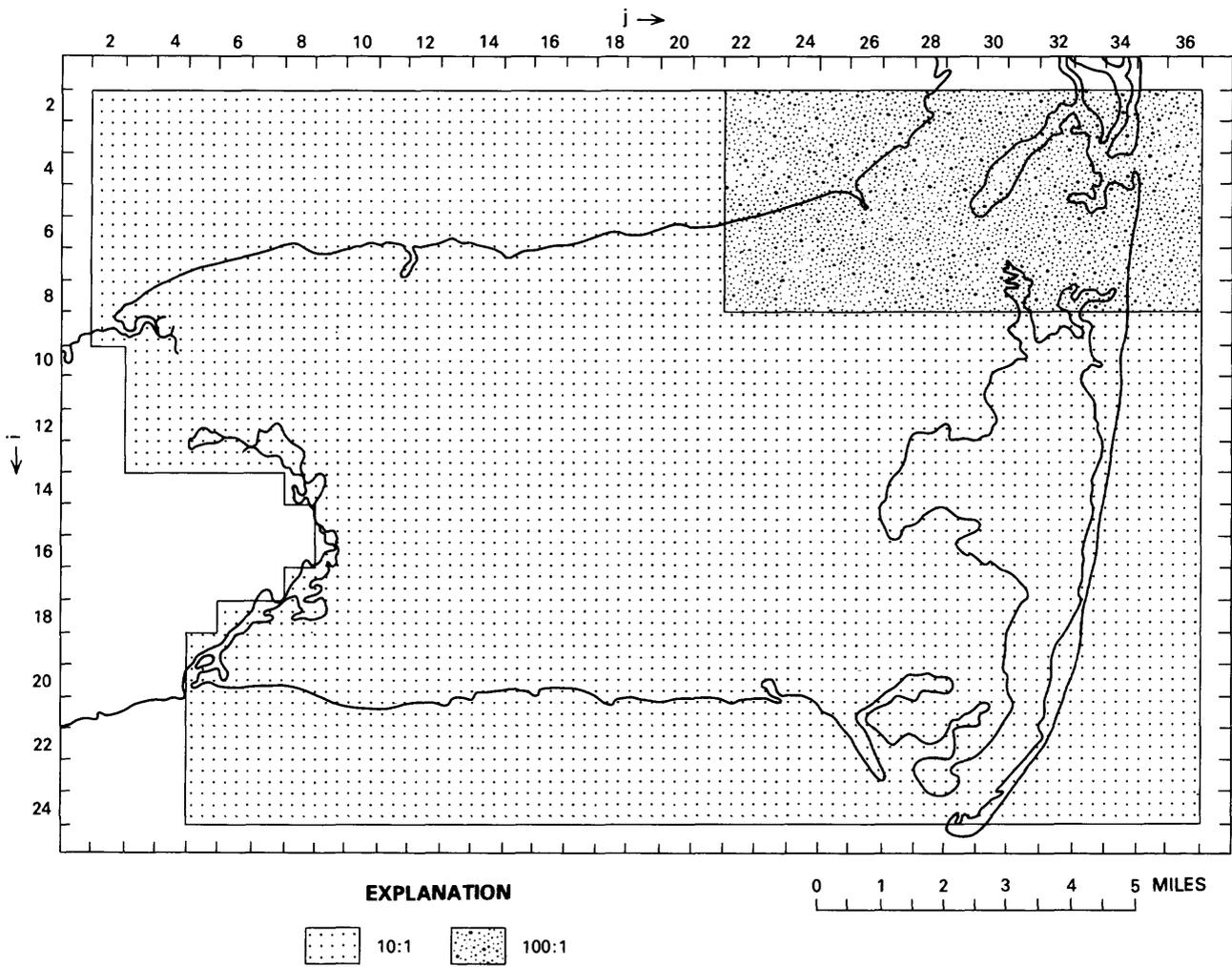


Figure 35. Ratio of lateral to vertical hydraulic conductivity, ECAPE model, layer 3.

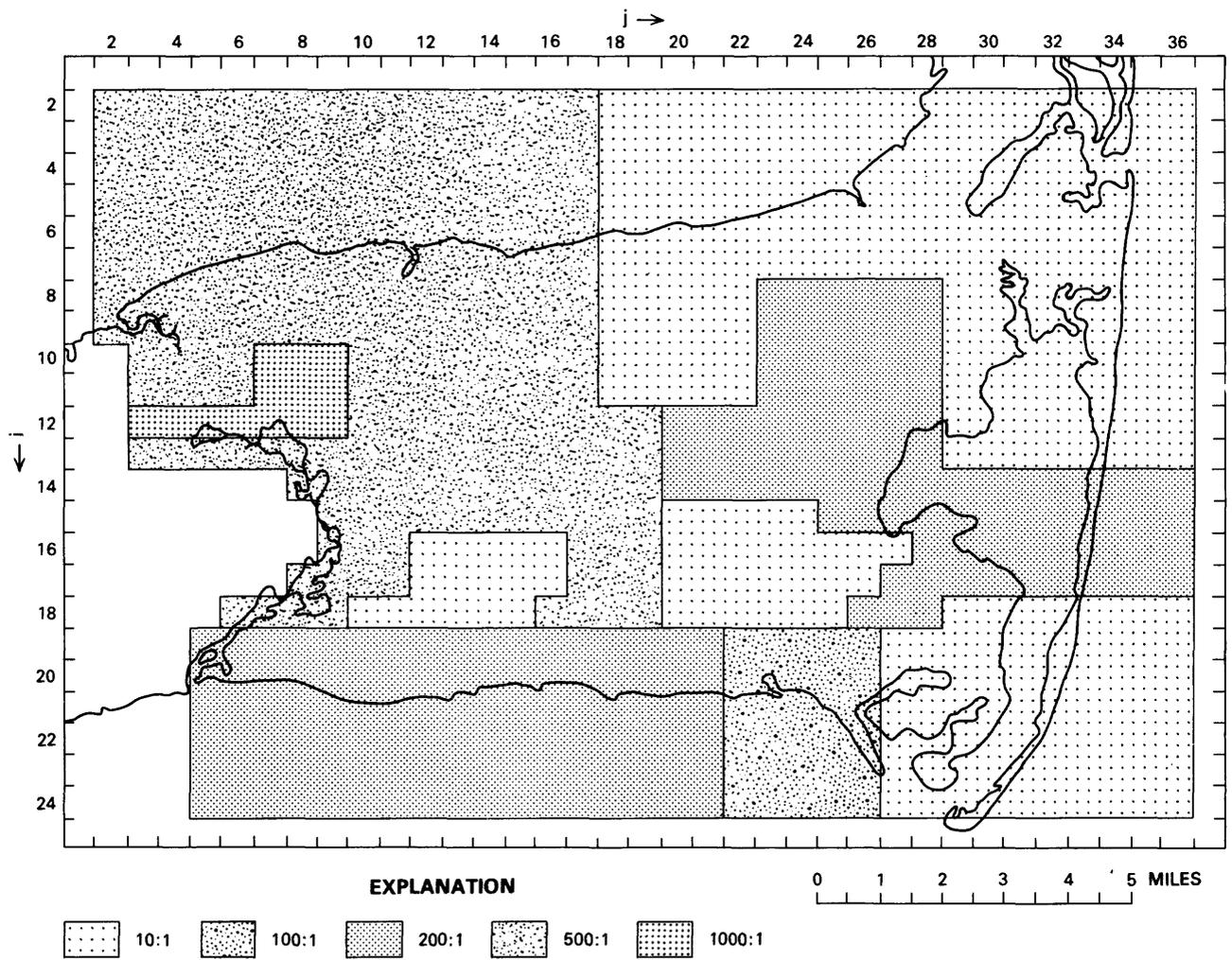


Figure 36. Ratio of lateral to vertical hydraulic conductivity, ECAPE model, layer 4.

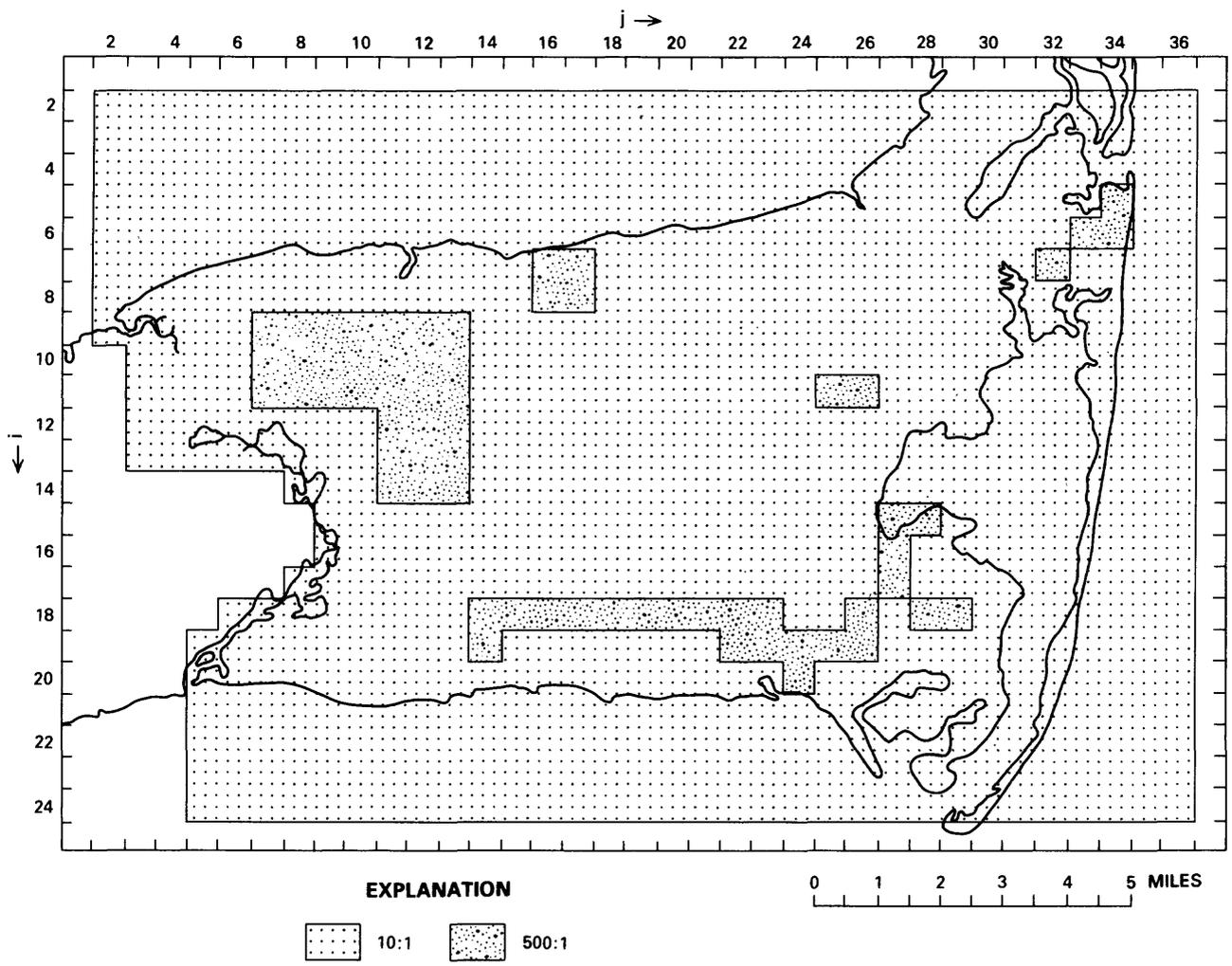


Figure 37. Ratio of lateral to vertical hydraulic conductivity, ECAPE model, layer 5.

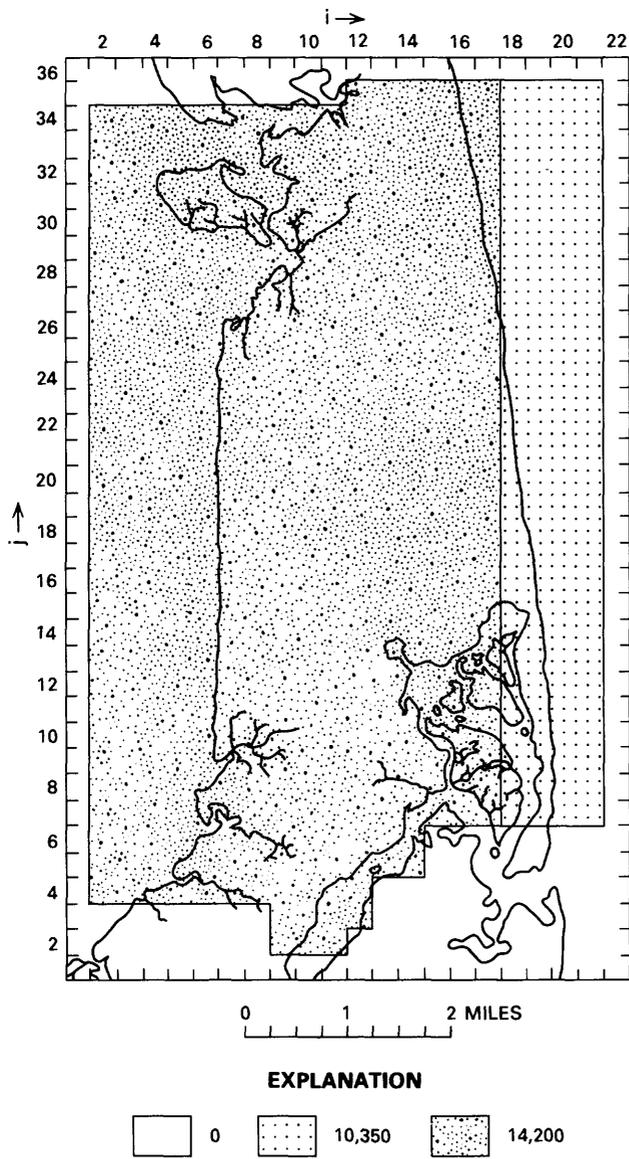


Figure 38. Transmissivity, in feet squared per day, ESTHM model, layer 5.

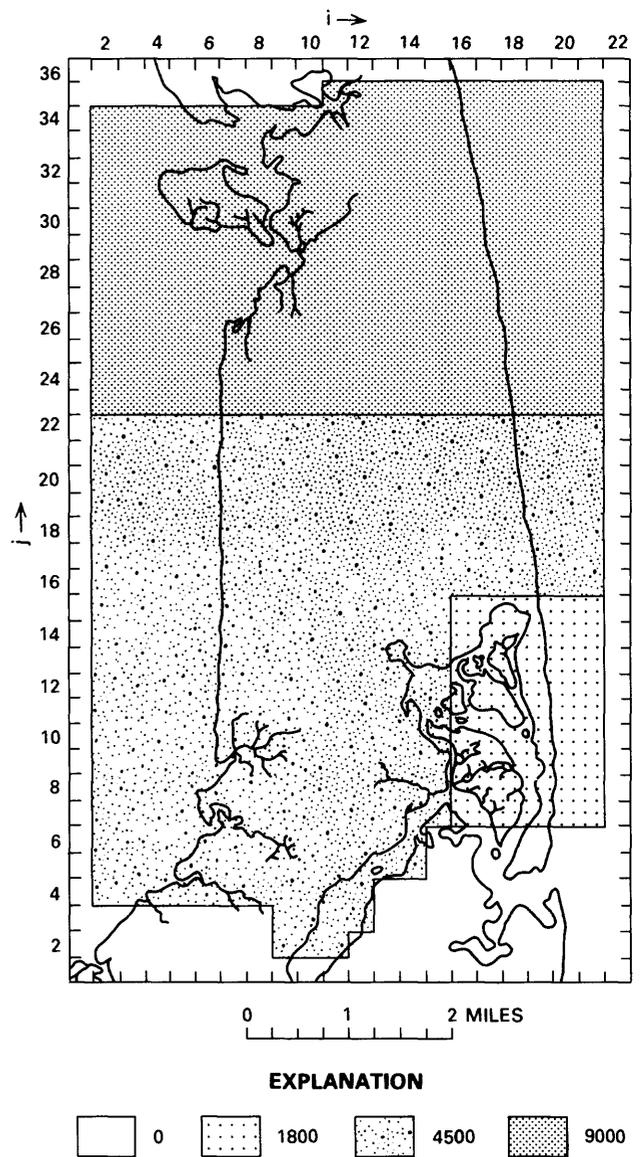


Figure 39. Transmissivity, in feet squared per day, ESTHM model, layer 6.

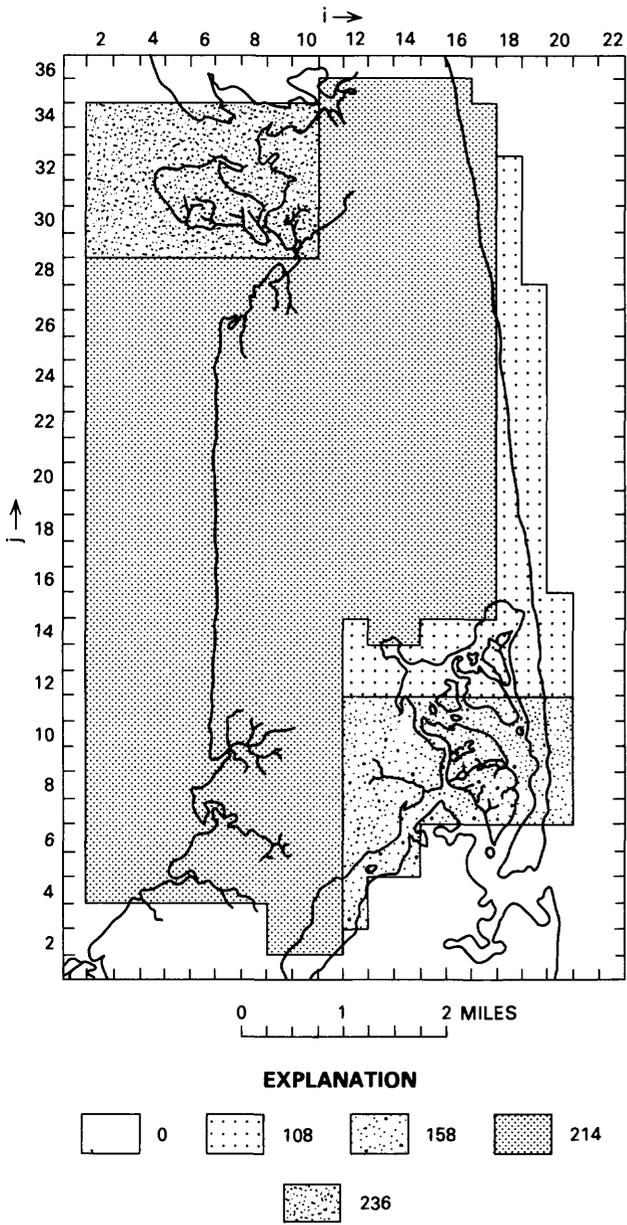


Figure 40. Hydraulic conductivity, in feet per day, ESTHM model, layer 7.

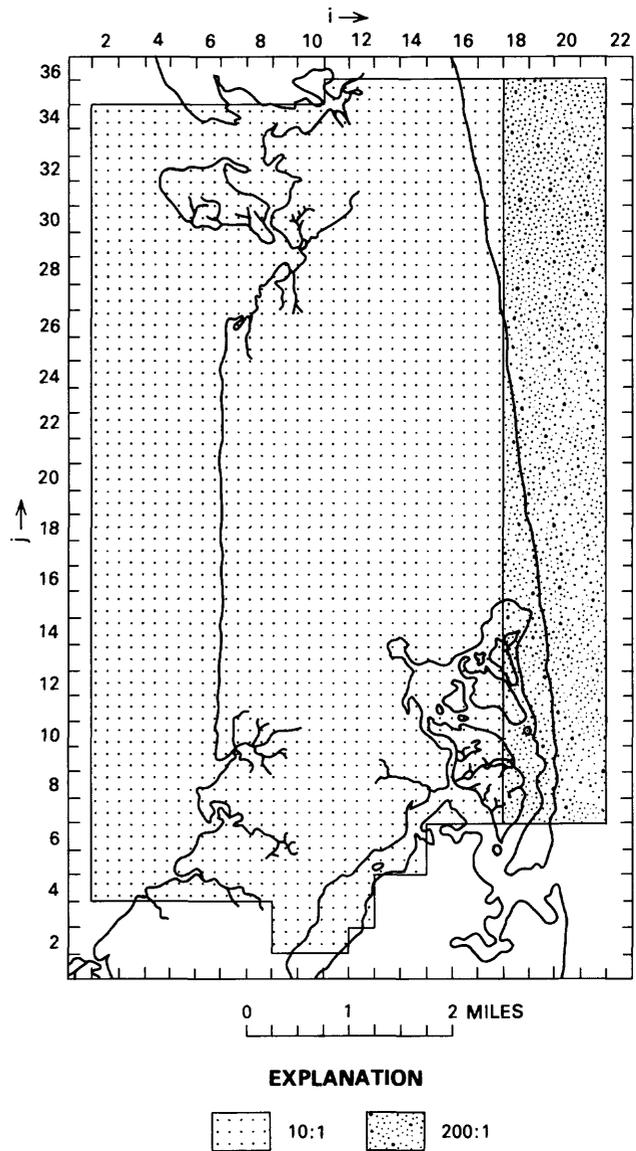


Figure 41. Ratio of lateral to vertical hydraulic conductivity, ESTHM model, layer 5.

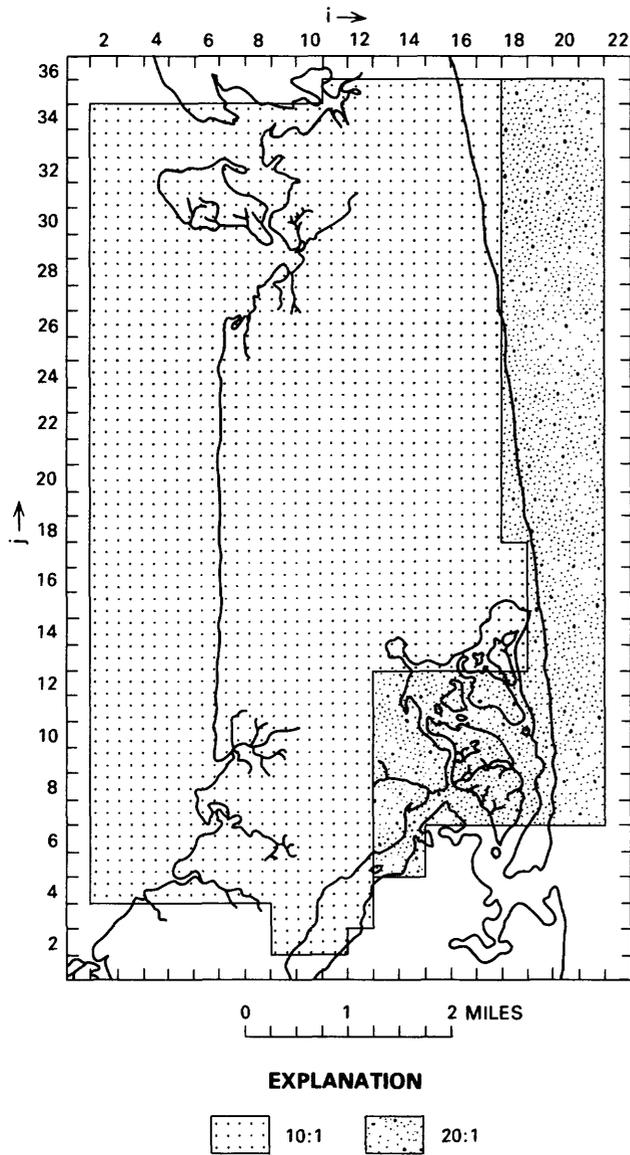


Figure 42. Ratio of lateral to vertical hydraulic conductivity, ESTHM model, layer 6.

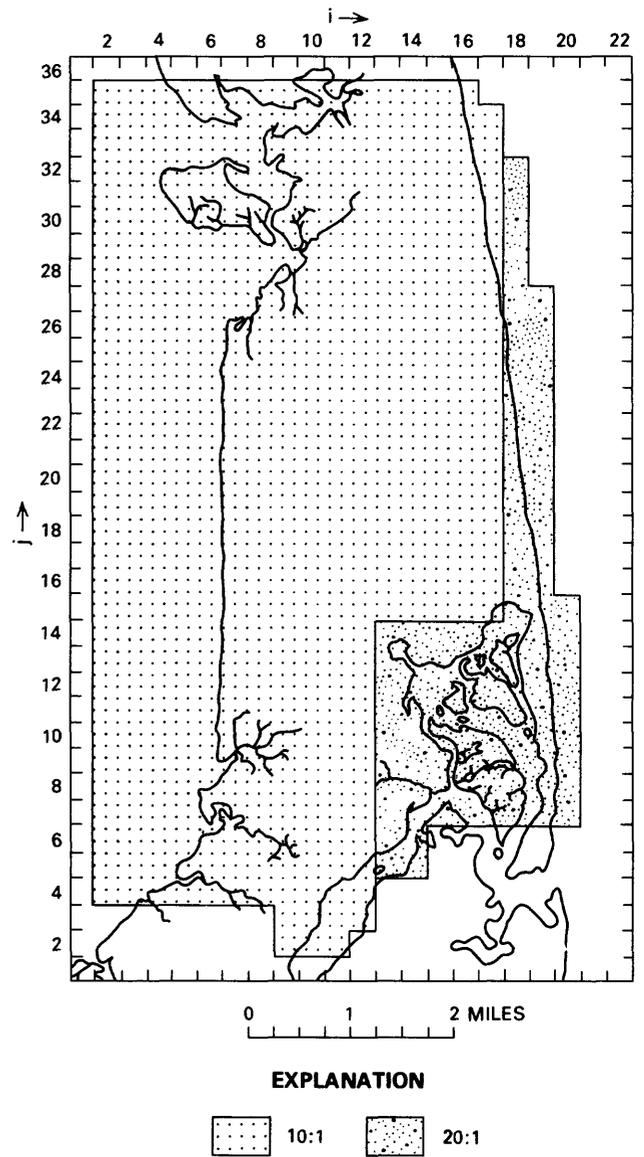


Figure 43. Ratio of lateral to vertical hydraulic conductivity, ESTHM model, layer 7.

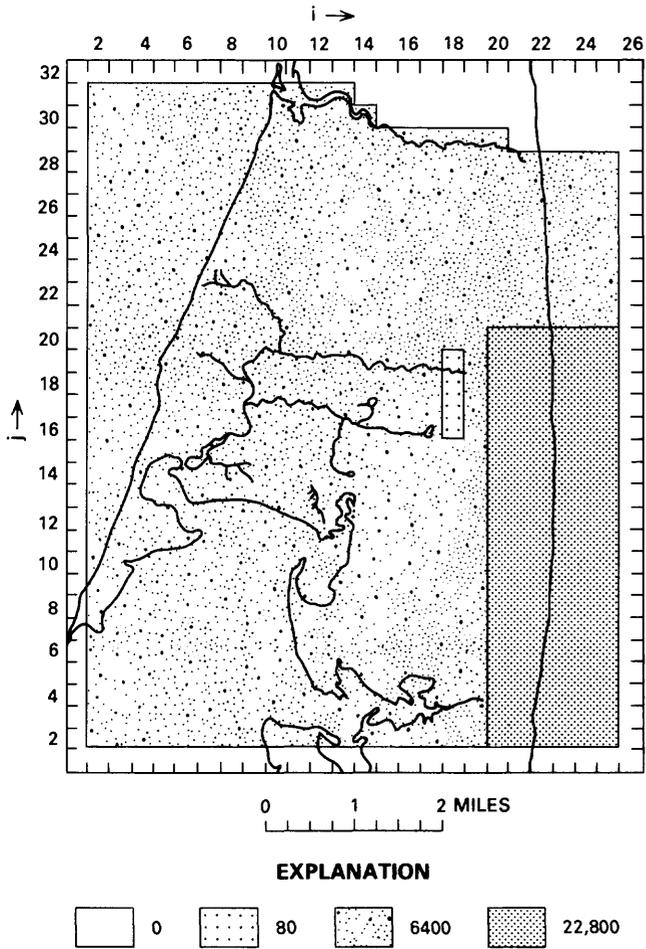


Figure 44. Transmissivity, in feet squared per day, WLFLT model, layer 4.

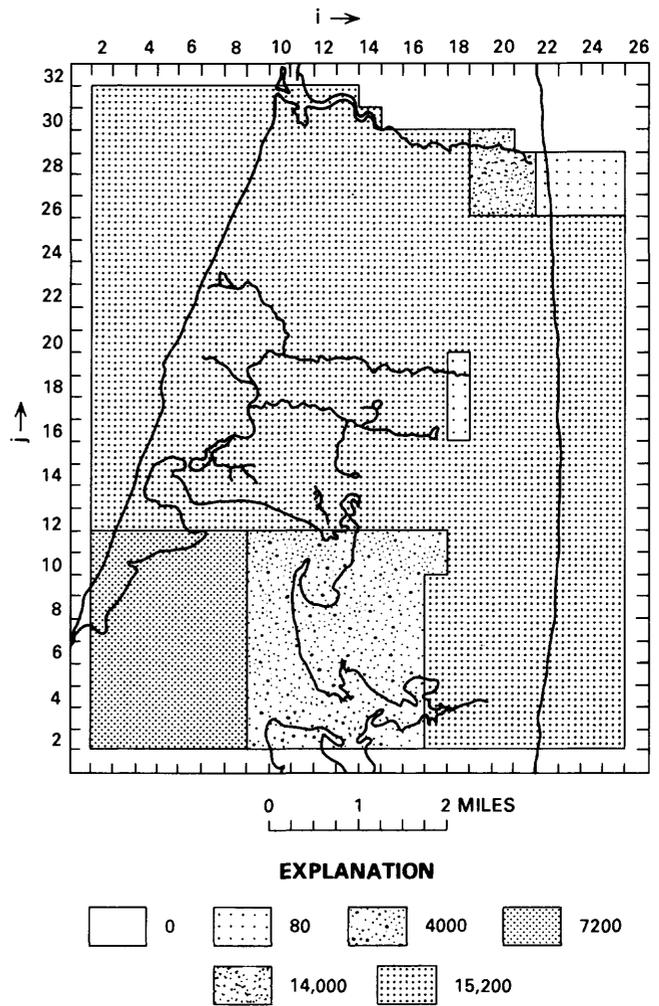


Figure 45. Transmissivity, in feet squared per day, WLFLT model, layer 5.

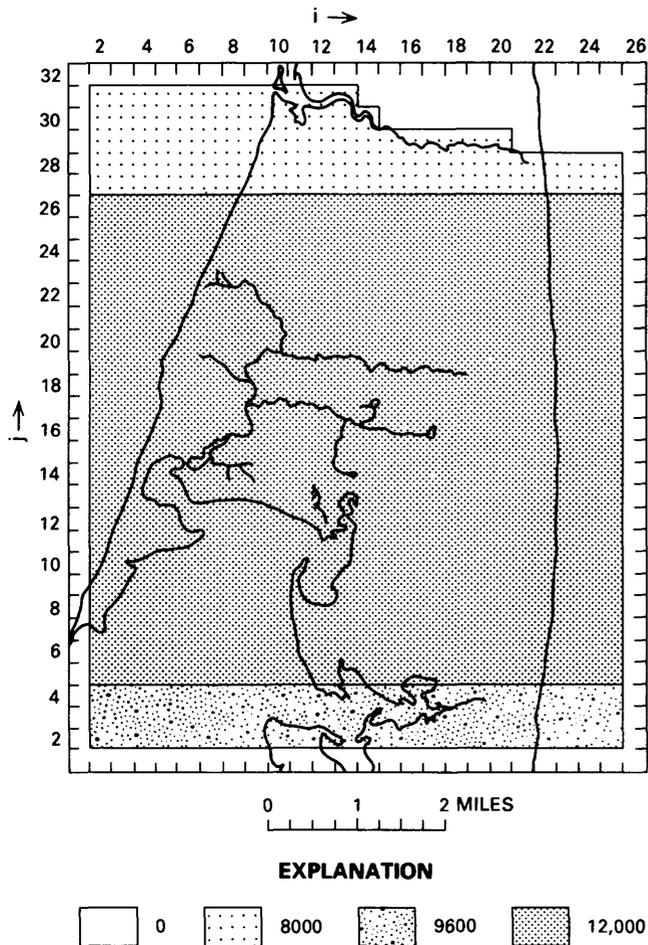


Figure 46. Transmissivity, in feet squared per day, WFLT model, layer 6.

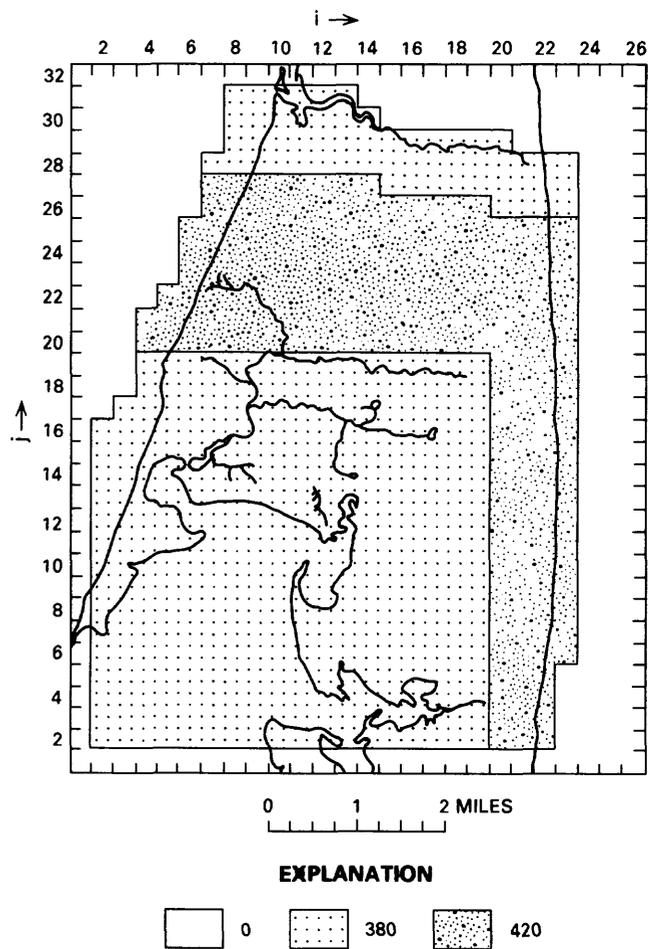


Figure 47. Hydraulic conductivity, in feet per day, WFLT model, layer 7.

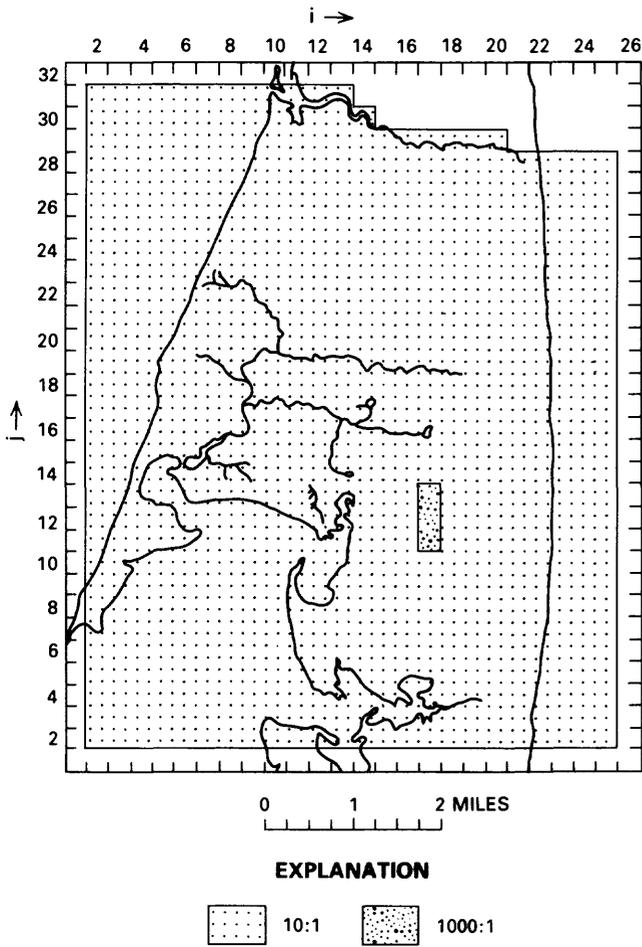


Figure 48. Ratio of lateral to vertical hydraulic conductivity, WLF LT model, layer 5.

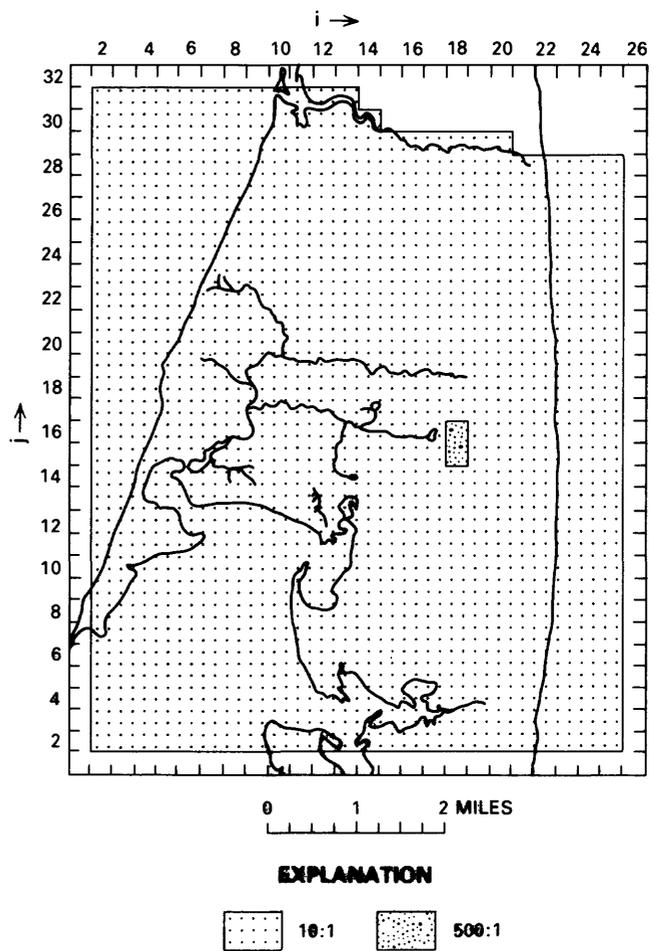


Figure 49. Ratio of lateral to vertical hydraulic conductivity, WLF LT model, layer 6.

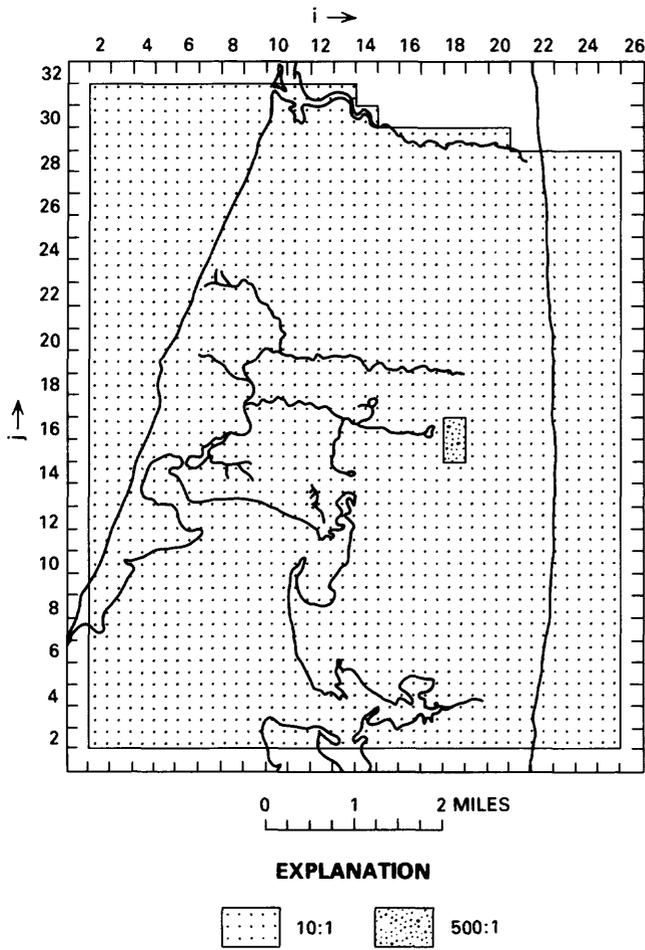


Figure 50. Ratio of lateral to vertical hydraulic conductivity, WLFLT model, layer 7.

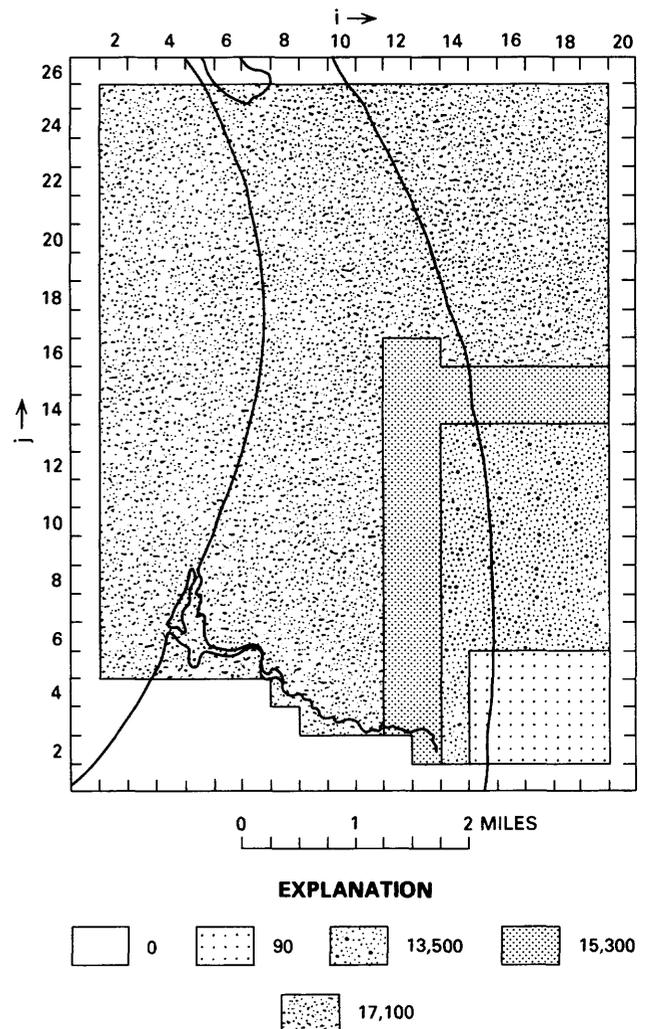


Figure 51. Transmissivity, in feet squared per day, TRURO model, layer 5.

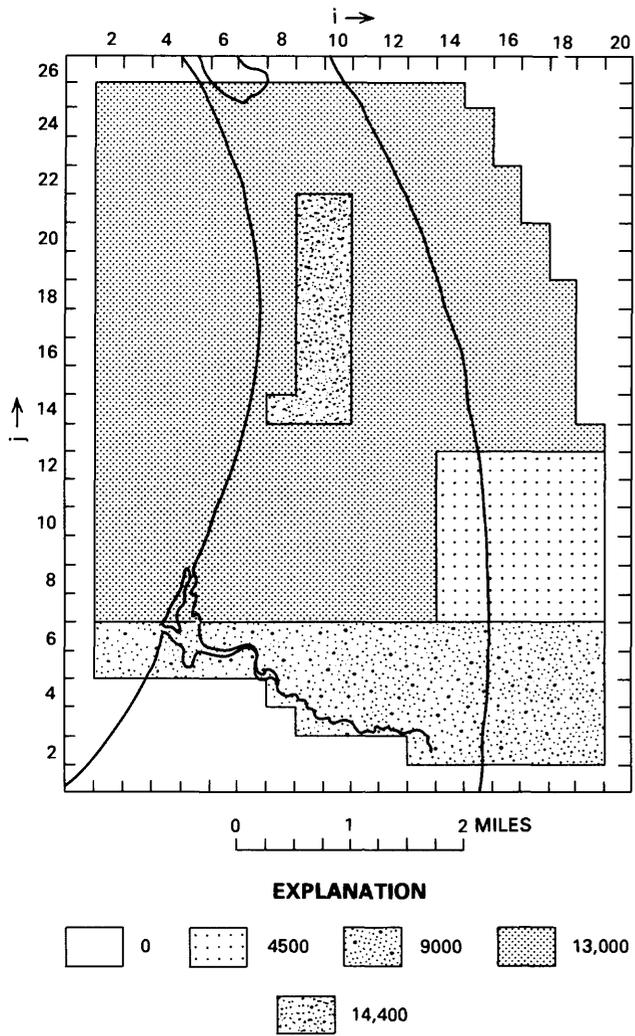


Figure 52. Transmissivity, in feet squared per day, TRURO model, layer 6.

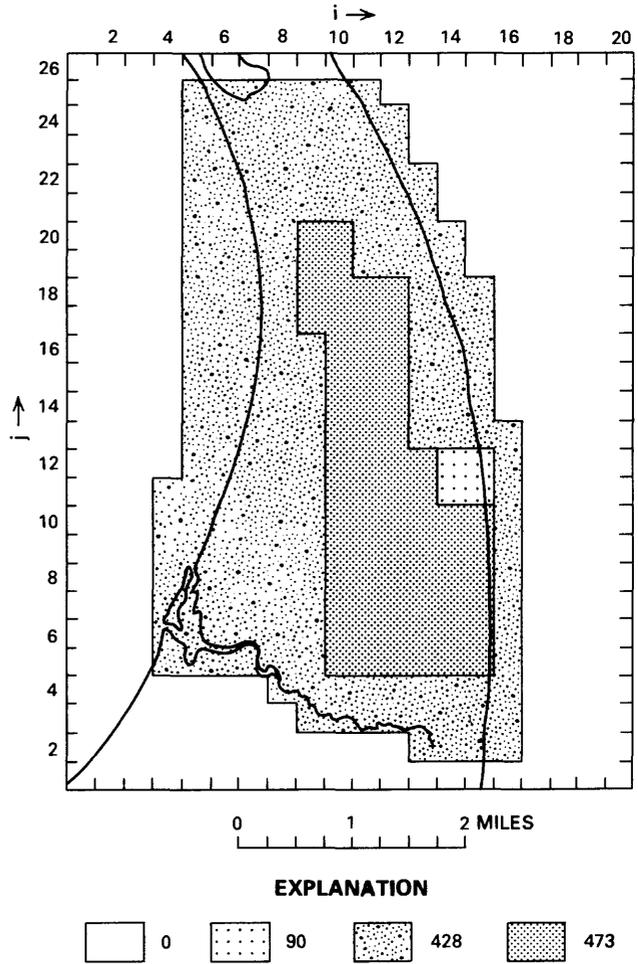


Figure 53. Hydraulic conductivity, in feet per day, TRURO model, layer 7.

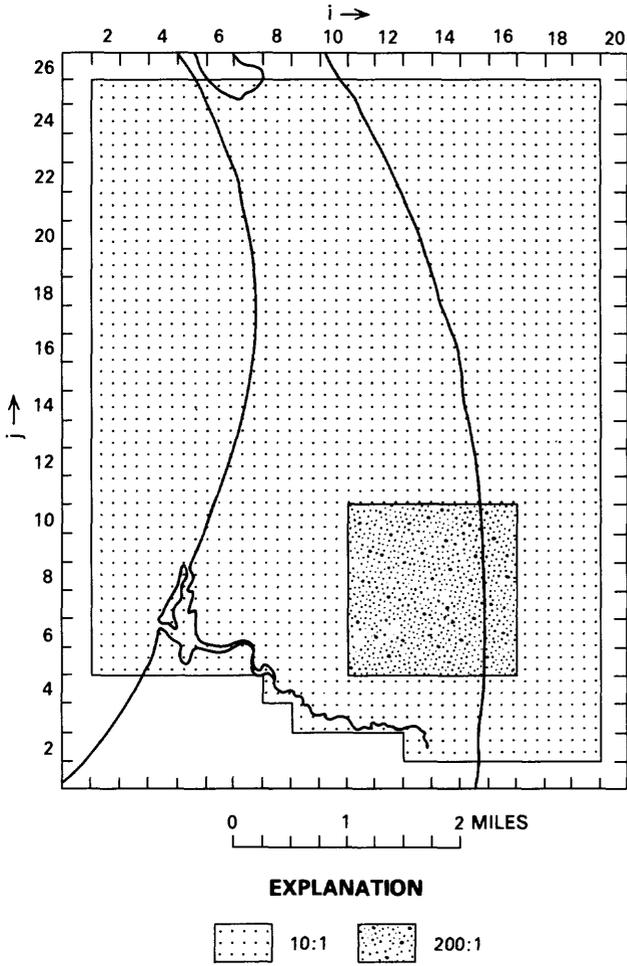


Figure 54. Ratio of lateral to vertical hydraulic conductivity, TRURO model, layer 5.

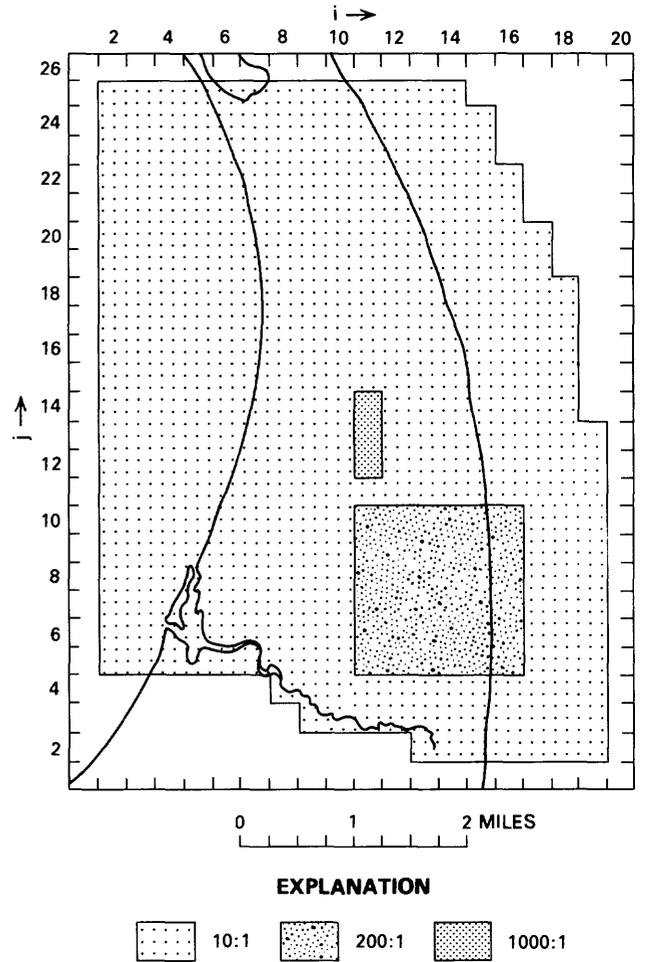


Figure 55. Ratio of lateral to vertical hydraulic conductivity, TRURO model, layer 6.

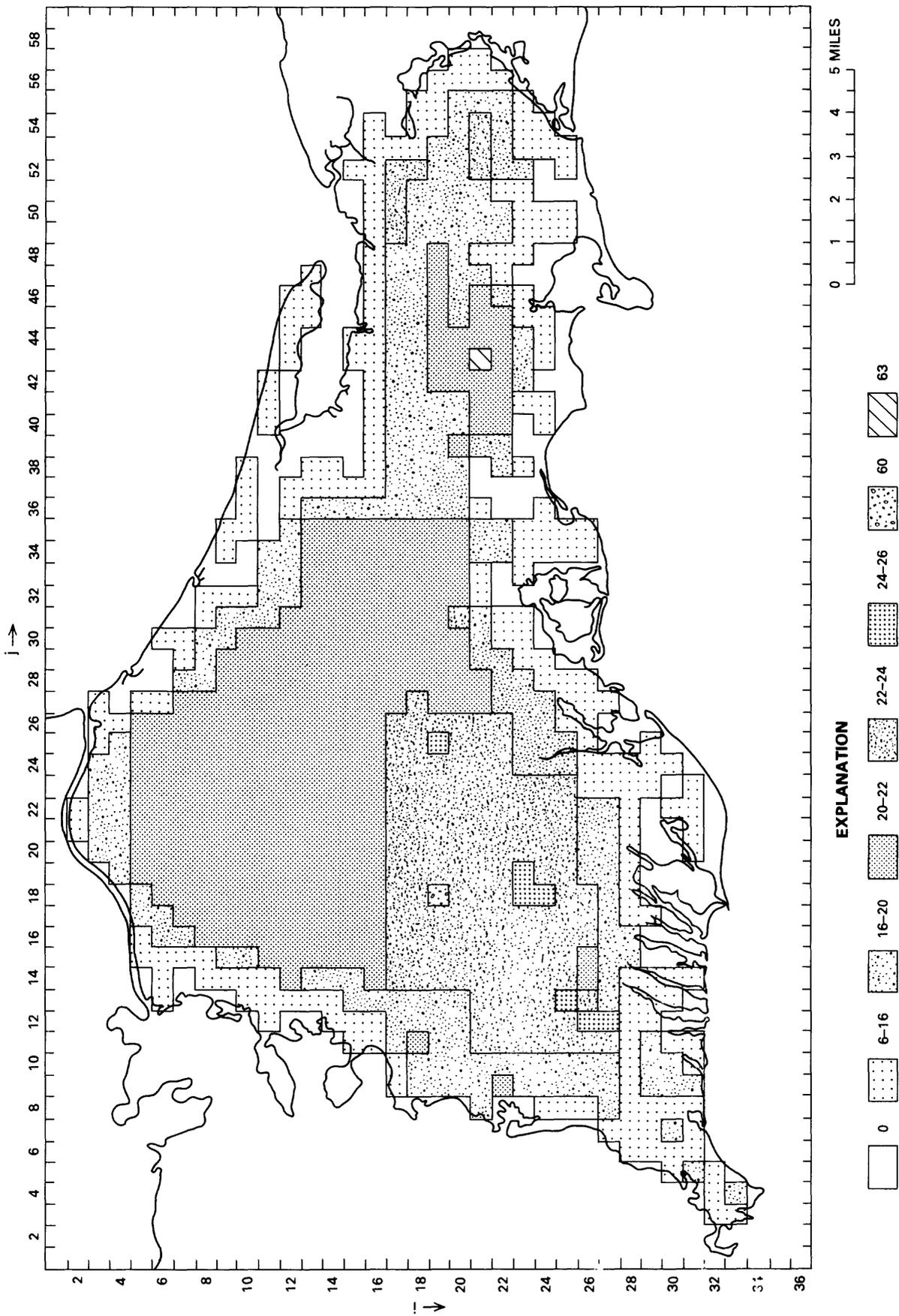


Figure 56. Steady-state recharge, in inches per year, WCAPE model.

Table 10. Summary of well discharges represented in models
(An asterisk indicates multiple well. A dash indicates well not pumped during 1975-76.)

| Node (i,j,k) | U.S. Geo- logical Survey well No. | Dis- charge (ft ³ /s) | Total node discharge (ft ³ /s) | Node (i,j,k) | U.S. Geo- logical Survey well No. | Dis- charge (ft ³ /s) | Total node discharge (ft ³ /s) |
|-----------------|--|--|--|-----------------|--|--|--|
| Model: WCAPE | | | | | | | |
| 3,24,4 | BHW 22,232* | 0.124 | 0.124 | 21,56,4 | YAW 53 | 0.223 | |
| 4,26,4 | SDW 249,250 | .062 | .062 | | YAW 144 | .256 | |
| 7,28,4 | SDW 27 | .104 | .104 | | YAW 146 | .238 | 0.717 |
| 7,28,5 | SDW 37 | .320 | .320 | 22,29,5 | A1W 369 | .113 | .113 |
| 9,15,4 | BHW 233 | .079 | .079 | 22,46,4 | A1W 377 | 1.165 | |
| 10,14,4 | BHW 1-3,136 | .336 | .336 | | A1W 385,386 | .109 | 1.274 |
| 10,15,5 | BHW 199 | .239 | .239 | 22,49,4 | YAW 128 | .125 | |
| 15,3,4 | BHW 137 | .288 | .288 | | YAW 195 | -- | .125 |
| 16,20,5 | SDW 155 | .433 | .433 | 22,50,4 | YAW 193 | -- | |
| 17,16,5 | BHW 23 | .507 | .507 | | YAW 194 | -- | 0 |
| 17,43,5 | A1W 228 | .172 | .172 | 22,51,4 | YAW 130 | -- | 0 |
| 18,52,4 | YAW 103* | .223 | .223 | 22,53,4 | YAW 64 | .345 | |
| 18,52,5 | YAW 103* | .223 | .223 | | YAW 65 | .309 | .654 |
| 19,35,5 | A1W 371 | .170 | .170 | 23,17,5 | FSW 214 | -- | 0 |
| 19,36,5 | A1W 372 | .156 | .156 | 23,28,4 | A1W 251 | .112 | .112 |
| 19,45,5 | A1W 370 | .269 | .269 | 23,34,5 | A1W 159 | .133 | |
| 19,46,5 | A1W 402 | .072 | .072 | | A1W 107 | .400 | .533 |
| 19,49,4 | YAW 41 | -- | 0 | 23,34,5 | A1W 249 | .371 | |
| 19,49,5 | YAW 42 | .248 | | | A1W 158 | .133 | |
| | YAW 43 | .266 | .514 | | A1W 160 | .133 | .637 |
| 19,55,4 | YAW 126 | .046 | | 23,41,4 | A1W 226 | .071 | .071 |
| | YAW 127 | .027 | .073 | 23,41,5 | A1W 227 | .048 | |
| 20,46,5 | A1W 403 | .065 | | | A1W 368 | .119 | .167 |
| | A1W 383 | .305 | | 23,42,4 | A1W 376 | .453 | .453 |
| | A1W 387 | .173 | .543 | 24,29,5 | A1W 224 | .046 | .046 |
| 20,56,4 | YAW 54 | .467 | .467 | 24,42,4 | A1W 229 | .210 | |
| 21,29,5 | A1W 59 | .007 | .007 | | A1W 384 | .966 | 1.176 |
| 21,36,4 | A1W 373 | .031 | .031 | 26,9,5 | Long Pond | 1.04 | 1.04 |
| 21,36,5 | A1W 259 | .471 | .471 | 26,10,5 | Long Pond | 1.45 | 1.45 |
| 21,50,4 | YAW 58 | .310 | .310 | 27,10,5 | Long Pond | 1.66 | 1.66 |
| 21,52,4 | YAW 61 | .279 | | 29,24,5 | MIW 32 | .048 | .048 |
| | YAW 63 | .208 | .487 | 30,23,4 | MIW 35 | .150 | .150 |

Table 10. Summary of well discharges represented in models—Continued

| Node (i,j,k) | U.S. Geo- logical Survey well No. | Dis- charge (ft ³ /s) | Total node discharge (ft ³ /s) | Node (i,j,k) | U.S. Geo- logical Survey well No. | Dis- charge (ft ³ /s) | Total node discharge (ft ³ /s) |
|-----------------|--|--|--|-----------------|--|--|--|
| Model: ECAPE | | | | | | | |
| 8,28,4 | OSW 11 | 0.322 | | 13,12,5 | DGW 77 | 0.273 | |
| | OSW 14 | .206 | | | DGW 87 | .150 | 0.423 |
| | OSW 15 | .277 | 0.805 | 14,9,5 | DGW 1-5 | .246 | .246 |
| 9,28,4 | OSW 42 | .041 | | 14,10,5 | DGW 66 | .161 | .161 |
| | OSW 43 | .088 | .129 | 16,12,4 | DGW 205 | -- | 0 |
| 11,9,5 | DGW 232 | .015 | | 18,20,4 | HJW 160 | .120 | .120 |
| | DGW 67 | .059 | .074 | 18,20,5 | HJW 49 | .529 | |
| 11,11,5 | DGW 244 | -- | 0 | | HJW 55 | .148 | |
| 11,24,4 | BMW 37 | .287 | .287 | | HJW 56 | .373 | |
| 12,9,5 | DGW 112 | .390 | .390 | | HJW 1-4* | .023 | 1.073 |
| 12,10,4 | DGW 79 | .527 | .527 | 18,22,4 | HJW 162 | .118 | |
| 12,11,5 | DGW 85 | .232 | .232 | | HJW 163 | .126 | .244 |
| 12,12,5 | DGW 86 | .139 | .139 | 18,22,5 | HJW 161 | .032 | .132 |
| 12,24,4 | BMW 41 | .231 | .231 | 18,26,4 | CGW 211 | .776 | .776 |
| 13,11,5 | DGW 56 | .226 | | 19,22,4 | CGW 153 | .085 | .085 |
| | DGW 57 | .013 | | 19,22,5 | CGW 1,2 | .171 | .171 |
| | DGW 58 | .123 | 0.362 | | | | |
| Model: TRURO | | | | | | | |
| 8,21,6 | TSW 115* | 0.696 | 0.696 | 11,14,6 | TSW 78* | 0.358 | 0.358 |
| 10,14,6 | TSW 78* | .358 | .358 | | | | |

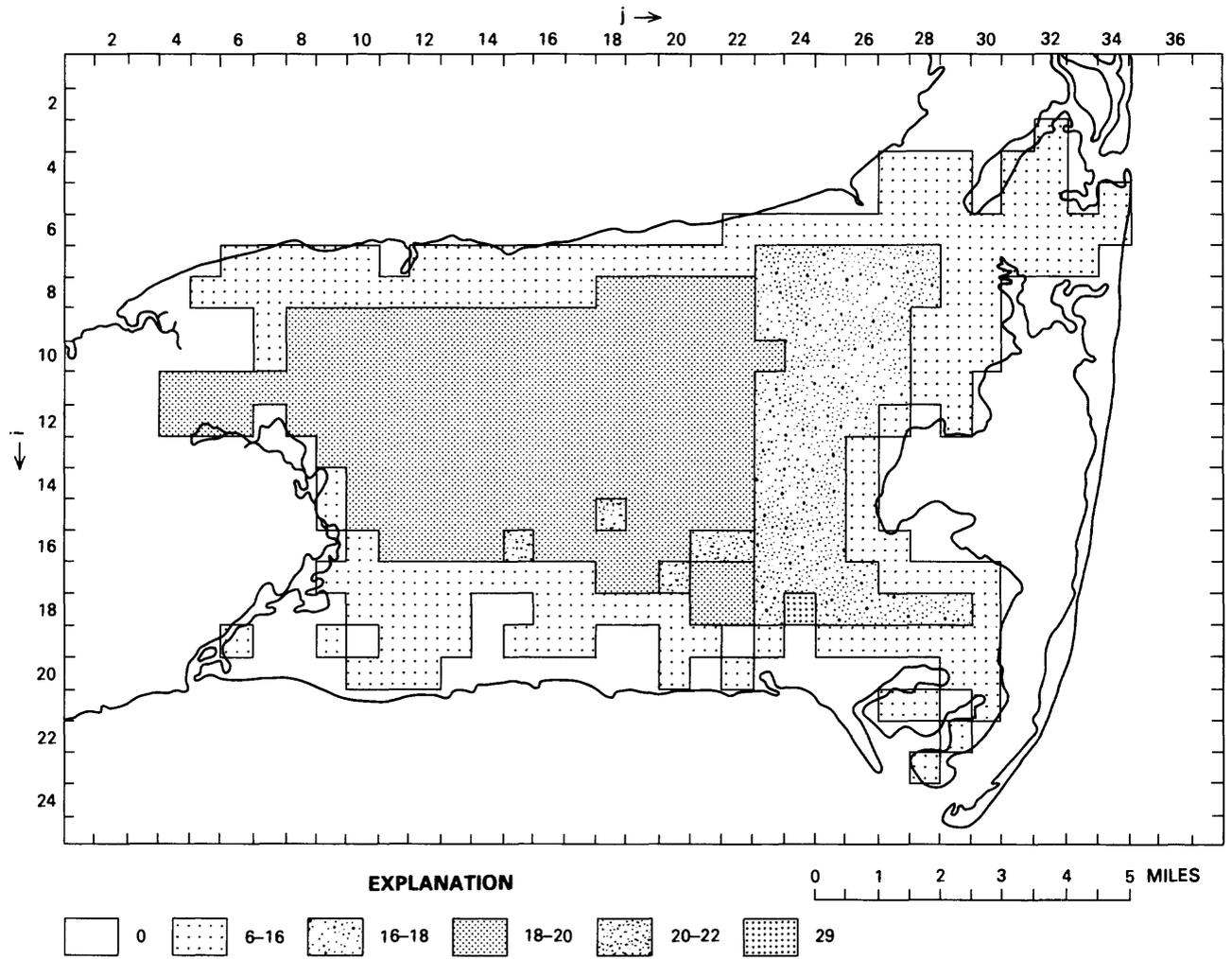


Figure 57. Steady-state recharge, in inches per year, ECAPE model.

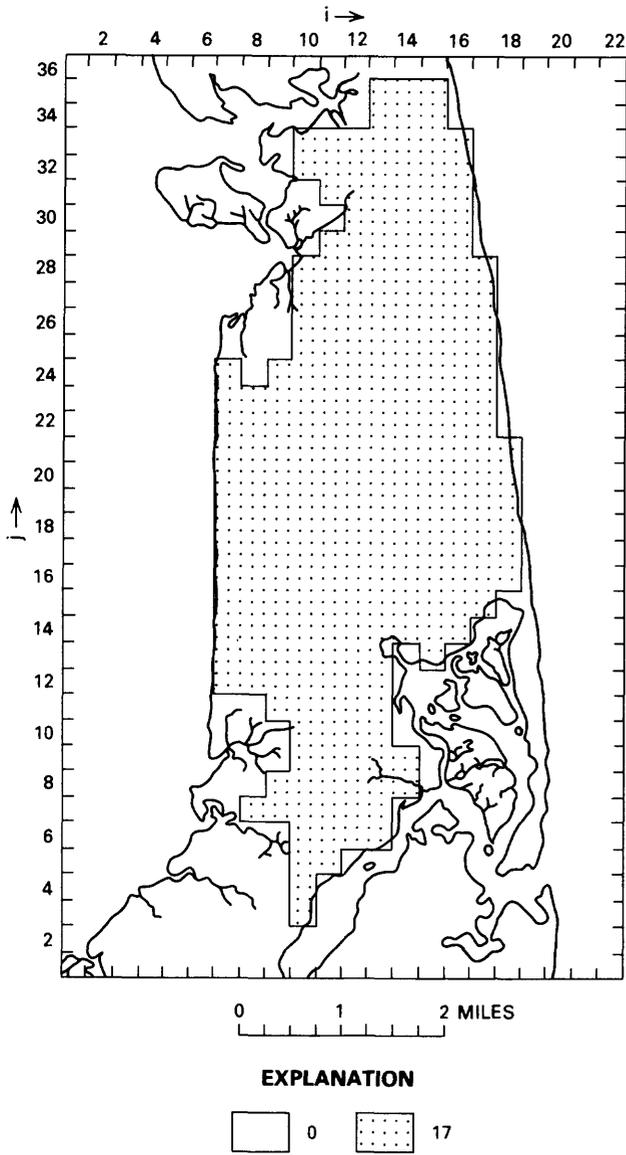


Figure 58. Steady-state recharge, in inches per year, ESTHM model.

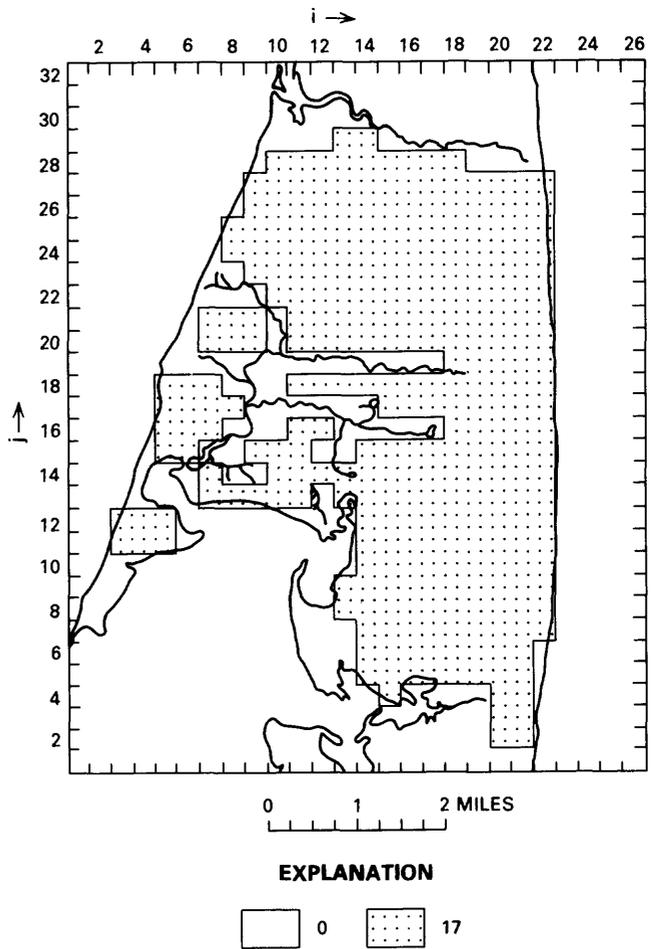


Figure 59. Steady-state recharge, in inches per year, WLFIT model.

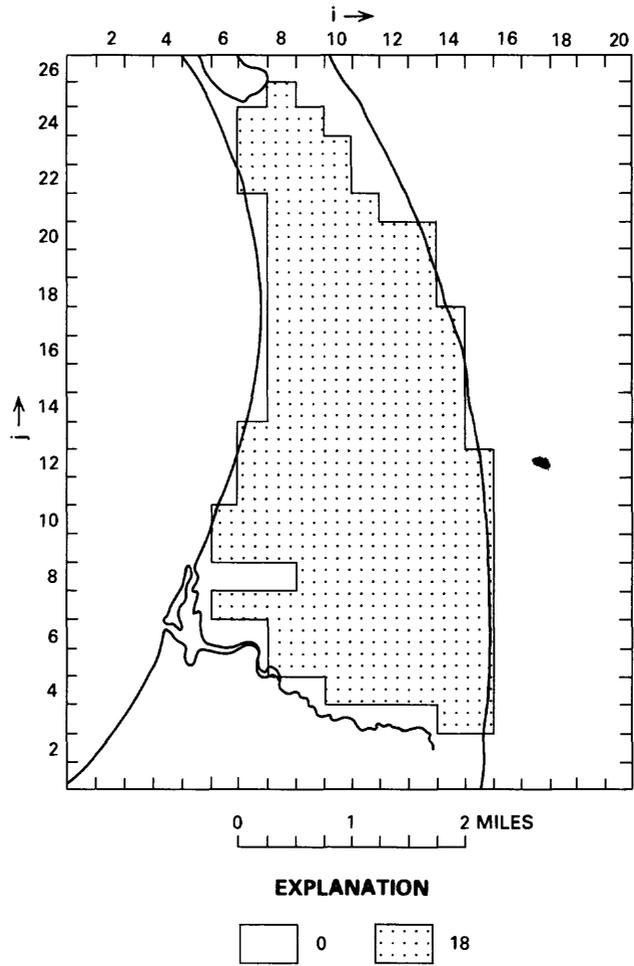


Figure 60. Steady-state recharge, in inches per year, TRURO model.

Table 11. Input documentation

Group I: Title, Simulation Options, and Problem Dimensions
 This group of cards, which are read by the main program, contains data required to dimension the model. To specify an option on card 4 punch the characters underlined in the definition. For an option not used, that section of the card 4 can be left blank.

NOTE: Default typing of variables applies for all data input.

| CARD | COLUMNS | FORMAT | VARIABLE | DEFINITION |
|-------|---------|--------|--|---|
| 1 | 1-80 | 20A4 | HEADING | Any title the user wishes to print on one line at the start of output |
| 2 | 1-52 | 13A4 | HEADING | |
| 3 | 1-10 | I10 | IO | Number of rows |
| | 11-20 | I10 | JO | Number of columns |
| | 21-30 | I10 | KO | Number of layers |
| | 31-40 | I10 | ITMAX | Maximum number of iterations per time step |
| | 41-50 | I10 | NCH | Number of constant head nodes |
| | 51-60 | I10 | NKODE | = 1 for interface solution; ≠ 1 if freshwater/saline water boundary is not to be considered |
| | 61-70 | G10.0 | BETA1 | Parameter to dampen numerical oscillation (usually 0.6 to 1.0) |
| | 71-75 | I5 | KFLOW | = 1 to print flow rates to individual constant head and leaky nodes |
| | 76-80 | I5 | NRIV | Number of leaky "river" nodes |
| | 4 | 1-4 | A4 | IDRAW |
| 6-9 | | A4 | IHEAD | <u>HEAD</u> to print hydraulic head |
| 11-14 | | A4 | IFLOW | <u>MASS</u> to compute a mass balance |
| 16-18 | | A3 | IDK1 | <u>DK1</u> to read initial head, elapsed time, and mass balance parameters from unit 4 on disk |
| 21-23 | | A3 | IDK2 | <u>DK2</u> to write computed head, elapsed time and mass balance parameters on unit 4 (disk) |
| 26-29 | | A4 | IWATER | <u>WATE</u> if the upper hydrologic unit is unconfined |
| 31-34 | | A4 | IQRE | <u>RECH</u> for a constant recharge that may be a function of space |
| 36-39 | | A4 | IPU1 | <u>PUN1</u> to read initial head, elapsed time, and mass balance parameters from cards |
| 41-44 | | A4 | IPU2 | <u>PUN2</u> to punch computed head, elapsed time, and mass balance parameters on cards |
| 46-49 | | A4 | ITK | <u>ITKR</u> to read the values of TK (I,J,K); for simulations in which confining layers are not represented by layers of nodes, $TK(I,J,K) = K_{ZZ}/b$; where variation in aquifer anisotropy is to be approximated, $TK(I,J,K) = 2.0 * K_{ZZ}(K+1) * K_{ZZ}(K) / (K_{ZZ}(K+1) * DELZ(K) + K_{ZZ}(K) * DELZ(K+1))$ (Trescott and Larson, 1976) |
| 51-54 | A4 | IEQN | <u>EQN3</u> if equation 3 is being solved; otherwise it is assumed that equation 4 is being solved | |

Group II: Scalar Parameters

The parameters required in every problem are underlined. The other parameters are required as noted; when not required, their location on the card can be left blank. The G format is used to read E, F, and I format data. Minimize mistakes by always right-justifying data in the field. If F format data do not contain significant figures to the right of the decimal point, the decimal point can be omitted.

| <u>CARD</u> | <u>COLUMNS</u> | <u>FORMAT</u> | <u>VARIABLE</u> | <u>DEFINITION</u> |
|-------------|---|---------------|------------------|--|
| 1 | 1-10 | G10.0 | <u>MPER</u> | Number of pumping periods for the simulation |
| | 11-20 | G10.0 | <u>KTH</u> | Number of times steps between printouts |
| | <u>NOTE:</u> To print only the results for the final time step in a pumping period, make KTH greater than the expected number of time steps. The program always prints the results for the final time step. | | | |
| | 21-30 | G10.0 | <u>ERR</u> | Error criterion for closure (L) |
| | <u>NOTE:</u> When the head change at all nodes on subsequent iterations is less than this value (for example, 0.01 foot), the program has converged to a solution for the time step. | | | |
| | 31-40 | G10.0 | <u>LENGTH</u> | Number of iteration parameters |
| 2 | 1-10 | G10.0 | <u>XSCALE</u> | Factor to convert model length unit to unit used in X direction on maps (e.g. to convert from feet to miles, XSCALE = 5280). <u>For no maps, card 2 is blank</u> |
| | 11-20 | G10.0 | <u>YSCALE</u> | Factor to convert model length unit used in Y direction on maps |
| | 21-30 | G10.0 | <u>DINCH</u> | Number of map units per inch |
| | 31-40 | G10.0 | <u>FACT1</u> | Factor to adjust value of drawdown printed* |
| | 41-49 | 9I1 | <u>LEVEL1(I)</u> | Layers for which drawdown maps are to be printed. List layers starting in column 41; the first zero entry terminates the printing of drawdown maps |
| | 51-60 | G10.D | <u>FACT2</u> | Factor to adjust value of head printed* |
| | 61-69 | 9I1 | <u>LEVEL2(I)</u> | Layers for which head maps are to be printed. List layers starting in column 61; the first zero entry terminates the printing of head maps. |
| | 71-78 | A8 | <u>MESUR</u> | Name of map length unit |

| <u>CARD</u> | <u>COLUMNS</u> | <u>FORMAT</u> | <u>VARIABLE</u> | <u>DEFINITION</u> |
|-------------|----------------|---------------|-----------------|--|
| 3 | 1-20 | G20.10 | SUM | Parameters in which elapsed time and cumulative volumes for mass balance are stored. For the start of a simulation, insert three blank cards. For continuation of a previous run using cards as input, replace the three blank cards with the first three cards of punched output from the previous run. Using data from disk for input, leave the three blank cards in the data deck. |
| | 21-40 | G20.10 | SUMP | |
| | 41-60 | G20.10 | PUMPT | |
| | 61-80 | G20.10 | CFLUXT | |
| 4 | 1-20 | G20.10 | QRET | |
| | 21-40 | G20.10 | CHST | |
| | 41-60 | G20.10 | CHDT | |
| | 61-80 | G20.10 | FLUXT | |
| 5 | 1-20 | G20.10 | STORT | |
| | 21-40 | G20.10 | ETFLXT | |
| | 41-60 | G20.10 | FLXNT | |

* Value of

| drawdown or head | FACT 1 or FACT 2 | Printed value |
|---------------------|------------------------|------------------|
| | 0.01 | 1 |
| | 0.1 | 5 |
| 52.57 | 1.0 | 53 |
| | 10.0 | 526 |
| | 100.0 | *** |

Group III: Array Data

Each of the following data sets (except data set 1) consists of a parameter card, and if the data set contains variable data, a set of data cards. If the data set requires data for each layer, a parameter card and data cards (for layers with variable data) are required for each layer. Each parameter card contains at least five variables.

| <u>CARD</u> | <u>COLUMNS</u> | <u>FORMAT</u> | <u>VARIABLE</u> | <u>DEFINITION</u> |
|----------------------------|----------------|---------------|-----------------|---|
| Every Parameter Card | 1-10 | G10.0 | FAC | If IVAR = 0, FAC is the value assigned to every element of the matrix for this layer If IVAR = 1, FAC is the multiplication factor for the following sets of data cards for this layer |
| | 11-20 | G10.0 | IVAR | = 0 if no data cards are to be read for this layer = 1 if data cards for this layer follow |
| | 21-30 | G10.0 | IPRM | = 0 if input data for this layer are to be printed = 1 if input data for the layer are <u>not</u> to be printed |

Transmissivity Parameter Cards also have these Variables:

| | | | |
|-------|-------|-----------|---|
| 31-40 | G10.0 | FACT(K,1) | Multiplication factor for transmissivity in x direction |
| 41-50 | G10.0 | FACT(K,2) | Multiplication factor for transmissivity in y direction |
| 51-60 | G10.0 | FACT(K,3) | Multiplication factor for hydraulic conductivity in the z direction (Not used when confining bed nodes are eliminated and TK values are read) |

| <u>CARD</u> | <u>COLUMNS</u> | <u>FORMAT</u> | <u>VARIABLE</u> | <u>DEFINITION</u> |
|----------------------------|----------------|---------------|-----------------|---|
| Every Parameter Card | 61-70 | G10.0 | IRECS | = 0 if the matrix is being read from cards or if each element is being set equal to FAC = 1 if the matrix is to be read from disk (unit 2) |
| Every Parameter Card | 71-80 | G10.0 | IRECD | = 0 if the matrix is <u>not</u> to be stored on disk = 1 if the matrix being read from cards or set equal to FAC <u>is</u> to be stored on disk (unit 2) for later retrieval |

When data cards are included, start each row on a new card. To prepare a set of data cards for an array that is a function of space, the general procedure is to overlay the finite-difference grid on a contoured map of the parameter and record the average value of the parameter for each finite-difference block, on coding forms according to the appropriate format. In general, record only significant digits and no decimal points (except for data set 2); use the multiplication factor to convert the data to their appropriate values. For example, if DELX ranges from 1000 to 15000 feet, coded values should range from 1-15; the multiplication factor (FAC) would be 1000.

DATA

| <u>SET</u> | <u>COLUMNS</u> | <u>FORMAT</u> | <u>VARIABLE</u> | <u>DEFINITION</u> |
|--|----------------|---------------|-----------------|--|
| 1 | 1-80 | 8F 10.4 | PHI(I,J,K) | Head values for continuation of a previous run (L) |
| NOTE: For a new simulation this data set is omitted. Do not include a parameter card with this data set. | | | | |
| 2 | 1-80 | 8F 10.4 | STRT(I,J,K) | Starting head matrix (L) |
| 3 | 1-80 | 20F 4.0 | S (I,J,K) | Storage coefficient (dimensionless). If equation 3 is to be solved, read S _s instead of storage coefficient |

NOTE: This matrix is also used to locate constant head boundaries by coding a negative number at constant head nodes. At these nodes, T must be greater than zero.

DATA

| <u>SET</u> | <u>COLUMNS</u> | <u>FORMAT</u> | <u>VARIABLE</u> | <u>DEFINITION</u> |
|------------|----------------|---------------|-----------------|--|
| 4 | 1-80 | 8F 10.4 | T(I,J,K) | Transmissivity (L ² /T). If equation 3 is to be solved, read hydraulic conductivity instead of transmissivity |

- NOTE: 1) Zero values are required around the perimeter of the T matrix for each layer for reasons inherent in the computational scheme. This is done automatically by the program.
- 2) See the previous page for additional requirements on the parameter cards for this data set.
- 3) If the upper active layer is unconfined and PERM and BOTTOM are to be read for this layer, insert a parameter card for this layer with only the values for FACT on it.

| | | | | |
|---|------|---------|-----------|------------|
| 5 | 1-80 | 8F 10.4 | TK(I,J,K) | K_{zz}/b |
|---|------|---------|-----------|------------|

NOTE: This data set is read only if specified in the options. The number of layers of TK values = K0-1. See the discussion of the treatment of confining layers.

| | | | | |
|---|------|---------|-----------|--|
| 6 | 1-80 | 8F 10.4 | PERM(I,J) | Hydraulic conductivity (L/T) (see note 1 for data set 4) |
|---|------|---------|-----------|--|

| | | | | |
|---|------|---------|-------------|---|
| 7 | 1-80 | 20F 4.0 | BOTTOM(I,J) | Elevation of bottom of water-table unit (L) |
|---|------|---------|-------------|---|

NOTE: Data sets 6 and 7 are required only for simulating unconfined conditions in the upper hydrologic unit.

| | | | | |
|---|------|---------|----------|---------------------|
| 8 | 1-80 | 20F 4.0 | QRE(I,J) | Recharge rate (L/T) |
|---|------|---------|----------|---------------------|

NOTE: Omit if not used

DATA

| <u>SET</u> | <u>COLUMNS</u> | <u>FORMAT</u> | <u>VARIABLE</u> | <u>DEFINITION</u> |
|------------|----------------|---------------|-----------------|--|
| 9 | 1-80 | 8F10.0 | DELX(J) | Grid spacing in x direction (L) |
| 10 | 1-80 | 8G10.0 | DELY(I) | Grid spacing in y direction (L) |
| 11 | 1-80 | 8G10.0 | DELZ(K) | Grid spacing in z direction (L) |
| 12 | 1-80 | 80I1 | IDR(I,J,K) | Flag for leaky "river" nodes. = 1 for leaky nodes |
| 13 | 1-10 | G10.0 | FAC | Multiplication factor for head on leaky layer |
| | 1-80 | 20F 4.0 | RH(I) | Head on leaky layer |
| 14 | 1-10 | G10.0 | FAC | Multiplication factor for elevation of bottom of leaky layer |
| | 1-80 | 20F 4.0 | RB(I) | Elevation of bottom of leaky layer |
| 15 | 1-10 | G10.0 | FAC | Multiplication factor for leakance coefficient |
| | 1-80 | 10F 8.0 | RC(I) | Leakance coefficient (K_2/M) |

Group IV: Parameters that Change with the Pumping Period

The program has two options for the simulation period:

1. To simulate a given number of time steps, set TMAX to a value larger than the expected simulation period. The program will use NUMT, CDLT, and DELT as coded.
2. To simulate a given pumping period, set NUMT larger than the number required for the simulation period (for example, 50). The program will compute the exact DELT (which will be \leq DELT coded) and NUMT to arrive exactly at TMAX on the last step.

| <u>CARD</u> | <u>COLUMNS</u> | <u>FORMAT</u> | <u>VARIABLE</u> | <u>DEFINITION</u> |
|----------------------------------|----------------|---------------|-----------------|---|
| 1 | 1-10 | G10.0 | KP | Number of the pumping period |
| | 11-20 | G10.0 | KPM1 | Number of the previous pumping period |
| NOTE: KPM1 is currently not used | | | | |
| | 21-30 | G10.0 | NWEL | Number of wells for this pumping period |
| | 31-40 | G10.0 | TMAX | Number of days in this pumping period |
| | 41-50 | G10.0 | NUMT | Number of time steps |
| | 51-60 | G10.0 | CDLT | Multiplying factor for DELT |
| NOTE: 1.5 is commonly used | | | | |
| | 61-70 | G10.0 | DELT | Initial time step in hours |

If NWEL = 0 the following set of cards is omitted

| <u>DATA SET 1</u> | | | | (NWEL cards) |
|-------------------|---------------|-----------------|---|--------------|
| <u>COLUMNS</u> | <u>FORMAT</u> | <u>VARIABLE</u> | <u>DEFINITION</u> | |
| 1-10 | G10.0 | K | Layer in which well is located | |
| 11-20 | G10.0 | I | Row location of well | |
| 21-30 | G10.0 | J | Column location of well | |
| 31-40 | G10.0 | WELL (I,J,K) | Pumping rate (L^3/T), negative for a pumping well | |

For each additional pumping period, another set of group IV cards is required (that is, NPER sets of group IV cards are required).

Group V: Interface and Optional Print Parameters

(These data cards follow the first set of Group IV cards)

| <u>CARD</u> | <u>COLUMNS</u> | <u>FORMAT</u> | <u>VARIABLE</u> | <u>DEFINITION</u> |
|-------------|----------------|---------------|-----------------|--|
| 1 | 1-10 | I10 | NTAD | Transmissivity adjustment code. Specifies frequency of transmissivity adjustment (e.g. = 1 every iteration; = 4 every fourth iteration) |
| | 11-20 | G10.0 | RF | Density factor |
| | 21-30 | I10 | KODCHP | = 1 Transmissivity adjustment proceeds across grid from north to south; otherwise adjustment proceeds from both north and south to mid-point of grid |
| | 31-40 | I10 | KPRNTT | = 1 write to file 9 final PHI, T, PERM and TK matrices |

DATA SET

| | | | | |
|---|------|---------|-----------|---|
| 1 | 1-80 | 16F 5.0 | DBOT(I,J) | Elevation of bottom of aquifer (positive downward). Read similar to array data. |
| 2 | 1-80 | 80I1 | ISEA(I,J) | Flag for "ocean" nodes. Assign value equal to node layer. Ocean nodes which "leak" into aquifer are treated as saline-water nodes |
| 3 | 1-80 | 8G10.0 | DTOP(K) | Elevation of top of layers (positive downward) |

Group VI: Additional Print Parameters

CARD

| | | | | |
|---|-------|-------------------|--------|--|
| 1 | 1-10 | I10 | NODKOD | = 1 to print head values at each iteration for selected nodes |
| | 11-20 | I10 | NMBRND | Number of nodes (10 maximum) for which head values are to be printed |
| 2 | 1-80 | 10(I3, I3, I2) | AA(I) | I, J, K location of nodes for which head values are to be printed. If NODKOD not equal to 1 omit this card |
| 3 | 1-10 | I10 | IHEDKD | = 1 write to file 8 final head values for selected nodes |

DATA

| <u>SET</u> | <u>COLUMNS</u> | <u>FORMAT</u> | <u>VARIABLE</u> | <u>DEFINITION</u> |
|------------|----------------|---------------|-----------------|---|
| 1 | 1-80 | 80I1 | IFNLHD | (I,J,K) Index array to identify those nodes for which final head values are to be printed in table form. Two passes are made through the array. The first pass is made only on the top layer and prints head values for those nodes where IFNLHD = 1. The second pass is made through the entire array and prints head values for those nodes where IFNLHD = 2. If IHEDKD is not equal to 1, omit this data set |

Table 12. Computer source code

(*Indicates a change from Trescott, 1975, and Trescott and Larson, 1976.)

```

C -----MAN0010
C FINITE-DIFFERENCE MODEL FOR SIMULATION OF GROUND-WATER FLOW IN MAN0020
C THREE DIMENSIONS, SEPTEMBER 1975 BY P.C. TRESMOTT, U. S. G. S. MAN0030
C WITH CONTRIBUTIONS TO MAIN, DATAI AND SOLVE BY S.P. LARSON MAN0040
C AND MODIFICATIONS NOTED BY ASTERISKS BY J.H.GUSWA AND D.R.LEBLANC MAN0045*
C -----MAN0050
C
C SPECIFICATIONS: MAN0060
C REAL *8YSTR MAN0070
C
C DIMENSION Y(177500), L(29), HEADNG(33), NAME(42), INFT(2,2), IOFT(MAN0100*
19,4), DUM(3) MAN0110
C
C EQUIVALENCE (YSTR,Y(1)) MAN0120
C
C COMMON /INTEGR/ IO,J0,K0,I1,J1,K1,I,J,K,NPER,KTH,ITMAX,LENGTH,KP,NMAN0150
1WEL,NUMT,IFINAL,IT,KT,IHEAD,IDRAW,IFLO,IERR,I2,J2,K2,IMAX,ITMX1,NCMAN0160
2H,IDK1,IDK2,IWATER,IQRE,IP,JP,IQ,JQ,IK,JK,K5,IPU1,IPU2,ITK,IEQN MAN0170
3,NKODE,KFLOW MAN0180*
COMMON /SPARAM/ TMAX,CDLT,DELT,ERR,TEST,SUM,SUMP,QR,BETA1 MAN0190*
COMMON /SARRAY/ ICHK(13),LEVEL1(9),LEVEL2(9) MAN0200
C
C DATA NAME/2*4H ,4H S,4HTART,4HING ,4HHEAD,4H ,4H STO,4HHRAGMAN0220
1E,4H COE,4HFFIC,4HIENT,2*4H ,4H TR,4HANSM,4HISSI,4HVITY,5*4H MAN0230
2 ,4H TK,4H HY,4HDRAU,4HLIC ,4HCOND,4HUCTI,4HVITY,2*4H ,4HBOTMAN0240
3T,4HOM E,4HLEVA,4HTION,2*4H ,4H R,4HECHA,4HRGE ,4HRATE/ MAN0250
DATA INFT/4H(20F,4H4.0),4H(8F1,4H0.4)/ MAN0260
DATA IOFT 4H(1H0,4H,I2,,4H2X,2,4HOF6.,4H1/(5,4HX,20,4HF6.1,4H)) ,MAN0270
14H ,4H(1H0,4H,I5,,4H14F9,4H.5/(,4H1H ,4H5X,1,4H4F9.,4H5)) ,4H MAN0280
2 ,4H(1H0,4H,I5,,4H10E1,4H2.5/,4H(1H ,4H,5X,,4H10E1,4H2.5),4H) MAN0290
3,4H(1H0,4H,I5,,4H10E1,4H1.3/,4H(1H ,4H,5X,,4H10E1,4H1.3),4H) / MAN0300
C
C DEFINE FILE 2(8,1520,U,KKK) MAN0310
C ..... MAN0320
C ..... MAN0330
C ..... MAN0340
C ---READ TITLE, PROGRAM SIZE AND OPTIONS--- MAN0350
C READ (5,200) HEADNG MAN0360
C WRITE (6,190) HEADNG MAN0370
C READ (5,160) IO,J0,K0,ITMAX,NCH,NKODE,BETA1,KFLOW,NRIV MAN0380*
C WRITE (6,180) IO,J0,K0,ITMAX,NCH,NKODE,BETA1,KFLOW,NRIV MAN0390*
C READ (5,210) IDRAW,IHEAD,IFLO,IDK1,IDK2,IWATER,IQRE,IPU1,IPU2,ITK MAN0400
1,IEQN MAN0410
C WRITE (6,220) IDRAW,IHEAD,IFLO,IDK1,IDK2,IWATER,IQRE,IPU1,IPU2,ITKMAN0420
1,IEQN MAN0430
C IERR=0 MAN0440
C
C ---COMPUTE DIMENSIONS FOR ARRAYS--- MAN0450
C J1=J0-1 MAN0460
C I1=I0-1 MAN0470
C K1=K0-1 MAN0480
C I2=I0-2 MAN0490
C J2=J0-2 MAN0500
C K2=K0-2 MAN0510
C IMAX=MAX0(IO,J0) MAN0520
C NCD=MAX0(1,NCH) MAN0530
C ITMX1=ITMAX+1 MAN0540
C ISIZ=IO*J0*K0 MAN0550
C IK1=IO*J0 MAN0570

```

| | |
|---------------------------------|---------|
| IK2=MAX0(IK1*K1,1) | MAN0580 |
| ISUM=2*ISIZ+1 | MAN0590 |
| L(1)=1 | MAN0600 |
| DO 30 I=2,14 | MAN0610 |
| IF (I.NE.8) GO TO 20 | MAN0620 |
| L(8)=ISUM | MAN0630 |
| ISUM=ISUM+IK2 | MAN0640 |
| IF (IK2.EQ.1) GO TO 10 | MAN0650 |
| IK=I0 | MAN0660 |
| JK=J0 | MAN0670 |
| K5=K1 | MAN0680 |
| GO TO 30 | MAN0690 |
| 10 IK=1 | MAN0700 |
| JK=1 | MAN0710 |
| K5=1 | MAN0720 |
| GO TO 30 | MAN0730 |
| 20 L(I)=ISUM | MAN0740 |
| ISUM=ISUM+ISIZ | MAN0750 |
| 30 CONTINUE | MAN0760 |
| L(15)=ISUM | MAN0770 |
| ISUM=ISUM+J0 | MAN0780 |
| L(16)=ISUM | MAN0790 |
| ISUM=ISUM+I0 | MAN0800 |
| L(17)=ISUM | MAN0810 |
| ISUM=ISUM+K0 | MAN0820 |
| L(18)=ISUM | MAN0830 |
| ISUM=ISUM+IMAX | MAN0840 |
| L(19)=ISUM | MAN0850 |
| ISUM=ISUM+K0*3 | MAN0860 |
| L(20)=ISUM | MAN0870 |
| ISUM=ISUM+ITMX1 | MAN0880 |
| L(21)=ISUM | MAN0890 |
| ISUM=ISUM+3*NCD | MAN0900 |
| L(22)=ISUM | MAN0910 |
| ISUM=ISUM+NCD | MAN0920 |
| L(23)=ISUM | MAN0930 |
| IF (IWATER.NE.ICHK(6)) GO TO 40 | MAN0940 |
| ISUM=ISUM+IK1 | MAN0950 |
| L(24)=ISUM | MAN0960 |
| ISUM=ISUM+IK1 | MAN0970 |
| IP=I0 | MAN0980 |
| JP=J0 | MAN0990 |
| GO TO 50 | MAN1000 |
| 40 ISUM=ISUM+1 | MAN1010 |
| L(24)=ISUM | MAN1020 |
| ISUM=ISUM+1 | MAN1030 |
| IP=1 | MAN1040 |
| JP=1 | MAN1050 |
| 50 L(25)=ISUM | MAN1060 |
| IF (IQRE.NE.ICHK(7)) GO TO 60 | MAN1070 |
| ISUM=ISUM+IK1 | MAN1080 |
| IQ=I0 | MAN1090 |
| JQ=J0 | MAN1100 |
| GO TO 70 | MAN1110 |
| 60 ISUM=ISUM+1 | MAN1120 |
| IQ=1 | MAN1130 |
| JQ=1 | MAN1140 |

| | | |
|----|--|----------|
| 70 | IF(NRIV.EQ.0) GO TO 75 | MAN1150* |
| | L(26)=ISUM | MAN1160* |
| | ISUM=ISUM+ISIZ | MAN1170* |
| | L(27)=ISUM | MAN1180* |
| | ISUM=ISUM+NRIV | MAN1190* |
| | L(28)=ISUM | MAN1200* |
| | ISUM=ISUM+NRIV | MAN1210* |
| | L(29)=ISUM | MAN1220* |
| | ISUM=ISUM+NRIV | MAN1230* |
| | GO TO 79 | MAN1240* |
| 75 | L(26)=ISUM | MAN1250* |
| | L(27)=ISUM+1 | MAN1260* |
| | L(28)=ISUM+2 | MAN1270* |
| | L(29)=ISUM+3 | MAN1280* |
| | ISUM=ISUM+4 | MAN1290* |
| 79 | WRITE (6,170) ISUM | MAN1300* |
| | | MAN1310 |
| C | --- <td>MAN1320</td> | MAN1320 |
| C | CALL DATAI(Y(L(1)),Y(L(2)),Y(L(3)),Y(L(4)),Y(L(5)),Y(L(6)),Y(L(7)), | MAN1330 |
| | 1,Y(L(8)),Y(L(9)),Y(L(15)),Y(L(16)),Y(L(17)),Y(L(19)),Y(L(23)),Y(L(| MAN1340 |
| | 224)),Y(L(25)), | MAN1350 |
| | 3Y(L(26)),Y(L(27)),Y(L(28)),Y(L(29))) | MAN1360* |
| | CALL STEP(Y(L(1)),Y(L(2)),Y(L(3)),Y(L(4)),Y(L(5)),Y(L(6)),Y(L(7)), | MAN1370 |
| | 1Y(L(8)),Y(L(9)),Y(L(15)),Y(L(16)),Y(L(17)),Y(L(19)),Y(L(18)),Y(L(2 | MAN1380 |
| | 20))) | MAN1390 |
| | CALL SOLVE(Y(L(1)),Y(L(2)),Y(L(3)),Y(L(4)),Y(L(5)),Y(L(6)),Y(L(7)), | MAN1400 |
| | 1,Y(L(8)),Y(L(9)),Y(L(15)),Y(L(16)),Y(L(17)),Y(L(19)),Y(L(10)),Y(L(| MAN1410 |
| | 211)),Y(L(12)),Y(L(13)),Y(L(14)),Y(L(20)),Y(L(25)),Y(L(24)), | MAN1420 |
| | 3Y(L(23)),Y(L(26)),Y(L(27)),Y(L(28)),Y(L(29)),NRIV) | MAN1430* |
| | CALL COEF(Y(L(1)),Y(L(2)),Y(L(3)),Y(L(4)),Y(L(5)),Y(L(6)),Y(L(7)), | MAN1440 |
| | 1Y(L(8)),Y(L(9)),Y(L(15)),Y(L(16)),Y(L(17)),Y(L(19)),Y(L(23)),Y(L(2 | MAN1450 |
| | 24)),Y(L(25))) | MAN1460 |
| | CALL CHECKI(Y(L(1)),Y(L(2)),Y(L(3)),Y(L(4)),Y(L(5)),Y(L(6)),Y(L(7)), | MAN1470 |
| | 1),Y(L(8)),Y(L(9)),Y(L(15)),Y(L(16)),Y(L(17)),Y(L(19)),Y(L(21)),Y(L(| MAN1480 |
| | 2(22)),Y(L(25)), | MAN1490 |
| | 3Y(L(26)),Y(L(27)),Y(L(28)),Y(L(29)),NRIV) | MAN1500* |
| | CALL PRNTAI(Y(L(1)),Y(L(2)),Y(L(4)),Y(L(5)),Y(L(9)),Y(L(15)),Y(L(1 | MAN1510 |
| | 16))) | MAN1520 |
| C | | MAN1530 |
| C | ---START COMPUTATIONS--- | MAN1540 |
| C | ***** | MAN1550 |
| C | ---READ AND WRITE DATA FOR GROUPS II AND III--- | MAN1560 |
| | CALL DATAIN | MAN1570 |
| | IRN=1 | MAN1580 |
| | NIJ=I0*J0 | MAN1590 |
| | DO 80 K=1,K0 | MAN1600 |
| | LOC=L(2)+(K-1)*NIJ | MAN1610 |
| 80 | CALL ARRAY(Y(LOC),INFT(1,2),IOFT(1,1),NAME(1),IRN,DUM) | MAN1620 |
| | DO 90 K=1,K0 | MAN1630 |
| | LOC=L(5)+(K-1)*NIJ | MAN1640 |
| 90 | CALL ARRAY(Y(LOC),INFT(1,1),IOFT(1,2),NAME(7),IRN,DUM) | MAN1650 |
| | DO 100 K=1,K0 | MAN1660 |
| | LOC=L(4)+(K-1)*NIJ | MAN1670 |
| | L1=L(19)+K-1 | MAN1680 |
| | L2=L(19)+K0+K-1 | MAN1690 |
| | L3=L(19)+2*K0+K-1 | MAN1700 |
| | CALL ARRAY(Y(LOC),INFT(1,2),IOFT(1,2),NAME(13),IRN,DUM) | MAN1710 |

| | | |
|-----|--|----------|
| | Y(L1)=DUM(1) | MAN1720 |
| | Y(L2)=DUM(2) | MAN1730 |
| | Y(L3)=DUM(3) | MAN1740 |
| 100 | WRITE (6,230) K,Y(L1),Y(L2),Y(L3) | MAN1750 |
| | IF (ITK.NE.ICHK(10)) GO TO 120 | MAN1760 |
| | DO 110 K=1,K1 | MAN1770 |
| | LOC=L(8)+(K-1)*NIJ | MAN1780 |
| 110 | CALL ARRAY(Y(LOC),INFT(1,2),IOFT(1,3),NAME(19),IRN,DUM) | MAN1790 |
| 120 | IF (IWATER.NE.ICHK(6)) GO TO 130 | MAN1800 |
| | K = K0 | MAN1810 |
| | CALL ARRAY(Y(L(23)),INFT(1,2),IOFT(1,4),NAME(25),IRN,DUM) | MAN1820 |
| | CALL ARRAY(Y(L(24)),INFT(1,1),IOFT(1,1),NAME(31),IRN,DUM) | MAN1830 |
| 130 | IF (IQRE.EQ.ICHK(7)) CALL ARRAY(Y(L(25)),INFT(1,1),IOFT(1,4),NAME(| MAN1840 |
| | 137),IRN,DUM) | MAN1850 |
| | CALL MDAT | MAN1860 |
| | IF(NRIV.NE.0) CALL DDAT2(NRIV) | MAN1870* |
| C | | MAN1880 |
| C | ---COMPUTE TRANSMISSIVITY FOR UNCONFINED LAYER--- | MAN1890 |
| | IF (IWATER.EQ.ICHK(6)) CALL TRANS(1) | MAN1900 |
| C | | MAN1910 |
| C | ---COMPUTE T COEFFICIENTS--- | MAN1920 |
| | CALL TCOF | MAN1930 |
| C | | MAN1940 |
| C | ---COMPUTE ITERATION PARAMETERS--- | MAN1950 |
| | CALL ITER | MAN1960 |
| C | | MAN1970 |
| C | ---READ TIME PARAMETERS AND PUMPING DATA FOR A NEW PUMPING PERIOD--- | MAN1980 |
| 140 | CALL NEWPER | MAN1990 |
| C | | MAN2000 |
| | KT=0 | MAN2010 |
| | IFINAL=0 | MAN2020 |
| C | | MAN2030 |
| C | ---START NEW TIME STEP COMPUTATIONS--- | MAN2040 |
| 150 | CALL NEWSTP | MAN2050 |
| C | | MAN2060 |
| C | ---START NEW ITERATION IF MAXIMUM NO. ITERATIONS NOT EXCEEDED--- | MAN2070 |
| | CALL NEWITA | MAN2080 |
| C | | MAN2090 |
| C | ---PRINT OUTPUT AT DESIGNATED TIME STEPS--- | MAN2100 |
| | CALL OUTPUT | MAN2110 |
| C | | MAN2120 |
| C | ---LAST TIME STEP IN PUMPING PERIOD ?--- | MAN2130 |
| | IF (IFINAL.NE.1) GO TO 150 | MAN2140 |
| C | | MAN2150 |
| C | ---CHECK FOR NEW PUMPING PERIOD--- | MAN2160 |
| | IF (KP.LT.NPER) GO TO 140 | MAN2170 |
| C | | MAN2180 |
| | STOP | MAN2190 |
| C | | MAN2200 |
| C | ---FORMATS--- | MAN2210 |
| C | | MAN2220 |
| C | | MAN2230 |
| C | | MAN2240 |
| 160 | FORMAT (6I10,G10.0,2I5) | MAN2250* |
| 170 | FORMAT ('0',54X,'WORDS OF VECTOR Y USED =',I7) | MAN2260 |
| 180 | FORMAT ('0',62X,'NUMBER OF ROWS =',I5/60X,'NUMBER OF COLUMNS =',I5 | MAN2270 |
| | 1/61X,'NUMBER OF LAYERS =',I5//39X,'MAXIMUM PERMITTED NUMBER OF ITEM | MAN2280 |

```

2RATIONS =',I5//48X,'NUMBER OF CONSTANT HEAD NODES =',I5//72X,      MAN2290
3'NKODE =',I5//53X,'BETA ITERATION PARAMETER =',F5.2,                MAN2300*
4//72X,'KFLOW =',I5,                                                 MAN2310*
5//56X,'NUMBER OF LEAKY NODES =',I5)                                  MAN2320*
190 FORMAT ('1',33A4)                                                MAN2330
200 FORMAT (20A4)                                                     MAN2340
210 FORMAT (16(A4,1X))                                               MAN2350
220 FORMAT ('-SIMULATION OPTIONS: ',11(A4,4X))                       MAN2360
230 FORMAT (1H0,44X,'DIRECTIONAL TRANSMISSIVITY MULTIPLICATION FACTORSMAN2370
1 FOR LAYER',I3,/,76X,'X =',G15.7/76X,'Y =',G15.7/76X,'Z =',G15.7) MAN2380
END                                                                    MAN2390

```

```

SUBROUTINE DATAI(PHI,STRT,OLD,T,S,TR,TC,TK,WELL,DELX,DELY,DELZ,FACDAT0010
1T,PERM,BOTTOM,QRE,                                DAT0020*
2IDR,RH,RC,RB)                                    DAT0030*
-----DAT0040
C READ AND WRITE DATA                                DAT0050
C -----DAT0060
C SPECIFICATIONS:                                    DAT0070
C REAL *8PHI                                          DAT0080
C REAL *8PHI                                          DAT0090
C REAL *8XLABEL,YLABEL,TITLE,XN1,MESUR              DAT0100
C                                                     DAT0110
DIMENSION PHI(I0,J0,K0),STRT(I0,J0,K0),OLD(I0,J0,K0),T(I0,J0,K0)DAT0120
1),S(I0,J0,K0),TR(I0,J0,K0),TC(I0,J0,K0),TK(IK,JK,K5),WELL(I0,DATA0130
2J0,K0),DELX(J0),DELY(I0),DELZ(K0),FACT(K0,3),PERM(IP,JP),BOTDATA0140
3TOM(IP,JP),QRE(IQ,JQ),TF(3),A(I0,J0),IN(6),IOFT(9),INFT(2) DAT0150*
4,IDR(I0,J0,K0),RH(1),RC(1),RB(1)                DAT0160*
C                                                     DAT0170
COMMON /INTEGR/ I0,J0,K0,I1,J1,K1,I,J,K,NPER,KTH,ITMAX,LENGTH,KP,NDATA0180
1WEL,NUMT,IFINAL,IT,KT,IHEAD,IDRAW,IFLO,IERR,I2,J2,K2,IMAX,ITMX1,MCDATA0190
2H,IDK1,IDK2,IWATER,IQRE,IP,JP,IQ,JQ,IK,JK,K5,IPU1,IPU2,ITK,IEQN  DAT0200
3,NKODE,KFLOW                                     DAT0210*
COMMON /SPARAM/ TMAX,CDLT,DELT,ERR,TEST,SUM,SUMP,QR,BETA1  DAT0220*
COMMON /SARRAY/ ICHK(13),LEVEL1(9),LEVEL2(9)             DAT0230
COMMON /CK/ RLFLXT,STORT,QRET,CHST,CHDT,FLUXT,PUMPT,CFLUXT,FLXNT DAT0240*
COMMON /PR/ XLABEL(3),YLABEL(6),TITLE(6),XN1,MESUR,PRNT(122),BLANKDATA0250
1(60),DIGIT(122),VF1(6),VF2(6),VF3(7),XSCALE,DINCH,SYM(17),XM(100),DATA0260
2YN(13),NA(4),N1,N2,N3,YSCALE,FACT1,FACT2             DAT0270
RETURN                                                 DAT0280
C .....DAT0290
C *****DAT0300
C ENTRY DATAIN                                       DAT0310
C *****DAT0320
C -----DAT0330
C ---READ AND WRITE SCALAR PARAMETERS---             DAT0340
C READ (5,330) NPER,KTH,ERR,LENGTH                   DAT0350
C WRITE (6,340) NPER,KTH,ERR                          DAT0360
C READ (5,460) XSCALE,YSCALE,DINCH,FACT1,(LEVEL1(I),I=1,9),FACT2,(LEDA0370
1VEL2(I),I=1,9),MESUR                                DAT0380
C IF (XSCALE.NE.0.) WRITE (6,470) XSCALE,YSCALE,MESUR,MESUR,DINCH,FADATA0390
1CT1,LEVEL1,FACT2,LEVEL2                             DAT0400
C                                                     DAT0410
C ---READ CUMULATIVE MASS BALANCE PARAMETERS---      DAT0420
C READ (5,450) SUM,SUMP,PUMPT,CFLUXT,QRET,CHST,CHDT,FLUXT,STORT,ETFLDATA0430
1XT,FLXNT                                             DAT0440
C IF (IDK1.EQ.ICHK(4)) GO TO 20                       DAT0450
C IF (IPU1.NE.ICHK(8)) GO TO 50                       DAT0460
C                                                     DAT0470
C -----DAT0480
C ---READ INITIAL HEAD VALUES FROM CARDS---         DAT0490
C DO 10 K=1,K0                                        DAT0500
C DO 10 I=1,I0                                        DAT0510
C 10 READ (5,360) (PHI(I,J,K),J=1,J0)                DAT0520
C GO TO 30                                            DAT0530
C -----DAT0540
C ---READ INITIAL HEAD AND MASS BALANCE PARAMETERS FROM DISK--- DAT0550
C 20 READ (4) PHI,SUM,SUMP,PUMPT,CFLUXT,QRET,CHST,CHDT,FLUXT,STORT,ETFLDATA0550
1XT,FLXNT                                             DAT0560
C REWIND 4                                            DAT0570

```

```

30 WRITE (6,430) SUM
DO 40 K=1,K0
WRITE (6,440) K
DO 40 I=1,I0
40 WRITE (6,350) I,(PHI(I,J,K),J=1,J0)
C
50 DO 60 K=1,K0
DO 60 I=1,I0
DO 60 J=1,J0
WELL(I,J,K)=0.
TR(I,J,K)=0.
TC(I,J,K)=0.
IF (K.NE.K0) TK(I,J,K)=0.
60 CONTINUE
RETURN
C *****
ENTRY ARRAY(A,INFT,IOFT,IN,IRN,TF)
C *****
READ (5,330) FAC,IVAR,IPRN,TF,IRECS,IRECD
IC=4*IRECS+2*IVAR+IPRN+1
GO TO (70,70,90,90,120,120), IC
70 DO 80 I=1,I0
DO 80 J=1,J0
80 A(I,J)=FAC
WRITE (6,280) IN,FAC,K
GO TO 140
90 IF (IC.EQ.3) WRITE (6,290) IN,K
DO 110 I=1,I0
READ (5,IMFT) (A(I,J),J=1,J0)
DO 100 J=1,J0
100 A(I,J)=A(I,J)*FAC
110 IF (IC.EQ.3) WRITE (6,IOFT) I,(A(I,J),J=1,J0)
GO TO 140
120 READ (2'IRN) A
IF (IC.EQ.6) GO TO 140
WRITE (6,290) IN,K
DO 130 I=1,I0
130 WRITE (6,IOFT) I,(A(I,J),J=1,J0)
140 IF (IRECD.EQ.1) WRITE (2'IRN) A
IRN=IRN+1
RETURN
C *****
ENTRY MDAT
C *****
DO 150 K=1,K0
DO 150 I=1,I0
DO 150 J=1,J0
IF (I.EQ.1.OR.I.EQ.I0.OR.J.EQ.1.OR.J.EQ.J0) T(I,J,K)=0.
IF (IDK1.NE.ICHK(4).AND.IPU1.NE.ICHK(8)) PHI(I,J,K)=STRT(I,J,K)
IF (K.NE.K0.OR.IWATER.NE.ICHK(6)) GO TO 150
IF (I.EQ.1.OR.I.EQ.I0.OR.J.EQ.1.OR.J.EQ.J0) PERM(I,J)=0.
150 CONTINUE
READ (5,330) FAC,IVAR,IPRN
C ..... DELX .....
IF (IVAR.EQ.1) READ (5,330) (DELX(J),J=1,J0)
DO 170 J=1,J0
IF (IVAR.NE.1) GO TO 160

```

```

DAT0580
DAT0590
DAT0600
DAT0610
DAT0620
DAT0630
DAT0640
DAT0650
DAT0660
DAT0670
DAT0680
DAT0690
DAT0700
DAT0710
DAT0720
DAT0730
DAT0740
DAT0750
DAT0760
DAT0770
DAT0780
DAT0790
DAT0800
DAT0810
DAT0820
DAT0830
DAT0840
DAT0850
DAT0860
DAT0870
DAT0880
DAT0890
DAT0900
DAT0910
DAT0920
DAT0930
DAT0940
DAT0950
DAT0960
DAT0970
DAT0980
DAT0990
DAT1000
DAT1010
DAT1020
DAT1030
DAT1040
DAT1050
DAT1060
DAT1070
DAT1080
DAT1090
DAT1100
DAT1110
DAT1120
DAT1130
DAT1140

```

| | | |
|-----|---|----------|
| | DELX(J)=DELX(J)*FAC | DAT1150 |
| | GO TO 170 | DAT1160 |
| 160 | DELX(J)=FAC | DAT1170 |
| 170 | CONTINUE | DAT1180 |
| | IF (IVAR.EQ.1.AND.IPRN.NE.1) WRITE (6,370) (DELX(J),J=1,J0) | DAT1190 |
| | IF (IVAR.EQ.0) WRITE (6,300) FAC | DAT1200 |
| C | DELY | DAT1210 |
| | READ (5,330) FAC,IVAR,IPRN | DAT1220 |
| | IF (IVAR.EQ.1) READ (5,330) (DELY(I),I=1,I0) | DAT1230 |
| | DO 190 I=1,I0 | DAT1240 |
| | IF (IVAR.NE.1) GO TO 180 | DAT1250 |
| | DELY(I)=DELY(I)*FAC | DAT1260 |
| | GO TO 190 | DAT1270 |
| 180 | DELY(I)=FAC | DAT1280 |
| 190 | CONTINUE | DAT1290 |
| | IF (IVAR.EQ.1.AND.IPRN.NE.1) WRITE (6,380) (DELY(I),I=1,I0) | DAT1300 |
| | IF (IVAR.EQ.0) WRITE (6,310) FAC | DAT1310 |
| C | DELZ | DAT1320 |
| | READ (5,330) FAC,IVAR,IPRN | DAT1330 |
| | IF (IVAR.EQ.1) READ (5,330) (DELZ(K),K=1,K0) | DAT1340 |
| | DO 210 K=1,K0 | DAT1350 |
| | IF (IVAR.NE.1) GO TO 200 | DAT1360 |
| | DELZ(K)=DELZ(K)*FAC | DAT1370 |
| | GO TO 210 | DAT1380 |
| 200 | DELZ(K)=FAC | DAT1390 |
| 210 | CONTINUE | DAT1400 |
| | IF (IVAR.EQ.1.AND.IPRN.NE.1) WRITE (6,390) (DELZ(K),K=1,K0) | DAT1410 |
| | IF (IVAR.EQ.0) WRITE (6,320) FAC | DAT1420 |
| C | | DAT1430 |
| C | ---INITIALIZE VARIABLES--- | DAT1440 |
| | B=0. | DAT1450 |
| | D=0. | DAT1460 |
| | F=0. | DAT1470 |
| | H=0. | DAT1480 |
| | SU=0. | DAT1490 |
| | Z=0. | DAT1500 |
| | IF (XSCALE.NE.0.) CALL MAP | DAT1510 |
| | RETURN | DAT1520 |
| | ENTRY DDAT2(NRIV) | DAT1530* |
| | NK=1 | DAT1540* |
| | DO 580 K=1,K0 | DAT1550* |
| | DO 580 I=1,I0 | DAT1560* |
| | READ(5,510) (IDR(I,J,K),J=1,J0) | DAT1570* |
| | DO 580 J=1,J0 | DAT1580* |
| | IF (IDR(I,J,K).EQ.0) GO TO 580 | DAT1590* |
| | IDR(I,J,K)=NK | DAT1600* |
| | NK=NK+1 | DAT1610* |
| 580 | CONTINUE | DAT1620* |
| | NK=NK-1 | DAT1630* |
| | IF (NK.EQ.NRIV) GO TO 600 | DAT1640* |
| | WRITE(6,520) NK,NRIV | DAT1650* |
| | STOP | DAT1660* |
| 600 | READ(5,330) FAC | DAT1670* |
| | READ(5,560) (RH(I),I=1,NRIV) | DAT1680* |
| | DO 610 I=1,NRIV | DAT1690* |
| 610 | RH(I)=RH(I)*FAC | DAT1700* |
| | WRITE(6,540) (RH(I),I=1,NRIV) | DAT1710* |

```

        READ(5,330) FAC
        READ(5,560) (RB(I),I=1,NRIV)
        DO 620 I=1,NRIV
620  RB(I)=RB(I)*FAC
        WRITE(6,545) (RB(I),I=1,NRIV)
        READ(5,330) FAC
        READ(5,625) (RC(I),I=1,NRIV)
        DO 630 I=1,NRIV
630  RC(I)=RC(I)*FAC
        WRITE(6,635) (RC(I),I=1,NRIV)
        RETURN
C      .....
C      ---READ TIME PARAMETERS AND PUMPING DATA FOR A NEW PUMPING PERIOD---
C      *****
C      ENTRY NEWPER
C      *****
C
C      READ (5,330) KP,KPM1,NWEL,TMAX,NUMT,CDLT,DELT
C
C      ---COMPUTE ACTUAL DELT AND NUMT---
        DT=DELT/24.
        TM=0.0
        DO 220 I=1,NUMT
        DT=CDLT*DT
        TM=TM+DT
        IF (TM.GE.TMAX) GO TO 230
220  CONTINUE
        GO TO 240
230  DELT=TMAX/TM*DELT
        NUMT=I
240  WRITE (6,400) KP,TMAX,NUMT,DELT,CDLT
        DELT=DELT*3600.
        TMAX=TMAX*86400.
        SUMP=0.0
C
C      ---READ AND WRITE WELL PUMPING RATES---
        WRITE (6,410) NWEL
        IF (NWEL.EQ.0) GO TO 260
        DO 245 K = 1,K0
        DO 245 I = 1,I0
        DO 245 J = 1,J0
245  WELL(I,J,K) = 0.0
        DO 250 II=1,NWEL
        READ (5,330) K,I,J,WELL(I,J,K)
        WRITE (6,420) K,I,J,WELL(I,J,K)
250  WELL(I,J,K)=WELL(I,J,K)/(DELX(J)*DELY(I))
260  RETURN
C
C      ---FORMATS---
C
C
C
280  FORMAT (1H0,52X,6A4,' =',G15.7,' FOR LAYER',I3)
290  FORMAT (1H1,45X,6A4,' MATRIX, LAYER',I3/46X,41('-'))
300  FORMAT ('0',72X,'DELX =',G15.7)
310  FORMAT ('0',72X,'DELY =',G15.7)
320  FORMAT ('0',72X,'DELZ =',G15.7)

```

```

DAT1720*
DAT1730*
DAT1740*
DAT1750*
DAT1760*
DAT1770*
DAT1780*
DAT1790*
DAT1800*
DAT1810*
DAT1820*
DAT1830
DAT1840
DAT1850
DAT1860
DAT1870
DAT1880
DAT1890
DAT1900
DAT1910
DAT1920
DAT1930
DAT1940
DAT1950
DAT1960
DAT1970
DAT1980
DAT1990
DAT2000
DAT2010
DAT2020
DAT2030
DAT2040
DAT2050
DAT2060
DAT2070
DAT2080
DAT2090
DAT2100
DAT2110
DAT2120
DAT2130
DAT2140
DAT2150
DAT2160
DAT2170
DAT2180
DAT2190
DAT2200
DAT2210
DAT2220
DAT2230
DAT2240
DAT2250
DAT2260
DAT2270
DAT2280

```

```

330 FORMAT (8G10.0) DAT2290
340 FORMAT ('0',51X,'NUMBER OF PUMPING PERIODS =',I5/49X,'TIME STEPS BETWEEN PRINTOUTS =',I5//51X,'ERROR CRITERIA FOR CLOSURE =',G15.7//)DAT2300
350 FORMAT ('0',I2,2X,20F6.1/(5X,20F6.1)) DAT2310
360 FORMAT (8F10.4) DAT2320
370 FORMAT (1H1,46X,40HGRID SPACING IN PROTOTYPE IN X DIRECTION/47X,40)DAT2330
1('-')//('0',12F10.0)) DAT2340
380 FORMAT (1H-,46X,40HGRID SPACING IN PROTOTYPE IN Y DIRECTION/47X,40)DAT2350
1('-')//('0',12F10.0)) DAT2360
390 FORMAT (1H-,46X,40HGRID SPACING IN PROTOTYPE IN Z DIRECTION/47X,40)DAT2370
1('-')//('0',12F10.0)) DAT2380
400 FORMAT ('-',50X,'PUMPING PERIOD NO.',I4,':',F10.2,' DAYS'/51X,38(')DAT2390
1-')//53X,'NUMBER OF TIME STEPS=',I6//59X,'DELT IN HOURS =',F10.3//)DAT2400
253X,'MULTIPLIER FOR DELT =',F10.3) DAT2410
410 FORMAT ('-',63X,I4,' WELLS'/65X,9('-')//50X,'K',9X,'I',9X,'J PUDAT2420
1MPING RATE'/) DAT2430
420 FORMAT (4I1X,3I10,2F13.2) DAT2440
430 FORMAT ('-',40X,' CONTINUATION - HEAD AFTER ',G20.7,' SEC PUMPING DAT2450
1'/42X,58('-')) DAT2460
440 FORMAT ('1',55X,'INITIAL HEAD MATRIX, LAYER',I3/56X,30('-')) DAT2470
450 FORMAT (4G20.10) DAT2480
460 FORMAT (3G10.0,2(G10.0,9I1,1X),A8) DAT2490
470 FORMAT ('0',30X,'ON ALPHAMERIC MAP:'/40X,'MULTIPLICATION FACTOR FODAT2500
1R X DIMENSION =',G15.7/40X,'MULTIPLICATION FACTOR FOR Y DIMENSION DAT2510
2=',G15.7/55X,'MAP SCALE IN UNITS OF ',A11/50X,'NUMBER OF ',A8,' PDAT2520
3ER INCH =',G15.7/43X,'MULTIPLICATION FACTOR FOR DRAWDOWN =',G15.7,DAT2530
4' PRINTED FOR LAYERS',9I2/47X,'MULTIPLICATION FACTOR FOR HEAD =',GDAT2540
515.7,' PRINTED FOR LAYERS',9I2) DAT2550
510 FORMAT(80I1) DAT2560
520 FORMAT(' ERROR**** NK.NE.NRIV NK=',I5,5X,'NRIV=',I5) DAT2570*
540 FORMAT('1',54X,'RH MATRIX',/'0',54X,9('-'),//(' ',10(E10.4,2X))) DAT2580*
545 FORMAT('1',54X,'RB MATRIX',/'0',54X,9('-'),//(' ',10(E10.4,2X))) DAT2590*
560 FORMAT(20F4.0) DAT2600*
625 FORMAT(10F8.0) DAT2610*
635 FORMAT('1',54X,'RC MATRIX',/'0',54X,9('-'),//(' ',10(E10.4,2X))) DAT2620*
END DAT2630*

```

```

SUBROUTINE STEP(PHI,STRT,OLD,T,S,TR,TC,TK,WELL,DELX,DELY,DELZ,FACTSTP0010
1,DDN,TEST3) STP0020
C -----STP0030
C INITIALIZE DATA FOR A NEW TIME STEP AND PRINT RESULTS STP0040
C -----STP0050
C STP0060
C SPECIFICATIONS: STP0070
C REAL *8PHI STP0080
C REAL *8XLABEL,YLABEL,TITLE,XN1,MESUR STP0090
C STP0100
C DIMENSION PHI(IO,J0,K0),STRT(IO,J0,K0),OLD(IO,J0,K0),T(IO,J0,K0)STP0110
1),S(IO,J0,K0),TR(IO,J0,K0),TC(IO,J0,K0),TK(IK,JK,K5),WELL(IO,STP0120
2J0,K0),DELX(J0),DELY(IO),DELZ(K0),FACT(K0,3),DDN(IMAX),TEST3STP0130
3(ITMX1),ITTO(50) STP0140
C STP0150
C COMMON /INTEGR/ IO,J0,K0,I1,J1,K1,I,J,K,NPER,KTH,ITMAX,LENGTH,KP,NSTP0160
1WEL,NUMT,IFINAL,IT,KT,IHEAD,IDRAW,IFLO,IERR,I2,J2,K2,IMAX,ITMX1,NCSTP0170
2H,IDK1,IDK2,IWATER,IQRE,IP,JP,IQ,JQ,IK,JK,K5,IPU1,IPU2,ITK,IEQN STP0180
3,NKODE,KFLOW STP0190*
C COMMON /SPARAM/ TMAX,CDLT,DELT,ERR,TEST,SUM,SUMP,QR,BETA1 STP0200*
C COMMON /SARRAY/ ICHK(13),LEVEL1(9),LEVEL2(9) STP0210
C COMMON /CK/ RLFLXT,STORT,QRET,CHST,CHDT,FLUXT,PUMPT,CFLUXT,FLXNT STP0220*
C COMMON /PR/ XLABEL(3),YLABEL(6),TITLE(6),XN1,MESUR,PRNT(122),BLANKSTP0230
1(60),DIGIT(122),VF1(6),VF2(6),VF3(7),XSCALE,DINCH,SYM(17),XN(100),STP0240
2YN(13),NA(4),N1,N2,N3,YSCALE,FACT1,FACT2 STP0250
C RETURN STP0260
C .....STP0270
C *****STP0280
C ENTRY NEWSTP STP0290
C *****STP0300
C KT=KT+1 STP0310
C IT=0 STP0320
C DO 10 K=1,K0 STP0330
C DO 10 I=1,I0 STP0340
C DO 10 J=1,J0 STP0350
10 OLD(I,J,K)=PHI(I,J,K) STP0360
C DELT=CDLT*DELT STP0370
C SUM=SUM+DELT STP0380
C SUMP=SUMP+DELT STP0390
C DAYSP=SUMP/86400. STP0400
C YRSP=DAYSP/365. STP0410
C HRS=SUM/3600. STP0420
C SMIN=HRS*60. STP0430
C DAYS=HRS/24. STP0440
C YRS=DAYS/365. STP0450
C RETURN STP0460
C STP0470
C ---PRINT OUTPUT AT DESIGNATED TIME STEPS--- STP0480
C *****STP0490
C ENTRY OUTPUT STP0500
C *****STP0510
C IF (KT.EQ.NUMT) IFINAL=1 STP0520
C ITTO(KT)=IT STP0530
C IF (IT.LE.ITMAX) GO TO 20 STP0540
C IT=IT-1 STP0550
C ITTO(KT)=IT STP0560
C IERR=2 STP0570

```

| | | |
|---|---|---------|
| C | | STP0580 |
| C | ---IF MAXIMUM ITERATIONS EXCEEDED,WRITE RESULTS ON DISK OR CARDS--- | STP0590 |
| | IF (IDK2.EQ.ICHK(5)) WRITE (4) PHI,SUM,SUMP,PUMPT,CFLUXT,QRET,CHST | STP0600 |
| | 1,CHDT,FLUXT,STORT,RLFLXT,FLXNT | STP0610 |
| | IF (IPU2.EQ.ICHK(9)) WRITE (7,230) SUM,SUMP,PUMPT,CFLUXT,QRET,CHST | STP0620 |
| | 1,CHDT,FLUXT,STORT,RLFLXT,FLXNT | STP0630 |
| C | | STP0640 |
| | 20 IF (IFLO.EQ.ICHK(3)) CALL CHECK | STP0650 |
| | IF (IERR.EQ.2) GO TO 30 | STP0660 |
| | IF (MOD(KT,KTH).NE.0.AND.IFINAL.NE.1) RETURN | STP0670 |
| | 30 WRITE (6,210) KT,DELT,SUM,SMIN,HRS,DAYS,YRS,DAYSP,YRSP | STP0680 |
| | IF (IFLO.EQ.ICHK(3)) CALL CWRITE | STP0690 |
| | IT=IT+1 | STP0700 |
| | WRITE (6,180) (TEST3(J),J=1,IT) | STP0710 |
| | I3=1 | STP0720 |
| | I5=0 | STP0730 |
| | 352 I5=I5+40 | STP0740 |
| | I4=MIN0(KT,I5) | STP0750 |
| | WRITE (6,240) (I,I=I3,I4) | STP0760 |
| | WRITE (6,260) | STP0770 |
| | WRITE (6,250) (ITTO(I),I=I3,I4) | STP0780 |
| | WRITE (6,260) | STP0790 |
| | IF(KT.LE.I5) GO TO 353 | STP0800 |
| | I3=I3+40 | STP0810 |
| | GO TO 352 | STP0820 |
| C | | STP0830 |
| C | ---PRINT MAPS--- | STP0840 |
| | 353 IF (XSCALE.EQ.0.) GO TO 70 | STP0850 |
| | IF (FACT1.EQ.0.) GO TO 50 | STP0860 |
| | DO 40 IA=1,9 | STP0870 |
| | II=LEVEL1(IA) | STP0880 |
| | IF (II.EQ.0) GO TO 50 | STP0890 |
| | 40 CALL PRNTA(1,II) | STP0900 |
| | 50 IF (FACT2.EQ.0.) GO TO 70 | STP0910 |
| | DO 60 IA=1,9 | STP0920 |
| | II=LEVEL2(IA) | STP0930 |
| | IF (II.EQ.0) GO TO 70 | STP0940 |
| | 60 CALL PRNTA(2,II) | STP0950 |
| | 70 IF (IDRAW.NE.ICHK(1)) GO TO 100 | STP0960 |
| C | | STP0970 |
| C | ---PRINT DRAWDOWN--- | STP0980 |
| | DO 90 K=1,K0 | STP0990 |
| | WRITE (6,200) K | STP1000 |
| | DO 90 I=1,I0 | STP1010 |
| | DO 80 J=1,J0 | STP1020 |
| | 80 DDM(J)=STRT(I,J,K)-PHI(I,J,K) | STP1030 |
| | 90 WRITE (6,170) I,(DDM(J),J=1,J0) | STP1040 |
| | 100 IF (IHEAD.NE.ICHK(2)) GO TO 120 | STP1050 |
| C | | STP1060 |
| C | ---PRINT HEAD MATRIX--- | STP1070 |
| | DO 110 K=1,K0 | STP1080 |
| | WRITE (6,190) K | STP1090 |
| | DO 110 I=1,I0 | STP1100 |
| | 110 WRITE (6,170) I,(PHI(I,J,K),J=1,J0) | STP1110 |
| C | | STP1120 |
| C | ---WRITE ON DISK--- | STP1130 |
| | 120 IF (IERR.EQ.2) GO TO 130 | STP1140 |

```

IF (KP.LT.NPER.OR.IFINAL.NE.1) RETURN STP1150
IF (IDK2.EQ.ICHK(5)) WRITE (4) PHI,SUM,SUMP,PUMPT,CFLUXT,QRET,CHST STP1160
1,CHDT,FLUXT,STORT,RLFLXT,FLXNT STP1170
C STP1180
C ---PUNCHED OUTPUT--- STP1190
130 IF (IPU2.NE.ICHK(9)) GO TO 160 STP1200
IF (IERR.EQ.2) GO TO 140 STP1210
WRITE (7,230) SUM,SUMP,PUMPT,CFLUXT,QRET,CHST,CHDT,FLUXT,STORT,ETP STP1220
1LXT,FLXNT STP1230
140 DO 150 K=1,K0 STP1240
DO 150 I=1,I0 STP1250
150 WRITE (7,220) (PHI(I,J,K),J=1,J0) STP1260
160 IF (IERR.EQ.2) STOP STP1270
RETURN STP1280
C STP1290
C ---FORMATS--- STP1300
C STP1310
C STP1320
C STP1330
170 FORMAT ('0',I4,18F7.2/(5X,18F7.2)) STP1340
180 FORMAT ('0MAXIMUM HEAD CHANGE FOR EACH ITERATION: '/' ',39('-')/('0 STP1350
1',10F12.4)) STP1360
190 FORMAT ('1',55X,'HEAD MATRIX, LAYER',I3/56X,21('-')) STP1370
200 FORMAT ('1',55X,' DRAWDOWN, LAYER',I3/59X,18('-')) STP1380
210 FORMAT (1H1,44X,57('-')/45X,'1',14X,'TIME STEP NUMBER =',I9,14X,'1 STP1390
1'/45X,57('-')/50X,29HSIZE OF TIME STEP IN SECONDS=',F14.2//55X,'TOSTP1400
2TAL SIMULATION TIME IN SECONDS=',F14.2/80X,8HMINUTES=',F14.2/82X,6HSTP1410
3HOURS=',F14.2/83X,5HSDAYS=',F14.2/82X,'YEARS=',F14.2//45X,'DURATION STP1420
4OF CURRENT PUMPING PERIOD IN DAYS=',F14.2/82X,'YEARS=',F14.2//) STP1430
220 FORMAT (10F8.4) STP1440
230 FORMAT (4G20.10) STP1450
240 FORMAT ('0TIME STEP :',40I3) STP1460
250 FORMAT ('0ITERATIONS:',40I3) STP1470
260 FORMAT (' ',10('-')) STP1480
END STP1490

```

```

SUBROUTINE SOLVE(PHI,STRT,OLD,T,S,TR,TC,TK,WELL,DELX,DELY,DELZ,FACSP30010
1T,EL,FL,GL,V,XI,TEST3,QRE,BOTTOM, SP30020
2PERM,IDR,RH,RC,RB,IRIV) SP30030*
-----SP30040
C SOLUTION BY THE STRONGLY IMPLICIT PROCEDURE SP30050
C -----SP30060
C SPECIFICATIONS: SP30070
C REAL *8PHI,RHO,B,D,F,H,Z,SU,RHOP,W,WMIN,RHO1,RHO2,RHO3,XPART,YPARTSP30080
1,ZPART,DMIN1,WMAX,XT,YT,ZT,DABS,DMAX1,DEN,TXM,TYM,TDEN SP30100
REAL *8E,AL,BL,CL,A,C,G,WU,TU,U,DL,RES,SUPH,GLXI,ZPHI SP30110
C SP30120
DIMENSION PHI(1),STRT(1),OLD(1),T(1),S(1),TR(1),TC(1),TK(1)SP30130
1,WELL(1),DELX(1),DELY(1),DELZ(1),FACT(K0,3),RHOP(20),TEST3(SP30140
21),EL(1),FL(1),GL(1),V(1),XI(1),QRE(1),BOTTOM(1) SP30150
3,PERM(1),IDR(1),RH(1),RC(1),RB(1) SP30160*
DIMENSION IN(36,59,7),TT(36,59,6),DBOT(36,59),ISLA(36,59) SP30170*
2,DTOP(9),IAA(30),HEDSTR(10),BTM(36,59),KCHOP2(9),IFNLHD(36,59,7),TSP30180*
3DEN(8) SP30183*
C SP30190
COMMON /INTEGR/ IO,J0,K0,I1,J1,K1,I,J,K,NPER,KTH,ITMAX,LENGTH,KP,NSP30200
1WEL,NUMT,IFINAL,IT,KT,IHEAD,IDRAW,IFLO,IERR,I2,J2,K2,IMAX,ITMX1,NCSP30210
2H,IDK1,IDK2,IWATER,IQRE,IP,JP,IQ,JQ,IK,JK,K5,IPU1,IPU2,ITK,IEQN SP30220
3,NKODE,KFLOW SP30230*
COMMON /SPARAM/ TMAX,CDLT,DELT,ERR,TEST,SUM,SUMP,QR,BETA1 SP30240*
COMMON /SARRAY/ ICHK(13),LEVEL1(9),LEVEL2(9) SP30250
RETURN SP30260
C .....SP30270
C ***** SP30280
ENTRY ITER SP30290
C ***** SP30300
C ---COMPUTE AND PRINT ITERATION PARAMETERS--- SP30310
WRITE (6,240) SP30320
WMIN=1.D0 SP30330
DELT=1. SP30340
P2=LENGTH-1 SP30350
NT=IO*J0*K0 SP30360
NIJ=IO*J0 SP30370
XT=3.141593**2/(2.*J2*J2) SP30380
YT=3.141593**2/(2.*I2*I2) SP30390
ZT=3.141593**2/(2.*K0*K0) SP30400
RHO1=0.D0 SP30410
RHO2=0.D0 SP30420
RHO3=0.D0 SP30430
DO 40 K=1,K0 SP30440
DO 40 I=2,I1, SP30450
DO 40 J=2,J1 SP30460
N=I+(J-1)*IO+(K-1)*NIJ SP30470
IF (T(N).EQ.0.) GO TO 40 SP30480
D=TR(N-IO)/DELX(J) SP30490
F=TR(N)/DELX(J) SP30500
B=TC(N-1)/DELY(I) SP30510
H=TC(N)/DELY(I) SP30520
SU=0.D0 SP30530
Z=0.D0 SP30540
IF (K.NE.1) Z=TK(N-NIJ)/DELZ(K) SP30550
IF (K.NE.K0) SU=TK(N)/DELZ(K) SP30560

```

| | | |
|----|--|----------|
| 10 | CONTINUE | SP30570 |
| | TXM=DMAX1(D,F) | SP30580 |
| | TYM=DMAX1(B,H) | SP30590 |
| | TZM=DMAX1(SU,Z) | SP30600 |
| | DEN=DMIN1(D,F) | SP30610 |
| | IF (DEN.EQ.0.D0) DEN=TXM | SP30620 |
| | IF (DEN.EQ.0.D0) GO TO 20 | SP30630 |
| | RHO1=DMAX1(RHO1, TYM/DEN) | SP30640 |
| 20 | DEN=DMIN1(B,H) | SP30650 |
| | IF (DEN.EQ.0.D0) DEN=TYM | SP30660 |
| | IF (DEN.EQ.0.D0) GO TO 30 | SP30670 |
| | RHO2=DMAX1(RHO2, TXM/DEN) | SP30680 |
| 30 | DEN=DMIN1(SU,Z) | SP30690 |
| | IF (DEN.EQ.0.D0) DEN=TZM | SP30700 |
| | IF (DEN.EQ.0.D0) GO TO 40 | SP30710 |
| | RHO3=DMAX1(RHO3, TXM/DEN) | SP30720 |
| 40 | CONTINUE | SP30730 |
| | XPART=XT/(1.D0+RHO1) | SP30740 |
| | YPART=YT/(1.D0+RHO2) | SP30750 |
| | ZPART=ZT/(1.D0+RHO3) | SP30760 |
| | WMIN=DMIN1(WMIN, XPART, YPART, ZPART) | SP30770 |
| | WMAX=1.D0-WMIN | SP30780 |
| | PJ=-1. | SP30790 |
| | DO 50 I=1, LENGTH | SP30800 |
| | PJ=PJ+1. | SP30810 |
| 50 | RHOP(I)=1.D0-(1.D0-WMAX)**(PJ/P2) | SP30820 |
| | WRITE (6,230) LENGTH, (RHOP(J), J=1, LENGTH) | SP30830 |
| | RETURN | SP30840 |
| C | | SP30850 |
| C | | SP30860 |
| C | ---INITIALIZE DATA FOR A NEW ITERATION--- | SP30870 |
| 60 | IT=IT+1 | SP30880 |
| C | | SP30890 |
| C | PRINT HEAD VALUES FOR SELECTED NODES | SP30900 |
| C | | SP30910 |
| | ITT=IT-1 | SP30920 |
| | IF(MODKOD.NE.1) GO TO 68 | SP30930* |
| | IF(ITT.LT.1) WRITE(6,2970) (IAA(I), I=1, MNOD) | SP30940* |
| | J=1 | SP30950* |
| | DO 64 I=3, MNOD, 3 | SP30960* |
| | N=IAA(I-2)+(IAA(I-1)-1)*I0+(IAA(I)-1)*NIJ | SP30970* |
| | HEDSTR(J)=PHI(N) | SP30980* |
| | J=J+1 | SP30990* |
| 64 | CONTINUE | SP31000* |
| | WRITE(6,2980) ITT, (HEDSTR(J), J=1, NMBRND) | SP31010* |
| C | | SP31020 |
| 68 | IF (IT.LE.ITMAX) GO TO 70 | SP31030 |
| | WRITE (6,220) | SP31040 |
| | CALL OUTPWT | SP31050 |
| 70 | IF (MOD(IT,LENGTH)) 80,80,90 | SP31060 |
| C | ***** | SP31070 |
| | ENTRY NEWITA | SP31080 |
| C | ***** | SP31090 |
| 80 | NTH=0 | SP31100 |
| 90 | NTH=NTH+1 | SP31110 |
| | W=RHOP(NTH) | SP31120 |
| | TEST3(IT+1)=0. | SP31130 |

| | |
|---|----------|
| TEST=0.0 | SP31140 |
| BIG=0. | SP31150 |
| DO 100 I=1,NT | SP31160 |
| EL(I)=0. | SP31170 |
| FL(I)=0. | SP31180 |
| GL(I)=0. | SP31190 |
| V(I)=0. | SP31200 |
| 100 XI(I)=0. | SP31210 |
| C | SP31220 |
| C | SP31230 |
| C | SP31240 |
| --- | SP31250 |
| COMPUTE TRANSMISSIVITY AND T COEFFICIENTS FOR UPPER | SP31260 |
| HYDROLOGIC UNIT WHEN IT IS UNCONFINED--- | SP31270 |
| IF (IWATER.NE.ICHK(6)) GO TO 110 | SP31280 |
| CALL TRANS(0) | SP31290 |
| CALL TCOF | SP31300 |
| C | SP31310 |
| C | SP31320 |
| --- | SP31330 |
| CHOOSE SIP NORMAL OR REVERSE ALGORITHM--- | SP31340 |
| 110 IF (MOD(IT,2)) 120,120,170 | SP31350 |
| 120 DO 150 K=1,K0 | SP31360 |
| DO 150 I=2,I1 | SP31370 |
| DO 150 J=2,J1 | SP31380 |
| N=I+(J-1)*I0+(K-1)*NIJ | SP31390 |
| NIA=N+1 | SP31400 |
| NIB=N-1 | SP31410 |
| NJA=N+I0 | SP31420 |
| NJB=N-I0 | SP31430 |
| NKA=N+NIJ | SP31440 |
| NKB=N-NIJ | SP31450 |
| C | SP31460 |
| C | SP31470 |
| --- | SP31480 |
| SKIP COMPUTATIONS IF NODE OUTSIDE MODEL--- | SP31490 |
| IF (T(N).EQ.0. .OR.S(N).LT.0.) GO TO 150 | SP31500 |
| C | SP31510 |
| C | SP31520 |
| --- | SP31530 |
| COMPUTE COEFFICIENTS--- | SP31540 |
| D=TR(NJB)/DELX(J) | SP31550 |
| F=TR(N)/DELX(J) | SP31560 |
| B=TC(NIB)/DELY(I) | SP31570 |
| H=TC(N)/DELY(I) | SP31580 |
| SU=0.D0 | SP31590* |
| Z=0.D0 | SP31600* |
| IF(K.EQ.1) GO TO 124 | SP31610* |
| Z=TK(NKB) | SP31620* |
| IF(IEQN.EQ.ICHK(11)) Z=Z/DELZ(K) | SP31630* |
| 124 IF(K.EQ.K0) GO TO 125 | SP31640* |
| SU=TK(N) | SP31650* |
| IF(IEQN.EQ.ICHK(11))SU=SU/DELZ(K) | SP31660* |
| 125 RHO=S(N)/DELT | SP31670* |
| QR=0. | SP31680* |
| UXR=0. | SP31690* |
| IF (K.NE.K0) GO TO 127 | SP31700* |
| IF (IQRE.EQ.ICHK(7)) QR=QRE(I+(J-1)*I0) | |
| 127 IF(IRIV.LE.0) GO TO 130 | |
| ND=IDR(N) | |
| IF(ND.EQ.0) GO TO 130 | |
| IF(PHI(N).GT.RB(ND)) GO TO 128 | |
| QR=QR+RC(ND)*(RH(ND)-RB(ND)) | |
| GO TO 130 | |
| 128 UXR=RC(ND) | |
| QR=QR+RC(ND)*RH(ND) | |

| | | |
|-----|--|----------|
| C | | SP31710 |
| C | ---SIP NORMAL ALGORITHM--- | SP31720 |
| C | ---FORWARD SUBSTITUTE, COMPUTING INTERMEDIATE VECTOR V--- | SP31730 |
| 130 | E=-B-D-F-H-SU-Z-RHO-UXR | SP31740 |
| | BL=B/(1.+W*(EL(NIB)+GL(NIB))) | SP31750 |
| | CL=D/(1.+W*(FL(NJB)+GL(NJB))) | SP31760 |
| | C=BL*EL(NIB) | SP31770 |
| | G=CL*FL(NJB) | SP31780 |
| | WU=CL*GL(NJB) | SP31790 |
| | U=BL*GL(NIB) | SP31800 |
| | IF (K.EQ.1) GO TO 140 | SP31810 |
| | AL=Z/(1.+W*(EL(NKB)+FL(NKB))) | SP31820 |
| | A=AL*EL(NKB) | SP31830 |
| | TU=AL*FL(NKB) | SP31840 |
| | DL=E+W*(A+C+G+WU+TU+U)-CL*EL(NJB)-BL*FL(NIB)-AL*GL(NKB) | SP31850 |
| | IF(DABS(DL).LT.1.E-10) WRITE(8,8100) I,J,K,DL,E,W,CL,BL,AL | SP31860* |
| | IF(DABS(DL).LT.1.0E-10) WRITE(8,8125) B,D,F,H,SU,Z,RHO,UXR | SP31870* |
| | IF(DABS(DL).LT.1.0E-10) WRITE(8,8130) A,C,G,WU,TU,U | SP31880* |
| | IF(DABS(DL).LT.1.0E-10) WRITE(8,8150) T(N),T(NIB),T(NIA),T(NJB), | SP31890* |
| | 1T(NJA),T(NKB),T(NKA) | SP31900* |
| | EL(N)=(F-W*(A+C))/DL | SP31910 |
| | FL(N)=(H-W*(G+TU))/DL | SP31920 |
| | GL(N)=(SU-W*(WU+U))/DL | SP31930 |
| | SUPH=0.DO | SP31940 |
| | IF (K.NE.K0) SUPH=SU*PHI(NKA) | SP31950 |
| | RES=-B*PHI(NIB)-D*PHI(NJB)-E*PHI(N)-F*PHI(NJA)-H*PHI(NIA)-SUPH-Z*P | SP31960 |
| | 1HI(NKB)-WELL(N)-RHO*OLD(N)-QR | SP31970 |
| | RES=RES*BETA 1 | SP31980* |
| | V(N)=(RES-AL*V(NKB)-BL*V(NIB)-CL*V(NJB))/DL | SP31990 |
| | GO TO 150 | SP32000 |
| 140 | DL=E+W*(C+G+WU+U)-CL*EL(NJB)-BL*FL(NIB) | SP32010 |
| | EL(N)=(F-W*C)/DL | SP32020 |
| | FL(N)=(H-W*G)/DL | SP32030 |
| | GL(N)=(SU-W*(WU+U))/DL | SP32040 |
| | SUPH=0.DO | SP32050 |
| | IF (K.NE.K0) SUPH=SU*PHI(NKA) | SP32060 |
| | RES=-B*PHI(NIB)-D*PHI(NJB)-E*PHI(N)-F*PHI(NJA)-H*PHI(NIA)-SUPH-WEL | SP32070 |
| | 1L(N)-RHO*OLD(N)-QR | SP32080 |
| | RES=RES*BETA 1 | SP32090* |
| | V(N)=(RES-BL*V(NIB)-CL*V(NJB))/DL | SP32100 |
| 150 | CONTINUE | SP32110 |
| C | | SP32120 |
| C | ---BACK SUBSTITUTE FOR VECTOR XI--- | SP32130 |
| | DO 160 K=1,K0 | SP32140 |
| | K3=K0-K+1 | SP32150 |
| | DO 160 I=1,I2 | SP32160 |
| | I3=I0-I | SP32170 |
| | DO 160 J=1,J2 | SP32180 |
| | J3=J0-J | SP32190 |
| | N=I3+(J3-1)*I0+(K3-1)*NIJ+I-I | SP32200 |
| | IF (T(N).EQ.0. .OR.S(N).LT.0.) GO TO 160 | SP32210 |
| | GLXI=0.DO | SP32220 |
| | IF (K3.NE.K0) GLXI=GL(N)*XI(N+NIJ) | SP32230 |
| | XI(N)=V(N)-EL(N)*XI(N+I0)-FL(N)*XI(N+1)-GLXI | SP32240 |
| C | | SP32250 |
| C | ---COMPARE MAGNITUDE OF CHANGE WITH CLOSURE CRITERIA--- | SP32260 |
| | TCHK=ABS(XI(N)) | SP32270 |

| | |
|---|----------|
| IF (TCHK.GT.BIG) BIG=TCHK | SP32280 |
| PHI(N)=PHI(N)+XI(N) | SP32290 |
| 160 CONTINUE | SP32300 |
| IF (BIG.GT.ERR) TEST=1. | SP32310 |
| TEST3(IT+1)=BIG | SP32320 |
| C | SP32330* |
| C ---GROUP I CARDS--- | SP32340* |
| C | SP32350* |
| C | SP32360* |
| C CHECK IF THIS IS FIRST TIME STEP OF FIRST PUMPING PERIOD. | SP32370* |
| C IF NOT SKIP THIS SECTION. | SP32371* |
| IF(KP.GT.1) GO TO 2090 | SP32380* |
| IF(KT.GT.1) GO TO 2090 | SP32385* |
| C | SP32390* |
| C CHECK IF THIS IS FIRST SOLUION. IF IT IS PROCEED WITH INITIAL CHECK | SP32400* |
| IF(IT.NE.0) GO TO 2090 | SP32410* |
| C | SP32420* |
| C INTERFACE DEFINITION: DEFINE PARAMETERS, READ DATA, INITIALIZE IN | SP32430* |
| C ARRAY AND PERFORM FIRST CHOP | SP32440* |
| C | SP32450* |
| C | SP32460* |
| C DEFINITION OF PARAMETERS | SP32470* |
| C | SP32480* |
| C NTAD = NUMBER OF ITERATIONS BETWEEN T ADJUSTMENTS | SP32490* |
| C DBOT(I,J) = BASE OF AQUIFER SYSTEM IN TERMS OF + NUMBERS BECAUSE | SP32500* |
| C Z IS + DOWNWARD | SP32510* |
| C RF = COEFFICIENT OF PHI TO GIVE INTERFACE DEPTH BELOW SEA LEVEL | SP32520* |
| C = ((RHOS-RHOF)/RHOF) | SP32530* |
| C DTOP(K) = DEPTH BELOW SEA LEVEL TO TOP OF LAYERS IN TERMS OF + | SP32540* |
| C NUMBERS BECAUSE Z IS + DOWNWARD | SP32550* |
| C Z2 = DISTANCE TO TOP OF F.D. BLOCK BELOW SEA LEVEL | SP32560* |
| C IH = IO/2 FOR TWO-SIDED CHOP, = I1 FOR ONE-SIDED CHOP | SP32570* |
| C (E.G. CROSS SECTIONS) | SP32580* |
| C Z = DEPTH BELOW S.L. TO INTERFACE | SP32590* |
| C IN = 4 = BLOCK WITH ADJUSTED T | SP32600* |
| C = 6 = BLOCK DROPPED DURING CHOP | SP32610* |
| C = 7 = BLOCK DROPPED BECAUSE OCEAN LEAKAGE INTO AQUIFER | SP32620* |
| C = 8 = INITIAL T = 0 | SP32630* |
| C KODCHP = CODE TO INDICATE WHETHER CHOPPING SHOULD PROCEED FROM | SP32640* |
| C TWO SIDES OR ONE (1=ONE-SIDED, NE 1=TWO-SIDED) | SP32650* |
| C READ AND WRITE INPUT DATA | SP32660* |
| READ(5,2901) NTAD,RF,KODCHP,KPRNTT | SP32670* |
| WRITE(6,2902) NTAD,RF,KODCHP,KPRNTT | SP32680* |
| C | SP32690* |
| C READ AND PRINT AQUIFER BASE | SP32700* |
| READ(5,2909) FAC,IVAR,IPRN | SP32710* |
| IF(IVAR.NE.1) GO TO 2002 | SP32720* |
| DO 2001 I=1,IO | SP32730* |
| 2001 READ(5,2903) (DBOT(I,J),J=1,J0) | SP32740* |
| 2002 DO 2004 I=1,IO | SP32750* |
| DO 2004 J=1,J0 | SP32760* |
| IF(IVAR.NE.1) GO TO 2003 | SP32770* |
| DBOT(I,J)=DBOT(I,J)*FAC | SP32780* |
| GO TO 2004 | SP32790* |
| 2003 DBOT(I,J)=FAC | SP32800* |
| 2004 CONTINUE | SP32810* |
| IF(IPRN.EQ.1) GO TO 2006 | SP32820* |

| | |
|--|----------|
| WRITE(6,2904) | SP32830* |
| DO 2005 I=1,I0 | SP32840* |
| 2005 WRITE(6,2905) I,(DBOT(I,J),J=1,J0) | SP32850* |
| C | SP32860* |
| C READ AND PRINT CONSTANT HEAD OCEAN NODES | SP32870* |
| 2006 DO 2007 I=1,I0 | SP32880* |
| 2007 READ(5,2906) (ISEA(I,J),J=1,J0) | SP32890* |
| WRITE(6,2907) | SP32900* |
| DO 2008 I=1,I0 | SP32910* |
| 2008 WRITE(6,2908) I,(ISEA(I,J),J=1,J0) | SP32920* |
| C | SP32930* |
| C READ AND PRINT DEPTH TO LAYER TOPS | SP32940* |
| READ(5,2909) (DTOP(K),K=1,K0) | SP32950* |
| DO 2010 K=1,K0 | SP32960* |
| 2010 WRITE(6,2910) K,DTOP(K) | SP32970* |
| C READ NODES FOR WHICH HEAD VALUES AT EACH ITERATION ARE TO BE PRINTED | SP32980* |
| READ(5,2909) NODKOD,NMBRND | SP32990* |
| IF(NODKOD.NE.1) GO TO 2012 | SP33000* |
| MNOD=3*NMBRND | SP33010* |
| READ(5,2960) (IAA(I),I=1,MNOD) | SP33020* |
| C | SP33030* |
| C READ NODES FOR WHICH FINAL HEADS ARE TO BE PRINTED INDIVIDUALLY | SP33040* |
| C | SP33050* |
| 2012 READ(5,2909) IHEDKD | SP33060* |
| IF(IHEDKD.NE.1) GO TO 2014 | SP33070* |
| DO 2013 K=1,K0 | SP33080* |
| DO 2013 I=1,I0 | SP33090* |
| 2013 READ(5,2906) (IFNLHD(I,J,K),J=1,J0) | SP33100* |
| C CHECK IF INTERFACE SOLUTION. IF NOT SKIP THIS SECTION | SP33110* |
| 2014 IF(NKODE.NE.1) GO TO 2095 | SP33120* |
| C | SP33130* |
| C | SP33140* |
| C INITIALIZE IN,TT, AND SCALARS | SP33150* |
| IH=I0/2 | SP33160* |
| IF(KODCHP.EQ.1) IH=I1 | SP33170* |
| NIJ=I0*J0 | SP33180* |
| KCHOP2(K0)=0 | SP33190* |
| DO 2015 K=1,K1 | SP33200* |
| KCHOP2(K)=0 | SP33210* |
| DO 2015 I=1,I0 | SP33220* |
| DO 2015 J=1,J0 | SP33230* |
| N=I+(J-1)*I0+(K-1)*NIJ | SP33240* |
| IN(I,J,K)=0 | SP33250* |
| 2015 TT(I,J,K)=T(N) | SP33260* |
| C | SP33270* |
| DO 2017 I=1,I0 | SP33280* |
| DO 2017 J=1,J0 | SP33290* |
| IN(I,J,K0)=0 | SP33300* |
| M=I+(J-1)*I0 | SP33310* |
| 2017 BTM(I,J)=BOTTOM(M) | SP33320* |
| C | SP33330* |
| C NORTH SIDE CHECK | SP33340* |
| C | SP33350* |
| DO 2026 K=1,K0 | SP33360* |
| DO 2025 J=2,J1 | SP33370* |
| DO 2020 I=2,IH | SP33380* |
| N=I+(J-1)*I0+(K-1)*NIJ | SP33390* |

| | |
|---|----------|
| IF(T(N).EQ.0.) IN(I,J,K)=8 | SP33400* |
| 2020 CONTINUE | SP33410* |
| 2025 CONTINUE | SP33420* |
| 2026 CONTINUE | SP33430* |
| C | SP33440* |
| IF(KODCHP.EQ.1) GO TO 2038 | SP33450* |
| C | SP33460* |
| C SOUTH SIDE CHECK: | SP33470* |
| C | SP33480* |
| DO 2036 K=1,K0 | SP33490* |
| DO 2035 J=2,J1 | SP33500* |
| DO 2030 II=1,IH | SP33510* |
| I=I0-II | SP33520* |
| M=I+(J-1)*I0+(K-1)*NIJ | SP33530* |
| IF(T(N).EQ.0.) IN(I,J,K)=8 | SP33540* |
| 2030 CONTINUE | SP33550* |
| 2035 CONTINUE | SP33560* |
| 2036 CONTINUE | SP33570* |
| C | SP33580* |
| C PRINT IN ARRAY FOR EACH LAYER | SP33590* |
| 2038 DO 2042 K=1,K0 | SP33600* |
| WRITE(6,2911) K | SP33610* |
| DO 2040 I=1,I0 | SP33620* |
| 2040 WRITE(6,2912) I,(IN(I,J,K),J=1,J0) | SP33630* |
| 2042 CONTINUE | SP33640* |
| C | SP33650* |
| C ADJUST T? | SP33660* |
| 2090 IF(NKODE.LT.1) GO TO 2095 | SP33670* |
| IF(TEST.EQ.0.) GO TO 2700 | SP33680* |
| IF(IT.EQ.0) GO TO 60 | SP33690* |
| IF(MOD(IT,NTAD).EQ.0) GO TO 2100 | SP33700* |
| 2095 IF(TEST.EQ.0.) RETURN | SP33710 |
| GO TO 60 | SP33720 |
| C | SP33730 |
| 170 DO 200 KK=1,K0 | SP33740 |
| K=K0-KK+1 | SP33750 |
| DO 200 II=1,I2 | SP33760 |
| I=I0-II | SP33770 |
| DO 200 J=2,J1 | SP33780 |
| M=I+(J-1)*I0+(K-1)*NIJ | SP33790 |
| NIA=N+1 | SP33800 |
| NIB=N-1 | SP33810 |
| NJA=N+I0 | SP33820 |
| NJB=N-I0 | SP33830 |
| NKA=N+NIJ | SP33840 |
| NKB=N-NIJ | SP33850 |
| C | SP33860 |
| C ---SKIP COMPUTATIONS IF NODE OUTSIDE AQUIFER--- | SP33870 |
| IF (T(N).EQ.0. .OR.S(N).LT.0.) GO TO 200 | SP33880 |
| C | SP33890 |
| C ---COMPUTE COEFFICIENTS--- | SP33900 |
| D=TR(NJB)/DELX(J) | SP33910 |
| F=TR(N)/DELX(J) | SP33920 |
| B=TC(NIB)/DELY(I) | SP33930 |
| H=TC(N)/DELY(I) | SP33940 |
| SU=0.D0 | SP33950 |
| Z=0.D0 | SP33960 |

| | | |
|-----|--|----------|
| | IF(K.EQ.1) GO TO 174 | SP33970 |
| | Z=TK(NKB) | SP33980 |
| | IF(IEQN.EQ.ICHK(11)) Z=Z/DELZ(K) | SP33990 |
| 174 | IF(K.EQ.K0) GO TO 175 | SP34000 |
| | SU=TK(N) | SP34010 |
| | IF(IEQN.EQ.ICHK(11))SU=SU/DELZ(K) | SP34020 |
| 175 | RHO=S(N)/DELT | SP34030 |
| | QR=0. | SP34040* |
| | UXR=0. | SP34050* |
| | IF (K.NE.K0) GO TO 177 | SP34060* |
| | IF (IQRE.EQ.ICHK(7)) QR=QRE(I+(J-1)*I0) | SP34070* |
| 177 | IF(IRIV.LE.0) GO TO 180 | SP34080* |
| | ND=IDR(N) | SP34090* |
| | IF(ND.EQ.0) GO TO 180 | SP34100* |
| | IF(PHI(N).GT.RB(ND)) GO TO 178 | SP34110* |
| | QR=QR+RC(ND)*(RH(ND)-RB(ND)) | SP34120* |
| | GO TO 180 | SP34130* |
| 178 | UXR=RC(ND) | SP34140* |
| | QR=QR+RC(ND)*RH(ND) | SP34150* |
| C | | SP34160 |
| C | ---SIP REVERSE ALGORITHM--- | SP34170 |
| C | ---FORWARD SUBSTITUTE, COMPUTING INTERMEDIATE VECTOR V--- | SP34180 |
| 180 | E=-B-D-F-H-SU-Z-RHO-UXR | SP34190 |
| | BL=H/(1.+W*(EL(NIA)+GL(NIA))) | SP34200 |
| | CL=D/(1.+W*(FL(NJB)+GL(NJB))) | SP34210 |
| | C=BL*EL(NIA) | SP34220 |
| | G=CL*FL(NJB) | SP34230 |
| | WU=CL*GL(NJB) | SP34240 |
| | U=BL*GL(NIA) | SP34250 |
| | IF (K.EQ.K0) GO TO 190 | SP34260 |
| | AL=SU/(1.+W*(EL(NKA)+FL(NKA))) | SP34270 |
| | A=AL*EL(NKA) | SP34280 |
| | TU=AL*FL(NKA) | SP34290 |
| | DL=E+W*(C+G+A+WU+TU)-AL*GL(NKA)-BL*FL(NIA)-CL*EL(NJB) | SP34300 |
| | IF(DABS(DL).LT.1.E-10) WRITE(8,8200) I,J,K,DL,E,W,CL,BL,AL | SP34310* |
| | IF(DABS(DL).LT.1.0E-10) WRITE(8,8125) B,D,F,H,SU,Z,RHO,UXR | SP34320* |
| | IF(DABS(DL).LT.1.0E-10) WRITE(8,8130) A,C,G,WU,TU,U | SP34330* |
| | IF(DABS(DL).LT.1.0E-10) WRITE(8,8150) T(N),T(NIB),T(NIA),T(NJB), | SP34340* |
| | T(NJA),T(NKB),T(NKA) | SP34350* |
| | EL(N)=(F-W*(C+A))/DL | SP34360 |
| | FL(N)=(B-W*(G+TU))/DL | SP34370 |
| | GL(N)=(Z-W*(WU+U))/DL | SP34380 |
| | ZPHI=0.DO | SP34390 |
| | IF (K.NE.1) ZPHI=Z*PHI(NKB) | SP34400 |
| | RES=-B*PHI(NIB)-D*PHI(NJB)-E*PHI(N)-F*PHI(NJA)-H*PHI(NIA)-SU*PHI(N | SP34410 |
| | 1KA)-ZPHI-WELL(N)-RHO*OLD(N)-QR | SP34420 |
| | RES=RES*BETA 1 | SP34430* |
| | V(N)=(RES-AL*V(NKA)-BL*V(NIA)-CL*V(NJB))/DL | SP34440 |
| | GO TO 200 | SP34450 |
| 190 | DL=E+W*(C+G+WU+U)-BL*FL(NIA)-CL*EL(NJB) | SP34460 |
| | EL(N)=(F-W*C)/DL | SP34470 |
| | FL(N)=(B-W*G)/DL | SP34480 |
| | GL(N)=(Z-W*(WU+U))/DL | SP34490 |
| | ZPHI=0.DO | SP34500 |
| | IF (K.NE.1) ZPHI=Z*PHI(NKB) | SP34510 |
| | RES=-B*PHI(NIB)-D*PHI(NJB)-E*PHI(N)-F*PHI(NJA)-H*PHI(NIA)-ZPHI-WEL | SP34520 |
| | 1L(N)-RHO*OLD(N)-QR | SP34530 |

| | | |
|------|---|----------|
| | RES=RES*BETA1 | SP34540* |
| | V(N)=(RES-BL*V(NIA)-CL*V(NJB))/DL | SP34550 |
| 200 | CONTINUE | SP34560 |
| C | | SP34570 |
| C | ---BACK SUBSTITUTE FOR VECTOR XI--- | SP34580 |
| | DO 210 K=1,K0 | SP34590 |
| | DO 210 I=2,I1 | SP34600 |
| | DO 210 J=1,J2 | SP34610 |
| | J3=J0-J | SP34620 |
| | N=I+(J3-1)*I0+(K-1)*NIJ | SP34630 |
| | IF (T(N).EQ.0. .OR.S(N).LT.0.) GO TO 210 | SP34640 |
| | GLXI=0.DO | SP34650 |
| | IF (K.NE.1) GLXI=GL(N)*XI(N-NIJ) | SP34660 |
| | XI(N)=V(N)-EL(N)*XI(N+I0)-FL(N)*XI(N-1)-GLXI | SP34670 |
| C | | SP34680 |
| C | ---COMPARE MAGNITUDE OF CHANGE WITH CLOSURE CRITERIA--- | SP34690 |
| | TCHK=ABS(XI(N)) | SP34700 |
| | IF (TCHK.GT.BIG) BIG=TCHK | SP34710 |
| | PHI(N)=PHI(N)+XI(N) | SP34720 |
| 210 | CONTINUE | SP34730 |
| | IF (BIG.GT.ERR) TEST=1. | SP34740 |
| | TEST3(IT+1)=BIG | SP34750 |
| C | | SP34760 |
| C | ---GROUP II CARDS--- | SP34770* |
| C | | SP34780* |
| | IF(NKODE.LT.1) GO TO 2900 | SP34790* |
| | IF(TEST.EQ.0.) GO TO 2700 | SP34800* |
| | IF(MOD(IT,NTAD).GT.0) GO TO 60 | SP34810* |
| C | | SP34820* |
| C | ADJUSTMENT OF T: | SP34830* |
| C | NORTH SIDE | SP34840* |
| C | | SP34850* |
| 2100 | WRITE(6,2920) IT | SP34860* |
| | DO 2300 K=1,K0 | SP34870* |
| | DO 2280 J=2,J1 | SP34880* |
| | DO 2260 I=2,IH | SP34890* |
| | N=I+(J-1)*I0+(K-1)*NIJ | SP34900* |
| | M=I+(J-1)*I0 | SP34910* |
| | Z2=DTOP(K) | SP34920* |
| | IF(T(N).EQ.0.) GO TO 2260 | SP34930* |
| C | | SP34940* |
| C | COMPUTE INTERFACE DEPTH BELOW SEA LEVEL | SP34950* |
| | Z=RF*PHI(N) | SP34960* |
| C | | SP34970* |
| C | DROP A BLOCK IF OCEAN LEAKAGE IS INTO AQUIFER | SP34980* |
| C | | SP34990* |
| C | CHECK LOWER LAYER | SP35000* |
| | IF(IN(I,J,K-1).EQ.0.OR.IN(I,J,K-1).EQ.4) GO TO 2105 | SP35010* |
| | ND=IDR(N) | SP35020* |
| | IF(ND.EQ.0) GO TO 2105 | SP35030* |
| | IF(ISEA(I,J).EQ.0) GO TO 2105 | SP35040* |
| | IF(PHI(N).GE.RH(MD)) GO TO 2105' | SP35050* |
| | T(N)=0. | SP35060* |
| | IN(I,J,K)=7 | SP35070* |
| | RC(MD)=0. | SP35080* |
| | IF(K.EQ.K0) PERM(M)=0. | SP35090* |
| C | | SP35100* |

| | |
|--|----------|
| GO TO 2260 | SP35110* |
| C NEXT FEW STATEMENTS ARE LAYER DEPENDENT | SP35120* |
| C | SP35130* |
| 2105 IF(K.EQ.1) GO TO 2110 | SP35140* |
| IF(K.EQ.K0) GO TO 2130 | SP35150* |
| GO TO 2120 | SP35160* |
| C | SP35170* |
| C LAYER 1 | SP35180* |
| C DROP A BLOCK WHERE INTERFACE IS ABOVE THE TOP | SP35190* |
| 2110 IF(Z.LE.Z2) GO TO 2210 | SP35200* |
| C | SP35210* |
| C INTERFACE BELOW BLOCK BOTTOM IN LAYER 1 | SP35220* |
| IF(Z.GE.DBOT(I,J)) GO TO 2240 | SP35230* |
| C | SP35240* |
| C ADJUST T IN BLOCK OF LAYER 1 | SP35250* |
| T(N)=TT(I,J,K)*(Z-Z2)/(DBOT(I,J)-Z2) | SP35260* |
| IF(ND.NE.0) T(N)=TT(I,J,K) | SP35270* |
| GO TO 2160 | SP35280* |
| C | SP35290* |
| C | SP35300* |
| C MIDDLE LAYERS | SP35310* |
| C | SP35320* |
| C CHECK LOWER LAYER | SP35330* |
| 2120 IF(IN(I,J,K-1).EQ.0.OR.IN(I,J,K-1).EQ.4) GO TO 2260 | SP35340* |
| C | SP35350* |
| C DROP A BLOCK WHERE INTERFACE IS ABOVE THE TOP | SP35360* |
| IF(Z.LE.Z2) GO TO 2210 | SP35370* |
| C | SP35380* |
| C INTERFACE BELOW BLOCK BOTTOM | SP35390* |
| IF(Z.GE.DTOP(K-1)) GO TO 2240 | SP35400* |
| C | SP35410* |
| C ADJUST T FOR LAYER K | SP35420* |
| T(N)=TT(I,J,K)*(Z-Z2)/(DTOP(K-1)-DTOP(K)) | SP35430* |
| IF(ND.NE.0) T(N)=TT(I,J,K) | SP35440* |
| GO TO 2160 | SP35450* |
| C | SP35460* |
| C TOP LAYER | SP35470* |
| C | SP35480* |
| C CHECK LOWER LAYER | SP35490* |
| 2130 IF(IN(I,J,K-1).EQ.0.OR.IN(I,J,K-1).EQ.4) GO TO 2260 | SP35500* |
| C | SP35510* |
| C DROP A BLOCK WHERE INTERFACE IS ABOVE THE TOP | SP35520* |
| IF(Z.LE.Z2) GO TO 2210 | SP35530* |
| C | SP35540* |
| C INTERFACE BELOW BLOCK BOTTOM | SP35550* |
| IF(Z.GE.DTOP(K-1)) GO TO 2240 | SP35560* |
| C | SP35570* |
| C ADJUST T FOR LAYER K0 | SP35580* |
| BOTTOM(M)=Z*(-1.0) | SP35590* |
| IF(ND.NE.0) BOTTOM(M)=BTM(I,J) | SP35600* |
| T(N)=PERM(M)*(PHI(N)-BOTTOM(M)) | SP35610* |
| C CHOP NODES A'ID NOTE AN ADJUSTED T | SP35620* |
| C | SP35630* |
| 2160 IN(I,J,K)=4 | SP35640* |
| GO TO 2260 | SP35650* |
| 2210 T(N)=0. | SP35660* |
| IN(I,J,K)=6 | SP35670* |

| | |
|---|----------|
| GO TO 2260 | SP35680* |
| 2240 IF(K.NE.K0) T(N)=TT(I,J,K) | SP35690* |
| IF(K.EQ.K0) T(N)=PERM(M)*(PHI(N)-BTM(I,J)) | SP35700* |
| IF(K.EQ.K0) BOTTOM(M)=BTM(I,J) | SP35710* |
| IN(I,J,K)=0 | SP35720* |
| 2260 CONTINUE | SP35730* |
| 2280 CONTINUE | SP35740* |
| 2300 CONTINUE | SP35750* |
| C | SP35760* |
| IF (KODCHP.EQ.1) GO TO 60 | SP35770* |
| C | SP35780* |
| C SOUTH SIDE | SP35790* |
| C | SP35800* |
| DO 2500 K=1,K0 | SP35810* |
| DO 2480 J=2,J1 | SP35820* |
| DO 2460 II=1,II | SP35830* |
| I=I0-II | SP35840* |
| N=I+(J-1)*I0+(K-1)*NIJ | SP35850* |
| M=I+(J-1)*I0 | SP35860* |
| Z2=DTOP(K) | SP35870* |
| IF(T(N).EQ.0.) GO TO 2460 | SP35880* |
| C | SP35890* |
| C COMPUTE INTERFACE DEPTH BELOW SEA LEVEL | SP35900* |
| Z=RF*PHI(N) | SP35910* |
| C | SP35920* |
| C DROP A BLOCK IF OCEAN LAYER IS LEAKING INTO AQUIFER | SP35930* |
| C | SP35940* |
| C CHECK LOWER LAYER | SP35950* |
| IF(IN(I,J,K-1).EQ.0.OR.IN(I,J,K-1).EQ.4) GO TO 2305 | SP35960* |
| ND=IDR(N) | SP35970* |
| IF(ND.EQ.0) GO TO 2305 | SP35980* |
| IF(ISEA(I,J).EQ.0) GO TO 2305 | SP35990* |
| IF(PHI(N).GE.RH(ND)) GO TO 2305 | SP36000* |
| T(N)=0. | SP36010* |
| IN(I,J,K)=7 | SP36020* |
| RC(ND)=0. | SP36030* |
| IF(K.EQ.K0) PERM(M)=0. | SP36040* |
| GO TO 2460 | SP36050* |
| C | SP36060* |
| C THE NEXT FEW STATEMENTS ARE LAYER DEPENDENT | SP36070* |
| C | SP36080* |
| 2305 IF(K.EQ.1) GO TO 2310 | SP36090* |
| IF(K.EQ.K0) GO TO 2330 | SP36100* |
| GO TO 2320 | SP36110* |
| C | SP36120* |
| C LAYER 1 | SP36130* |
| C DROP A BLOCK WHERE INTERFACE IS ABOVE THE TOP | SP36140* |
| 2310 IF(Z.LE.Z2) GO TO 2410 | SP36150* |
| C | SP36160* |
| C INTERFACE BELOW BLOCK BOTTOM IN LAYER 1 | SP36170* |
| IF(Z.GE.DBOT(I,J)) GO TO 2440 | SP36180* |
| C | SP36190* |
| C ADJUST T IN BLOCK OF LAYER 1 | SP36200* |
| T(N)=TT(I,J,K)*(Z-Z2)/(DBOT(I,J)-Z2) | SP36210* |
| IF(ND.NE.0) T(N)=TT(I,J,K) | SP36220* |
| GO TO 2360 | SP36230* |
| C | SP36240* |

| | | |
|---|--|----------|
| C | MIDDLE LAYERS | SP36250* |
| C | | SP36260* |
| C | CHECK LOWER LAYER | SP36270* |
| | 2320 IF(IN(I,J,K-1).EQ.0.OR.IN(I,J,K-1).EQ.4) GO TO 2460 | SP36280* |
| C | | SP36290* |
| C | DROP A BLOCK WHERE INTERFACE IS ABOVE THE TOP | SP36300* |
| | IF(Z.LE.Z2) GO TO 2410 | SP36310* |
| C | | SP36320* |
| C | INTERFACE BELOW BLOCK BOTTOM | SP36330* |
| | IF(Z.GE.DTOP(K-1))GO TO 2440 | SP36340* |
| C | | SP36350* |
| C | ADJUST T FOR LAYER K | SP36360* |
| | T(N)=TT(I,J,K)*(Z-Z2)/(DTOP(K-1)-DTOP(K)) | SP36370* |
| | IF(ND.NE.0) T(N)=TT(I,J,K) | SP36380* |
| | GO TO 2360 | SP36390* |
| C | | SP36400* |
| C | TOP LAYER | SP36410* |
| C | | SP36420* |
| C | CHECK LOWER LAYER | SP36430* |
| | 2330 IF(IN(I,J,K-1).EQ.0.OR.IN(I,J,K-1).EQ.4) GO TO 2460 | SP36440* |
| C | | SP36450* |
| C | DROP A BLOCK WHERE INTERFACE IS ABOVE THE TOP | SP36460* |
| | IF(Z.LE.Z2) GO TO 2410 | SP36470* |
| C | | SP36480* |
| C | INTERFACE BELOW BLOCK BOTTOM | SP36490* |
| | IF(Z.GE.DTOP(K-1)) GO TO 2440 | SP36500* |
| C | | SP36510* |
| C | ADJUST T FOR LAYER K0 | SP36520* |
| | BOTTOM(M)=Z*(-1.) | SP36530* |
| | IF(ND.NE.0) BOTTOM(M)=BTM(I,J) | SP36540* |
| | T(N)=PERM(M)*(PHI(N)-BOTTOM(M)) | SP36550* |
| C | | SP36560* |
| C | CHOP NODES AND NOTE AN ADJUSTED T | SP36570* |
| C | | SP36580* |
| | 2360 IN(I,J,K)=4 | SP36590* |
| | GO TO 2460 | SP36600* |
| | 2410 T(N)=0. | SP36610* |
| | IN(I,J,K)=6 | SP36620* |
| | GO TO 2460 | SP36630* |
| | 2440 IF(K.NE.K0) T(N)=TT(I,J,K) | SP36640* |
| | IF(K.EQ.K0) T(N)=PERM(M)*(PHI(N)-BTM(I,J)) | SP36650* |
| | IF(K.EQ.K0) BOTTOM(M)=BTM(I,J) | SP36660* |
| | IN(I,J,K)=0 | SP36670* |
| | 2460 CONTINUE | SP36680* |
| | 2480 CONTINUE | SP36690* |
| | 2500 CONTINUE | SP36700* |
| C | | SP36710* |
| | 2600 GO TO 60 | SP36720* |
| C | | SP36730* |
| C | | SP36740* |
| C | PRINT CONTENTS OF IN ARRAY AND LOCATION OF ADJUSTED T | SP36750* |
| | 2700 DO 2710 K=1,K0 | SP36760* |
| | DO 2710 I=2,I1 | SP36770* |
| | DO 2710 J=2,J1 | SP36780* |
| | 2710 IF(IN(I,J,K).EQ.6) KCHOP2(K)=KCHOP2(K)+1 | SP36790* |
| | DO 2740 K=1,K0 | SP36800* |
| | WRITE(6,2925) K,KCHOP2(K) | SP36810* |

| | |
|--|----------|
| DO 2720 I=1,I0 | SP36820* |
| 2720 WRITE(6,2930) I,(IN(I,J,K),J=1,J0) | SP36830* |
| 2740 CONTINUE | SP36840* |
| C | SP36850* |
| WRITE(6,2940) | SP36860* |
| DO 2800 K=1,K0 | SP36870* |
| DO 2800 I=1,I0 | SP36880* |
| DO 2800 J=1,J0 | SP36890* |
| N=I+(J-1)*I0+(K-1)*NIJ | SP36900* |
| IF(IN(I,J,K).NE.4) GO TO 2800 | SP36910* |
| Z=RF*PHI(N) | SP36920* |
| WRITE(6,2950) K,I,J,Z,T(N) | SP36930* |
| 2800 CONTINUE | SP36940* |
| C | SP36950* |
| C PRINT FINAL HEAD VALUES FOR SELECTED NODES | SP36960* |
| IF(IHEDKD.NE.1) GO TO 2825 | SP36970* |
| WRITE(8,2985) | SP36980* |
| C UPPER LAYER ONLY | SP36990* |
| K=K0 | SP37000* |
| DO 2820 I=2,I1 | SP37010* |
| DO 2820 J=2,J1 | SP37020* |
| IF(IFNLHD(I,J,K).NE.1) GO TO 2820 | SP37030* |
| N=I+(J-1)*I0+(K-1)*NIJ | SP37040* |
| WRITE(8,2990) I,J,K,PHI(N) | SP37050* |
| 2820 CONTINUE | SP37060* |
| C ALL LAYERS | SP37070* |
| DO 2823 K=1,K0 | SP37080* |
| DO 2823 I=2,I1 | SP37090* |
| DO 2823 J=2,J1 | SP37100* |
| IF(IFNLHD(I,J,K).NE.2) GO TO 2823 | SP37110* |
| N=I+(J-1)*I0+(K-1)*NIJ | SP37120* |
| WRITE(8,2990) I,J,K,PHI(N) | SP37130* |
| 2823 CONTINUE | SP37140* |
| C | SP37145* |
| C PRINT FINAL PHI, T, PERM, AND TK MATRICES | SP37150* |
| 2825 IF(KPRNTT.NE.1) GO TO 2895 | SP37160* |
| I0J0=I0*J0 | SP37170* |
| L1=1 | SP37175* |
| DO 2893 M1=1,3 | SP37180* |
| K0A=K0 | SP37185* |
| IF(M1.GT.2) K0A=K0A-1 | SP37190* |
| DO 2890 K=1,K0A | SP37195* |
| WRITE(9,2830) | SP37200* |
| 2830 FORMAT(9X,'1',9X,'1',9X,'0',9X,'1',9X,'1',9X,'1',9X,'0',9X,'0') | SP37205* |
| DO 2885 I=1,I0 | SP37210* |
| DO 2880 J=1,J0 | SP37215* |
| N=I+(J-1)*I0+(K-1)*I0J0 | SP37220* |
| GO TO (2835,2840,2845),M1 | SP37225* |
| 2835 TDEN(L1)=PHI(N) | SP37230* |
| GO TO 2850 | SP37235* |
| 2840 IF(K.LT.K0A) GO TO 2843 | SP37240* |
| M=I+(J-1)*I0 | SP37243* |
| TDEN(L1)=PERM(M) | SP37246* |
| GO TO 2850 | SP37250* |
| 2843 TDEN(L1)=T(N) | SP37253* |
| GO TO 2850 | SP37256* |
| 2845 TDEN(L1)=TK(N) | SP37260* |

```

2850 IF(L1.LT.8)GO TO 2855                                SP37263*
      GO TO 2860                                           SP37266*
2855 IF(J.EQ.J0)GO TO 2860                                SP37270*
      L1=L1+1                                              SP37273*
      GO TO 2880                                           SP37276*
2860 WRITE(9,2865)(TDEN(N2), N2=1,8)                     SP37280*
2865 FORMAT(8E10.4)                                       SP37283*
      DO 2870 N2=1,8                                       SP37286*
      TDEN(N2)=0.0                                         SP37290*
2870 CONTINUE                                             SP37293*
      L1=1                                                 SP37296*
2880 CONTINUE                                             SP37300*
2885 CONTINUE                                             SP37303*
      IF(N1.NE.2)GO TO 2888                                SP37306*
      IF(IWATER.NE.ICHK(6))GO TO 2888                     SP37310*
      KOA1=KOA-1                                           SP37313*
      IF(K.NE.KOA1)GO TO 2888                              SP37316*
      WRITE(9,2887)                                         SP37320*
2887 FORMAT(39X,'1',9X,'1',9X,'1'/'ENDOFMEMBER')        SP37323*
      GO TO 2890                                           SP37326*
2888 IF(N1.EQ.1.AND.K.NE.KOA)GO TO 2890                 SP37330*
      WRITE(9,2889)                                         SP37333*
2889 FORMAT('ENDOFMEMBER')                                SP37336*
2890 CONTINUE                                             SP37340*
2893 CONTINUE                                             SP37345*
2895 RETURN                                               SP37350*
2900 CONTINUE                                             SP37360*
      IF(TEST.EQ.0.) RETURN                                SP37370*
      GO TO 60                                             SP37380
C ..... SP37390
C ..... SP37400
C ---FORMATS--- SP37410
C ..... SP37420
C ..... SP37430
C ..... SP37440
220 FORMAT ('0EXCEEDED PERMITTED NUMBER OF ITERATIONS'/' ',39('*')) SP37450
230 FORMAT (///1H0,I5,22H ITERATION PARAMETERS:,6E15.7/(/28X,6E15.7/))SP37460
240 FORMAT ('-',44X,'SOLUTION BY THE STRONGLY IMPLICIT PROCEDURE'/45X,SP37470
      143('_'))                                             SP37480
2901 FORMAT(I10,G10.0,2I10)                                SP37490*
2902 FORMAT('0','NTAD =',I10,/,3X,'RF =',F10.2,/,1X,'KODCHP =',I10,
      1/1X,'KPRNTT =',I10)                                SP37500*
2903 FORMAT(16F5.0)                                       SP37520*
2904 FORMAT('1','BOTTOM OF AQUIFER SYSTEM:'//)           SP37530*
2905 FORMAT('0',I4,2X,20F6.1/(7X,20F6.1))                SP37540*
2906 FORMAT(80I1)                                          SP37550*
2907 FORMAT('1','SPECIFIED-HEAD OCEAN NODES'//5X,'NUMBERS REPRESENT LAYS SP37560*
      1ER CONTAINING THE SPECIFIED HEAD OCEAN NODE'/7X,'0 = NO OCEAN NODES SP37563*
      2 AT THIS LOCATION'//)                               SP37566*
2908 FORMAT('0',I4,2X,60(I1,1X)/(7X,60(I1,1X)))          SP37570*
2909 FORMAT(8G10.0)                                       SP37580*
2910 FORMAT('0',5X,'DEPTH TO TOP OF LAYER',I5,' = ',F6.1) SP37590*
2911 FORMAT('1','PRELIMINARY IN ARRAY FOR LAYER',I5//' INITIAL TRANSMSP37600*
      1ISSIVITY SET EQUAL TO ZERO ALONG PERIMETER'/' AND WHERE CODE =SP37610*
      2 8.'//)                                             SP37615*
2912 FORMAT('0',I4,2X,60I2/(7X,60I2))                    SP37620*
2920 FORMAT('0','T ADJUSTED ON ITERATION ',I4)           SP37630*

```

```

2925 FORMAT('1','IN ARRAY FOR LAYER',I3,5X,I5,' BLOCKS CHOPPED'//'      8=SP37640*
1 INITIAL TRANSMISSIVITY EQUALS ZERO '/'      6= BLOCK DROPPED DURING SP37643*
2CHOP'/'      7= BLOCK DROPPED BECAUSE OF OCEAN LEAKAGE INTO AQUIFER'/SP37646*
3'      4= BLOCK WITH ADJUSTED TRANSMISSIVITY'///)      SP37648*
2930 FORMAT('0',6I12/(3X,60I2))      SP37650*
2940 FORMAT('1','SUMMARY OF NODES WITH ADJUSTED TRANSMISSIVITY'/'      L ASP37660*
1YER      ROW      COLUMN      Z      T(I,J,K)')      SP37670*
2950 FORMAT('0',5X,I2,2(8X,I2),6X,F6.2,6X,F8.4)      SP37680*
2960 FORMAT(10(I3,I3,I2))      SP37690*
2970 FORMAT('1','HEAD VALUES AT SELECTED NODES AFTER EACH ITERATION'//1SP37700*
14X,'NODE(I,J,K):',5X,10('(',I2,',',I2,',',I1,')'))      SP37705*
2980 FORMAT('0',' HEAD AT ITERATION',2X,I5,5X,10(2X,F6.2,2X))      SP37710*
2985 FORMAT('1',10X,'FINAL HEAD VALUES FOR SELECTED NODES')      SP37720*
2990 FORMAT(' ',3I5,5X,F7.2)      SP37730*
8100 FORMAT(' ', 'I=',I3,2X, 'J=',I3,2X, 'K=',I3,2X, 'DL=',E10.4, ' E=',      SP37740*
1E10.4, ' W=',E10.4, ' CL=',E10.4, ' BL=',E10.4, ' AL=',E10.4, 'SIPNORM' SP37750*
2)      SP37760*
8125 FORMAT(' ', 'B,D,F,H,SU,Z,RHO,UXR=',8(E10.4,2X))      SP37770*
8130 FORMAT(' ', 'A,C,G,WU,TU,U=',6(E10.4,2X))      SP37780*
8150 FORMAT(' ',5X, 'T(N)=',E10.4, ' T(NIB)=',E10.4, ' T(NIA)=',E10.4,      SP37790*
1' T(NJB)=',E10.4, ' T(NJA)=',E10.4, ' T(NKB)=',E10.4, ' T(NKA)='      SP37800*
2,E10.4,/)      SP37810*
8200 FORMAT(' ', 'I=',I3,2X, 'J=',I3,2X, 'K=',I3,2X, 'DL=',E10.4, ' E=',      SP37820*
1E10.4, ' W=',E10.4, ' CL=',E10.4, ' BL=',E10.4, ' AL=',E10.4, 'SIPREV') SP37830*
8300 FORMAT('1',' I ', ' J ', ' T(I,J,1) ', ' T(I,J,2) ', ' T(I,J,3)      SP37840*
1', ' T(I,J,4) ', ' T(I,J,5) ')      SP37850*
8400 FORMAT(' ',2I3,5E12.4)      SP37860*
8500 FORMAT('1',' I ', ' J ', ' TK(I,J,1) ', ' TK(I,J,2) ', ' TK(I,J,3)      SP37870*
1', ' TK(I,J,4)')      SP37880*
8600 FORMAT(' ',2I3,4E12.4)      SP37890*
END      SP37900

```

```

SUBROUTINE COEF(PHI,STRT,OLD,T,S,TR,TC,TK,WELL,DELX,DELY,DELZ,FACTCOF0010
1,PERM,BOTTOM,QRE)
C-----COF0020
C COMPUTE COEFFICIENTS
C-----COF0030
C-----COF0040
C-----COF0050
C-----COF0060
C SPECIFICATIONS:
C-----COF0070
C REAL *8PHI
C-----COF0080
C-----COF0090
C DIMENSION PHI(IO,J0,K0),STRT(IO,J0,K0),OLD(IO,J0,K0),T(IO,J0,K0COF0100
1),S(IO,J0,K0),TR(IO,J0,K0),TC(IO,J0,K0),TK(IK,JK,K5),WELL(IO,COF0110
2J0,K0),DELX(J0),DELY(IO),DELZ(K0),FACT(K0,3),PERM(IP,JP),BOTCOF0120
3TOM(IP,JP),QRE(IQ,JQ)
C-----COF0130
C-----COF0140
C COMMON /INTEGR/ IO,J0,K0,I1,J1,K1,I,J,K,NPER,KTH,ITMAX,LENGTH,KP,NCOF0150
1WEL,NUMT,IFINAL,IT,KT,IHEAD,IDRAW,IFLO,IERR,I2,J2,K2,IMAX,ITMX1,NCCOF0160
2H,IDK1,IDK2,IWATER,IQRE,IP,JP,IQ,JQ,IK,JK,K5,IPU1,IPU2,ITK,IEQN COF0170
3,NKODE,KFLOW
C-----COF0180*
C COMMON /SPARAM/ TMAX,CDLT,DELT,ERR,TEST,SUM,SUMP,QR,BETA1
C-----COF0190*
C COMMON /SARRAY/ ICHK(13),LEVEL1(9),LEVEL2(9)
C-----COF0200
C N3 = 1
C-----COF0210
C N7=-1
C-----COF0220
C RETURN
C-----COF0230
C-----COF0240
C ---COMPUTE TRANSMISSIVITY FOR UPPER HYDROLOGIC UNIT WHEN
C-----COF0250
C IT IS UNCONFINED---
C-----COF0260
C *****
C-----COF0270
C ENTRY TRANS(N3)
C-----COF0280
C *****
C-----COF0290
C DO 10 I=2,I1
C-----COF0300
C DO 10 J=2,J1
C-----COF0310
C IF (PERM(I,J).EQ.0.) GO TO 10
C-----COF0320
C T(I,J,K0)=PERM(I,J)*(PHI(I,J,K0)-BOTTOM(I,J))
C-----COF0330
C IF (T(I,J,K0).GT.0.) GO TO 10
C-----COF0340
C IF (WELL(I,J,K0).LT.0.) WRITE (6,60) I,J,K0
C-----COF0350
C IF (WELL(I,J,K0).GE.0.) WRITE (6,70) I,J,K0
C-----COF0360
C PERM(I,J)=0.
C-----COF0370
C T(I,J,K0)=0.
C-----COF0380
C TR(I,J-1,K0)=0.
C-----COF0390
C TR(I,J,K0)=0.
C-----COF0400
C TC(I,J,K0)=0.
C-----COF0410
C TC(I-1,J,K0)=0.
C-----COF0420
C IF (K0.NE.1) TK(I,J,K1)=0.
C-----COF0430
C PHI(I,J,K0)=1.D30
C-----COF0440
10 CONTINUE
C-----COF0450
C IF (N3.EQ.1) RETURN
C-----COF0460
C N1=K0
C-----COF0470
C N2=K0
C-----COF0480
C N4=K1
C-----COF0490
C GO TO 20
C-----COF0500
C ---COMPUTE T COEFFICIENTS---
C-----COF0510
C *****
C-----COF0520
C ENTRY TCOF
C-----COF0530
C *****
C-----COF0540
C N7=N7+1
C-----COF0550
C N1=1
C-----COF0560
C N2=K0
C-----COF0570

```

```

      IF(N7.GT.0) N2=K1
      N4=1
20 DO 40 K=N1,N2
      DO 40 I=1,I1
      DO 40 J=1,J1
      IF (T(I,J,K).EQ.0.) GO TO 32
      IF (T(I,J+1,K).EQ.0.) GO TO 30
      TR(I,J,K)=(2.*T(I,J+1,K)*T(I,J,K))/(T(I,J,K)*DELX(J+1)+T(I,J+1,K)*
1DELX(J))*FACT(K,1)
30 IF (T(I+1,J,K).EQ.0.) GO TO 32
      TC(I,J,K)=(2.*T(I+1,J,K)*T(I,J,K))/(T(I,J,K)*DELY(I+1)+T(I+1,J,K)*
1DELY(I))*FACT(K,2)
      IF(T(I,J+1,K).EQ.0.) GO TO 32
      GO TO 40
32 IF(T(I,J,K).EQ.0.) GO TO 33
      GO TO 34
33 TR(I,J,K)=0.
      TR(I,J-1,K)=0.
      TC(I,J,K)=0.
      TC(I-1,J,K)=0.
      TK(I,J,K)=0.
      IF(WELL(I,J,K).LT.0.)GO TO 80
34 IF(T(I+1,J,K).EQ.0.) GO TO 35
      GO TO 36
35 TC(I,J,K)=0.
      TC(I+1,J,K)=0.
      TK(I+1,J,K)=0.
36 IF(T(I,J+1,K).EQ.0.) GO TO 37
      GO TO 40
37 TR(I,J,K)=0.
      TR(I,J+1,K)=0.
      TK(I,J+1,K)=0.
40 CONTINUE
      IF (K0.EQ.1.OR.ITK.EQ.ICHK(10).OR.N3.EQ.0) RETURN
      DO 50 K=N4,K1
      DO 50 I=2,I1
      DO 50 J=2,J1
      IF (T(I,J,K+1).EQ.0.) GO TO 50
      T1=T(I,J,K)*FACT(K,3)
      T2=T(I,J,K+1)*FACT(K+1,3)
      TK(I,J,K)=(2.*T2*T1)/(T1*DELZ(K+1)+T2*DELZ(K))
50 CONTINUE
      GO TO 100
80 WRITE(6,73) I,J,K
      WRITE(6,75)
      IERR=2
      CALL OUTPJT
100 RETURN

60 FORMAT ('-',20('*'),'WELL',2I3,' IN LAYER',I3,' GOES DRY',20('*'))COF1020
70 FORMAT ('-',20('*'),'NODE',2I3,' IN LAYER',I3,' GOES DRY',20('*'))COF1030
73 FORMAT ('-',20('*'),' WELL',2I3,' IN LAYER',I3,' GOES DRY--DEWATECOF1031*
1RED OR SALINE-WATER INTRUSION ',20('*')) COF1032*
75 FORMAT ('-',5X,'*** EXECUTION TERMINATING-DISCHARGING WELL NODE (NOCOF1033*
1T IN TOP LAYER) GOES DRY--CHECK FOR SW INTRUSION ***'/10X,'THE FOLCOF1034*
2LOWING OUTPUT REPRESENTS STATE OF SOLUTION WHEN WELL WENT DRY') COF1035*
      END

```

C
C

```

SUBROUTINE CHECKI(PHI,STRT,OLD,T,S,TR,TC,TK,WELL,DELX,DELY,DELZ,FACHK0010
1CT,JFLO,FLOW,QRE,CHK0020
2IDR,RH,RC,RB,IRIV)CHK0030*
C -----CHK0040
C COMPUTE A VOLUMETRIC BALANCECHK0050
C -----CHK0060
C SPECIFICATIONS:CHK0070
C REAL *8PHICHK0080
C CHK0090
C CHK0100
C DIMENSION PHI(IO,J0,K0),STRT(IO,J0,K0),OLD(IO,J0,K0),T(IO,J0,K0)CHK0110
1),S(IO,J0,K0),TR(IO,J0,K0),TC(IO,J0,K0),TK(IK,JK,K5),WELL(IO,CHK0120
2J0,K0),DELX(J0),DELY(IO),DELZ(K0),FACT(K0,3),JFLO(NCH,3),FLOCHK0130
3W(NCH),QRE(IQ,JQ)CHK0140
4,IDR(IO,J0,K0),RH(1),RC(1),RB(1)CHK0150*
C CHK0160
C COMMON /INTEGR/ IO,J0,K0,I1,J1,K1,I,J,K,NPER,KTH,ITMAX,LENGTH,KP,NCHK0170
1WEL,NUMT,IFINAL,IT,KT,IHEAD,IDRAW,IFLO,IERR,I2,J2,K2,IMAX,ITMX1,NCCHK0180
2H,IDK1,IDK2,IWATER,IQRE,IP,JP,IQ,JQ,IK,JK,K5,IPU1,IPU2,ITK,IEQNCHK0190
3,NKODE,KFLOWCHK0200*
C COMMON /SPARAM/ TMAX,CDLT,DELT,ERR,TEST,SUM,SUMP,QR,BETA1CHK0210*
C COMMON /SARRAY/ ICHK(13),LEVEL1(9),LEVEL2(9)CHK0220
C COMMON /CK/ RLFLXT,STORT,QRET,CHST,CHDT,FLUXT,PUMPT,CFLUXT,FLXNTCHK0230*
C RETURNCHK0240
C .....CHK0250
C *****CHK0260
C ENTRY CHECKCHK0270
C *****CHK0280
C ---INITIALIZE VARIABLES---CHK0290
C PUMP=0.CHK0300
C STOR=0.CHK0310
C FLUXS=0.0CHK0320
C CHD1=0.0CHK0330
C CHD2=0.0CHK0340
C QREFLX=0.CHK0350
C CFLUX=0.CHK0360
C FLUX=0.CHK0370
C RLFLUX=0.CHK0380*
C FLXN=0.0CHK0390
C II=0CHK0400
C .....CHK0410
C IF(MOD(KT,KTH).EQ.0.OR.IFINAL.EQ.1) WRITE(6,5900)CHK0425
C CHK0420
C ---COMPUTE RATES,STORAGE AND PUMPAGE FOR THIS STEP---CHK0430
C DO 220 K=1,K0CHK0440
C DO 220 I=2,I1CHK0450
C DO 220 J=2,J1CHK0460
C INDEX=0CHK0470
C IF (T(I,J,K).EQ.0.) GO TO 220CHK0480
C AREA=DELX(J)*DELY(I)CHK0490
C VOLUME=AREA*DELZ(K)CHK0500
C IF (S(I,J,K).GE.0.) GO TO 180CHK0510
C CHK0520
C ---COMPUTE FLOW RATES TO AND FROM CONSTANT HEAD BOUNDARIES---CHK0530
C II=II+1CHK0540
C FLOW(II)=0.CHK0550
C JFLO(II,1)=KCHK0560

```

| | |
|---|----------|
| JFLO(II,2)=I | CHK0570 |
| JFLO(II,3)=J | CHK0580 |
| IF (S(I,J-1,K).LT.0..OR.T(I,J-1,K).EQ.0.) GO TO 30 | CHK0590 |
| X=(PHI(I,J,K)-PHI(I,J-1,K))*TR(I,J-1,K)*DELY(I) | CHK0600 |
| IF(IEQN.EQ.ICHK(11)) X=X*DELZ(K) | CHK0610 |
| FLOW(II)=FLOW(II)+X | CHK0620 |
| IF(KFLOW.NE.1) GO TO 5 | CHK0630* |
| IF(ABS(X).LT.0.00001) GO TO 5 | CHK0640* |
| M=J-1 | CHK0650* |
| WRITE(6,6000) I,J,K,I,M,K,X | CHK0660* |
| INDEX=1 | CHK0670* |
| 5 IF (X) 10,30,20 | CHK0690 |
| 10 CHD1=CHD1+X | CHK0700 |
| GO TO 30 | CHK0710 |
| 20 CHD2=CHD2+X | CHK0720 |
| 30 IF (S(I,J+1,K).LT.0..OR.T(I,J+1,K).EQ.0.) GO TO 60 | CHK0730 |
| X=(PHI(I,J,K)-PHI(I,J+1,K))*DELY(I)*TR(I,J,K) | CHK0740 |
| IF(IEQN.EQ.ICHK(11)) X=X*DELZ(K) | CHK0750 |
| FLOW(II)=FLOW(II)+X | CHK0760 |
| IF(KFLOW.NE.1) GO TO 35 | CHK0770* |
| IF(ABS(X).LT.0.00001) GO TO 35 | CHK0780* |
| M=J+1 | CHK0790* |
| IF(INDEX.EQ.0) WRITE(6,6000) I,J,K,I,M,K,X | CHK0800* |
| INDEX=1 | CHK0810* |
| WRITE(6,6100) I,M,K,X | CHK0820* |
| 35 IF (X) 40,60,50 | CHK0830 |
| 40 CHD1=CHD1+X | CHK0840 |
| GO TO 60 | CHK0850 |
| 50 CHD2=CHD2+X | CHK0860 |
| 60 IF (K.EQ.1) GO TO 90 | CHK0870 |
| IF (S(I,J,K-1).LT.0..OR.T(I,J,K-1).EQ.0.) GO TO 90 | CHK0880 |
| X=(PHI(I,J,K)-PHI(I,J,K-1))*TK(I,J,K-1)*AREA | CHK0890 |
| FLOW(II)=FLOW(II)+X | CHK0900 |
| IF(KFLOW.NE.1) GO TO 65 | CHK0910* |
| IF(ABS(X).LT.0.00001) GO TO 65 | CHK0920* |
| M=K-1 | CHK0930* |
| IF(INDEX.EQ.0) WRITE(6,6000) I,J,K,I,J,M,X | CHK0940* |
| INDEX=1 | CHK0950* |
| WRITE(6,6100) I,J,M,X | CHK0960* |
| 65 IF (X) 70,90,80 | CHK0970 |
| 70 CHD1=CHD1+X | CHK0980 |
| GO TO 90 | CHK0990 |
| 80 CHD2=CHD2+X | CHK1000 |
| 90 IF (K.EQ.K0) GO TO 120 | CHK1010 |
| IF (S(I,J,K+1).LT.0..OR.T(I,J,K+1).EQ.0.) GO TO 120 | CHK1020 |
| X=(PHI(I,J,K)-PHI(I,J,K+1))*TK(I,J,K)*AREA | CHK1030 |
| FLOW(II)=FLOW(II)+X | CHK1040 |
| IF(KFLOW.NE.1) GO TO 95 | CHK1050* |
| IF(ABS(X).LT.0.00001) GO TO 95 | CHK1060* |
| M=K+1 | CHK1070* |
| IF(INDEX.EQ.0) WRITE(6,6000) I,J,K,I,J,M,X | CHK1080* |
| INDEX=1 | CHK1090* |
| WRITE(6,6100) I,J,M,X | CHK1100* |
| 95 IF (X) 100,120,110 | CHK1110 |
| 100 CHD1=CHD1+X | CHK1120 |
| GO TO 120 | CHK1130 |
| 110 CHD2=CHD2+X | CHK1140 |

| | | |
|-----|--|----------|
| 120 | IF (S(I-1,J,K).LT.0..OR.T(I-1,J,K).EQ.0.) GO TO 150 | CHK1150 |
| | X=(PHI(I,J,K)-PHI(I-1,J,K))*TC(I-1,J,K)*DELX(J) | CHK1160 |
| | IF(IEQN.EQ.ICHK(11)) X=X*DELZ(K) | CHK1170 |
| | FLOW(II)=FLOW(II)+X | CHK1180 |
| | IF(KFLOW.NE.1) GO TO 125 | CHK1190 |
| | IF(ABS(X).LT.0.00001) GO TO 125 | CHK1200* |
| | M=I-1 | CHK1210* |
| | IF(INDEX.EQ.0) WRITE(6,6000) I,J,K,M,J,K,X | CHK1220* |
| | INDEX=1 | CHK1230* |
| | WRITE(6,6100) M,J,K,X | CHK1240* |
| 125 | IF (X) 130,150,140 | CHK1250 |
| 130 | CHD1=CHD1+X | CHK1260 |
| | GO TO 150 | CHK1270 |
| 140 | CHD2=CHD2+X | CHK1280 |
| 150 | IF (S(I+1,J,K).LT.0..OR.T(I+1,J,K).EQ.0.) GO TO 220 | CHK1290 |
| | X=(PHI(I,J,K)-PHI(I+1,J,K))*TC(I,J,K)*DELX(J) | CHK1300 |
| | IF(IEQN.EQ.ICHK(11)) X=X*DELZ(K) | CHK1310 |
| | FLOW(II)=FLOW(II)+X | CHK1320 |
| | IF(KFLOW.NE.1) GO TO 155 | CHK1330* |
| | IF(ABS(X).LT.0.00001) GO TO 155 | CHK1340* |
| | M=I+1 | CHK1350* |
| | IF(INDEX.EQ.0) WRITE(6,6000) I,J,K,M,J,K,X | CHK1360* |
| | INDEX=1 | CHK1370* |
| | WRITE(6,6100) M,J,K,X | CHK1380* |
| 155 | IF (X) 160,220,170 | CHK1390 |
| 160 | CHD1=CHD1+X | CHK1400 |
| | GO TO 220 | CHK1410 |
| 170 | CHD2=CHD2+X | CHK1420 |
| | GO TO 220 | CHK1430 |
| C | | CHK1440 |
| C | ---CHECK FOR EQUATION BEING SOLVED--- | CHK1450 |
| 180 | IF(IEQN.EQ.ICHK(11)) GO TO 211 | CHK1460 |
| C | | CHK1470 |
| C | ---EQUATION 4--- | CHK1480 |
| C | ---RECHARGE AND WELLS--- | CHK1490 |
| | IF (K.EQ.K0.AND.IQRE.EQ.ICHK(7)) QREFLX=QREFLX+QRE(I,J)*AREA | CHK1500 |
| | IF (WELL(I,J,K)) 190,210,200 | CHK1510 |
| 190 | PUMP=PUMP+WELL(I,J,K)*AREA | CHK1520 |
| | GO TO 210 | CHK1530 |
| 200 | CFLUX=CFLUX+WELL(I,J,K)*AREA | CHK1540 |
| C | | CHK1550 |
| C | ---COMPUTE VOLUME FROM STORAGE--- | CHK1560 |
| 210 | STOR=STOR+S(I,J,K)*(OLD(I,J,K)-PHI(I,J,K))*AREA | CHK1570 |
| | IF(IRIV.LE.0) GO TO 220 | CHK1580* |
| C | | CHK1585* |
| C | ---COMPUTE 'RIVER' LEAKAGE--- | CHK1590* |
| C | | CHK1595* |
| | ND=IDR(I,J,K) | CHK1600* |
| | IF(ND.EQ.0) GO TO 220 | CHK1610* |
| | IF(PHI(I,J,K).GT.RB(ND)) GO TO 217 | CHK1620* |
| | FLXRAT=RC(ND)*(RH(ND)-RB(ND))*AREA | CHK1630* |
| | IF(KFLOW.EQ.1) WRITE(6,6200) I,J,K,FLXRAT | CHK1640* |
| | RLFLUX=RLFLUX+FLXRAT | CHK1650* |
| | GO TO 220 | CHK1660* |
| 217 | FLXRAT=RC(ND)*(RH(ND)-PHI(I,J,K))*AREA | CHK1670* |
| | IF(KFLOW.EQ.1) WRITE(6,6200) I,J,K,FLXRAT | CHK1680* |
| | RLFLUX=RLFLUX+FLXRAT | CHK1690* |

| | | |
|---|--|----------|
| | GO TO 220 | CHK1700 |
| C | | CHK1710 |
| C | ---EQUATION 3--- | CHK1720 |
| C | ---RECHARGE AND WELLS--- | CHK1730 |
| | 211 IF (K.EQ.K0.AND.IQRE.EQ.ICHK(7)) QREFLX=QREFLX+QRE(I,J)*VOLUME | CHK1740 |
| | IF(WELL(I,J,K)) 212,214,213 | CHK1750 |
| | 212 PUMP=PUMP+WELL(I,J,K)*VOLUME | CHK1760 |
| | GO TO 214 | CHK1770 |
| | 213 CFLUX=CFLUX+WELL(I,J,K)*VOLUME | CHK1780 |
| C | | CHK1790 |
| C | ---COMPUTE VOLUME FROM STORAGE--- | CHK1800 |
| | 214 STOR=STOR+S(I,J,K)*(OLD(I,J,K)-PHI(I,J,K))*VOLUME | CHK1810 |
| | 220 CONTINUE | CHK1820 |
| C | | CHK1830 |
| C | | CHK1840 |
| C | ---COMPUTE CUMULATIVE VOLUMES, TOTALS, AND DIFFERENCES--- | CHK1850 |
| | FLXPT=0.0 | CHK1860 |
| | RLFLXT=RLFLXT-RLFLUX*DELT | CHK1870* |
| | STORT=STORT+STOR | CHK1880 |
| | STOR=STOR/DELT | CHK1890 |
| | QRET=QRET+QREFLX*DELT | CHK1900 |
| | CHDT=CHDT-CHD1*DELT | CHK1910 |
| | CHST=CHST+CHD2*DELT | CHK1920 |
| | PUMPT=PUMPT-PUMP*DELT | CHK1930 |
| | CFLUXT=CFLUXT+CFLUX*DELT | CHK1940 |
| | TOTL1=STORT+QRET+CFLUXT+CHST+FLXPT | CHK1950 |
| | TOTL2=CHDT+PUMPT+RLFLXT+FLXNT | CHK1960* |
| | SUMR=QREFLX+CFLUX+CHD2+CHD1+PUMP+RLFLUX+FLUXS+STOR | CHK1970* |
| | DIFF=TOTL2-TOTL1 | CHK1980 |
| | PERCNT=0.0 | CHK1990 |
| | IF (TOTL2.EQ.0.) GO TO 230 | CHK2000 |
| | PERCNT=DIFF/TOTL2*100. | CHK2010 |
| | 230 RETURN | CHK2020 |
| C | | CHK2030 |
| C | | CHK2040 |
| C | ---PRINT RESULTS--- | CHK2050 |
| C | ***** | CHK2060 |
| | ENTRY CWRITE | CHK2070 |
| C | ***** | CHK2080 |
| C | | CHK2090 |
| | WRITE (6,260) STOR,QREFLX,STORT,CFLUX,QRET,PUMP,CFLUXT,RLFLUX,CHST | CHK2100* |
| | 1,FLXPT,CHD2,TOTL1,CHD1,FLUX,FLUXS,RLFLXT,CHDT,SUMR,PUMPT,FLXNT,TOTL | CHK2110* |
| | 2L2,DIFF,PERCNT | CHK2120 |
| | IF (NCH.EQ.0) GO TO 240 | CHK2130 |
| | WRITE (6,270) | CHK2140 |
| | WRITE (6,280) ((JFLO(I,J),J=1,3),FLOW(I),I=1,NCH) | CHK2150 |
| C | | CHK2160 |
| C | ---COMPUTE VERTICAL FLOW--- | CHK2170 |
| | 240 X=0. | CHK2180 |
| | Y=0. | CHK2190 |
| | IF (K0.EQ.1) RETURN | CHK2200 |
| | DO 250 I=2,I1 | CHK2210 |
| | DO 250 J=2,J1 | CHK2220 |
| | X=X+(PHI(I,J,1)-PHI(I,J,2))*TK(I,J,1)*DELX(J)*DELY(I) | CHK2230 |
| | 250 Y=Y+(PHI(I,J,K1)-PHI(I,J,K0))*TK(I,J,K1)*DELX(J)*DELY(I) | CHK2240 |
| | WRITE (6,290) Y,X | CHK2250 |
| | RETURN | CHK2260 |

C
C
C
C
C
C
C

```

---FORMATS---
-----
260 FORMAT ('0',10X,'CUMULATIVE MASS BALANCE:',16X,'L**3',23X,'RATES FCHK2340
10R THIS TIME STEP:',16X,'L**3/T'/11X,24('-'),43X,25('-')//20X,'SOUCHK2350
2RCES:',69X,'STORAGE =',F20.4/20X,8('-'),68X,'RECHARGE =',F20.4/27XCHK2360
3,'STORAGE =',F20.2,35X,'CONSTANT FLUX =',F20.4/26X,'RECHARGE =',F2CHK2370
40.2,41X,'PUMPING =',F20.4/21X,'CONSTANT FLUX =',F20.2,30X,' RICHK2380*
5VER LEAKAGE =',F20.4/21X,'CONSTANT HEAD =',F20.2,34X,'CONSTANT HEACHK2390*
6D:'/27X,'LEAKAGE =',F20.2,46X,'IN =',F20.4/21X,'TOTAL SOURCES =',FCHK2400
720.2,45X,'OUT =',F20.4/96X,'LEAKAGE:'/20X,'DISCHARGES:',45X,'FROM CHK2410
8PREVIOUS PUMPING PERIOD =',F20.4/20X,11('-'),68X,'TOTAL =',F20.4/1CHK2420
96X,' RIVER LEAKAGE =',F20.2/21X,'CONSTANT HEAD =',F20.2,36X,'SCHK2430*
$UM OF RATES =',F20.4/19X'QUANTITY PUMPED =',F20.2/27X,'LEAKAGE =',CHK2440
$F20.2/19X,'TOTAL DISCHARGE =',F20.2//17X,'DISCHARGE-SOURCES =',F20CHK2450
$.2/15X,'PER CENT DIFFERENCE =',F20.2//)
CHK2460
270 FORMAT ('0FLOW RATES TO CONSTANT HEAD NODES:'/ ' ',34('-')// ' ',3(9CHK2470
1X,'K',4X,'I',4X,'J',5X,'RATE (L**3/T)')/' ',3(9X,'-',4X,'-',4X,'-'CHK2480
2,5X,13('-'))//)
CHK2490
280 FORMAT (/('1X,3(I10,2I5,G18.7)))
CHK2500
290 FORMAT ('0FLOW TO TOP LAYER =',G15.7,' FLOW TO BOTTOM LAYER =',GCHK2510
115.7,' POSITIVE UPWARD')
CHK2520
5900 FORMAT('1',' I J K FROM I J K CONSTANT HEAD RATE CHK2530*
1 RIVER LEAKAGE'//)
CHK2540*
6000 FORMAT(' ',3I4,7X,3I4,8X,G12.4)
CHK2550*
6100 FORMAT(' ',19X,3I4,8X,G12.4)
CHK2560*
6200 FORMAT(' ',3I4,48X,G12.4)
CHK2565*
END
CHK2570
```

```

SUBROUTINE PRNTAI(PHI,STRT,T,S,WELL,DELX,DELY) PRN0010
-----PRN0020
C PRINT MAPS OF DRAWDOWN AND HYDRAULIC HEAD PRN0030
C -----PRN0040
C SPECIFICATIONS: PRN0050
C REAL *8PHI,Z,XLABEL,YLABEL,TITLE,XN1,MESUR PRN0060
C REAL *4K PRN0070
C PRN0080
C PRN0090
DIMENSION PHI(IO,J0,K0), STRT(IO,J0,K0), S(IO,J0,K0), WELL(IO,J0,K0) PRN0100
10), DELX(J0), DELY(IO), T(IO,J0,K0) PRN0110
C PRN0120
COMMON /INTEGR/ IO,J0,K0,I1,J1,K1,I,J,K,NPER,KTH,ITMAX,LENGTH,KP,NPRN0130
1WEL,NUMT,IFINAL,IT,KT,IHEAD,IDRAW,IFLO,IERR,I2,J2,K2,IMAX,ITMX1,NCPRN0140
2H,IDK1,IDK2,IWATER,IQRE,IP,JP,IQ,JQ,IK,JK,K5,IPU1,IPU2,ITK,IEQN PRN0150
3,NKODE,KFLOW PRN0160*
COMMON /PR/ XLABEL(3),YLABEL(6),TITLE(6),XN1,MESUR,PRNT(122),BLANKPRN0170
1(60),DIGIT(122),VF1(6),VF2(6),VF3(7),XSCALE,DINCH,SYM(17),XN(100),PRN0180
2YN(13),NA(4),N1,N2,N3,YSCALE,FACT1,FACT2 PRN0190
RETURN PRN0200
C ..... PRN0210
C ---INITIALIZE VARIABLES FOR PLOT--- PRN0220
C ***** PRN0230
C ENTRY MAP PRN0240
C ***** PRN0250
C YDIM=0. PRN0260
C WIDTH=0. PRN0270
C DO 10 J=2,J1 PRN0280
10 WIDTH=WIDTH+DELX(J) PRN0290
C DO 20 I=2,I1 PRN0300
20 YDIM=YDIM+DELY(I) PRN0310
30 XSF=DINCH*XSCALE PRN0320
YSF=DINCH*YSCALE PRN0330
NYD=YDIM/YSF PRN0340
IF (NYD*YSF.LE.YDIM-DELY(I1)/2.) NYD=NYD+1 PRN0350
IF (NYD.LE.12) GO TO 40 PRN0360
DINCH=YDIM/(12.*YSCALE) PRN0370
WRITE (6,330) DINCH PRN0380
IF (YSCALE.LT.1.0) WRITE (6,340) PRN0390
GO TO 30 PRN0400
40 MXD=WIDTH/XSF PRN0410
IF (MXD*XSF.LE.WIDTH-DELX(J1)/2.) MXD=MXD+1 PRN0420
N4=MXD*N1+1 PRN0430
N5=MXD+1 PRN0440
N6=NYD+1 PRN0450
N8=N2*NYD+1 PRN0460
NA(1)=N4/2-1 PRN0470
NA(2)=N4/2 PRN0480
NA(3)=N4/2+3 PRN0490
NC=(N3-N8-10)/2 PRN0500
ND=NC+N8 PRN0510
NE=MAX0(N5,N6) PRN0520
VF1(3)=DIGIT(ND) PRN0530
VF2(3)=DIGIT(ND) PRN0540
VF3(3)=DIGIT(ND) PRN0550
XLABEL(3)=MESUR PRN0560
PRN0570

```

| | | |
|-----|--|---------|
| | YLABEL(6)=MESUR | PRN0580 |
| | DO 60 I=1,NE | PRN0590 |
| | NNX=N5-I | PRN0600 |
| | NMY=I-1 | PRN0610 |
| | IF (NMY.GE.N6) GO TO 50 | PRN0620 |
| | YN(I)=YSF*NMY/YSCALE | PRN0630 |
| 50 | IF (NNX.LT.0) GO TO 60 | PRN0640 |
| | XN(I)=XSF*NNX/YSCALE | PRN0650 |
| 60 | CONTINUE | PRN0660 |
| | RETURN | PRN0670 |
| C | | PRN0680 |
| C | ***** | PRN0690 |
| C | ENTRY PRNTA(NG,LA) | PRN0700 |
| C | ***** | PRN0710 |
| C | ---VARIABLES INITIALIZED EACH TIME A PLOT IS REQUESTED--- | PRN0720 |
| | DIST=WIDTH-DELX(J1)/2. | PRN0730 |
| | JJ=J1 | PRN0740 |
| | LL=1 | PRN0750 |
| | Z=NXD*XSF | PRN0760 |
| | IF (NG.EQ.1) WRITE (6,300) (TITLE(I),I=1,3),LA | PRN0770 |
| | IF (NG.EQ.2) WRITE (6,300) (TITLE(I),I=4,6),LA | PRN0780 |
| | DO 290 I=1,N4 | PRN0790 |
| | | PRN0800 |
| C | | PRN0810 |
| C | ---LOCATE X AXES--- | PRN0820 |
| | IF (I.EQ.1.OR.I.EQ.N4) GO TO 70 | PRN0830 |
| | PRNT(1)=SYM(12) | PRN0840 |
| | PRNT(N8)=SYM(12) | PRN0850 |
| | IF ((I-1)/N1*N1.NE.I-1) GO TO 90 | PRN0860 |
| | PRNT(1)=SYM(14) | PRN0870 |
| | PRNT(N8)=SYM(14) | PRN0880 |
| | GO TO 90 | PRN0890 |
| C | | PRN0900 |
| C | ---LOCATE Y AXES--- | PRN0910 |
| 70 | DO 80 J=1,N8 | PRN0920 |
| | IF ((J-1)/N2*N2.EQ.J-1) PRNT(J)=SYM(14) | PRN0930 |
| 80 | IF ((J-1)/N2*N2.NE.J-1) PRNT(J)=SYM(13) | PRN0940 |
| C | | PRN0950 |
| C | ---COMPUTE LOCATION OF NODES AND DETERMINE APPROPRIATE SYMBOL--- | PRN0960 |
| 90 | IF (DIST.LT.0..OR.DIST.LT.Z-XN1*XSF) GO TO 240 | PRN0970 |
| | YLEN=DELY(2)/2. | PRN0980 |
| | DO 220 L=2,I1 | PRN0990 |
| | J=YLEN*N2/YSF+1.5 | PRN1000 |
| | IF (T(L,JJ,LA).EQ.0.) GO TO 160 | PRN1010 |
| | IF (S(L,JJ,LA).LT.0.) GO TO 210 | PRN1020 |
| | INDX3=0 | PRN1030 |
| | GO TO (100,110), NG | PRN1040 |
| 100 | K=(STRT(L,JJ,LA)-PHI(L,JJ,LA))*FACT1 | PRN1050 |
| C | -TO CYCLE SYMBOLS FOR DRAWDOWN, REMOVE C FROM COL. 1 OF NEXT CARD- | PRN1060 |
| C | K=AMOD(K,10.) | PRN1070 |
| | GO TO 120 | PRN1080 |
| 110 | K=PHI(L,JJ,LA)*FACT2 | PRN1090 |
| 120 | IF (K) 130,160,140 | PRN1100 |
| 130 | IF (J-2.GT.0) PRNT(J-2)=SYM(13) | PRN1110 |
| | N=-K+.5 | PRN1120 |
| | IF (N.LT.100) GO TO 150 | PRN1130 |
| | GO TO 190 | PRN1140 |

| | | |
|-----|--|---------|
| 140 | N=K+.5 | PRN1150 |
| | IF (N.LT.100) GO TO 150 | PRN1160 |
| | IF (N.GT.999) GO TO 190 | PRN1170 |
| | INDX3=N/100 | PRN1180 |
| | IF (J-2.GT.0) PRNT(J-2)=SYM(INDX3) | PRN1190 |
| | N=N-INDX3*100 | PRN1200 |
| 150 | INDX1=MOD(N,10) | PRN1210 |
| | IF (INDX1.EQ.0) INDX1=10 | PRN1220 |
| C | -TO CYCLE SYMBOLS FOR DRAWDOWN, REMOVE C FROM COL. 1 OF NEXT CARD- | PRN1230 |
| C | IF (NG.EQ.1) GO TO 170 | PRN1240 |
| | INDX2=N/10 | PRN1250 |
| | IF (INDX2.GT.0) GO TO 180 | PRN1260 |
| | INDX2=10 | PRN1270 |
| | IF (INDX3.EQ.0) INDX2=15 | PRN1280 |
| | GO TO 180 | PRN1290 |
| 160 | INDX1=15 | PRN1300 |
| 170 | INDX2=15 | PRN1310 |
| 180 | IF (J-1.GT.0) PRNT(J-1)=SYM(INDX2) | PRN1320 |
| | PRNT(J)=SYM(INDX1) | PRN1330 |
| | GO TO 220 | PRN1340 |
| 190 | DO 200 II=1,3 | PRN1350 |
| | JI=J-3+II | PRN1360 |
| 200 | IF (JI.GT.0) PRNT(JI)=SYM(11) | PRN1370 |
| 210 | IF (S(L,JJ,LA).LT.0.) PRNT(J)=SYM(16) | PRN1380 |
| 220 | YLEN=YLEN+(DELY(L)+DELY(L+1))/2. | PRN1390 |
| 230 | DIST=DIST-(DELX(JJ)+DELX(JJ-1))/2. | PRN1400 |
| | JJ=JJ-1 | PRN1410 |
| | IF (JJ.EQ.0) GO TO 240 | PRN1420 |
| | IF (DIST.GT.Z-XN1*XSF) GO TO 230 | PRN1430 |
| 240 | CONTINUE | PRN1440 |
| C | | PRN1450 |
| C | ---PRINT AXES,LABELS, AND SYMBOLS--- | PRN1460 |
| | IF (I-NA(LL).EQ.0) GO TO 260 | PRN1470 |
| | IF ((I-1)/N1*N1-(I-1)) 270,250,270 | PRN1480 |
| 250 | WRITE (6,VF1) (BLANK(J),J=1,NC),(PRNT(J),J=1,N8),XN(1+(I-1)/6) | PRN1490 |
| | GO TO 280 | PRN1500 |
| 260 | WRITE (6,VF2) (BLANK(J),J=1,NC),(PRNT(J),J=1,N8),XLABEL(LL) | PRN1510 |
| | LL=LL+1 | PRN1520 |
| | GO TO 280 | PRN1530 |
| 270 | WRITE (6,VF2) (BLANK(J),J=1,NC),(PRNT(J),J=1,N8) | PRN1540 |
| C | | PRN1550 |
| C | ---COMPUTE NEW VALUE FOR Z AND INITIALIZE PRNT--- | PRN1560 |
| 280 | Z=Z-2.*XN1*XSF | PRN1570 |
| | DO 290 J=1,N8 | PRN1580 |
| 290 | PRNT(J)=SYM(15) | PRN1590 |
| C | | PRN1600 |
| C | ---NUMBER AND LABEL Y AXIS AND PRINT LEGEND--- | PRN1610 |
| | WRITE (6,VF3) (BLANK(J),J=1,NC),(YN(I),I=1,N6) | PRN1620 |
| | WRITE (6,320) (YLABEL(I),I=1,6) | PRN1630 |
| | IF (NG.EQ.1) WRITE (6,310) FACT1 | PRN1640 |
| | IF (NG.EQ.2) WRITE (6,310) FACT2 | PRN1650 |
| | RETURN | PRN1660 |
| C | | PRN1670 |
| C | ---FORMATS--- | PRN1680 |
| C | | PRN1690 |
| C | ----- | PRN1700 |
| C | | PRN1710 |

```

C
300 FORMAT ('1',49X,3A8,'LAYER',I4//) PRN1720
310 FORMAT ('0EXPLANATION'/' ',11('-')//' R = CONSTANT HEAD BOUNDARY'/PRN1730
1' *** = VALUE EXCEEDED 3 FIGURES'/' MULTIPLICATION FACTOR =',F8.3)PRN1740
320 FORMAT ('0',39X,6A8) PRN1750
330 FORMAT ('0',25X,10('*'),' TO FIT MAP WITHIN 12 INCHES, DINCH REVISPRN1760
1ED TO',G15.7,1X,10('*')) PRN1770
340 FORMAT ('0',45X,'NOTE: GENERALLY SCALE SHOULD BE > OR = 1.0') PRN1780
END PRN1790
PRN1800

```

```

BLOCK DATA
-----
SPECIFICATIONS:
REAL *8XLABEL, YLABEL, TITLE, XN1, MESUR
COMMON /SARRAY/ ICHK(13), LEVEL1(9), LEVEL2(9)
COMMON /PR/ XLABEL(3), YLABEL(6), TITLE(6), XN1, MESUR, PRNT(122), BLANK
1(60), DIGIT(122), VF1(6), VF2(6), VF3(7), XSCALE, DINCH, SYM(17), XN(100),
2YN(13), NA(4), N1, N2, N3, YSCALE, FACT1, FACT2
*****
DATA ICHK/'DRAW', 'HEAD', 'MASS', 'DK1', 'DK2', 'WATE', 'RECH', 'PUN1', 'PBLK
1UN2', 'ITKR', 'EQN3', 2*0/
DATA SYM/'1', '2', '3', '4', '5', '6', '7', '8', '9', '0', '*', '|', '-', '+',
1 ' ', 'R', 'W'/
DATA PRNT/122*' ', N1, N2, N3, XN1/6, 10, 133, .8333333333D-1/, BLANK/60*'
1 ' ', NA(4)/1000/
DATA XLABEL/' X DIS- ', 'TANCE IN', ' MILES ', YLABEL/'DISTANCE', '
1FROM OR', 'IGIN IN ', 'Y DIRECT', 'ION, IN ', 'MILES ', TITLE/'PLOT
2OF ', 'DRAWDOWN', ' ', 'PLOT OF ', 'HYDRAULI', 'C HEAD'/
DATA DIGIT/'1', '2', '3', '4', '5', '6', '7', '8', '9', '10', '11', '12', '13'
1, '14', '15', '16', '17', '18', '19', '20', '21', '22', '23', '24', '25', '26',
2'27', '28', '29', '30', '31', '32', '33', '34', '35', '36', '37', '38', '39',
340', '41', '42', '43', '44', '45', '46', '47', '48', '49', '50', '51', '52', '5
43', '54', '55', '56', '57', '58', '59', '60', '61', '62', '63', '64', '65', '6
5', '67', '68', '69', '70', '71', '72', '73', '74', '75', '76', '77', '78', '
6', '80', '81', '82', '83', '84', '85', '86', '87', '88', '89', '90', '91', '9
7, '93', '94', '95', '96', '97', '98', '99', '100', '101', '102', '103', '
8, '105', '106', '107', '108', '109', '110', '111', '112', '113', '114', '1
9, '116', '117', '118', '119', '120', '121', '122'/
DATA VF1/'(1H ', ' ', ' ', ' ', ' ', 'A1, F', '10.2', ')'/
DATA VF2/'(1H ', ' ', ' ', ' ', ' ', 'A1, 1', 'X, A8', ')'/
DATA VF3/'(1H0', ' ', ' ', ' ', ' ', 'A1, F', '3.1', ' ', '12F1', '0.2)'/
*****
END

```

```

BLK0010
BLK0020
BLK0030
BLK0040
BLK0050
BLK0060
BLK0070
BLK0080
BLK0090
BLK0100
BLK0110
BLK0120
BLK0130
BLK0140
BLK0150
BLK0160
BLK0170
BLK0180
BLK0190
BLK0200
BLK0210
BLK0220
BLK0230
BLK0240
BLK0250
BLK0260
BLK0270
BLK0280
BLK0290
BLK0300
BLK0310
BLK0320
BLK0330
BLK0340
BLK0350
BLK0360

```

FACTORS FOR CONVERTING INCH-POUND UNITS
TO INTERNATIONAL SYSTEM OF UNITS (SI)

The following factors can be used to convert inch-pound units to International System of Units (SI).

| Multiply inch-pound units | By | To obtain SI Units |
|--|--------------------------|--|
| foot (ft) | 0.3048 | meter (m) |
| foot per day (ft/d) | 0.3048 | meter per day (m/d) |
| foot per day per foot [(ft/d)/ft] | 0.3048 | meter per day per meter [(m/d)/m] |
| square foot per day (ft ² /d) | 9.290 x 10 ⁻² | square meter per day (m ² /d) |
| cubic foot per second (ft ³ /s) | 2.832 x 10 ⁻² | cubic meter per second (m ³ /s) |
| inch (in) | 2.54 | centimeter (cm) |
| inch per year (in/year) | 2.54 | centimeter per year (cm/year) |
| mile (mi) | 1.609 | kilometer (km) |
| gallon per minute (gal/min) | 6.310 x 10 ⁻² | liter per second (L/s) |
| ohm-foot | 0.3048 | ohm-meter |

National Geodetic Vertical Datum of 1929 (NGVD of 1929): A geodetic datum derived from a general adjustment of the first-order level nets of both the United States and Canada, formerly called "Mean Sea Level." NGVD of 1929 is referred to as sea level in this report.