# Effects of Simulated Ground-Water Pumping and Recharge on Ground-Water Flow in Cape Cod, Martha's Vineyard, and Nantucket Island Basins, Massachusetts

United States Geological Survey Water-Supply Paper 2447

Prepared in cooperation with the Massachusetts Department of Environmental Management, Office of Water Resources

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# Effects of Simulated Ground-Water Pumping and Recharge on Ground-Water Flow in Cape Cod, Martha's Vineyard, and Nantucket Island Basins, Massachusetts

By JOHN P. MASTERSON and PAUL M. BARLOW

Prepared in cooperation with the
Massachusetts Department of Environmental Management,
Office of Water Resources

U.S. GEOLOGICAL SURVEY WATER-SUPPLY PAPER 2447

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**Library of Congress Cataloging in Publication Data**

Masterson, John P
Effects of simulated ground-water pumping and recharge on ground-water flow in Cape Cod, Martha's Vineyard, and Nantucket Island Basins, Massachusetts / by John P Masterson and Paul M Barlow
p cm —(U S Geological Survey Water-Supply Paper , 2447)
"Prepared in cooperation with the Massachusetts Department of Environmental Management, Office of Water Resources "
Includes bibliographical references
Supt of Docs no I 19 13 2447
1 Groundwater flow--Massachusetts--Cape Cod--Measurement
2 Groundwater flow--Massachusetts--Martha's Vineyard--Measurement
3 Groundwater flow--Massachusetts--Nantucket Island--Measurement
I Barlow, Paul M II Massachusetts Office of Water Resources
III Title V Series
GB1197 7 M38 1996 95-9823
553 7'9'097449--dc20 CIP
ISBN 0-607-866330

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## CONVERSION FACTORS AND VERTICAL DATUM

### CONVERSION FACTORS

| Multiply | By | To obtain |
|---|---|---|
| acre | 4,047 | square meter |
| cubic foot per second ($ft^3/s$) | 0 02832 | cubic meter per second |
| foot | 0 3048 | meter |
| foot per day (ft/d) | 0 3048 | meter per day |
| foot per day per foot [(ft/d)/ft] | 0 3048 | meter per day per meter |
| foot squared per day ($ft^2/d$) | 0 09290 | meter squared per day |
| gallon per minute (gal/min) | 0 06309 | liter per second |
| inch (in ) | 25 4 | millimeter |
| | 2 54 | centimeter |
| inch per year (in/yr) | 25 4 | millimeter per year |
| mile (mi) | 1 609 | kilometer |
| million gallons per day (Mgal/d) | 0 04381 | cubic meter per second |
| per foot ($ft^{-1}$) | 0 3048 | per meter |
| square mile ($mi^2$) | 2 590 | square kilometer |

In this report, the unit of hydraulic conductivity is foot per day (ft/d), the mathematically reduced form of cubic foot per day per square foot [$(ft^3/d)/ft^2$] The unit of transmissivity is square foot per day ($ft^2/d$), the mathematically reduced form of cubic foot per day per square foot times foot of aquifer thickness [$(ft^3/d)/ft^2 \times ft$]

Chemical concentration is given in milligrams per liter Milligrams per liter is a unit expressing the solute per unit volume (liter) of water One milligram per liter is equivalent to 1,000 micrograms per liter Density is given in grams per cubic centimeter ($g/cm^3$)

### VERTICAL DATUM

**Sea Level:** In this report, "sea level" refers to the National Geodetic Vertical Datum of 1929 (NGVD of 1929)—a geodetic datum derived from a general adjustment of the first-order level nets of the United States and Canada, formerly called Sea Level Datum of 1929

# Effects of Simulated Ground-Water Pumping and Recharge on Ground-Water Flow in Cape Cod, Martha's Vineyard, and Nantucket Island Basins, Massachusetts

*By* John P Masterson *and* Paul M. Barlow

## Abstract

The management and protection of water resources of Cape Cod, Martha's Vineyard, and Nantucket Island water-resource planning basins are of concern to Massachusetts State and local officials because ground water is the sole source of drinking water in the basins. Significant growth in the number of summer and permanent residents has increased ground-water use during the last 30 years and placed stresses on the ground-water resources In particular, there is concern over the extent of long-term declines in ground-water and pond levels and in the quantity of streamflow, as well as in the possibility of saltwater intrusion from the surrounding ocean The effects of simulated ground-water pumping and recharge on the surface- and ground-water hydrology were assessed for the Cape Cod, Martha's Vineyard, and Nantucket Island Basins Five of the six flow cells of the Cape Cod Basin were assessed—the West Cape, East Cape, Eastham, Wellfleet, and Truro flow cells These effects are reported as (1) changes in water-table altitudes in the three basins, (2) changes in pond altitudes and streamflow for selected ponds and streams of the Cape Cod Basin, (3) changes in the sources and sinks of water in the Cape Cod Basin, and (4) changes in the position of the freshwater-saltwater interface in the West Cape, East Cape, and Truro flow cells of the Cape Cod Basin

Transient, three-dimensional, finite-difference models were developed to simulate freshwater and saltwater flow in the West Cape, East Cape, and Truro flow cells Model results indicate little change in the position of the freshwater-saltwater interface in the West Cape and East Cape flow cells for ground-water pumping and recharge conditions similar to those that occurred in the basin from predevelopment (assumed to have ended in 1950) to 1989 and for those estimated to occur from 1989 to 2020 Increases in pumping in the Truro flow cell also had a negligible effect on the position of the freshwater-saltwater interface except near the three areas of ground-water pumping in the flow cell

Transient, three-dimensional, finite-difference models also were developed to simulate freshwater flow in the West Cape, East Cape, Wellfleet, and Eastham flow cells for the period from predevelopment to the year 2020 Total average declines in the water table at 32 observation wells in the West Cape flow cell and 19 observation wells in the East Cape flow cell are 1 8 and 2 9 feet, respectively, for the simulation period Water-table altitudes range from 0 to nearly 75 feet in the West Cape flow cell and from 0 to nearly 45 feet in the East Cape flow cell Declines in the average water levels of ponds during the simulation period are less than those at observation wells because of the greater storage capacity of the ponds than of the surrounding aquifer material The average depletion in the rate of streamflow at the gaging points of eight of the largest streams in the West Cape and East Cape flow cells simulated in the models in the year 2020 was 14 percent of the model-calculated predevelopment streamflow in the rivers

Total sources and sinks of freshwater to the West Cape and East Cape flow cells increase from predevelopment flow conditions to the year 2020 because the total amount of ground-water pumping

and subsequent wastewater-return flow to each system increases with time The source of ground-water pumpage from the flow cells is freshwater removed from storage in the aquifers and decreased rates of freshwater discharge to streams and saltwater boundaries of the flow cells Sources and sinks of water to the Eastham and Wellfleet flow cells have not changed and are not projected to change significantly with time because there is no large-scale pumping in the flow cells for public supply

Declines in the altitude of the water table were calculated for Martha's Vineyard and Nantucket Island Basins for conditions of 180 days of no recharge for projected 2020 in-season ground-water pumping rates Drawdowns were largest at the pumping centers The largest decline for the Martha's Vineyard Basin was 2.5 feet near the proposed Manter site in Tisbury The largest decline for the Nantucket Island Basin was 1 9 feet near the Wannacomet well field

## INTRODUCTION

The peninsula of Cape Cod and the islands of Martha's Vineyard and Nantucket are in the southeasternmost part of Massachusetts (fig 1) Cape Cod covers an area of 440 $mi^2$ that extends into the Atlantic Ocean and is separated from the mainland by a sea-level canal Martha's Vineyard is about 5 mi south of the southwestern part of Cape Cod, and Nantucket Island is about 15 mi south of the central part of Cape Cod Martha's Vineyard covers 95 $mi^2$ and Nantucket Island covers 46 $mi^2$

Ground water is the principal source of freshwater for domestic, industrial, and agricultural use on Cape Cod, Martha's Vineyard, and Nantucket Island Basins In addition, the discharge of water from the aquifer supports freshwater ponds and streams that are important habitats for rare plants and fish spawning and maturation Water is withdrawn from shallow sand and gravel aquifers that are susceptible to saltwater intrusion and to contamination from anthropogenic sources

Six ground-water flow cells were delineated for Cape Cod (fig 2), each of which is hydraulically distinct under natural hydrologic conditions (LeBlanc and others, 1986) These six flow cells are the West Cape, East Cape, Eastham, Wellfleet, Truro, and Provincetown flow cells Martha's Vineyard and Nantucket Island each consist of a single, shallow principal flow system

Under Massachusetts Law 313 CMR 2 00, the Massachusetts Department of Environmental Management, Office of Water Resources (MOWR), is responsible for developing management plans for the efficient and environmentally sound use of water for the State's 27 water-resource planning basins These basins are designated by MOWR and typically coincide with existing river basins The Cape Cod, Martha's Vineyard, and Nantucket Island Basins are unique in that they are distinct hydrologic systems bounded by saltwater rather than topographic divides Therefore, the Cape Cod, Martha's Vineyard, and Nantucket Island Basins are of particular concern because ground water is the sole source of drinking water to residents in those basins and because significant growth in the number of summer and permanent residents has resulted in the increased use of water during the last 30 years Federal, State, and local officials responsible for managing and protecting water resources are concerned that with increased ground-water pumping, water-table and pond altitudes will decline, ground-water discharge to streams will decrease, and saltwater intrusion will occur In response to this concern, the U S Geological Survey (USGS), in cooperation with MOWR, began an investigation in 1989 to assess the effects of changing ground-water pumping and recharge to the surface- and ground-water hydrology of Cape Cod, Martha's Vineyard, and Nantucket Island Basins

### Purpose and Scope

The purpose of this report is to assess the effects of simulated ground-water pumping and recharge on ground-water flow of the Cape Cod, Martha's Vineyard, and Nantucket Island Basins These effects are reported as (1) changes in the water-table altitudes in the three basins, (2) changes in pond altitudes and streamflow for selected ponds and streams, of Cape Cod Basin, (3) changes in sources and sinks of water in Cape Cod Basin, and (4) changes in the position of the interface between freshwater and saltwater in the West Cape, East Cape, and Truro flow cells of Cape Cod Basin

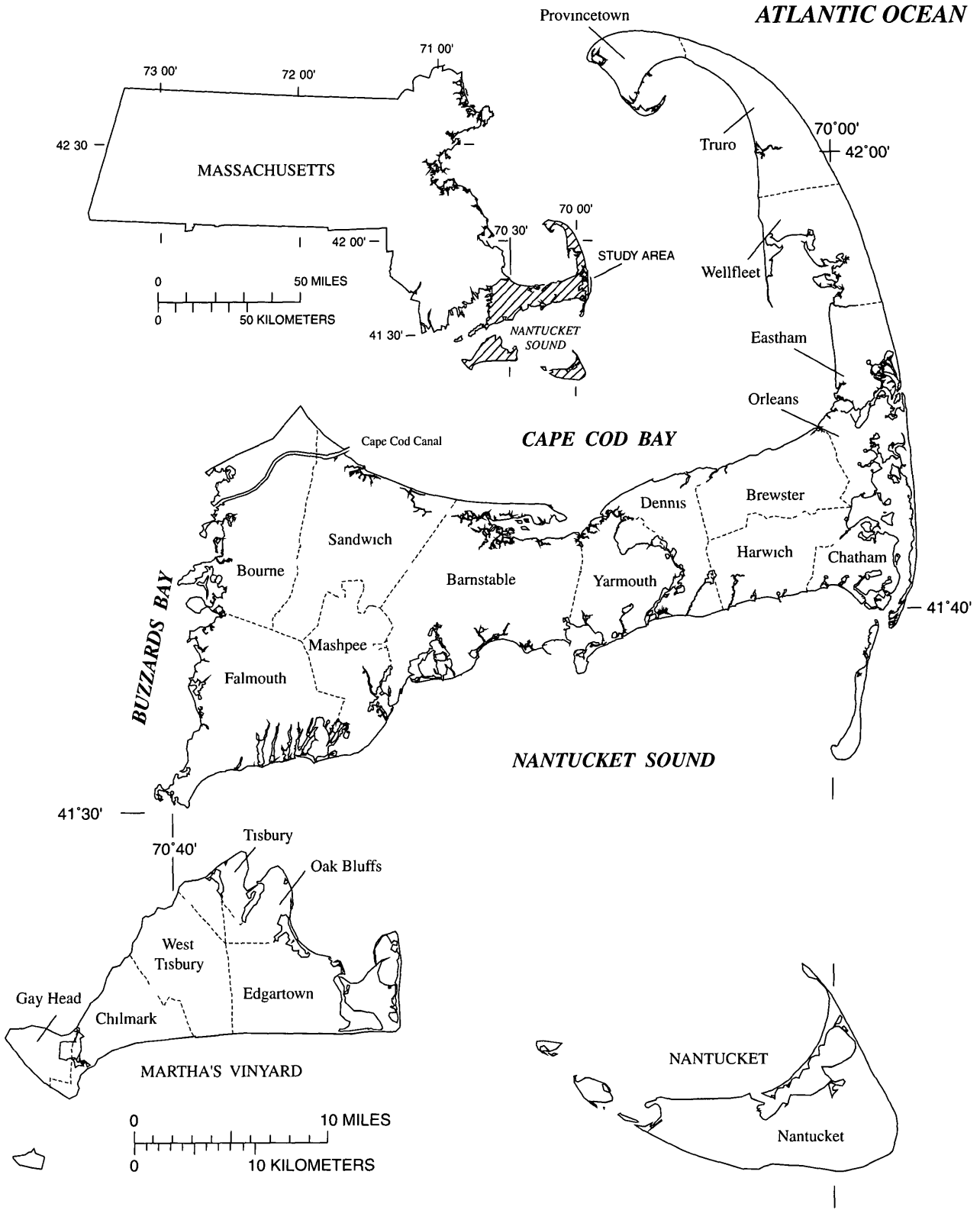


**Figure 1.** Location of Cape Cod, Martha's Vineyard, and Nantucket Island Basins, Massachusetts

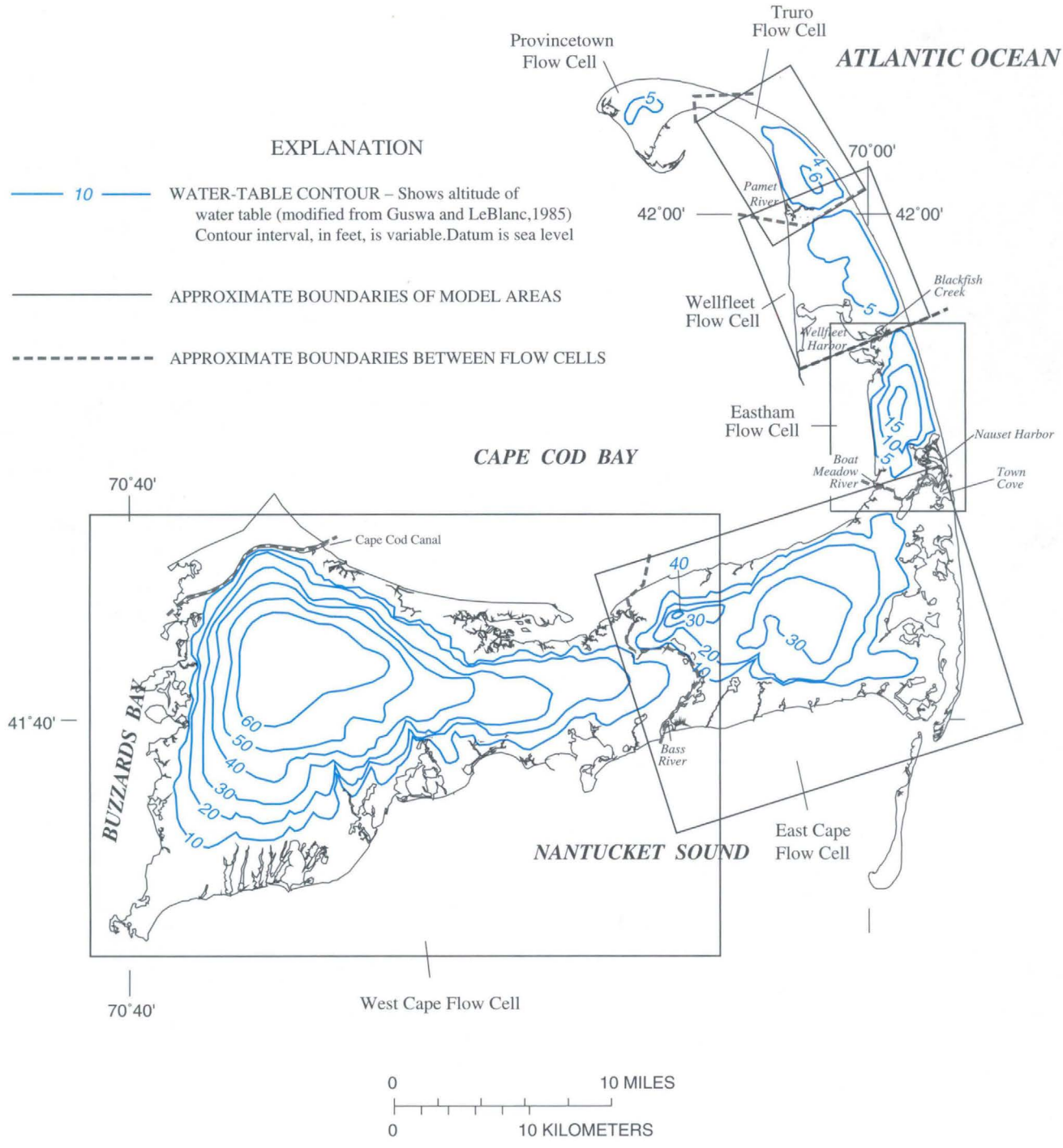


**Figure 2.** Location of flow cells, model boundaries, and water-table configuration on May 25–27, 1976, Cape Cod Basin, Massachusetts.

This report describes the hydrogeology of the Cape Cod Basin and the development and application of numerical ground-water-flow models for five flow cells of Cape Cod the West Cape, East Cape, Eastham, Wellfleet, and Truro flow cells Because water in the Provincetown flow cell is of poor quality, it is not used for public-water supplies, and no analysis of this flow cell is provided Ground-water-flow models that simulate freshwater and saltwater flow were developed for the West Cape, East Cape, and Truro flow cells of Cape Cod Flow models that simulate only freshwater flow were developed for the West Cape, East Cape, Eastham, and Wellfleet flow cells Each of these seven models is a three-dimensional, transient, finite-difference, ground-water-flow model The models developed for the West Cape, East Cape, and Truro flow cells simulate ground-water pumping and recharge conditions similar to those that existed during the period before large-scale development up to those conditions that existed in 1989 The models also simulate conditions that are projected to occur through the year 2020 The models developed for the Eastham and Wellfleet flow cells simulate only natural hydrologic conditions because there are currently no large-capacity wells in these two flow cells

This report also describes the hydrology of Martha's Vineyard and Nantucket Island Basins and the development of two-dimensional, finite-difference numerical flow models that simulate changes in the altitude of the water table Application of these models was used to estimate declines in the altitude of the water table near public-supply wells on Martha's Vineyard and Nantucket Island for ground-water pumping equal to that which is projected for the year 2020

Numerical flow models developed during this investigation provide information regarding regional-scale behavior of the ground-water-flow cells, including regional movement of the interface separating the freshwater- and saltwater-flow systems Although detailed analyses of local hydrologic conditions were beyond the scope of the investigation, the flow models may serve as starting points for more detailed modeling investigations of smaller areas of the flow cells

## Approach

Effects of simulated ground-water pumping and recharge to each of the three ground-water basins were determined using available hydrogeologic data and ground-water-flow models for each basin, as well as current and projected future ground-water-use data in each basin Because the annual pumping of ground water on Cape Cod is much greater than that on Martha's Vineyard and Nantucket Island, the need for assessing the effects of changing stress conditions is greater in the Cape Cod flow system than in the Martha's Vineyard or Nantucket Island flow systems The effects of changing stress conditions in the Cape Cod flow cells were assessed by use of three-dimensional, transient, ground-water-flow models The effects of changing stress conditions in the Martha's Vineyard and Nantucket Island flow systems were calculated by use of simple, two-dimensional, finite-difference models, called change models, in which declines in the altitude of the water table were determined for simulated pumping conditions that are projected to occur during summer months (June, July, and August) in the year 2020 Two-dimensional change models were used to evaluate these two flow systems because not enough hydrogeologic information was available for the island basins to develop calibrated, three-dimensional, ground-water-flow models

Of the five flow cells of Cape Cod, the West Cape and East Cape flow cells have had the greatest amount of domestic, industrial, and agricultural development within them and, consequently, the largest amount of ground-water use Two flow models were developed for each of these two flow cells to assess the effects of changing stress conditions The first model developed for each flow cell was a transient, three-dimensional, finite-difference model that simulates freshwater and saltwater flow separated by a sharp interface (referred to in the remainder of the report as the SHARP models) (Essaid, 1990) The SHARP models were used to assess the response of the boundary between the freshwater- and saltwater-flow systems to changing stress conditions The

SHARP models provide estimates of the location of the interface between freshwater and saltwater and of the rate of discharge of freshwater to zones of the aquifer that contain saltwater Because the SHARP models indicated little movement of the interface between freshwater and saltwater for the stress conditions simulated, it was assumed subsequently that a static interface exists between the freshwater and saltwater zones of the aquifer The calculated location of the freshwater-saltwater interface and the rate of discharge to zones of the aquifer that contain saltwater were then used as lateral boundary conditions for freshwater-flow models of the West Cape and East Cape flow cells The freshwater-flow models are transient, three-dimensional, finite-difference models that use the USGS modular ground-water-flow computer model (McDonald and Harbaugh, 1988), they are referred to in the remainder of the report as the MODFLOW models Freshwater-flow models were developed for the West Cape and East Cape flow cells because they require less computational effort than the SHARP models and may be more easily applied to the analysis of the effects of ground-water pumping on the flow systems than the computationally intensive SHARP models

The Truro flow cell is the smallest of the five flow cells of Cape Cod that are used for water supply and is the only source of water for the communities of Truro and Provincetown Although the sand and gravel aquifer that constitutes the Truro flow cell extends to bedrock, the freshwater-flow system is bounded at depth by the transition zone between freshwater and underlying saltwater (Guswa and LeBlanc, 1985, LeBlanc and others, 1986) Contamination of public-water supplies by saltwater intrusion caused by ground-water pumping has been recorded at the Knowles Crossing well field, located 1,500 ft from Cape Cod Bay (LeBlanc and others, 1986) Evidence of saltwater intrusion precludes the assumption of a static interface between freshwater and saltwater and the use of MODFLOW for the analysis of changing stress conditions in the Truro flow cell Consequently, changing stress conditions in the flow cell were evaluated by means of the SHARP model alone

The Eastham and Wellfleet flow cells currently (1996) do not have any large-capacity wells, and less data are available on the hydrogeology of these flow cells than for the West Cape, East Cape, or Truro flow cells Consequently, a simpler modeling approach was used for the analysis of these two flow cells than was used for the West Cape, East Cape, or Truro flow cells Freshwater (MODFLOW) models were developed to simulate ground-water flow in each of these flow cells These models simulate predevelopment flow conditions in the flow cells—conditions that are assumed to exist at this time (1996) Users of these models are urged to review the section "Limitations of the Numerical Modeling Analyses for the Cape Cod Basin" prior to applying the models to any assessment of the flow cells

## Previous Investigations

The basinwide hydrogeology of Cape Cod has been described by Strahler (1972), Ryan (1980), and LeBlanc and others (1986) A detailed summary of the geology of Cape Cod is provided by Oldale and Barlow (1986) The hydrogeology of Martha's Vineyard and Nantucket Island has been described by Delaney (1980) and Walker (1980), respectively More recently, the hydrogeology of Nantucket Island has been described by Horsley Witten Hegemann, Inc (1990)

In addition to these basinwide studies, recent hydrogeologic investigations of the West Cape flow system have been reported by LeBlanc (1984a and 1984b), Barlow (1994), and Barlow and Hess (1993), of the East Cape flow cell by Johnson (1990), of the Eastham flow cell by Barlow (1994), and of the Truro flow cell by LeBlanc (1982) and Cape Cod Planning and Economic Development Commission (1989)

Numerical models of ground-water flow were developed previously by Guswa and LeBlanc (1985) for the West Cape, East Cape, Eastham, Wellfleet, and Truro flow cells and by Barlow (1994) for the Eastham and part of the West Cape flow cells The flow models developed by Guswa and LeBlanc (1985) used a modified version of the three-dimensional flow model of Trescott (1975) that solved for the steady-state position of the interface between freshwater and saltwater The models of Guswa and LeBlanc (1985) were not used in this investigation because they cannot simulate (1) transient flow conditions, (2) ground-water flow in the saltwater zones of the aquifer, or (3) freshwater discharge to saltwater zones of the aquifer such as occurs beneath Cape Cod Bay

## Acknowledgments

The authors thank several individuals who provided data or assisted in the acquisition of data during this investigation William Wilcox, Martha's Vineyard Commission, Tony Maevsky, USGS (retired), Joseph Bergin, Massachusetts Division of Fisheries and Wildlife, James Cook, Provincetown Water Department, and facilities personnel at the Falmouth, Hyannis, Otis, and Chatham wastewater-treatment facilities

# HYDROGEOLOGIC SETTING

## Cape Cod Basin

### Geologic Setting

The Cape Cod Basin consists of glacial drift ranging in size from clay to boulders deposited during the glaciation of the Pleistocene Epoch Ice sheets advancing south from northern New England and Canada transported eroded rock debris scoured from the underlying Paleozoic crystalline bedrock until reaching the southernmost extent at Martha's Vineyard and Nantucket Island

During the late Wisconsinan stage of the Pleistocene glaciation, the coalescing Buzzards Bay, Cape Cod Bay, and South Channel glacial lobes deposited the glacial drift that now constitutes Cape Cod (fig 3) The glacial drift overlies an igneous and metamorphic basement complex The altitude of the top of this complex ranges from 100 ft below sea level near Cape Cod Canal to more than 900 ft below sea level near the town of Provincetown (Oldale, 1969) A veneer of compact basal till was deposited on the bedrock as the ice sheets advanced south

Hydrogeologic sections of glacial drift were made for eight north-south trending transects and one east-west trending transect across the Cape Cod Basin to characterize the vertical and spatial distribution of grain sizes (pl 1) Although lithologic variations over short distances can be extreme, the general trend in grain distribution is an overall decrease in grain size with depth

Overlying the basal till are thick deposits of fine sand, silt, and clay that were deposited in a proglacial lake, located in front of the retreating ice sheets, that was dammed in the south by glacial-drift deposits of Martha's Vineyard and Nantucket Island These fine sand, silt, and clay deposits vary in thickness and are as much as 200 ft thick in the eastern part of Cape Cod (Oldale, 1984)

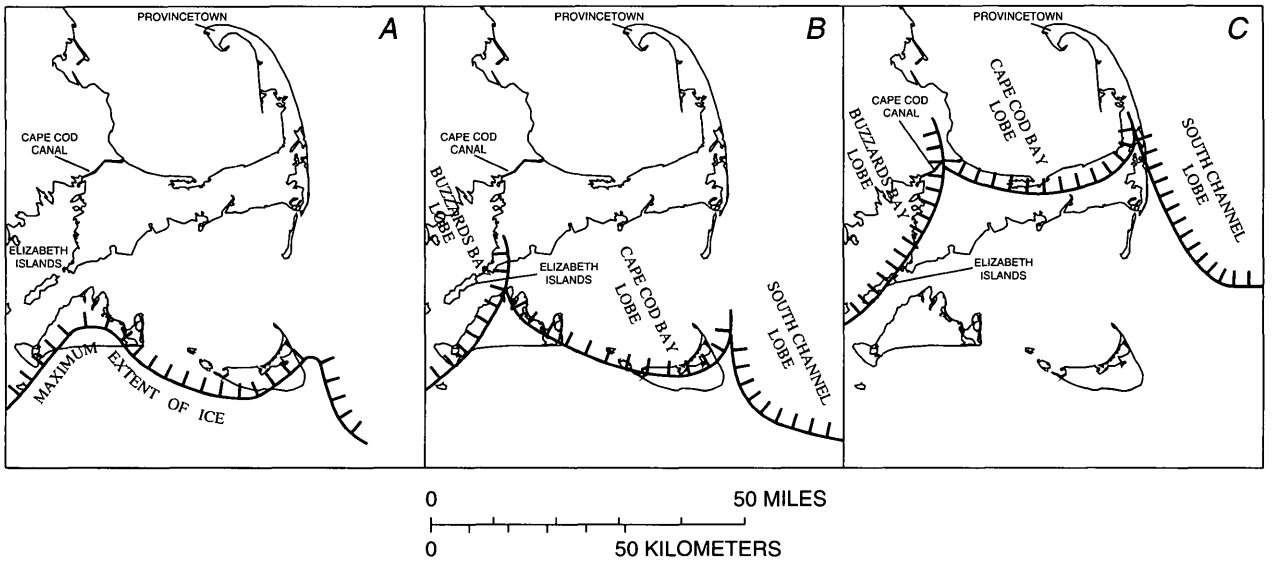


**Figure 3** Ice recession and lobe formation in southeastern Massachusetts (modified from Oldale and Barlow, 1986) *A* Approximately 21,000 to 16,000 years before present *B* Approximately 16,000 years before present *C* 15,000 years before present

As the melting ice sheets receded to what is now the northern and western shores of Cape Cod, isolated blocks of buried ice left by the receding glaciers melted to form depressions in which kettle ponds and marshes are located throughout present-day Cape Cod Stratified drift was extensively deposited on the fine-grained lake-bottom deposits by meltwater streams flowing from the Buzzards Bay and Cape Cod Bay glacial lobes during a period of ice standstill (fig 3*C*) The stratified-drift deposits show a decrease in the percentage of coarse-grained material from the Buzzards Bay and Sandwich moraines south and southeastward to Nantucket Sound

The Buzzards Bay and Sandwich glacial moraines (pl 1), which border the western and northern shores of Cape Cod, were deposited during minor advances of the ice lobes that occurred during this period of standstill The readvancing lobes thrust outwash material on previously deposited outwash plain deposits, forming ridges of reworked outwash material that consist of unsorted sediment ranging from clay to boulders The lobes then advanced over these ice-thrust ridges, depositing a veneer of pulverized rock or glacial till over the reworked outwash deposits (Oldale, 1984)

Between the retreating Cape Cod Bay glacial lobe and the Sandwich moraine, fine sand, silt, and clay were deposited in a proglacial lake occupying what is now Cape Cod Bay These fine-grained glaciolacustrine deposits rim the south shore of Cape Cod Bay from Sandwich to Orleans Localized topographic highs of unsorted sand, gravel, boulders, and lenses of fine silt and clay lie between the moraine and lacustrine deposits (pl 1)

Outer Cape Cod consists of stratified drift from meltwaters flowing westward from the South Channel glacial lobe that was located east of present-day outer Cape Cod (fig 3*C*) This outwash material was deposited as deltas prograded into the glacial lake occupying the area left by the retreating Cape Cod Bay lobe These deposits consist of stratified fine-medium sand, medium-coarse sand, and gravel with lenses of fine silt and clay that overlie the glaciolacustrine deposits consisting of fine sand, silt, and clay

After the retreat of the last ice sheets, sea level rose during the Holocene Epoch, nearly 300 ft to its present-day altitude The rise in sea level flooded the low-lying areas surrounding Cape Cod, including the proglacial lakes of Cape Cod Bay and Nantucket Sound Postglacial wave erosion and redeposition has had a pronounced effect on the shoreline of Outer Cape Cod (Oldale, 1984)

### Hydrologic System

The Cape Cod ground-water-flow system consists of six flow cells that are hydraulically distinct under present hydrologic conditions (Guswa and LeBlanc, 1985) These flow cells are bounded laterally by saltwater bodies that include Cape Cod Bay, Nantucket Sound, and the Atlantic Ocean (fig 1) Freshwater is less dense than the surrounding saltwater and forms lens-shaped bodies that are underlain by saltwater An interface separates the freshwater- and saltwater-flow systems within the unconsolidated deposits except in areas where the freshwater lens in the unconsolidated deposits is truncated by bedrock This interface consists of a zone of mixing between the two flow systems that generally is thin in comparison to the total thickness of each aquifer

Under natural hydrologic conditions, the freshwater- and saltwater-flow systems are assumed to be in hydrodynamic equilibrium—ground-water discharge from the freshwater system is balanced by aquifer recharge from precipitation, which results in a static interface between the two flow systems Decreases in aquifer recharge and (or) increases in ground-water pumping may result in a decrease in the rate of coastal freshwater discharge and a consequent landward movement of the interface position

The source of freshwater to the Cape Cod Basin is precipitation, which ranges from about 40 to 47 in/yr An estimated 45 to 48 percent of this amount (or about 18 to 22 in/yr) recharges the ground-water-flow system (LeBlanc and others, 1986), and precipitation that is not recharged to the aquifer evaporates and is transpired by plants Surface-water runoff is assumed to be negligible because of the highly permeable soils of Cape Cod

The water-table configuration in the Cape Cod Basin is characterized by six oblong mounds, one in each flow cell (fig 2) Water-table contours approximately follow the shape of the coast Ground water flows radially from the centers of the mounds toward coastal discharge areas The altitude of the water table fluctuates by as much as 7 ft because of seasonal changes in aquifer recharge and ground-water pumping (Letty, 1984) Annual water-table altitudes are highest during early spring when

recharge is high and ground-water pumping is low Water-table altitudes are lowest during late summer when ground-water recharge is low and water use is highest Water-table fluctuations at observation wells vary in magnitude across Cape Cod and are dependent on the proximity of the observation wells to the coast and to nearby pumping wells Annual water-table fluctuations are lowest near the coast where a nearly constant sea level keeps ground-water levels from fluctuating substantially Water-table fluctuations are highest near the center of each flow cell and near areas of pumping

Most lakes and streams of Cape Cod are hydraulically connected to the ground-water-flow system, and lake levels and streamflow fluctuate in consonance with ground-water levels In some areas, however, such as within the moraine deposits, discontinuous lenses of fine-grained material above the regional water table have caused perched ponds Water levels in these perched ponds do not reflect the regional water table

Many streams on Cape Cod are fed by ground-water discharge Most of these streams are ungaged, and the total quantity of ground water that discharges from the flow cells to the streams is unknown Continuous-measurement, USGS streamflow-gaging stations have been in operation on only two streams on Cape Cod These have been on the Herring River in Harwich (1967–88) and the Quashnet River in Falmouth (1989–present) Daily mean streamflow was 10 0 $ft^3/s$ on the Herring River during 1967–88 and 13 5 $ft^3/s$ on the Quashnet River during 1989–93 Streamflow is typically above average in the spring and below average during the summer and autumn Streamflow on Cape Cod remains relatively constant during the year in comparison to streamflow of other areas of New England because of the large infiltration and storage capacity of the glacial outwash deposits that cover the basin and tend to reduce the effect of climatic variability on streamflow (Barlow and Hess, 1993)

Streamflow measurements have been made only sporadically or intermittently on a few other streams in the West Cape and East Cape flow cells In the West Cape flow cell, measurements were made within 2,100 ft of the mouth of the Coonamessett, Backus, Bourne, and Childs Rivers on August 9, 1989, when ground-water levels and streamflow of the Quashnet River were near average conditions (Barlow and Hess, 1993) Streamflow was 8 2, 2 3, 1 5, and 6 0 $ft^3/s$ for the Coonamessett, Backus, Bourne, and Childs Rivers, respectively, on this date (Socolow and others, 1991, p 178–179) Streamflow on these rivers is highly dependent on cranberry-bog activities, however, and the reported streamflow may be unrepresentative of average streamflow conditions Only one streamflow measurement is recorded for the Mashpee River This measurement was made 1,250 ft upstream from the mouth of the river on August 4, 1978, and was 15 5 $ft^3/s$ (Socolow and others, 1991, p 180) Streamflow on the Quashnet River on the same date was 19 3 $ft^3/s$ Because the streamflow in the Quashnet River was much higher than average on the same date, it is assumed that streamflow on the Mashpee River also was much higher than average In the East Cape flow cell, streamflow on the Stony Brook in Brewster was 3 6 $ft^3/s$ in October 1978, at a point near the mouth of the river

### Aquifer Hydraulic Properties

Horizontal and vertical hydraulic conductivity, specific yield, specific storage, storage coefficient, and porosity are needed for an analysis of the response of the Cape Cod ground-water-flow system to changing stress conditions These hydraulic properties have been estimated for the flow system as part of this and previous investigations by analysis of aquifer tests, permeameter tests, and field tracer tests

Estimates of horizontal and vertical hydraulic conductivity of stratified drift on Cape Cod, as determined during previous investigations by analysis of aquifer tests, are summarized in table 1 The estimates indicate that there is a general increase in horizontal hydraulic conductivity with increase in grain size, from about 40 ft/d for fine sand and silt to 300 ft/d for medium-coarse sand and gravel The exceptionally high value of horizontal hydraulic conductivity of 380 ft/d at well site Falmouth 214 reported by LeBlanc and others (1988) may be the result of a particularly uniform sediment of fine to medium sand with little percentage of other grain sizes The ratio of vertical to horizontal hydraulic conductivity ranges from 1 1 to 1 30 for all sediment classes except fine sand and silt, for which the estimated ratio is 1 50 (table 1)

Little information is available on the hydraulic conductivity of the moraine and glaciolacustrine deposits of Cape Cod Estimates of horizontal and vertical hydraulic conductivity of glaciolacustrine deposits from Eastham were reported by Barlow

**Table 1** Estimates of hydraulic conductivity of stratified drift, as determined from analysis of aquifer tests, Cape Cod Basin, Massachusetts

[ft/d, foot per day --, no data <, actual value is less than value shown]

| Predominant grain size of tested interval | Aquifer test well | Latitude ° ′ ″ | Longitude ° ′ ″ | Horizontal hydraulic conductivity (ft/d) | Ratio of vertical to horizontal hydraulic conductivity | Source of data |
|---|---|---|---|---|---|---|
| Fine sand and silt | Mashpee 108 | 41 06 36 | 70 29 30 | 40 | 1 50 | Barlow and Hess (1993) |
| Fine sand | Barnstable 406 | 41 13 39 | 70 15 22 | 160 | 1 30 | Barlow (1994) |
| | Yarmouth 74 | 41 00 40 | 70 52 14 | 160 | 1 30 | Do |
| Fine to medium sand | Wellfleet 41 | 41 00 54 | 69 42 58 | 180 | 1 3–1 5 | Do |
| | Yarmouth 176 | 41 16 39 | 70 48 11 | 200 | -- | Guswa and LeBlanc (1985) |
| | Falmouth 214 | 41 03 37 | 70 00 33 | 380 | 1 2–1 5 | LeBlanc and others (1988) |
| Fine to coarse sand and gravel | Truro 200 | 42 51 00 | 70 48 02 | 220 | 1 1–1 5 | Guswa and Londquist (1976) |
| | Mashpee 108 | 41 06 36 | 70 29 30 | 240 | 1 3 | Barlow and Hess (1993) |
| | Yarmouth 59 | 41 10 40 | 70 53 13 | 220 | 1 10 | Barlow (1994) |
| | Yarmouth 129 | 41 22 40 | 70 19 14 | 240 | 1 3–1 5 | Do |
| Medium to coarse sand and gravel | Orleans 37 | 41 16 45 | 69 39 59 | 300 | <1 10 | Guswa and LeBlanc (1985) |

(1994) The horizontal and vertical hydraulic conductivity of these deposits varied depending on the percentage of clay present in the sample, however, the hydraulic conductivities for all of these glaciolacustrine deposits were 3 to 5 orders of magnitude less than those reported in table 1 for stratified drift

Estimates of specific yield, specific storage, and storage coefficient of the aquifer have been made at only a few locations on Cape Cod The storage coefficient is equal to the specific storage of the aquifer multiplied by its saturated thickness Analyses of aquifer tests in Truro by Guswa and Londquist (1976) and in Orleans by Guswa and LeBlanc (1985) indicate specific yields of 0 10 and 0 15, respectively Palmer (1977) reports a range of specific yield of 0 13 to 0 26 based on four aquifer tests in Falmouth Garabedian and others (1988) report a range in specific yield of 0 1 to 0 2 and a range of specific storage of $4\ 4\times10^{-5}$ to $8\ 7\times10^{-5}$ $ft^{-1}$ from the analysis of an aquifer test in Falmouth Dufresne-Henry, Inc (1990) reports a range of specific yield of 0 13 to 0 21 for an aquifer test at a well site in Mashpee, and Barlow and Hess (1993) report a specific yield of 0 25, a specific storage of $1\times10^{-6}$ $ft^{-1}$, and a storage coefficient of $2\times10^{-4}$ for the aquifer at a nearby test site These estimates of specific yield are similar to those reported for the stratified-drift sediments of Long Island, N Y, of 0 10 to 0 26 (Perlmutter and Geraghty, 1963, Getzen, 1977, Lindner and Reilly, 1983)

Porosity was estimated for stratified drift at two sites in Falmouth using ground-water tracer experiments and laboratory tests of cored sediment samples Estimates of porosity from the tracer tests range from 0 38 to 0 42 (Garabedian and others, 1988, LeBlanc and others, 1988, Barlow, 1989), whereas that of the cored samples was 0 32 (Wolf, 1988) Porosity also was determined for fine to medium sand in Orleans An average porosity of 0 34 was determined for five samples using laboratory volumetric measurements of cored samples (Desimone and others, 1996) These estimates are similar to those of 0 34 to 0 38 reported for the porosity of stratified drift on Long Island, N Y (Perlmutter and Lieber, 1970)

Horizontal hydraulic conductivity and specific yield of the stratified drift were estimated by analysis of 10 aquifer tests in the West Cape and East Cape flow cells Data used in the analysis were from engineers' reports of aquifer tests done as part of the process required for development of a new water supply The analysis was done to improve the existing data base of hydraulic conductivity and specific yield of the Cape Cod ground-water-flow system

The Cooper-Jacob approximation to the Theis solution of radial flow to a well was used in the analysis The approximation is applied subject to assumptions from Marsily (1986, p 164–165) (1) the aquifer is infinite, homogeneous, and isotropic, (2) if the aquifer is unconfined, then drawdowns are small relative to the saturated thickness of the aquifer, (3) hydraulic head does not vary in the vertical direction and water velocity is parallel to a horizontal, impermeable boundary, and (4) the pumped and observation wells fully penetrate the aquifer, the pumping rate of the well is constant, and the borehole radius of the pumped well is negligibly small Although several of these assumptions were not strictly met during the tests, the Cooper-Jacob method is a widely applied analysis technique that should provide reasonable estimates of horizontal hydraulic conductivity and specific yield A complete description of the Cooper-Jacob approximation is provided by Marsily (1986, p 163–165)

In the analyses, measured drawdowns were corrected to account for unconfined conditions according to the method reported by Kruseman and de Ridder (1983, p 107), and the following criterion was used to determine the time at which the Cooper-Jacob approximation was assumed valid (Marsily, 1986)

$$t \geq \frac{S_y r^2}{4Tu} \quad , \tag{1}$$

where

$t$ is the time since start of test (days),
$r$ is radial distance of observation well from pumped well (feet),
$S_y$ is specific yield (dimensionless),
$T$ is transmissivity (feet square per day), and
$u$ is Theis' dimensionless time

An upper limit of 0 02 was selected for the parameter $u$ because Marsily (1986, p 164) states that the error between the Cooper-Jacob approximation and the Theis solution is 0 5 percent at this limit Two additional criteria were used to account for the effect of partial penetration of the pumped and observation wells to the parameter estimates These criteria are (Neuman, 1974)

$$r \geq \frac{b}{\sqrt{\frac{K_v}{K_h}}} \tag{2}$$

and

$$t \geq \frac{10 S_y r^2}{T} \quad , \tag{3}$$

where

$b$ is saturated thickness of the aquifer near the well (feet),
$K_v$ is vertical hydraulic conductivity of the aquifer (feet per day), and
$K_h$ is horizontal hydraulic conductivity of the aquifer (feet per day)

The use of these two criteria is based on the results of Neuman (1974, p 309), who found that the effect of partial penetration on drawdown in an unconfined aquifer disappears completely at distances from the pumped well greater than $b/\sqrt{K_v/K_h}$ at times greater than $10S_y r^2/T$ The ratios of $K_v/K_h$ used to assess the effects of partial penetrations ranged from 1 3 to 1 5 for fine-medium sand to 1 30 for fine sand These values are based on the results of table 1 In practice, equation 1 was the more limiting time criterion compared to equation 3

Horizontal hydraulic conductivity was determined by dividing the transmissivity determined from the aquifer-test data by the saturated thickness of the aquifer at or near the site of the aquifer test Because the pumped and observation wells did not fully penetrate the aquifer, a saturated thickness was estimated from available information In the analysis, the saturated thickness of the aquifer was assumed to be the distance from the water table to an assumed impermeable boundary at the contact between overlying sand and gravel and underlying fine-grained sediments, such as silt and clay The fine-grained sediments were assumed to act as confining units that do not yield water to the overlying sand and gravel during the aquifer tests

Although 22 aquifer tests were analyzed using the above methodology, only the 10 reported in table 2 satisfied the three criteria of equations 1, 2, and 3 and had sufficient information to allow an estimate of the saturated thickness of the aquifer near the pumped well Values of horizontal hydraulic conductivity determined from the 10 aquifer tests (table 2) are similar to those determined by previous investigators, and tend to increase with increasing grain size of the aquifer Horizontal hydraulic conductivities estimated at wells Yarmouth 74 (180 ft/d) and Yarmouth 129 (220 ft/d) are within 13 percent of those determined for the aquifer at these two wells by Barlow (1994), who used a more comprehensive method of analysis than the Cooper-Jacob method, in which type curves were

**Table 2.** Hydraulic properties of stratified drift, as determined by use of the Cooper-Jacob method, Cape Cod Basin, Massachusetts

[ft, foot, ft/d, foot per day, $ft^2/d$, foot squared per day, gal/min, gallon per minute]

| Predominant grain size of tested interval | Aquifer test well | Length of test (days) | Well discharge (gal/min) | Latitude ° ′ ″ | Longitude ° ′ ″ | Transmissivity ($ft^2/d$) | Saturated thickness (ft) | Horizontal hydraulic conductivity (ft/d) | Specific yield |
|---|---|---|---|---|---|---|---|---|---|
| Fine sand | Yarmouth 155 | 6 | 299 | 41 10 40 | 70 18 14 | 5,000 | 40 | 125 | 0 15 |
| | Yarmouth 74 | 5 | 250 | 41 00 40 | 70 52 14 | 11,000 | 60 | 180 | 02 |
| Fine to coarse sand and gravel | Dennis 200 | 7 | 204 | 41 54 42 | 70 03 10 | 13,000 | 40 | 325 | 25 |
| | Dennis 211 | 8 | 500 | 41 42 42 | 70 21 08 | 8,000 | 30 | 270 | 17 |
| | Barnstable 253 | 7 | 300 | 41 42 38 | 70 23 19 | 9,000 | 40 | 225 | 20 |
| | Yarmouth 129 | 5 | 246 | 41 22 40 | 70 19 14 | 12,000 | 55 | 220 | 17 |
| Medium to coarse sand and gravel | Harwich 128 | 4 | 191 | 41 09 41 | 70 06 02 | 12,000 | 40 | 300 | 08 |
| | Dennis 225 | 6 | 271 | 41 53 42 | 70 25 08 | 12,000 | 65 | 185 | 23 |
| | Falmouth 177 | 6 | 350 | 41 44 35 | 70 04 32 | 18,000 | 50 | 360 | 12 |
| | Sandwich 282 | 5 | 505 | 41 04 42 | 70 28 44 | 32,000 | 100 | 320 | 12 |

developed for each pumped and observation well pair (Neuman, 1974) The strong agreement between the hydraulic conductivities determined by the two methods indicates that the Cooper-Jacob approximation and the three criteria used in this investigation constitute a valid estimation method for hydraulic conductivity of the Cape Cod ground-water-flow system

Estimates of specific yield range from 0 020 to 0 25 and are highest for fine to medium sand and gravel to medium to coarse sand The average value of specific yield determined for the 10 aquifer tests was 0 15

## Ground-Water Pumping

Ground water is the principal source of freshwater for domestic, industrial, and agricultural use on Cape Cod Public-supply systems service communities in the West Cape, East Cape, and Provincetown flow cells Water-supply needs for residents in the Wellfleet and Eastham flow cells, and most of the Truro flow cell are met by domestic wells Pumping-well locations and pumping rates for 1975–76 were reported by Guswa and LeBlanc (1985), and pumping-well locations and pumping rates for 1989 were obtained directly from public-water suppliers of Cape Cod (tables 3–5, figs 4 and 5) Projected pumping rates for the year 2020 were provided by MOWR, these rates are based on 1989 estimates of future water needs Agricultural and industrial pumpage have not been considered in this study because (1) they represent a small percentage of the total pumping, (2) daily demand information for pumping less than 0 1 Mgal/d is not readily available, and (3) pumped water is typically returned to the aquifer within the same model node from which it is pumped

Public-water supply systems began operation on Cape Cod as early as 1893 The total quantity of water pumped from the West Cape and East Cape flow cells increased substantially between 1975 and 1989, total pumpage is projected to continue to increase through the year 2020 as a result of the projected continuing increase in the number of permanent and summer residents throughout Cape Cod Average daily demand increased by more than 40 percent between 1975 and 1989 in the West Cape flow cell (table 3) and by more than 70 percent between 1975 and 1989 in the East Cape flow cell (table 4) Average daily demands are projected to increase by about 100 percent between 1989 and 2020 in the West Cape and East Cape flow cells (tables 3 and 4) Most wells in the West Cape and East Cape flow cells are screened at depths less than 70 ft below sea level and are more than 1 mi inland of the coasts (figs 4 and 5) Average daily demand for water pumped from the Truro flow cell actually decreased slightly from 1975 to 1989 but is projected to increase by more than 40 percent between 1989 and 2020 (table 5) Seasonal fluctuations in ground-water use are due to the substantial population increases during June, July, and August, resulting in average daily pumpage during the summer that is more than double that of the winter

**Table 3** Pumping rates of public-supply wells for 1975, 1989, and projected for 2020, represented in models of the West Cape flow cell, Cape Cod Basin, Massachusetts

[Map No Wells shown in bold are proposed wells, all other wells are existing Location of wells shown in figure 4 Model node Model grid shown in figure 8 Year and pumping rate ADD, average daily demand, IN, in-season pumping rates from June through August, OFF, off-season pumping rates from September through May Ab, abandoned well No , number --, no data]

| Map No | Well No | Well name | Model node | | | Year and pumping rate, in cubic feet per second | | | | | | |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| | | | Layer | Row | Column | 1975 ADD | 1989 IN | 1989 OFF | 1989 ADD | 2020 IN | 2020 OFF | 2020 ADD |
| **Barnstable Fire District** | | | | | | | | | | | | |
| 1 | A1W228 | Phinney's Lane | 1 | 34 | 87 | 0 172 | 0 148 | 0 068 | 0 077 | 0 188 | 0 091 | 0 123 |
| 2 | A1W370 | Old Barns Rd #2 | 2 | 38 | 90 | 269 | 148 | 068 | 077 | 376 | 182 | 247 |
| 3 | A1W416 | Route 132 #3 | 2 | 36 | 84 | -- | 829 | 380 | 433 | 502 | 243 | 329 |
| **4** | -- | Oak Street | 1 | 34 | 78 | -- | -- | -- | -- | 441 | 213 | 289 |
| **5** | -- | District Well #4 | 1 | 36 | 83 | -- | -- | -- | -- | 350 | 169 | 229 |
| | Total | | | | | 0 441 | 1 125 | 0 516 | 0 587 | 1 857 | 0 898 | 1 217 |
| **Barnstable Water Company** | | | | | | | | | | | | |
| 6 | A1W300 | Airport Well #1 | 2 | 41 | 92 | -- | 0 389 | 0 219 | 0 248 | 1 300 | 0 633 | 0 855 |
| 7 | A1W229 | Hyannisport | 2 | 48 | 84 | 0 210 | 753 | 425 | 480 | 648 | 316 | 515 |
| | A1W384 | Simmons Pond | 2 | 48 | 84 | 966 | 1 580 | 891 | 1 006 | 908 | 444 | 515 |
| 8 | A1W377 | Maher Well #1 | 2 | 44 | 92 | 1 165 | 729 | 411 | 464 | 648 | 316 | 542 |
| | A1W385 | Maher Well #2 | 2 | 44 | 92 | 055 | 839 | 473 | 534 | 909 | 444 | 542 |
| | A1W386 | Maher Well #3 | 2 | 44 | 92 | 055 | 839 | 473 | 534 | 909 | 444 | 542 |
| 9 | A1W402 | Mary Dunn #1 | 1 | 39 | 92 | 072 | 097 | 055 | 603 | 648 | 316 | 427 |
| | A1W383 | Mary Dunn #2 | 1 | 39 | 92 | 305 | 948 | 535 | 325 | 909 | 444 | 542 |
| 10 | A1W387 | Mary Dunn #3 | 1 | 39 | 91 | 173 | 022 | 012 | 014 | 648 | 316 | 427 |
| | A1W403 | Mary Dunn #4 | 1 | 39 | 91 | 065 | 510 | 288 | 328 | 648 | 316 | 427 |
| 11 | A1W376 | Straightway #1 | 4 | 47 | 84 | 453 | -- | -- | -- | 909 | 444 | 600 |
| **12** | -- | Straightway #2 | 2 | 46 | 84 | -- | -- | -- | -- | 909 | 444 | 600 |
| **13** | -- | Mary Dunn #5 | 1 | 38 | 92 | -- | -- | -- | -- | 648 | 316 | 426 |
| | -- | Mary Dunn #6 | 1 | 38 | 92 | -- | -- | -- | -- | 648 | 316 | 426 |
| **14** | -- | Airport | 2 | 40 | 91 | -- | -- | -- | -- | 909 | 444 | 600 |
| | -- | Mary Dunn #7 | 1 | 39 | 92 | -- | -- | -- | -- | 648 | 316 | 600 |
| | -- | Mary Dunn #8 | 1 | 39 | 92 | -- | -- | -- | -- | 648 | 316 | 442 |
| | Total | | | | | 3 519 | 6 706 | 3 782 | 4 536 | 13 494 | 6 585 | 9 028 |
| **Bourne Water District** | | | | | | | | | | | | |
| 15 | BHW1 | County Rd #1 | 2 | 19 | 26 | 0 084 | 0 209 | 0 102 | 0 116 | 0 418 | 0 148 | 0 238 |
| | BHW2 | -- | 2 | 19 | 26 | 084 | 208 | 102 | 116 | 418 | 148 | 238 |
| | BHW3 | -- | 1 | 19 | 26 | 084 | 290 | 102 | 116 | 418 | 148 | 238 |
| | BHW136 | -- | 1 | 19 | 26 | 084 | 208 | 102 | 116 | 418 | 148 | 238 |
| 16 | BHW137 | Route 28A #2 | 2 | 30 | 25 | 288 | 278 | 136 | 155 | 558 | 197 | 317 |
| 17 | BHW199 | Bourne Forest #3 | 1 | 19 | 29 | 239 | 361 | 177 | 201 | 725 | 256 | 412 |
| 18 | BHW233 | Bourne Forest #4 | 2 | 18 | 28 | 079 | 389 | 191 | 217 | 781 | 276 | 444 |
| 19 | -- | Route 28A #5 | 1 | 31 | 25 | -- | 528 | 260 | 294 | 1 060 | 374 | 603 |
| **20** | -- | Route 28A #6 | 2 | 20 | 27 | -- | -- | -- | -- | 732 | 286 | 441 |
| | Total | | | | | 0 942 | 2 471 | 1 172 | 1 331 | 5 528 | 1 981 | 3 169 |

**Table 3.** Pumping rates of public-supply wells for 1975, 1989, and projected for 2020, represented in models of the West Cape flow cell, Cape Cod Basin, Massachusetts—*Continued*

| Map No | Well No | Well name | Model node | | | Year and pumping rate, in cubic feet per second | | | | | | |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| | | | Layer | Row | Column | 1975 ADD | 1989 IN | 1989 OFF | 1989 ADD | 2020 IN | 2020 OFF | 2020 ADD |
| **Centerville/Osterville Water District** | | | | | | | | | | | | |
| 21 | A1W107 | McShane #1 | 2 | 46 | 67 | 0 400 | 0 008 | 0 003 | 0 005 | 0 889 | 0 390 | 0 555 |
| | -- | McShane #2 | 2 | 46 | 67 | -- | 024 | 009 | 011 | 889 | 390 | 555 |
| 22 | A1W160 | Arena #3 | 1 | 45 | 65 | 133 | -- | -- | -- | -- | -- | -- |
| | A1W158 | Arena #3 | 1 | 45 | 67 | 133 | 031 | 012 | 015 | 556 | 244 | 348 |
| | A1W159 | Arena #4 | 2 | 45 | 67 | 133 | 159 | 061 | 077 | 556 | 244 | 348 |
| 23 | A1W373 | Lumbert Mill #5 | 2 | 42 | 72 | 031 | 407 | 156 | 201 | 332 | 146 | 208 |
| | A1W259 | Lumbert Mill #9 | 1 | 42 | 72 | 471 | 781 | 300 | 387 | 471 | 207 | 295 |
| 24 | A1W368 | Craigville #7 | 2 | 46 | 81 | 119 | 374 | 144 | 186 | 386 | 169 | 199 |
| | A1W227 | Craigville #8 | 1 | 46 | 81 | 071 | -- | -- | -- | -- | -- | -- |
| | A1W226 | Craigville #11 | 1 | 46 | 81 | 048 | 622 | 239 | 310 | 247 | 108 | 196 |
| 25 | A1W249 | Davis #10 | 1 | 45 | 66 | 371 | 686 | 263 | 340 | 355 | 156 | 295 |
| 26 | A1W371 | Murray #12 | 1 | 38 | 70 | 170 | 622 | 239 | 310 | 386 | 169 | 242 |
| | A1W372 | Murray #13 | 1 | 38 | 70 | 156 | 526 | 202 | 263 | 386 | 169 | 242 |
| 27 | -- | Hayden #14 | 1 | 40 | 61 | -- | 1 212 | 466 | 603 | 780 | 342 | 485 |
| | -- | Hayden #15 | 1 | 40 | 61 | -- | 311 | 119 | 155 | 332 | 146 | 214 |
| | -- | Hayden #17 | 1 | 40 | 61 | -- | 1 060 | 407 | 527 | 780 | 342 | 485 |
| 28 | -- | Harrison #16 | 1 | 31 | 63 | -- | 1 148 | 441 | 573 | 556 | 244 | 348 |
| | -- | CO-16 | 1 | 31 | 63 | -- | -- | -- | -- | 780 | 342 | 488 |
| **29** | -- | CO-19 | 2 | 30 | 64 | -- | -- | -- | -- | 1 152 | 508 | 725 |
| **30** | -- | CO-19-83 | 2 | 39 | 60 | -- | -- | -- | -- | 780 | 342 | 488 |
| **31** | -- | CO40-8 | 2 | 40 | 59 | -- | -- | -- | -- | 780 | 342 | 488 |
| **32** | -- | CO41-89 | 2 | 39 | 59 | -- | -- | -- | -- | 780 | 342 | 488 |
| | Total | | | | | 2 236 | 7 971 | 3 061 | 3 963 | 12 173 | 5 342 | 7 692 |
| **Cotuit Fire District** | | | | | | | | | | | | |
| 33 | A1W251 | Electric Station | 2 | 45 | 57 | 0 112 | 0 273 | 0 116 | 0 128 | 0 292 | 0 139 | 0 190 |
| 34 | A1W369 | Electric Station | 1 | 45 | 56 | 113 | 243 | 104 | 139 | 292 | 139 | 190 |
| 35 | A1W224 | Electric Station | 1 | 48 | 57 | 046 | 243 | 104 | 128 | 174 | 083 | 095 |
| 36 | A1W59 | Electric Station | 1 | 44 | 57 | 007 | 304 | 129 | 155 | 292 | 139 | 190 |
| **37** | -- | C-130/28 | 3 | 43 | 53 | -- | -- | -- | -- | 609 | 289 | 396 |
| | Total | | | | | 0 278 | 1 063 | 0 453 | 0 550 | 1 659 | 0 789 | 1 061 |
| **Falmouth Department of Public Works, Water, and Sewer Commission** | | | | | | | | | | | | |
| 38 | -- | Long Pond | 2 | 52 | 18 | 0 692 | 0 837 | 0 447 | 0 524 | 1 536 | 0 987 | 1 170 |
| | -- | Long Pond | 2 | 53 | 18 | 692 | 837 | 447 | 524 | 1 536 | 987 | 1 170 |
| | -- | Long Pond | 2 | 54 | 18 | 692 | 837 | 447 | 524 | 1 536 | 987 | 1 170 |
| | -- | Long Pond | 2 | 55 | 18 | 692 | 837 | 447 | 524 | 1 536 | 987 | 1 170 |
| | -- | Long Pond | 2 | 53 | 19 | 692 | 837 | 447 | 524 | 1 536 | 987 | 1 170 |
| | -- | Long Pond | 2 | 54 | 19 | 692 | 837 | 447 | 524 | 1 536 | 987 | 1 170 |
| 39 | -- | Fresh Pond | 2 | 51 | 37 | -- | 1 831 | 978 | 1 145 | 853 | 548 | 650 |
| 40 | -- | Coonamessett | 1 | 44 | 28 | -- | 1 460 | 779 | 913 | 853 | 548 | 650 |
| **41** | -- | F6D | 2 | 50 | 23 | -- | -- | -- | -- | 350 | 225 | 267 |
| **42** | -- | F-4D | 1 | 42 | 25 | -- | -- | -- | -- | 350 | 225 | 267 |

**Table 3** Pumping rates of public-supply wells for 1975, 1989, and projected for 2020, represented in models of the West Cape flow cell, Cape Cod Basin, Massachusetts—*Continued*

| Map No | Well No | Well name | Model node | | | Year and pumping rate, in cubic feet per second | | | | | | |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| | | | Layer | Row | Column | 1975 ADD | 1989 IN | 1989 OFF | 1989 ADD | 2020 IN | 2020 OFF | 2020 ADD |
| **Falmouth Department of Public Works, Water, and Sewer Commission—*Continued*** | | | | | | | | | | | | |
| **43** | -- | F-F3 | 1 | 50 | 38 | -- | -- | -- | -- | 0 350 | 0 225 | 0 267 |
| **44** | -- | F-8A | 2 | 52 | 38 | -- | -- | -- | -- | 350 | 225 | 267 |
| **45** | -- | Beebe Woods | 1 | 58 | 15 | -- | -- | -- | -- | 350 | 225 | 267 |
| | Total | | | | | 4 152 | 8 313 | 4 439 | 5 202 | 12 672 | 8 143 | 9 655 |
| **Highwood Water Company, Mashpee** | | | | | | | | | | | | |
| 46 | M1W32 | Wading Place | 2 | 58 | 48 | 0 048 | 0 119 | 0 039 | 0 062 | 0 150 | 0 057 | 0 088 |
| 47 | M1W35 | Rock Landing #2 | 2 | 59 | 46 | 150 | 357 | 117 | 183 | 600 | 229 | 362 |
| | -- | Rock Landing #3 | 2 | 59 | 46 | -- | 477 | 156 | 248 | 600 | 229 | 362 |
| **48** | -- | High Wood #3 | 1 | 50 | 43 | -- | -- | -- | -- | 600 | 229 | 362 |
| | Total | | | | | 0 198 | 0 953 | 0 312 | 0 493 | 1 950 | 0 744 | 1 174 |
| **Mashpee Water District** | | | | | | | | | | | | |
| **49** | -- | T-4 | 1 | 45 | 51 | -- | -- | -- | -- | 0 735 | 0 294 | 0 441 |
| **50** | -- | P-1 | 1 | 51 | 41 | -- | -- | -- | -- | 735 | 294 | 441 |
| | Total | | | | | -- | -- | -- | -- | 1 470 | 0 588 | 0 882 |
| **Otis Air Force Base Water System** | | | | | | | | | | | | |
| 51 | BHW23 | Well G | 3 | 33 | 31 | 0 507 | Ab | Ab | Ab | Ab | Ab | Ab |
| 52 | SDW155 | Well J | 1 | 33 | 40 | 433 | 0 890 | 0 630 | 0 634 | 0 753 | 0 526 | 0 602 |
| **53** | -- | Airport | 2 | 38 | 43 | -- | -- | -- | -- | 269 | 187 | 214 |
| | Total | | | | | 0 940 | 0 890 | 0 630 | 0 634 | 1 022 | 0 713 | 0 816 |
| **Sandwich Water District** | | | | | | | | | | | | |
| 54 | SDW27 | BS #2 | 2 | 13 | 56 | 0 104 | 0 690 | 0 390 | 0 449 | 0 515 | 0 349 | 0 404 |
| | SDW37 | BS #3 | 2 | 13 | 56 | 320 | 660 | 380 | 433 | 515 | 349 | 404 |
| 55 | -- | Pinkham #4 | 2 | 25 | 55 | -- | 420 | 230 | 279 | 715 | 485 | 562 |
| | -- | Pinkham #6 | 2 | 25 | 55 | -- | 560 | 330 | 371 | 715 | 485 | 562 |
| 56 | SDW208 | Pinkham #5 | 1 | 29 | 42 | -- | 380 | 220 | 248 | 715 | 485 | 562 |
| 57 | SDW249 | Boiling Springs | 2 | 6 | 50 | 031 | -- | -- | -- | -- | -- | -- |
| | SDW250 | Boiling Springs | 2 | 6 | 50 | 031 | -- | -- | -- | -- | -- | -- |
| **58** | S23 | -- | 2 | 29 | 57 | -- | -- | -- | -- | 715 | 485 | 562 |
| **59** | S13 | -- | 2 | 20 | 63 | -- | -- | -- | -- | 715 | 485 | 562 |
| | -- | Boiling Springs(A) | 1 | 13 | 56 | -- | -- | -- | -- | 715 | 485 | 562 |
| | Total | | | | | 0 486 | 2 710 | 1 550 | 1 780 | 5 320 | 3 608 | 4 180 |
| **South Sagamore Water District** | | | | | | | | | | | | |
| 60 | BHW232 | Route 6A Tub Well | 2 | 5 | 46 | 0 124 | 0 304 | 0 129 | 0 155 | 0 366 | 0 202 | 0 243 |
| | Total | | | | | 0 124 | 0 304 | 0 129 | 0 155 | 0 366 | 0 202 | 0 243 |
| **Yarmouth Water Department** | | | | | | | | | | | | |
| 61 | YAW103 | Main Station | 2 | 35 | 104 | 0 223 | 0 918 | 0 418 | 0 528 | 0 887 | 0 410 | 0 854 |
| 62 | YAW41 | Higgins Crow PS #1 | 1 | 38 | 97 | -- | 243 | 111 | 139 | 222 | 103 | 297 |
| 63 | YAW42 | Higgins Crow PS #2 | 1 | 37 | 98 | 248 | 324 | 147 | 186 | 308 | 143 | 297 |
| | YAW43 | Higgins Crow PS #3 | 1 | 37 | 98 | 266 | 378 | 172 | 217 | 370 | 171 | 297 |
| 64 | YAW64 | Long Pond #4 | 2 | 43 | 107 | 345 | 324 | 147 | 186 | 308 | 143 | 217 |
| | YAW5 | Long Pond #5 | 2 | 43 | 107 | 309 | 351 | 160 | 201 | 370 | 171 | 217 |

**Table 3** Pumping rates of public-supply wells for 1975, 1989, and projected for 2020, represented in models of the West Cape flow cell, Cape Cod Basin, Massachusetts—*Continued*

| Map No | Well No | Well name | Model node | | | Year and pumping rate, in cubic feet per second | | | | | | |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| | | | Layer | Row | Column | 1975 ADD | 1989 IN | 1989 OFF | 1989 ADD | 2020 IN | 2020 OFF | 2020 ADD |
| **Yarmouth Water Department—*Continued*** | | | | | | | | | | | | |
| 65 | YAW53 | N Main St #6 | 2 | 40 | 112 | 0 223 | 0 270 | 0 123 | 0 155 | 0 222 | 0 103 | 0 192 |
| | YAW146 | N Main St #7 | 2 | 40 | 112 | 238 | 243 | 111 | 139 | 222 | 103 | 192 |
| | YAW144 | N Main St #8 | 2 | 40 | 112 | 256 | 270 | 123 | 155 | 222 | 103 | 192 |
| | YAW54 | N Main St #9 | 2 | 40 | 112 | 467 | 594 | 270 | 340 | 530 | 245 | 192 |
| 66 | YAW61 | Forest Rd #10 | 2 | 41 | 104 | 279 | 297 | 135 | 170 | 265 | 123 | 170 |
| 67 | YAW63 | Forest Rd #11 | 2 | 42 | 104 | 208 | 324 | 147 | 186 | 265 | 123 | 170 |
| 68 | YAW193 | Chickadee Ln #13 | 2 | 43 | 100 | -- | 459 | 234 | 263 | 401 | 185 | 285 |
| | YAW58 | Chickadee Ln #18 | 2 | 43 | 100 | 310 | 459 | 209 | 294 | 444 | 205 | 285 |
| | YAW194 | Chickadee Ln #19 | 2 | 43 | 100 | -- | 432 | 197 | 248 | 401 | 185 | 340 |
| 69 | YAW128 | Higgins Crow #14 | 2 | 43 | 97 | 125 | 378 | 172 | 217 | 308 | 143 | 198 |
| 70 | YAW126 | N Dennis Rd #15 | 2 | 37 | 110 | 045 | 459 | 290 | 263 | 444 | 205 | 284 |
| | YAW127 | N Dennis Rd #16 | 2 | 37 | 110 | 027 | 513 | 234 | 294 | 444 | 205 | 284 |
| 71 | YAW195 | Horse Pond #17 | 2 | 43 | 98 | -- | 621 | 283 | 356 | 530 | 245 | 340 |
| 72 | -- | Higgins Crow #20 | 2 | 39 | 98 | -- | 351 | 160 | 201 | 308 | 143 | 198 |
| | -- | Higgins Crow #23 | 2 | 39 | 97 | -- | 243 | 111 | 139 | 487 | 225 | 312 |
| 73 | -- | Higgins Crow #24 | 1 | 41 | 95 | -- | 162 | 074 | 093 | 487 | 225 | 312 |
| | -- | Higgins Crow #25 | 2 | 41 | 95 | -- | -- | -- | -- | 444 | 205 | 285 |
| **74** | -- | Y-DP | 3 | 35 | 100 | -- | -- | -- | -- | 444 | 205 | 285 |
| **75** | -- | Y-GP | 3 | 36 | 100 | -- | -- | -- | -- | 444 | 205 | 285 |
| **76** | -- | Y-MP | 3 | 34 | 101 | -- | -- | -- | -- | 444 | 205 | 285 |
| **77** | -- | YBP1 | 3 | 44 | 100 | -- | -- | -- | -- | 444 | 205 | 285 |
| | -- | YBP2 | 2 | 44 | 100 | -- | -- | -- | -- | 444 | 205 | 285 |
| | -- | YBP4 | 2 | 44 | 100 | -- | -- | -- | -- | 444 | 205 | 285 |
| **78** | -- | YBP3 | 2 | 44 | 101 | -- | -- | -- | -- | 444 | 205 | 285 |
| **79** | -- | Y-Union | 2 | 35 | 104 | -- | -- | -- | -- | 444 | 205 | 285 |
| **80** | -- | Y-Dennis | 2 | 37 | 109 | -- | -- | -- | -- | 444 | 205 | 285 |
| **81** | -- | Y-F Pond | 2 | 40 | 111 | -- | -- | -- | -- | 444 | 205 | 285 |
| | Total | | | | | 3 569 | 8 613 | 4 028 | 4 970 | 13 329 | 6 162 | 9 260 |

**West Cape Flow Cell Totals**

| Town name | Year and pumping rate, in cubic feet per second | | | | | | |
|---|---|---|---|---|---|---|---|
| | 1975 ADD | 1989 IN | 1989 OFF | 1989 ADD | 2020 IN | 2020 OFF | 2020 ADD |
| Barnstable | 3 960 | 7 831 | 4 298 | 5 123 | 15 351 | 7 483 | 10 245 |
| Bourne | 942 | 2 471 | 1 172 | 1 331 | 5 528 | 1 981 | 3 169 |
| Centerville/Osterville | 2 236 | 7 971 | 3 061 | 3 963 | 12 173 | 5 343 | 7 692 |
| Cotuit | 278 | 1 063 | 453 | 550 | 1 659 | 789 | 1 061 |
| Falmouth | 4 152 | 8 313 | 4 439 | 5 202 | 12 672 | 8 143 | 9 655 |
| Mashpee | 198 | 953 | 312 | 493 | 3 420 | 1 332 | 2 056 |
| Otis | 940 | 890 | 630 | 634 | 1 022 | 713 | 816 |
| Sandwich | 486 | 2 710 | 1 550 | 1 780 | 5 320 | 3 608 | 4 180 |
| South Sagamore | 124 | 304 | 129 | 155 | 366 | 202 | 243 |
| Yarmouth | 3 569 | 8 613 | 4 028 | 4 970 | 13 329 | 6 162 | 9 26 |
| Total for the West Cape flow cell | 16 885 | 41 119 | 20 072 | 24 201 | 70 840 | 35 756 | 48 377 |

**Table 4** Pumping rates of public-supply wells for 1975, and 1989, and projected for 2020, represented in models of the East Cape flow cell, Cape Cod Basin, Massachusetts

[Map No Wells shown in bold are proposed wells, all other wells are existing Location of wells shown in figure 5 Model node Model grid shown in figure 9 Year and pumping rate ADD, average daily demand, IN, in-season pumping rates from June through August, OFF, off-season pumping rates from September through May No , number --, no data]

| Map No | Well No | Well name | Model node | | | Year and pumping rate, in cubic feet per second | | | | | | |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| | | | Layer | Row | Column | 1975 ADD | 1989 IN | 1989 OFF | 1989 ADD | 2020 IN | 2020 OFF | 2020 ADD |
| **Brewster Water Department** | | | | | | | | | | | | |
| 1 | BMW37 | Well #1 | 2 | 22 | 47 | 0 287 | 0 830 | 0 390 | 0 495 | 1 300 | 0 528 | 0 785 |
| 2 | BMW41 | Freeman's Way #2 | 2 | 21 | 46 | 231 | 910 | 420 | 542 | 1 300 | 528 | 785 |
| 3 | BMW55 | Freeman's Way #3 | 2 | 20 | 49 | -- | 956 | 440 | 573 | 1 347 | 528 | 814 |
| 4 | -- | P-B-21-85 | 3 | 21 | 30 | -- | -- | -- | -- | 962 | 391 | 581 |
| 5 | -- | P-B5 | 2 | 24 | 32 | -- | -- | -- | -- | 722 | 294 | 437 |
| | Total | | | | | 0 518 | 2 696 | 1 250 | 1 610 | 5 631 | 2 269 | 3 402 |
| **Chatham Water Department** | | | | | | | | | | | | |
| 6 | CGW211 | Indian Hill W-1 | 2 | 35 | 52 | 0 776 | 1 096 | 0 520 | 0 681 | 0 471 | 0 337 | 0 382 |
| 7 | CGW153 | South Chatham #1 | 2 | 37 | 44 | 086 | -- | -- | -- | -- | -- | -- |
| | CGW1-3 | South Chatham #2 | 1 | 37 | 44 | 171 | 1 021 | 484 | 634 | 807 | 578 | 654 |
| 8 | -- | RCA | 2 | 33 | 53 | -- | 025 | 012 | 016 | 336 | 240 | 272 |
| 9 | -- | P-CWC-9 | 2 | 35 | 45 | -- | -- | -- | -- | 466 | 334 | 387 |
| | -- | P-CWC-13 | 2 | 35 | 45 | -- | -- | -- | -- | 466 | 334 | 387 |
| 10 | -- | P-CWC-RCA-N | 2 | 34 | 52 | -- | -- | -- | -- | 466 | 334 | 387 |
| 11 | -- | P-CWC-GP | 2 | 33 | 49 | -- | -- | -- | -- | 466 | 334 | 387 |
| | Total | | | | | 1 033 | 2 142 | 1 016 | 1 331 | 3 478 | 2 491 | 2 856 |
| **Dennis Water Department** | | | | | | | | | | | | |
| 12 | DGW1-5 | Main Station | 1 | 28 | 18 | 0 246 | 0 927 | 0 369 | 0 495 | 0 887 | 0 336 | 0 520 |
| 13 | DGW56 | Chatham Road #1 | 1 | 25 | 21 | 226 | 261 | 104 | 139 | 887 | 336 | 520 |
| | DGW57 | Chatham Road #2 | 1 | 25 | 21 | 013 | 116 | 046 | 062 | 887 | 336 | 520 |
| | DGW58 | Chatham Road #3 | 1 | 25 | 21 | 123 | 116 | 046 | 062 | 887 | 336 | 520 |
| 14 | DGW67 | Bass Road #4 | 1 | 22 | 18 | 059 | 290 | 115 | 155 | 887 | 336 | 520 |
| | DGW232 | Bass Road #6 | 1 | 22 | 18 | 015 | 058 | 023 | 031 | 193 | 073 | 113 |
| 15 | DGW66 | Route 134 #5 | 1 | 27 | 20 | 161 | 405 | 161 | 217 | 517 | 216 | 334 |
| 16 | DGW85 | Airline Road #7 | 2 | 24 | 22 | 232 | 492 | 196 | 263 | 571 | 216 | 334 |
| 17 | DGW86 | Airline Road #8 | 1 | 24 | 23 | 139 | 203 | 081 | 108 | 439 | 166 | 257 |
| 18 | DGW79 | Grassy Pond #9 | 2 | 22 | 20 | 527 | 521 | 207 | 279 | 825 | 313 | 484 |
| 19 | DGW77 | Airline Road #10 | 1 | 25 | 23 | 273 | 434 | 173 | 232 | 877 | 336 | 500 |
| 20 | DGW112 | Bass River #11 | 1 | 23 | 18 | 390 | 290 | 115 | 155 | 632 | 239 | 370 |
| 21 | DGW87 | Chatham #12 | 1 | 26 | 23 | 150 | 781 | 311 | 418 | 1 010 | 383 | 592 |
| 22 | DGW205 | Center Street #13 | 2 | 32 | 23 | -- | 405 | 161 | 217 | 632 | 239 | 370 |
| 23 | DGW244 | Bakers Pond Road #14 | 1 | 22 | 22 | -- | 347 | 138 | 186 | 571 | 216 | 334 |
| 24 | -- | Bakers Pond Road #15 | 1 | 22 | 21 | -- | 492 | 196 | 263 | 887 | 336 | 520 |
| 25 | -- | Timber Lane #16 | 1 | 23 | 21 | -- | 492 | 196 | 263 | 755 | 286 | 442 |
| 26 | -- | Timber Lane #17 | 1 | 23 | 23 | -- | -- | -- | -- | 887 | 337 | 520 |
| 27 | -- | Timber Lane #18 | 2 | 19 | 18 | -- | 637 | 253 | 340 | 887 | 337 | 520 |
| | Total | | | | | 2 554 | 7 267 | 2 891 | 3 885 | 14 118 | 5 373 | 8 290 |

**Table 4.** Pumping rates of public-supply wells for 1975, and 1989, and projected for 2020, represented in models of the East Cape flow cell, Cape Cod Basin, Massachusetts—*Continued*

| Map No | Well No | Well name | Model node | | | Year and pumping rate, in cubic feet per second | | | | | | |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| | | | Layer | Row | Column | 1975 ADD | 1989 IN | 1989 OFF | 1989 ADD | 2020 IN | 2020 OFF | 2020 ADD |
| **Harwich Water Department** | | | | | | | | | | | | |
| 28 | HJW1-5 | -- | 1 | 36 | 40 | 0 023 | 0 390 | 0 153 | 0 201 | 0 788 | 0 393 | 0 525 |
| 29 | HJW49 | PS #1 | 1 | 34 | 40 | 529 | 600 | 236 | 309 | 377 | 188 | 257 |
| | HJW55 | PS #2 | 1 | 34 | 40 | 148 | 300 | 118 | 155 | 377 | 188 | 257 |
| 30 | HJW56 | PS #3 | 1 | 35 | 40 | 373 | 510 | 200 | 263 | 460 | 229 | 334 |
| | HJW160 | PS #4 | 2 | 35 | 40 | 120 | 270 | 106 | 139 | 542 | 270 | 333 |
| 31 | HJW161 | PS #5 | 1 | 35 | 43 | 032 | 510 | 200 | 263 | 375 | 187 | 249 |
| | HJW162 | PS #6 | 2 | 35 | 43 | 118 | 480 | 189 | 248 | 375 | 187 | 249 |
| 32 | HJW163 | PS #7 | 2 | 36 | 44 | 126 | 540 | 212 | 279 | 432 | 216 | 288 |
| 33 | HJW8 | PS #8 | 2 | 30 | 50 | -- | 570 | 244 | 294 | 394 | 196 | 262 |
| | HJW243 | PS #9 | 2 | 30 | 50 | -- | 600 | 236 | 309 | 394 | 196 | 262 |
| 34 | -- | P-H-10 | 3 | 25 | 29 | -- | -- | -- | -- | 411 | 205 | 273 |
| 35 | -- | P-H-11 | 2 | 26 | 29 | -- | -- | -- | -- | 411 | 205 | 273 |
| 36 | -- | P-H-12 | 2 | 27 | 29 | -- | -- | -- | -- | 411 | 205 | 273 |
| 37 | -- | P-H-5R (Spruce Road) | 2 | 26 | 45 | -- | -- | -- | -- | 411 | 205 | 273 |
| 38 | -- | P-H-PBR1 (Pleasant Bay Road) | 2 | 27 | 49 | -- | -- | -- | -- | 411 | 205 | 273 |
| 39 | -- | P-H-PBR2 (Pleasant Bay Road) | 2 | 27 | 50 | -- | -- | -- | -- | 411 | 205 | 273 |
| | Total | | | | | 1 469 | 4 770 | 1 894 | 2 460 | 6 980 | 3 480 | 4 654 |
| **Orleans Water Department** | | | | | | | | | | | | |
| 40 | OSW11 | GP#1 | 2 | 16 | 56 | 0 322 | 0 179 | 0 148 | 0 108 | 0 447 | 0 226 | 0 300 |
| 41 | OSW14 | GP#2 | 2 | 15 | 56 | 206 | 307 | 086 | 186 | 323 | 163 | 216 |
| | -- | P-O-G2 | 2 | 15 | 56 | -- | -- | -- | -- | 323 | 163 | 216 |
| | OSW15 | GP#3 | 2 | 15 | 56 | 277 | 486 | 148 | 294 | 619 | 313 | 415 |
| 42 | OSW42 | GP#4 | 2 | 18 | 55 | 041 | 614 | 233 | 371 | 646 | 326 | 432 |
| 43 | OSW43 | GP#5 | 2 | 17 | 55 | 088 | 307 | 296 | 186 | 619 | 313 | 415 |
| | -- | P-0-G5 | 2 | 17 | 55 | -- | -- | -- | -- | 689 | 348 | 461 |
| 44 | -- | GP#6 | 2 | 16 | 55 | -- | 639 | 308 | 387 | 688 | 348 | 461 |
| | Total | | | | | 0 934 | 2 532 | 1 219 | 1 532 | 4 354 | 2 200 | 2 916 |

**Table 4** Pumping rates of public-supply wells for 1975, and 1989, and projected for 2020, represented in models of the East Cape flow cell, Cape Cod Basin, Massachusetts—*Continued*

| Map No | Well No | Well name | Model node | | | Year and pumping rate, in cubic feet per second | | | | | | |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| | | | Layer | Row | Column | 1975 ADD | 1989 IN | 1989 OFF | 1989 ADD | 2020 IN | 2020 OFF | 2020 ADD |
| **Yarmouth Water Department** | | | | | | | | | | | | |
| 45 | -- | Setucket Rd | 2 | 23 | 12 | -- | 0 432 | 0 197 | 0 248 | 0 401 | 0 185 | 0 257 |
| 46 | -- | Setucket Rd | 2 | 23 | 13 | -- | 513 | 234 | 294 | 487 | 225 | 352 |
| | Total | | | | | -- | 0 945 | 0 431 | 0 542 | 0 888 | 0 410 | 0 609 |

**East Cape Flow Cell Totals**

| Town name | Year and pumping rate, in cubic feet per second | | | | | | |
|---|---|---|---|---|---|---|---|
| | 1975 ADD | 1989 IN | 1989 OFF | 1989 ADD | 2020 IN | 2020 OFF | 2020 ADD |
| Brewster | 0 518 | 2 696 | 1 250 | 1 610 | 5 631 | 2 269 | 3 402 |
| Chatham | 1 033 | 2 142 | 1 016 | 1 331 | 3 478 | 2 491 | 2 856 |
| Dennis | 2 554 | 7 267 | 2 891 | 3 885 | 14 118 | 5 373 | 8 290 |
| Harwich | 1 469 | 4 770 | 1 894 | 2 460 | 6 980 | 3 480 | 4 654 |
| Orleans | 934 | 2 532 | 1 219 | 1 532 | 4 354 | 2 200 | 2 916 |
| Yarmouth | -- | 945 | 431 | 542 | 888 | 410 | 609 |
| Total for the East Cape flow cell | 6 508 | 20 352 | 8 701 | 11 360 | 35 449 | 16 223 | 22 727 |

**Table 5** Pumping rates of public-supply wells for 1975, and 1989, and projected for 2020, represented in models of the Truro flow cell, Cape Cod Basin, Massachusetts

[Map No Only existing wells are shown Location of wells shown in figure 22 Model node Model grid shown in figure 21 Year and pumping rate ADD, average daily demand, IN, in-season pumping rates from June through August, OFF, off-season pumping rates from September through May No , number --, no data]

| Map No | Well No | Well name | Model node | | | Year and pumping rate, in cubic feet per second | | | | | | |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| | | | Layer | Row | Column | 1975 ADD | 1989 IN | 1989 OFF | 1989 ADD | 2020 IN | 2020 OFF | 2020 ADD |
| **Provincetown Water Department** | | | | | | | | | | | | |
| 1 | TSW58 | Air Force | 3 | 14 | 12 | -- | 0 303 | 0 137 | 0 170 | 0 371 | 0 175 | 0 240 |
| 2 | TSW115 | Knowles Crossing | 1 | 7 | 21 | 0 696 | 331 | 150 | 186 | 405 | 190 | 262 |
| 3 | TSW78 | South Hollow | 2-3 | 10 | 14 | 716 | 1 516 | 688 | 851 | 1 855 | 873 | 1 200 |
| | Total | | | | | 1 412 | 2 150 | 0 975 | 1 207 | 2 631 | 1 238 | 1 702 |
| | Total for the Truro flow cell | | | | | 1 412 | 2 150 | 0 975 | 1 207 | 2 631 | 1 238 | 1 702 |

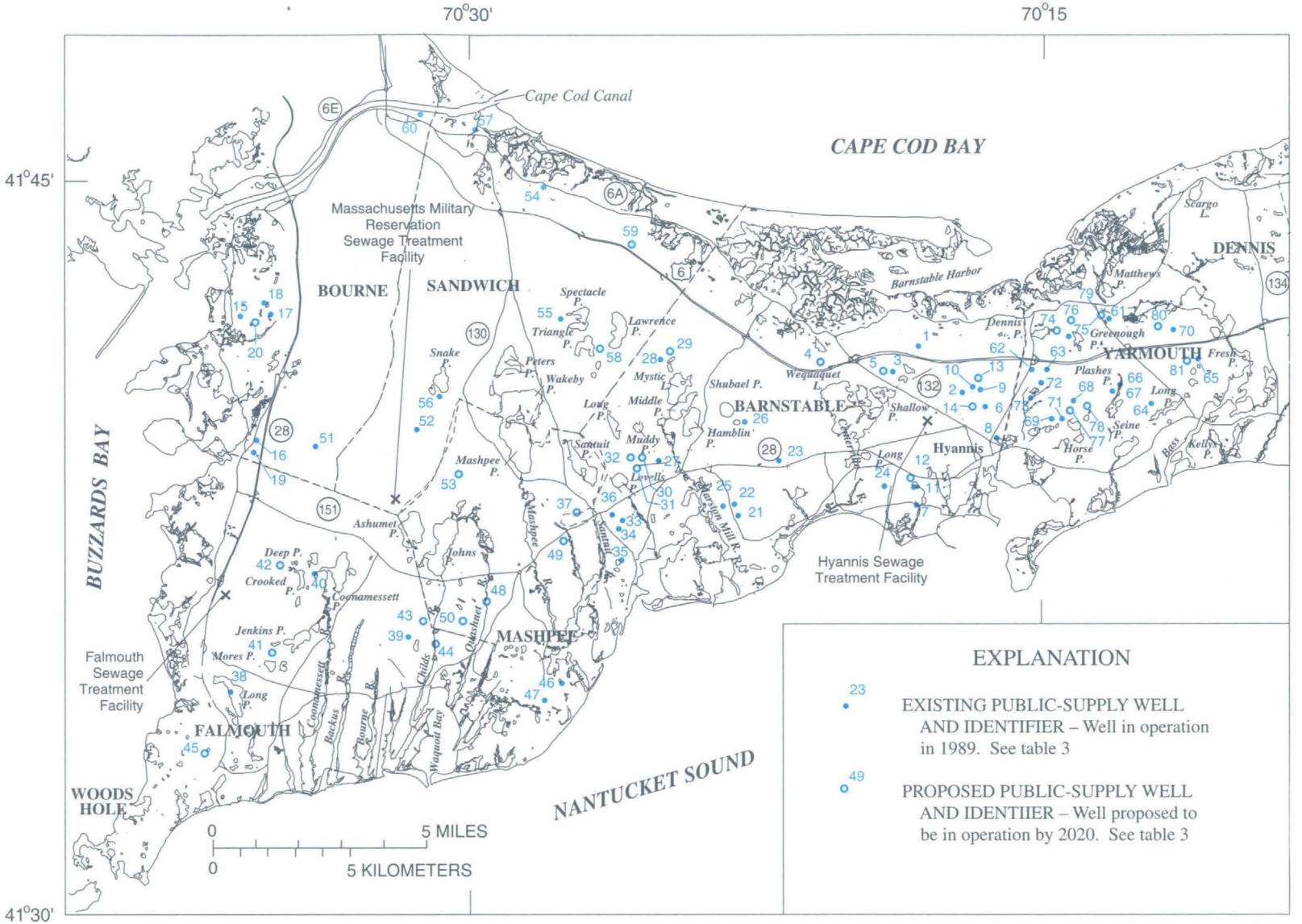


**Figure 4.** Existing and proposed public-supply wells in the West Cape flow cell, Cape Cod Basin, Massachusetts.

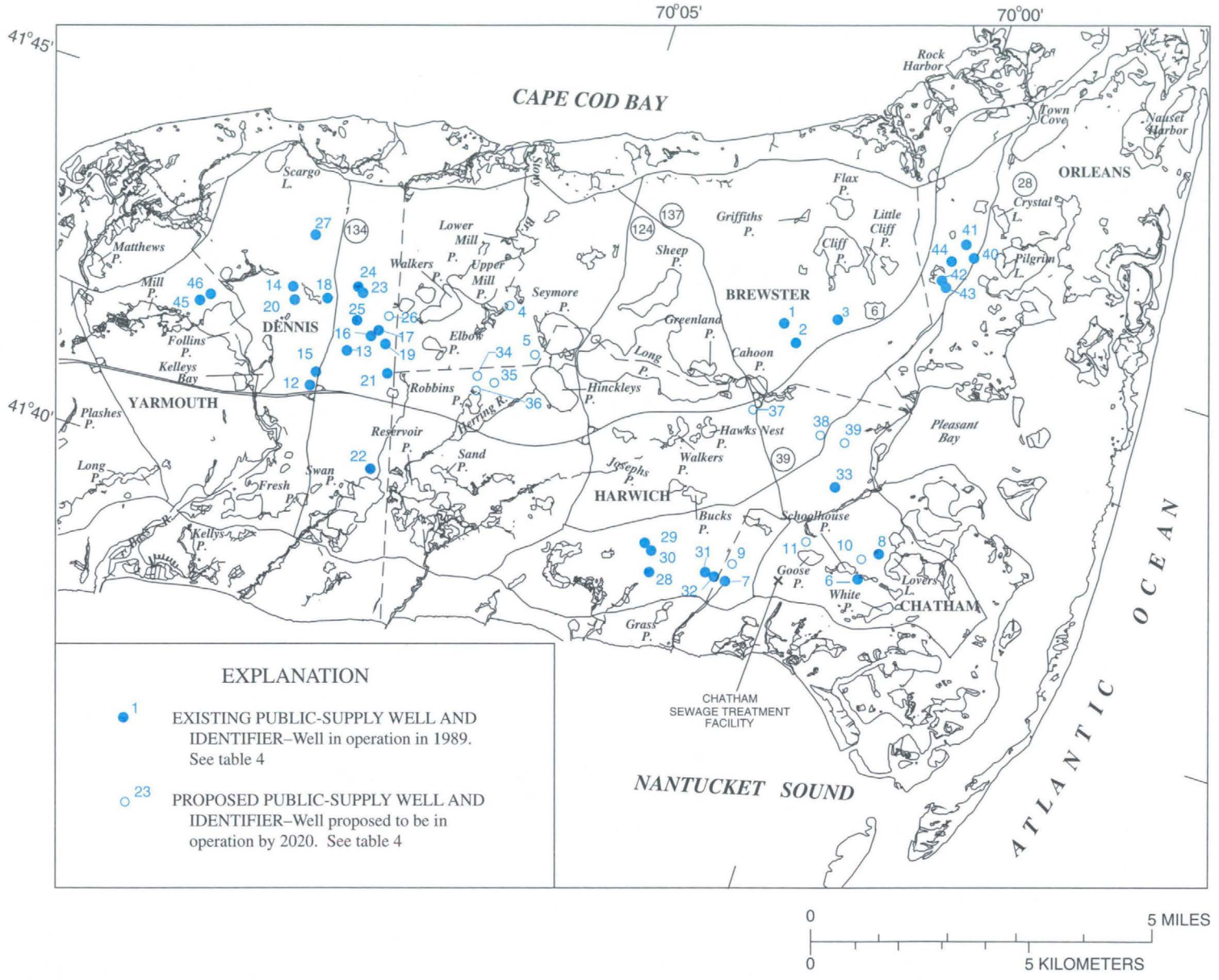


**Figure 5.** Existing and proposed public-supply wells in the East Cape flow cell, Cape Cod Basin, Massachusetts.

## Martha's Vineyard Basin

The Martha's Vineyard Basin is the second largest and second most populated of the three ground-water basins Unlike for Cape Cod Basin, only a single comprehensive investigation of the hydrogeology of the Martha's Vineyard Basin has been completed (Delaney, 1980) Most of the following information is based on that investigation

Martha's Vineyard consists of more than 600 ft of Cretaceous- and Tertiary-aged coastal-plain deposits that overlie crystalline bedrock (Delaney, 1980) These coastal-plain deposits are mantled by Pleistocene glacial formations deposited by the Buzzards Bay and Cape Cod Bay ice lobes (fig 3*B*) Martha's Vineyard has three principal physiographic regions (fig 6) the western moraine, the eastern moraine, and the central outwash plain The western moraine is a complex sequence of thrust sheets of coastal-plain deposits interbedded with till, gravel, sand, silt, and clay, with land-surface altitudes greater than 250 ft The eastern moraine is less rugged than the western moraine, with land-surface altitudes of only one-half those of the western moraine The eastern moraine consists of poorly sorted silt, sand, and till, which is overlain by sand and gravel outwash The central outwash plain consists of interbedded sand and gravel outwash formed by the meltwaters of the retreating ice lobes (Delaney, 1980)

The principal ground-water-flow system of Martha's Vineyard Basin consists of a primary and a secondary aquifer, both of which are glacial deposits within the upper 160 ft of saturated material (Delaney, 1980) The silty sands of the Cretaceous- and Tertiary-aged coastal-plain deposits and deposits of the western moraine are not considered part of the principal flow system The western moraine is not considered part of the principal flow system because, as Delaney (1980) reports, most ground-water levels in the western moraine are significantly higher than those of the central outwash plain and the eastern moraine Delaney (1980) determined that the primary sand and gravel aquifer extends from the water table to about 70 ft below sea level A 20-foot-thick unit of silty sand separates the primary aquifer from the underlying secondary aquifer, which consists of a 70-foot-thick deposit of interbedded fine to coarse sand

Martha's Vineyard Basin is completely surrounded by saltwater that forms the outer boundary of the fresh ground-water-flow system Because the island is hydraulically isolated, precipitation is the only source of freshwater to the ground-water-flow system Delaney (1980) reports that about one-half of the average annual precipitation (46 in ) is lost to evapotranspiration and that about 22 in/yr recharges the ground-water-flow system Martha's Vineyard has no significant streamflow or overland runoff

A water-table contour map was made from water levels measured on October 28–30, 1991 (fig 6) Ground water flows from the area that borders the western moraine to the Nantucket Sound and the Atlantic Ocean Local highs in the water table in the town of Edgartown represent perched conditions and are due to discontinuous deposits of low hydraulic conductivity in the eastern moraine Ground water is locally confined in parts of the towns of Oak Bluffs and Tisbury, where these confining conditions are present, ground-water seepage may occur when ground-water levels rise above the land surface

Ponds and streams on Martha's Vineyard are surface-water expressions of the principal ground-water-flow system Perched water bodies, however, are present in the western moraine and parts of the eastern moraine, where water levels overlying discontinuous deposits of low hydraulic conductivity intersect with the land surface

The horizontal hydraulic conductivity of the primary and secondary aquifers has been determined by analysis of aquifer tests made at public-supply wells and estimated from lithologic information Delaney (1980) reported horizontal hydraulic conductivities of 200 ft/d for the primary aquifer, 35 ft/d for the secondary aquifer, and less than 1 ft/d for the silt layer between the aquifers

Ground water is the principal source of drinking water for the residents of Martha's Vineyard Public-supply systems provide water for the residents of Tisbury, Edgartown, Oak Bluffs, and parts of West Tisbury Average annual pumping for 1989 and average annual and seasonal pumping from the public-supply wells of Martha's Vineyard projected by MOWR for the year 2020 are shown in table 6 Ground-water pumping rates during the summer months of 2020 are anticipated to be more than double those of the rest of the year, average daily demand for the year 2020 is projected to be nearly twice that of 1989

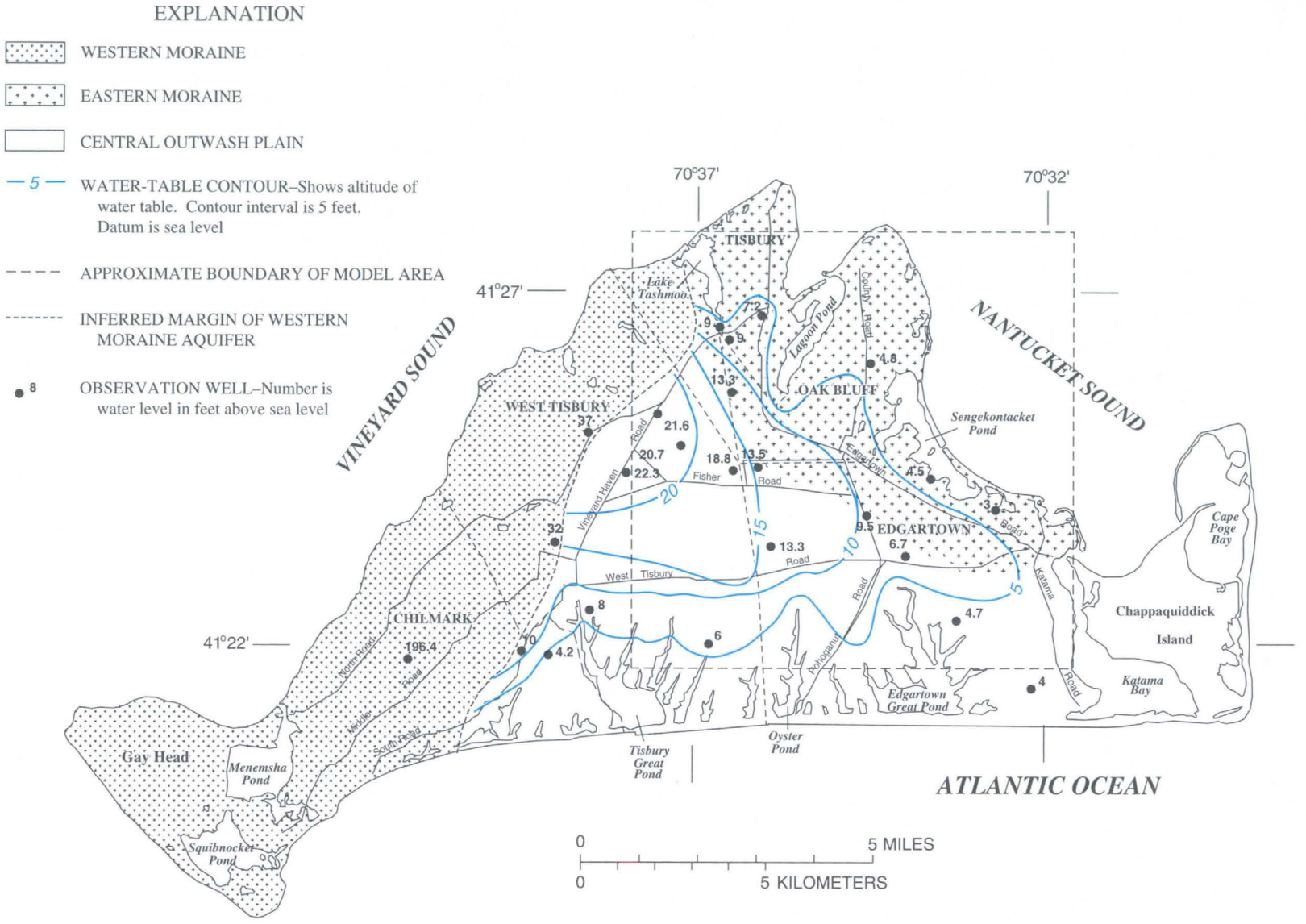


**Figure 6.** Water-table configuration on October 28–30, 1991, surficial geology, and location of modeled area, Martha's Vineyard Basin, Massachusetts.

**Table 6** Pumping rates of public-supply wells for 1989 and projected for 2020, represented in models of Martha's Vineyard and Nantucket Island Basins, Massachusetts

[Map No Wells shown in bold are proposed wells, all other wells are existing Location of wells shown in figures 32 and 33 Year and pumping rate ADD, Average daily demand, IN, in-season pumping rates from June through August, OFF, off-season pumping rates from September through May --, no pumping]

| Map No | Well name | Year | | | |
|---|---|---|---|---|---|
| | | 1989 ADD | 2020 IN | 2020 OFF | 2020 ADD |
| **Martha's Vineyard Basin** | | | | | |
| Edgartown Water Department | | | | | |
| 1 | Machacket Well | 0 217 | 0 758 | 0 248 | 0 402 |
| 2 | Lily Pond Well | 217 | 758 | 248 | 402 |
| 3 | Wintucket Well #1 | 634 | 1 13 | 379 | 596 |
| **4** | Wintucket Well #2 | -- | 1 13 | 379 | 595 |
| Oak Bluffs Water Department | | | | | |
| 5 | Lagoon Pond Well | 0 124 | 0 371 | 0 155 | 0 217 |
| 6 | Farm Neck Well | 124 | 371 | 155 | 217 |
| 7 | State Forest Well | 805 | 1 207 | 519 | 720 |
| **8** | State Forest Well | -- | 1 207 | 518 | 719 |
| Tisbury Water Works | | | | | |
| 9 | Sanborn Well | 0 495 | 0 712 | 0 294 | 0 433 |
| 10 | Tashmoo Well | 495 | 712 | 294 | 433 |
| **11** | Manter Well | -- | 1 470 | 619 | 882 |
| | Total | 3 111 | 9 826 | 3 808 | 5 616 |
| **Nantucket Island Basin** | | | | | |
| Siasconset Water District | | | | | |
| -- | Gravel Pack Well[1] | 0 186 | 0 464 | 0 046 | 0 186 |
| Wannacomet Water Company | | | | | |
| 1 | Well #1 | 1 222 | 1 370 | 0 689 | 0 842 |
| **2** | Well #2 | -- | 1 370 | 689 | 842 |
| | Total | 1 408 | 3 204 | 1 424 | 1 870 |

[1]Siasconset Water District Gravel Pack Well is not shown in figure 33 because drawdown is negligible

## Nantucket Island Basin

The Nantucket Island Basin is the smallest and least populated of the three basins Two comprehensive investigations of the hydrogeology of the basin have been undertaken (Walker, 1980, Horsley Witten Hegemann, Inc , 1990) Most of the following information is based on those investigations

Nantucket Island consists of nearly 1,500 ft of Cretaceous- and Tertiary-aged coastal-plain deposits overlying crystalline bedrock (Walker, 1980) The coastal-plain deposits are mantled by 150 to 250 ft of sand and gravel deposited by the Cape Cod Bay ice lobe (fig 3*B*) There are two principal physiographic regions on Nantucket Island (fig 7)—the moraine and the outwash plain The moraine consists of unsorted sand, gravel, silt, and clay that were deposited during the final readvancement of the Cape Cod Bay ice lobe The subsequent standstill and downwasting of the ice sheets in the area of the moraine account for its hummocky terrain, with altitudes greater than 100 ft The outwash plain consists of stratified sand and gravel deposited by the meltwaters of the retreating ice lobe

The ground-water-flow system of Nantucket Island can be divided into a shallow and a deep aquifer The upper 250 ft of sand and gravel outwash deposits constitutes the shallow aquifer The thick wedge of fine sand, silt, and clay of the underlying coastal-plain deposits contains the deep aquifer, which is poorly understood Walker (1980) reports a wide range in hydraulic conductivity for the coastal-plain deposits These hydraulic conductivity values generally are much lower than the hydraulic conductivity values for the glacial deposits

Nantucket Sound and the Atlantic Ocean completely surround Nantucket Island and form the outer boundary of the fresh ground-water-flow system Precipitation is the only source of freshwater to the shallow flow system Walker (1980) estimated that of the 44 in/yr of precipitation, 25 in are lost to evapotranspiration, resulting in 19 in of aquifer recharge Kohout and others (1977) explored the possibility that the deep aquifer is part of a larger ground-water-flow system that underlies Nantucket Sound and provides a hydraulic connection to remote recharge areas on Martha's Vineyard or Cape Cod The findings of the study proved to be inconclusive

A water-table contour map was made from water levels measured in observation wells between August 10 and 22, 1989, and reported by Horsley Witten Hegemann, Inc (1990) (fig 7) Ground water flows radially outward from the center of the island toward coastal discharge areas Local highs in the water table in the central part of the island may be caused by discontinuous lenses of low hydraulic conductivity in the moraine Ponds, streams, and

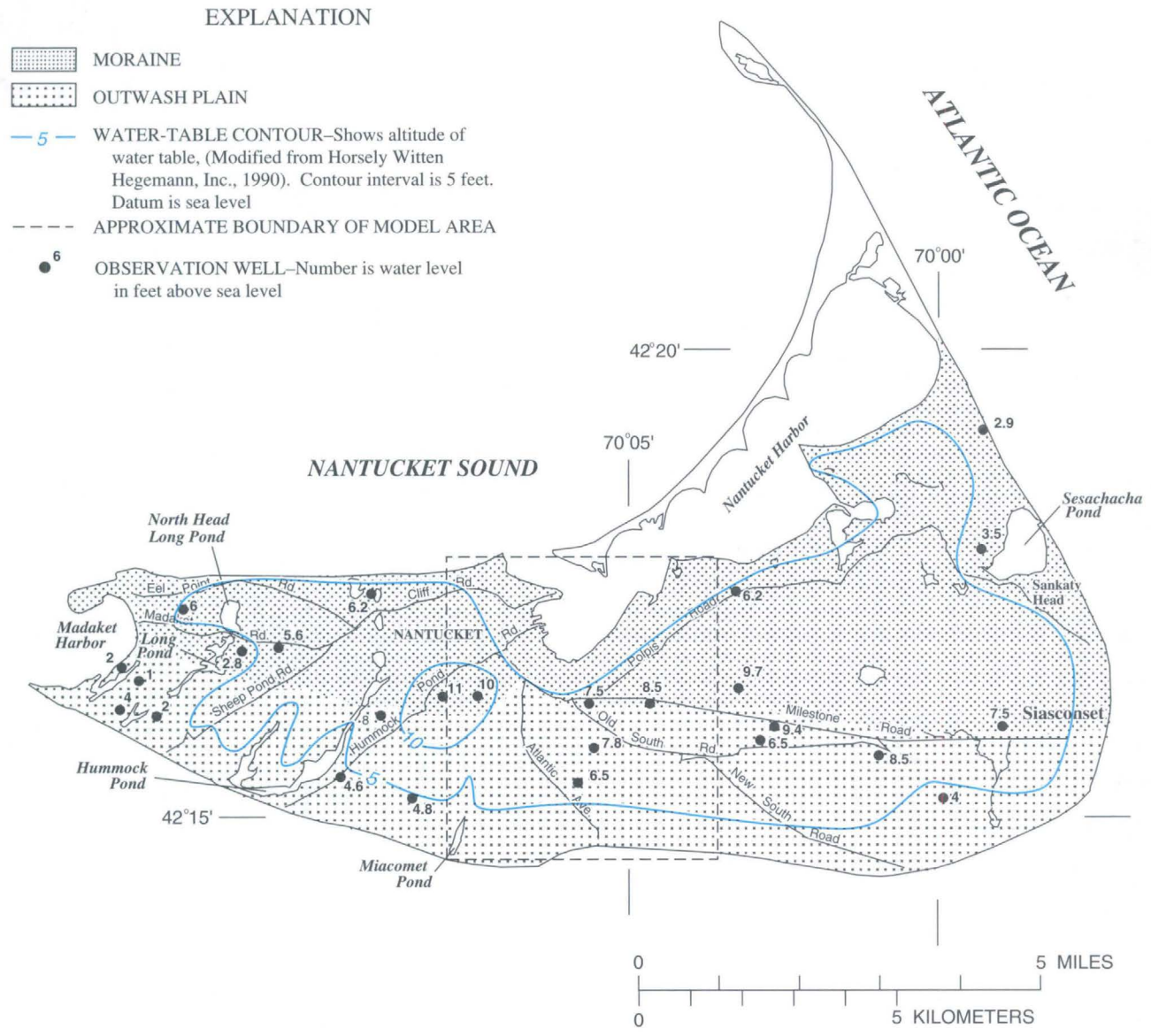


**Figure 7.** Water-table configuration on August 10–22, 1989, surficial geology, and location of modeled area, Nantucket Island Basin, Massachusetts.

swamps are surface-water expressions of the ground-water-flow system; perched water bodies present in the moraine may go dry in summer months.

The hydraulic properties of the shallow aquifer have been determined by analysis of aquifer tests made at public-supply wells and estimated from lithologic information. Walker (1980) reported a horizontal hydraulic conductivity of 970 ft/d and a specific yield of 0.25 for the shallow aquifer.

Ground water is the principal source of drinking water for the residents of Nantucket. Two public-water supply systems service nearly one-half the population of Nantucket. Average annual pumping rates from public-supply wells for 1989 and average annual and seasonal pumping rates projected by MOWR for the year 2020 are shown in table 6. Ground-water pumping increases nearly fourfold during the summer to accommodate the significant increase in seasonal population.

## EFFECTS OF SIMULATED GROUND-WATER PUMPING AND RECHARGE ON GROUND-WATER FLOW IN CAPE COD BASIN

This section describes the development and application of numerical flow models for the West Cape, East Cape, Eastham, Wellfleet, and Truro flow cells in the Cape Cod Basin Each flow system is conceptualized as a single, unconfined system that may be confined locally by units of low hydraulic conductivity, such as the unit of silt and clay that underlies Cape Cod Bay The entire thickness of the unconsolidated deposits of each flow cell is explicitly simulated in each model As discussed in the "Approach" section of this report, three different modeling strategies were used to analyze the effects of ground-water pumping and recharge on the ground-water-flow systems The analysis is divided into three sections, in which flow cells are grouped by similar modeling approaches the West Cape and East Cape flow cells are discussed first, followed by the Truro flow cell, and, finally, by the Eastham and Wellfleet flow cells

### West Cape and East Cape Flow Cells

The West Cape and East Cape flow cells are the largest flow cells on Cape Cod Flow models developed for the West Cape flow cell extend from the Cape Cod Canal eastward to the Bass River and from Cape Cod Bay southward to Nantucket Sound (fig 8) Flow models for the West Cape flow cell include the towns of Bourne, Sandwich, Falmouth, Mashpee, Barnstable, and most of Yarmouth Flow models developed for the East Cape flow cell adjoin the eastern edge of the West Cape flow models and extend from the Bass River in Dennis eastward to Town Cove and Rock Harbor in Orleans, and from the Cape Cod Bay southward to Nantucket Sound (fig 9) Flow models for the East Cape flow cell include a small part of Yarmouth and all of the towns of Dennis, Brewster, Harwich, Chatham, and Orleans

#### Modeling Approach

The analysis of changing stress conditions in the West Cape and East Cape flow cells consisted of three modeling phases Within each phase, a freshwater-saltwater flow model (SHARP model) was developed first, followed by a freshwater-flow model (MODFLOW model) The three phases are summarized as follows

First phase —Model data developed by Guswa and LeBlanc (1985) were used in the SHARP models of the first phase to determine an initial estimate of the location of the freshwater-saltwater interface and the rate of freshwater discharge to overlying saltwater zones of the aquifer for simulated predevelopment flow conditions through to those projected to occur in the year 2020 Details on recharge and pumping conditions used in the simulations are discussed in the section "Stresses and Stress Periods " The boundary conditions along the freshwater-saltwater interface calculated by the SHARP models then were used as boundary conditions in the MODFLOW models of the first phase Details on the incorporation of the freshwater-saltwater interface into the boundary conditions of the MODFLOW models are discussed in the section "Boundary Conditions " Next, model data developed by Guswa and LeBlanc (1985) that related to the hydraulic properties of the aquifer—including horizontal hydraulic conductivity, vertical conductance, and the contact between unconsolidated deposits and bedrock—were modified to reflect data made available since the work of Guswa and LeBlanc (1985) The MODFLOW models then were calibrated In the calibration process, initial estimates of hydraulic conductivity and vertical conductance were adjusted within reason such that model-calculated ground-water levels, pond levels, and streamflow were approximately equal to measured values Details on the calibration process are discussed in the section "Calibration "

Second phase —The same stress conditions used in the first phase were simulated with the SHARP models using the calibrated model hydraulic property data sets of the first phase These simulations provided a second estimate of boundary conditions along the freshwater-saltwater interface that was used in a second phase of MODFLOW simulations Model hydraulic property data sets were modified slightly in the MODFLOW models in a second calibration process using these updated freshwater-saltwater interface conditions

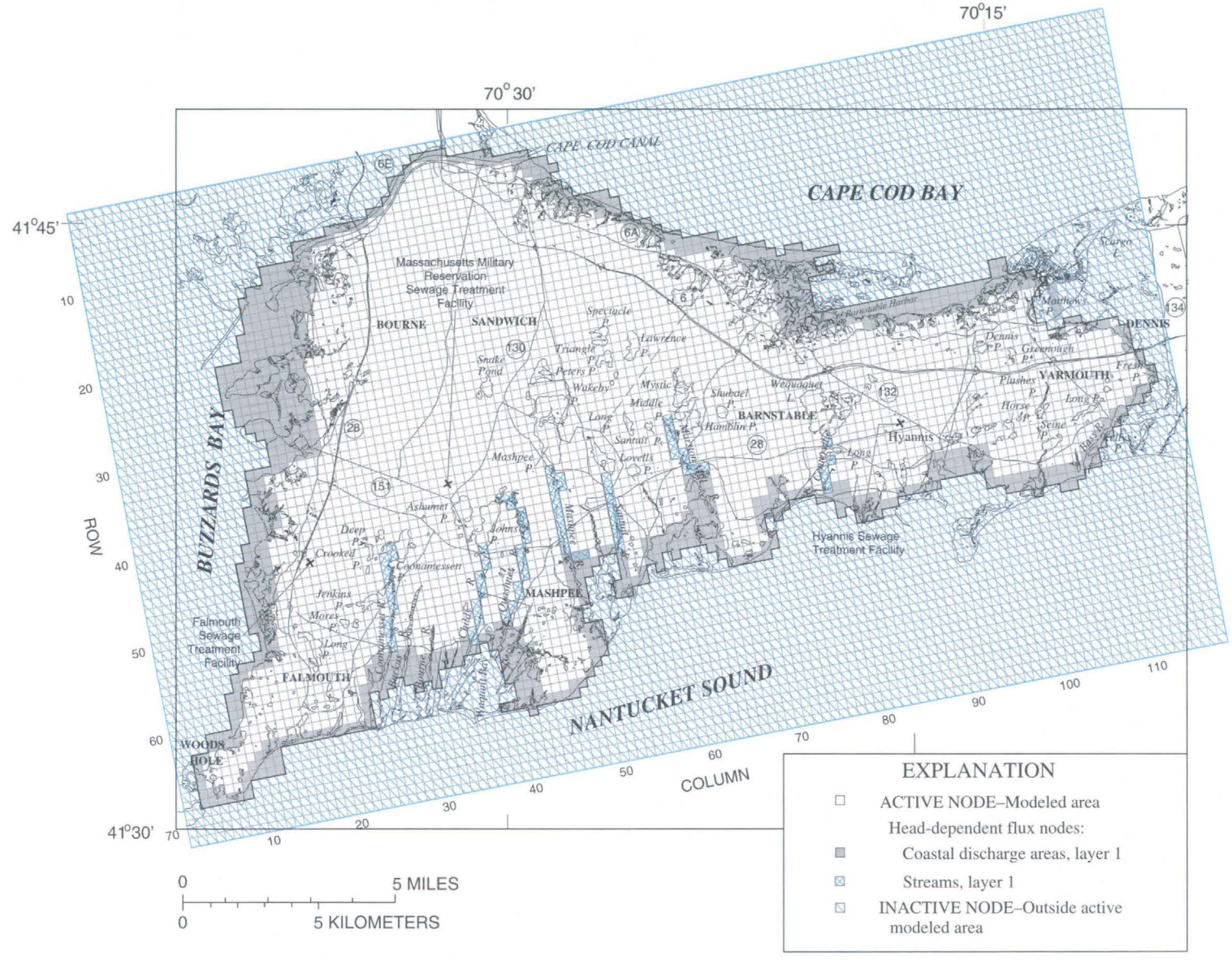


**Figure 8.** Grid and boundary conditions for the West Cape flow cell, Cape Cod Basin, Massachusetts.

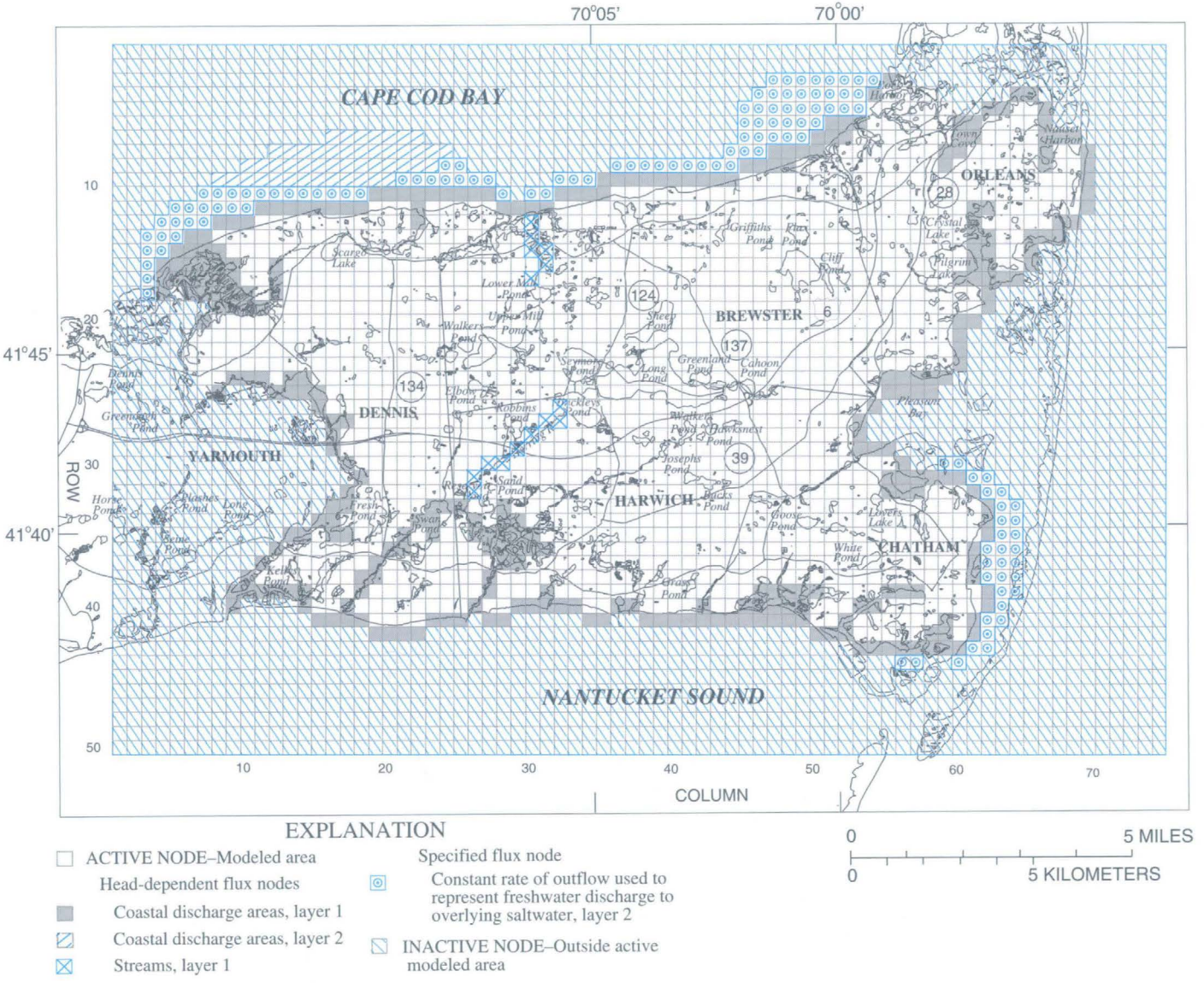


**Figure 9.** Grid and boundary conditions for the East Cape flow cell, Cape Cod Basin, Massachusetts.

Third phase —Calibrated model hydraulic property data sets of the second phase were simulated for a third time using the SHARP models The location of the freshwater-saltwater interface changed little between the second and third phases because changes made to the hydraulic property data sets in the second phase were minor Consequently, the SHARP and MODFLOW models developed in the second phase were considered to be calibrated models and were used in the final analysis of the flow system

The SHARP model assumes that freshwater and saltwater zones of each aquifer are immiscible fluids separated by a sharp interface, an assumption that implies that no transition zone separates the freshwater- and saltwater-flow systems This sharp-interface approximation is appropriate for the analysis of regional flow systems in which the width of the transition zone is very small relative to the total saturated thickness of the aquifer A sharp-interface approximation was assumed for the West Cape and East Cape flow cells because the thickness of the transition zone measured at 11 observation wells in the flow cells ranges from about 3 to 11 percent of the total saturated thickness of the aquifer, and the mean thickness of the zone of transition at these 11 wells is 8 percent of the total saturated thickness (LeBlanc and others, 1986)

## Description of Models

### Grids

The finite-difference grids of the two flow cells consist of uniform nodes 1,320 by 1,320 ft These nodes are one-quarter the size of those used by Guswa and LeBlanc (1985) and were necessary to improve the numerical stability of the freshwater-saltwater flow models (Essaid, 1990, p 54) The West Cape grid consists of 72 rows and 118 columns (fig 8), the East Cape grid consists of 50 rows and 74 columns (fig 9)

Each model consists of five layers (table 7), and the definition of layers in each model is similar to that used by Guswa and LeBlanc (1985) The bottom altitudes of model nodes in the top layer of each of the MODFLOW models that are coincident with ponds equal to or greater than one model node in size (about

**Table 7.** Vertical layering, horizontal hydraulic conductivity, and vertical conductance of calibrated models of the Cape Cod Basin, Massachusetts

[The SHARP model used in the Truro flow cell numbering convention shows model layer 7 as the top layer --, vertical conductance was not specified for the bottom layer]

| Model layer | Maximum depth of layer, in feet below sea level | Horizontal hydraulic conductivity, in feet per day | Vertical conductance, in $day^{-1}$ |
|---|---|---|---|
| **West Cape Flow Cell** | | | |
| 1 | 20 | [1]3–250 | 0 0005–0 080 |
| 2 | 70 | 1–200 | 0 001–0 068 |
| 3 | 140 | 1–150 | 0 001–0 280 |
| 4 | 240 | 10–125 | 0 0001–0 034 |
| 5 | 500 | 1–30 | -- |
| **East Cape Flow Cell** | | | |
| 1 | 20 | [1]3–250 | 0 00002–0 8000 |
| 2 | 70 | 1–150 | 0 0013–0 044 |
| 3 | 140 | 10–75 | 0 002–0 060 |
| 4 | 240 | 10–75 | 0 00001–0 050 |
| 5 | 500 | 1–30 | -- |
| **Truro Flow Cell** | | | |
| 1 | 900 | 75 | -- |
| 2 | 280 | 75 | 0 0125 |
| 3 | 200 | 75 | 0 0180 |
| 4 | 140 | 75 | 0 015–0 021 |
| 5 | 80 | 50–150 | 0 03–0 08 |
| 6 | 40 | 150–200 | 0 08–0 48 |
| 7 | 10 | 200–350 | 0 36–0 50 |
| **Eastham Flow Cell** | | | |
| 1 | 10 | [1]100–150 | 0 005–1 0 |
| 2 | 35 | 10–100 | 0 001–1 0 |
| 3 | 60 | 0 001–100 | 0 0001 |
| 4 | 90 | 0 001–100 | 0 00001 |
| **Wellfleet Flow Cell** | | | |
| 1 | 10 | [1]50–350 | 0 4–1 870 |
| 2 | 40 | 200–250 | 0 003–1 515 |
| 3 | 80 | 10–200 | 0 003–0 286 |
| 4 | 140 | 10–100 | 0 003–0 167 |
| 5 | 200 | 100 | 0 1430 |
| 6 | 280 | 100 | -- |

[1]Grid nodes underlying ponds represented in the models were assigned a horizontal hydraulic conductivity of 50,000 feet per day

40 acres) were set equal to measured pond-bottom altitudes where known (McCann, 1969) The bottom of each model is the contact between unconsolidated deposits and bedrock, bedrock altitudes have been mapped in the two flow cells from available lithologic and seismic-refraction data by B D Stone (U S Geological Survey, written commun , 1990)

### Hydraulic Properties

Horizontal hydraulic conductivity and vertical conductance (referred to as vertical leakance in the freshwater-saltwater models) were determined for the models by comparing hydrogeologic sections shown in plate 1 to values of hydraulic conductivity generalized for individual grain sizes from the results of the aquifer-test analyses shown in tables 1 and 2 The generalized values of horizontal hydraulic conductivity used were 40 ft/d for fine sand and silt, 150 ft/d for fine sand, 220 ft/d for fine-medium sand, and 350 ft/d for medium-coarse sand and gravel Fine silt and clay were assigned a horizontal hydraulic conductivity of $1\times10^{-3}$ ft/d, which is the average value reported by Barlow (1994) based on permeameter tests of fine silt and clay samples from the town of Eastham The generalized values of the ratio of vertical to horizontal hydraulic conductivity were 1 30 for very fine and fine sand and 1 5 for medium sand to gravel Vertical conductance between vertically adjacent nodes was calculated following McDonald and Harbaugh (1988, p 5–11), this conductance is based on layer thicknesses and vertical hydraulic conductivities

Zones of lithologically similar deposits were defined using the hydrogeologic sections and geologic maps of the study area (plate 1 of this report, Oldale and Barlow, 1986) Uniform values of horizontal hydraulic conductivity and vertical conductance were used for each zone Calibrated values of horizontal hydraulic conductivity and vertical conductance that were determined in the second phase of model calibration, are summarized in table 7 In general, calibrated model values are less than initial estimates

A uniform specific yield of 0 15, based on the results of the aquifer-test analyses reported in table 2, was specified in the models where unconfined conditions exist A uniform value of the storage coefficient of $2\times10^{-4}$, based on the work of Barlow and Hess (1993), was used for both flow models where confined conditions exist Porosity, which is required for the SHARP models, was set equal to 0 30

### Boundary Conditions

#### Freshwater-Saltwater Flow Models

The models of the West Cape and East Cape flow cells are bounded on top by the water table, which is a free-surface boundary that receives spatially and temporally variable rates of recharge Recharge rates specified for the models are discussed in the next section, "Stresses and Stress Periods " The lower boundary of each model was the contact between the unconsolidated glacial sediments and underlying bedrock, which was assumed to be impermeable Each model layer is bounded laterally by inactive nodes that separate modeled areas from unmodeled areas (figs 8 and 9)

Head-dependent flux boundary conditions were used to simulate saltwater discharge areas The head values specified at saltwater discharge boundaries are equivalent freshwater heads equal to the height of the column of saltwater overlying the seabed at the discharge boundary divided by 40 0, which is the ratio of the specific weight of freshwater (1 000 g/cm$^3$) to the difference between the specific weight of saltwater (1 025 g/cm$^3$) and freshwater (Essaid, 1990) The height of the column of saltwater in each node was determined from bathymetric maps of the area Head-dependent flux boundaries are specified at saltwater discharge areas in the top layer in the West Cape flow cell and in the top two layers in the East Cape flow cell A vertical leakance of 20 day$^{-1}$ for the seabed deposits was used in the simulations, which assumes a 1-foot-thick sediment deposit on the seabed with a vertical hydraulic conductivity of 20 ft/d This value of the vertical leakance of seabed deposits also was used by Guswa and LeBlanc (1985) in their models of the area

Streams also were modeled as head-dependent flux boundaries Heads specified at stream boundaries were estimated from topographic maps or, where available, from survey data Streambed conductance values were estimated from available data and modified during model calibration to match measured streamflow Streams represented in the models are the Coonamessett, Childs, Quashnet, Mashpee, Santuit, Centerville, and Marston Mill in the West Cape flow cell, and the Herring River and Stony Brook in the East Cape flow cell (figs 4, 5, 8, and 9)

### Freshwater-Flow Models

Several boundary conditions used for the freshwater-flow models were the same as those used for the freshwater-saltwater models. These include (1) recharge to the water table; (2) the contact between unconsolidated deposits and bedrock; and (3) the use of head-dependent flux boundaries at saltwater discharge areas, in which the heads specified at these boundaries were equivalent freshwater heads.

Boundary conditions at the freshwater-saltwater interface calculated by the SHARP models. were incorporated into the MODFLOW freshwater models. Figure 10 illustrates how these boundary conditions were incorporated into the MODFLOW models. Grid nodes determined to be completely filled with saltwater by the SHARP models were made inactive in the MODFLOW models. Grid nodes determined to be partially filled with saltwater by the SHARP models (called mixed nodes) were reduced in total thickness in the MODFLOW models to account for those parts of the aquifers containing saltwater. Coastal discharge of freshwater to the saltwater zones of the aquifer in the top layer of the model was simulated as a head-dependent flux boundary, as described in the previous section "Freshwater-Saltwater Flow Models." Subsea discharge of freshwater to overlying saltwater zones of the simulated aquifer in the bottom four layers of the SHARP models was removed from the MODFLOW models by creating freshwater sinks where this discharge occurred; these sinks were simulated using pumping wells. The pumping rate of each of these

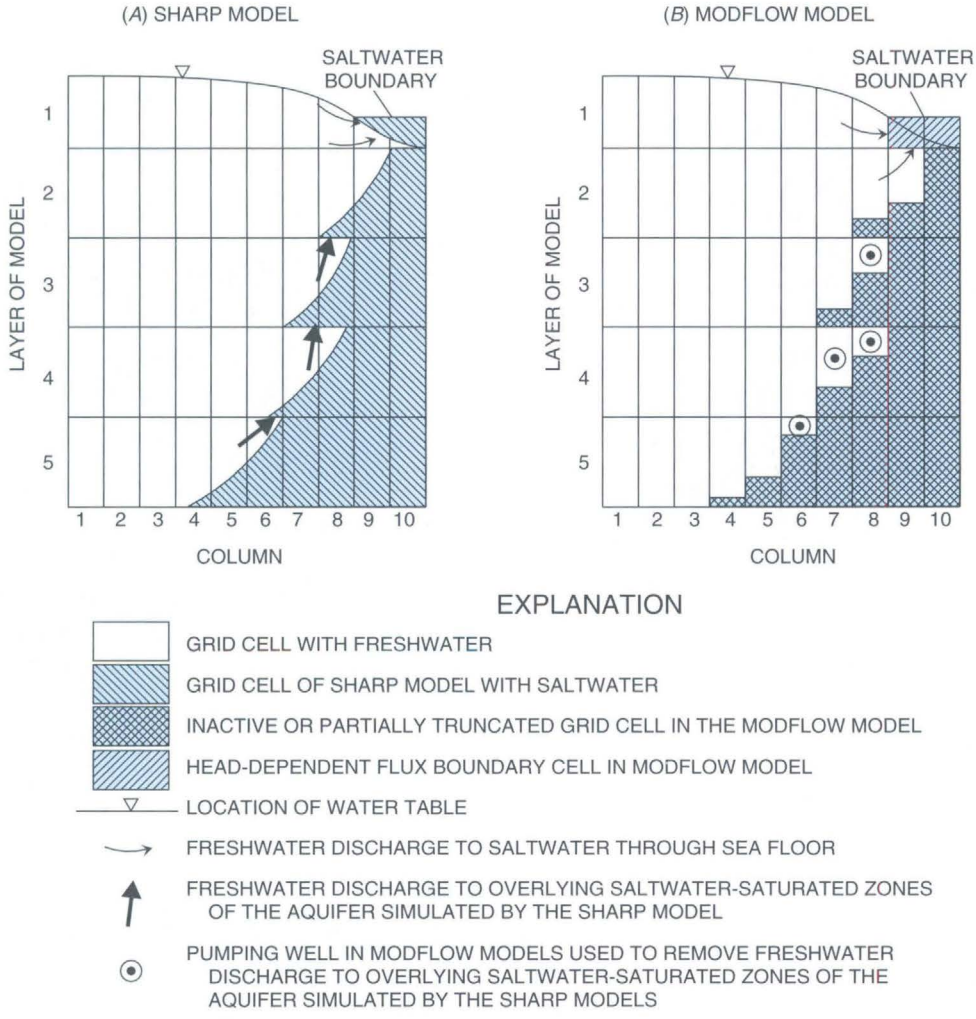


**Figure 10.** Incorporation of hydraulic dynamics along the freshwater-saltwater boundary as represented in the SHARP model (*A*) into the MODFLOW model and (*B*) for the West Cape and East Cape flow cells, Cape Cod Basin, Massachusetts.

wells was set equal to the model-calculated rate of freshwater discharged to the overlying zone containing saltwater The total model-calculated rate of freshwater discharge removed from the models by these wells was 4 percent of recharge for the West Cape model and 11 percent of recharge for the East Cape model

Freshwater streams were simulated using head-dependent flux boundaries Streams were simulated such that they could receive only ground-water discharge (that is, as gaining streams), water could not flow from simulated streams to the underlying aquifer Streams represented in the models were the same as those represented in the SHARP models (the Coonamessett, Childs, Quashnet, Mashpee, Santuit, Marston Mill, and Centerville in the West Cape flow cell and the Herring River and Stony Brook in the East Cape flow cell)

Freshwater ponds greater than 40 acres (1 model node) were represented as zones of high horizontal hydraulic conductivity (50,000 ft/d) with a specific yield equal to 1 0 The high value of horizontal hydraulic conductivity caused calculated hydraulic gradients in the ponds to be nearly zero, which is consistent with pond surfaces, the high value of specific yield caused pond levels to fluctuate less than heads in the surrounding aquifers because of the greater storage capacity of the ponds than of the surrounding aquifer

### Stresses and Stress Periods

Ground-water flow in the West Cape and East Cape flow cells was simulated using the SHARP models for predevelopment conditions and a 71-year stress condition that corresponds to 1950–2020 First, a simulation was completed in which heads throughout the ground-water-flow system were calculated for conditions of no pumping (predevelopment flow conditions)—conditions that are assumed to have occurred until 1950 These conditions were simulated for long time periods, until the change in storage in each model layer was virtually 0 This simulation was completed to determine heads throughout the flow cells that were then used as initial conditions for the 71-year stress-condition simulation This simulation also provided heads and freshwater-saltwater interface conditions to compare with the effects of development

Once the predevelopment heads and interface locations were calculated, a simulation for 1950–2020 was completed The 71-year period was divided into three stress periods 1950–81, 1982–2004, and 2005–2020 The stress period 1950–81 used pumping and recharge conditions of 1975–76, the stress period 1982–2004 used pumping and recharge conditions of 1989, and the stress period 2005–2020 used pumping and recharge conditions projected by MOWR to occur in the year 2020 These three pumping and recharge stress conditions are approximations of the actual stress conditions that have increased, and are assumed to continue to increase, incrementally with time These approximations were assumed to be sufficient for a regional analysis of changing stress conditions Public-supply well locations and pumping rates for 1975 and 1989 and those projected for the year 2020, which were used in the models, are summarized in tables 3 and 4

Once the 71-year stress condition simulation was completed, approximate steady-state conditions were simulated using the projected 2020 pumping and recharge rates The approximate steady-state distribution of heads and the freshwater-saltwater interface positions in each flow cell for the projected 2020 stress conditions was assumed to occur whenever calculated changes in storage within each model were negligible

Recharge to the flow cells consists of precipitation and wastewater-return flow from sources such as septic systems and wastewater-treatment facilities Annual precipitation rates were estimated to be about 45 and 43 in in the West Cape and East Cape flow cells, respectively (LeBlanc and others, 1986) Guswa and LeBlanc (1985) used the method of Thornthwaite and Mather (1957) to estimate recharge from precipitation for each of the flow systems The annual precipitation recharge rates estimated by Guswa and LeBlanc (1985) were not changed during this investigation, with the exception that recharge was reduced beneath ponds to account for free-water-surface potential evaporation, which is estimated to be 28 in/yr on Cape Cod (Farnsworth and others, 1982, pl 1) The free-water-surface potential evaporation was subtracted from annual precipitation rates in each flow cell to obtain net recharge rates of 17 and 15 in/yr beneath ponds in the West Cape and East Cape flow cells, respectively Finally, precipitation recharge rates were reduced by Guswa and LeBlanc (1985) in low-lying areas near the coast

to account for evapotranspiration that occurs where the water table is near the land surface Precipitation recharge rates used in the West Cape and East Cape models ranged from 6 to 22 in/yr, and were applied to the top layer of each model

Wastewater-return flow from septic systems and wastewater-treatment facilities was estimated for each node of the top layer of each model Wastewater-return flow from septic systems for the 1975, 1989, and 2020 stress periods was determined from maps of distribution lines for public-water supply and the average daily rate of water supplied to unsewered areas by water companies, according to

$$R_T = \left[\frac{Q_{wd}}{A_{node}} \times \frac{L_{node}}{L_{total}}\right] \times 0.9, \qquad (4)$$

where

$R_T$ is the return flow recharge (feet per day),

$Q_{wd}$ is the average daily rate of water distributed to unsewered areas by the water supplier in 1975, 1989, or 2020, in cubic feet per day,

$A_{node}$ is the area of the grid node, in square feet,

$L_{node}$ is the length of roads in unsewered areas in the node that are served by the water supplier, in feet, and

$L_{total}$ is the total length of roads in unsewered areas that are served by the water supplier, in feet

Ten percent of the water supplied to unsewered areas was assumed to be lost by consumptive use Recent data (M A Horn, U S Geological Survey, oral commun , 1992), however, suggest that this value of consumptive use may be low, and that a more realistic value of consumptive use would be 15 percent For the total recharge rate to each node of the two models, the difference between using a value of 10 percent and using a value of 15 percent is small (a maximum error of 1 5 percent of total recharge to each node), and therefore the estimate of 10 percent is acceptable for this investigation

Water discharged to sewers is returned to the aquifers through infiltration beds at three wastewater-treatment facilities in the West Cape flow cell (the Massachusetts Military Reservation facility in Sandwich, the Hyannis facility in Barnstable, and the Falmouth facility) and at a single facility (the Chatham facility) in the East Cape flow cell Daily rates of wastewater infiltration at these four facilities were obtained from facilities personnel, the average daily volume of wastewater at each facility was distributed evenly to the grid nodes that contain the infiltration beds Wastewater infiltration at these four facilities in 1989 was about 9 percent of the total rate of public-water supplies pumped from the flow cells

Recharge from precipitation is assumed to remain constant throughout the simulation period Simulated wastewater-return flow, however, was specified based on a percentage of the rate of simulated ground-water pumpage As a result, simulated recharge rates increased as simulated ground-water pumping rates increased with time

### Calibration

The West Cape and East Cape freshwater-flow models were calibrated during the first and second phases of the modeling analysis by comparison of model-calculated water levels to measured water levels at 62 well and pond locations in the West Cape flow cell and 49 well and pond locations in the East Cape flow cell (figs 11 and 12) Comparison also was made of model-calculated streamflow to measured streamflow for the Childs, Coonamessett, Mashpee, and Quashnet Rivers in the West Cape flow cell and the Herring River and Stony Brook in the East Cape flow cell Average water levels for 1963–76 for 32 observation wells in the West Cape flow cell and 19 observation wells in the East Cape flow cell were compared to water levels calculated by the MODFLOW models for 1975 Average water levels for 1963–76 were compared to model-calculated water levels for 1975 because (1) measured water-level information was not available for predevelopment flow conditions, (2) this time period is consistent with that used for model calibration by Guswa and LeBlanc (1985), and (3) the water levels measured in these observation wells in 1975 are representative of long-term average water levels reported for the observation wells for the 1963–76 period (Letty, 1984) Pond levels calculated by the MODFLOW models for 1975 were compared to those determined from topographic maps of the area Pond levels reported on the maps were assumed to be consistent with 1975 water levels Model-calculated streamflow for 1975 and 1989 were compared to period-of-record averages for the Quashnet and Herring Rivers, model-calculated streamflows for 1989 were compared to single discharge measurements made for the Coonamessett and Childs Rivers in August 1989 during a period of near-average hydrologic conditions (Barlow and Hess, 1993)

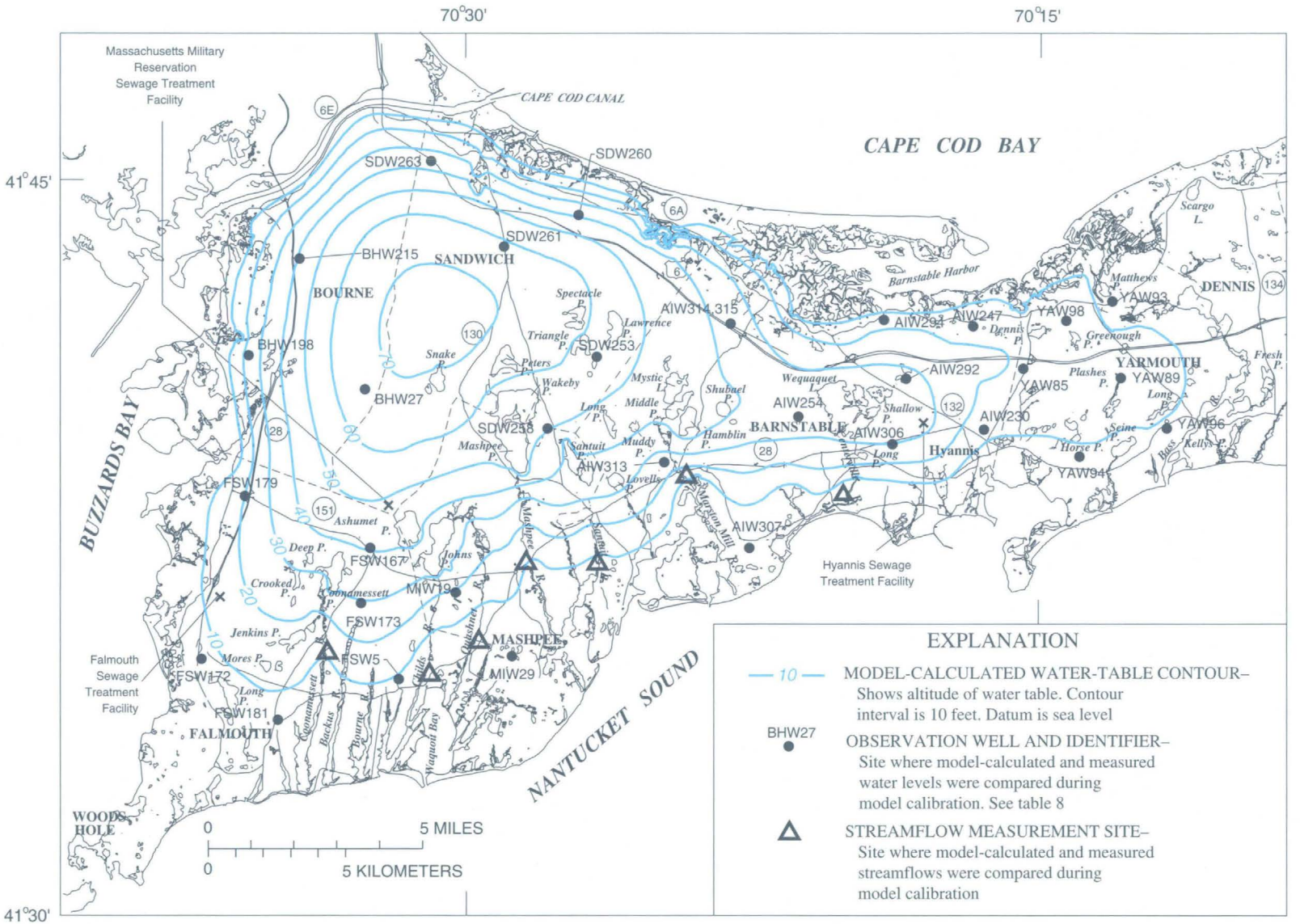


**Figure 11.** Model-calculated water-table configuration for the West Cape flow cell, Cape Cod Basin, Massachusetts, 1989.

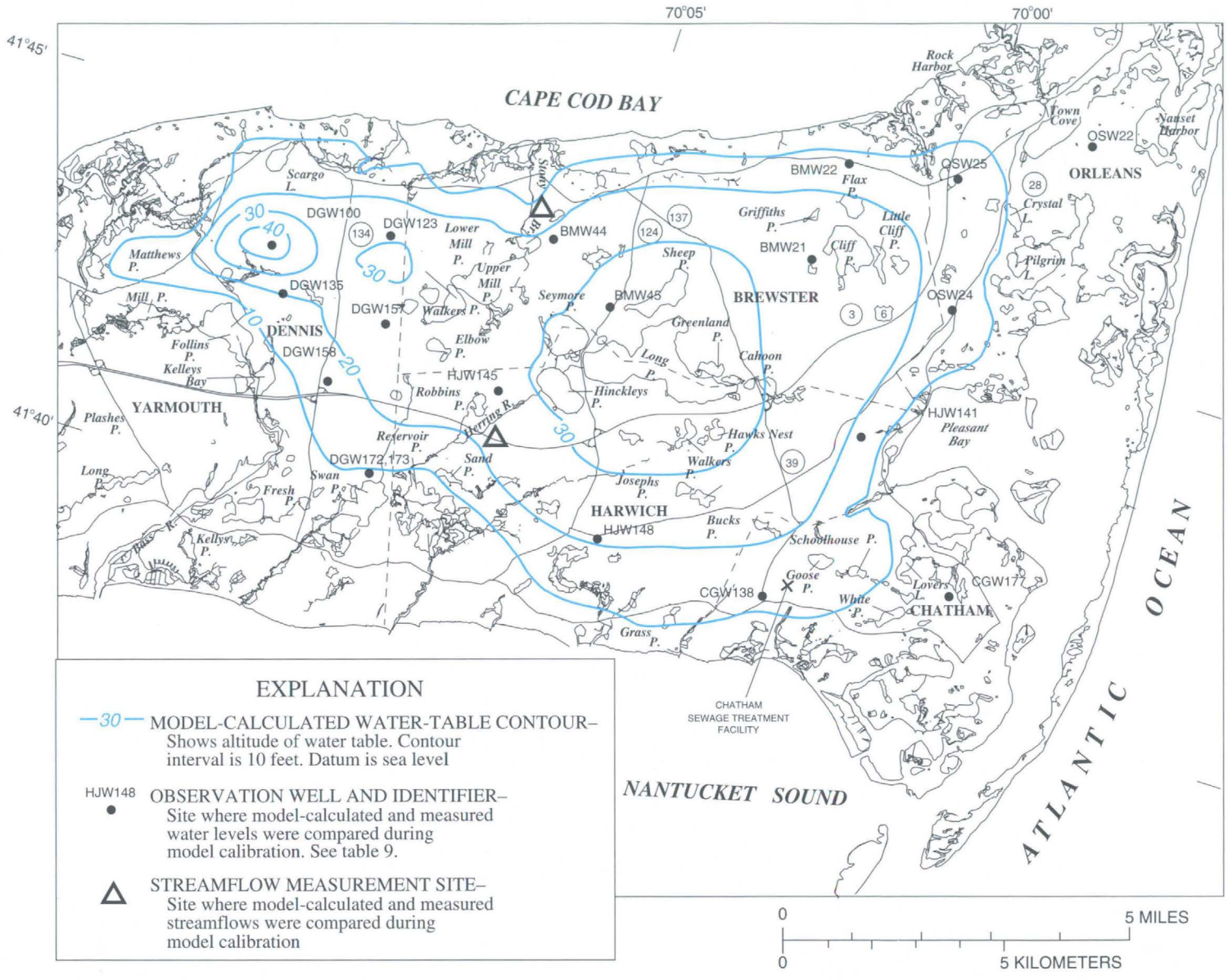


**Figure 12.** Model-calculated water-table configuration for the East Cape flow cell, Cape Cod Basin, Massachusetts, 1989.

Data reported by LeBlanc and others (1986) on the vertical location and shape of the freshwater-saltwater interface at 16 zone-of-transition observation wells in the West Cape and East Cape flow cells also were used to compare model-calculated with measured freshwater-saltwater interface locations The data reported by LeBlanc and others (1986) consist of chloride concentrations of water samples collected at different depths at the zone-of-transition wells Marsily (1986) reports that the $10^4$ mg/L isochlor generally is used as the demarcation between freshwater- and saltwater-flow systems

The SHARP model assumes a sharp interface between freshwater and saltwater Each node simulated in the SHARP model can consist completely of freshwater or saltwater, or can consist of freshwater in the top part of the node separated by the interface from saltwater in the bottom part of the node

Eight zone-of-transition wells were used for comparison in each flow cell Interface locations calculated by the SHARP models for 1975 were compared to measured data collected between January 1976 and April 1977, results for four representative wells in each flow cell are shown in figure 13 The measured data generally indicate that chloride concentrations increase with depth, a finding that is indicative of the transition between freshwater- and saltwater-flow systems The calculated vertical distribution of freshwater and saltwater at the 16 well sites generally agreed well with data presented by LeBlanc and others (1986) and is considered adequate for the purpose of this investigation

Initial estimates of horizontal hydraulic conductivity and vertical conductance were adjusted within reasonable limits during model calibration Generally, agreement between model-calculated and measured water levels at observation wells and ponds is close (tables 8–11) The mean error between model-calculated water levels for 1975 and measured average water levels in observation wells (which are known more accurately than pond levels) was 2 0 ft or 3 percent of the total relief of the water table for the West Cape flow cell (table 8), and 2 2 ft or 5 percent of the total relief of the water table in the East Cape flow cell (table 9)

Streambed leakances and the heads specified for streams were modified within reasonable limits during model calibration from those used in the freshwater-saltwater models to match measured streamflow Model-calculated streamflow for the Quashnet River at the site of the streamflow-gaging station was 13 6 $ft^3/s$ for 1975 and 13 5 $ft^3/s$ for 1989 (table 12), which compares favorably with the 1989–91 period-of-record daily mean streamflow of 13 8 $ft^3/s$ (Barlow and Hess, 1993) Model-calculated streamflow for the Herring River was 4 3 $ft^3/s$ for 1975 and 4 0 $ft^3/s$ for 1989, and was less than one-half of the period-of-record daily mean streamflow of 10 0 $ft^3/s$, the discrepancy likely is due to the fact that a significant part of the streamflow in the Herring River results from the overflow of water to the river from Hinckleys Pond, which was not simulated in the model Joseph Bergin (oral commun , 1992) of the Massachusetts Division of Fisheries and Wildlife estimated that nearly one-half of the discharge measured at the Herring River gaging station is due to overflow at the Hinckleys Pond spillway, an estimate that is consistent with that calculated by the model The model-calculated values of 4 3 and 4 0 $ft^3/s$ represent ground-water discharge to the river that occurs between the outlet of the pond and the site of the streamflow-gaging station Model-calculated 1989 streamflow for the Coonamessett and Childs Rivers at their points of measurement are 9 2 and 5 2 $ft^3/s$, respectively, these values compare reasonably well with the measured values in August 1989 of 8 2 and 6 0 $ft^3/s$, respectively August 1989 measurements for these rivers were assumed to be at near average conditions because the nearby Quashnet River was at near average conditions at the continuous-streamflow gaging station on the same date

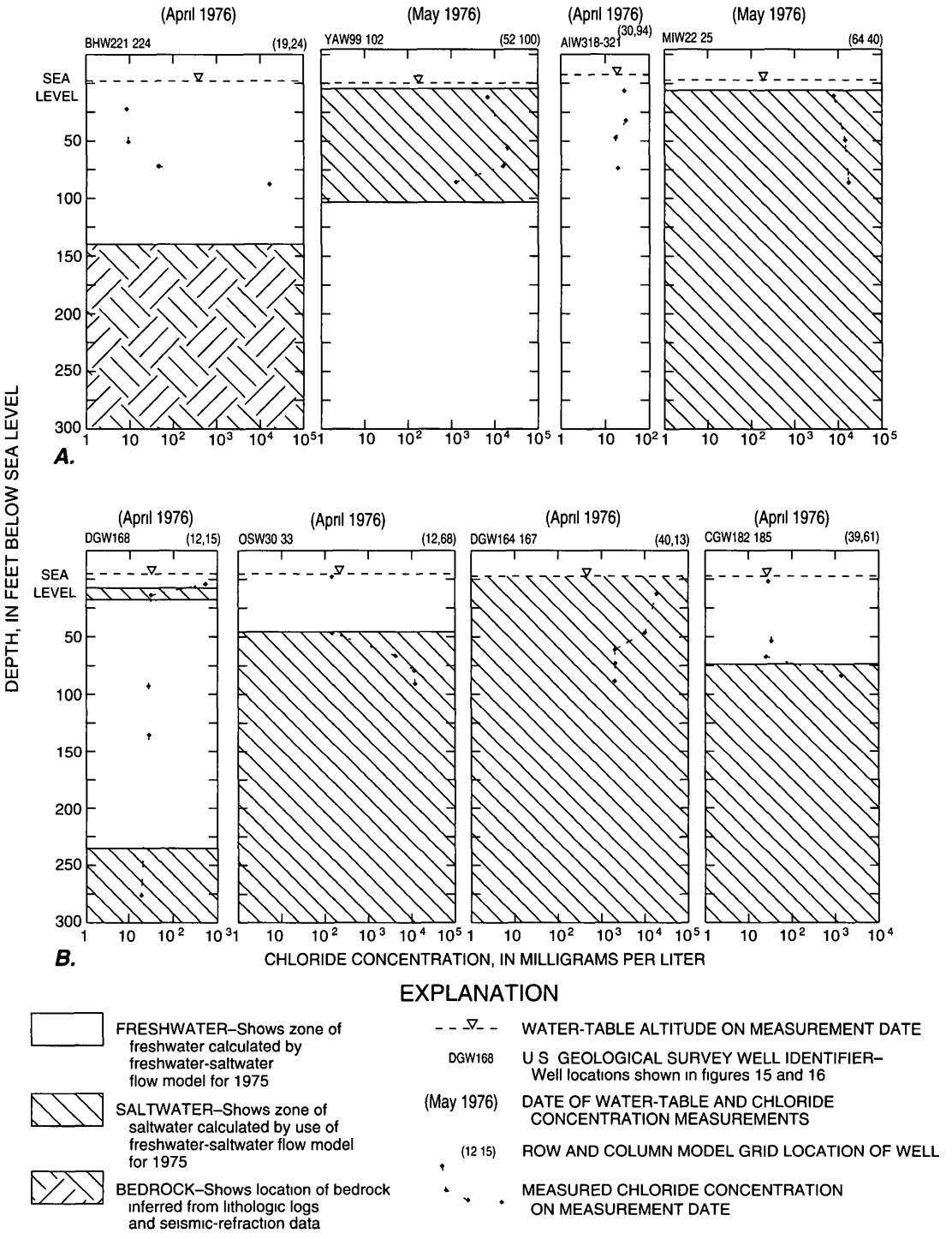


**Figure 13** Measured chloride concentrations and model-calculated freshwater and saltwater zones at selected observation wells in the (*A*) West Cape and (*B*) East Cape flow cells, Cape Cod Basin, Massachusetts

**Table 8** Average measured water levels for selected wells, 1963–76, and model-calculated water levels for 1975 and 1989, and 2020 for the West Cape flow cell, Cape Cod Basin, Massachusetts

[Location of wells shown in figure 11 Change in model-calculated water level between stress periods +, increase in the water level resulting from changing stress conditions, otherwise, a change is a decrease in water level]

| Well No | Model node | | | Average measured water level, in feet above sea level | Model-calculated water levels, in feet above sea level | | | | Change in model-calculated water level between stress periods, in feet | | |
|---|---|---|---|---|---|---|---|---|---|---|---|
| | | | | | | Year | | | | | |
| | Layer | Row | Column | | Predevelopment | 1975 | 1989 | 2020 | Predevelopment–1975 | 1975–1989 | 1989–2020 |
| A1W230 | 1 | 42 | 91 | 18 4 | 19 1 | 17 9 | 17 0 | 12 5 | 1 2 | 0 9 | 4 5 |
| A1W247 | 1 | 33 | 92 | 20 3 | 18 9 | 18 4 | 17 8 | 15 3 | 5 | 6 | 2 5 |
| A1W254 | 1 | 39 | 74 | 34 9 | 34 9 | 35 1 | 35 4 | 35 3 | +2 | +3 | 1 |
| A1W292 | 1 | 37 | 85 | 34 1 | 32 9 | 33 4 | 33 2 | 31 9 | +5 | 2 | 1 3 |
| A1W294 | 2 | 31 | 84 | 18 1 | 13 2 | 13 2 | 12 8 | 12 1 | 0 | 4 | 7 |
| A1W306 | 1 | 43 | 83 | 27 2 | 26 3 | 26 7 | 27 4 | 26 8 | +4 | +7 | 6 |
| A1W307 | 1 | 49 | 68 | 5 0 | 5 0 | 4 4 | 4 8 | 3 7 | 6 | +4 | 1 1 |
| A1W313 | 1 | 40 | 62 | 27 5 | 24 2 | 24 2 | 24 0 | 23 7 | 0 | 2 | 3 |
| A1W314 | 3 | 29 | 70 | 32 5 | 37 2 | 37 2 | 36 9 | 35 5 | 0 | 3 | 1 4 |
| A1W315 | 1 | 29 | 70 | 32 5 | 37 2 | 37 3 | 36 9 | 35 6 | +1 | 4 | 1 3 |
| BHW27 | 2 | 28 | 36 | 66 0 | 66 5 | 66 1 | 66 0 | 65 5 | 4 | 1 | 5 |
| BHW198 | 1 | 23 | 26 | 22 0 | 23 7 | 23 2 | 23 2 | 22 4 | 5 | 0 | 8 |
| BHW215 | 1 | 15 | 33 | 46 0 | 44 5 | 44 2 | 44 1 | 43 4 | 3 | 1 | 7 |
| FSW5 | 2 | 55 | 34 | 6 2 | 8 5 | 8 5 | 7 9 | 7 9 | 0 | 6 | 0 |
| FSW167 | 1 | 42 | 34 | 41 3 | 41 7 | 42 0 | 41 9 | 41 9 | +3 | 1 | 0 |
| FSW172 | 2 | 50 | 17 | 7 9 | 9 2 | 6 7 | 7 5 | 6 4 | 2 5 | +8 | 1 1 |
| FSW173 | 2 | 47 | 32 | 29 8 | 29 4 | 29 5 | 29 0 | 29 0 | +1 | 5 | 0 |
| FSW179 | 1 | 35 | 24 | 25 3 | 21 7 | 21 7 | 22 0 | 21 9 | 0 | +3 | 1 |
| FSW181 | 1 | 56 | 22 | 5 7 | 7 9 | 6 1 | 6 6 | 5 7 | 1 8 | +5 | 9 |
| MIW19 | 1 | 48 | 41 | 26 9 | 18 7 | 18 7 | 18 7 | 18 5 | 0 | 0 | 2 |
| MIW29 | 2 | 55 | 45 | 6 4 | 4 4 | 4 4 | 4 3 | 4 1 | 0 | 1 | 2 |
| SDW253 | 1 | 29 | 58 | 61 4 | 57 3 | 57 4 | 56 5 | 53 6 | +1 | 9 | 2 9 |
| SDW258 | 1 | 35 | 52 | 53 7 | 50 0 | 50 2 | 49 9 | 48 9 | +2 | 3 | 1 0 |
| SDW260 | 1 | 16 | 58 | 41 0 | 41 4 | 41 0 | 40 2 | 38 8 | 4 | 8 | 1 4 |
| SDW261 | 2 | 18 | 51 | 63 8 | 63 9 | 63 8 | 63 2 | 62 1 | 1 | 6 | 1 1 |
| SDW263 | 1 | 8 | 46 | 36 7 | 33 9 | 33 7 | 33 6 | 33 4 | 2 | 1 | 2 |
| YAW85 | 1 | 38 | 96 | 22 7 | 21 5 | 20 4 | 19 2 | 15 2 | 1 1 | 1 2 | 4 0 |
| YAW89 | 1 | 40 | 104 | 18 0 | 17 4 | 16 5 | 16 5 | 15 5 | 9 | 0 | 1 0 |
| YAW93 | 2 | 34 | 105 | 9 1 | 14 6 | 14 5 | 14 1 | 12 9 | 1 | 4 | 1 2 |
| YAW94 | 1 | 47 | 99 | 7 7 | 7 5 | 7 6 | 7 3 | 6 7 | +1 | 3 | 6 |
| YAW96 | 1 | 46 | 108 | 5 2 | 8 6 | 8 1 | 8 5 | 8 6 | 5 | +4 | +1 |
| YAW98 | 1 | 34 | 101 | 15 5 | 18 1 | 18 1 | 17 6 | 15 1 | 0 | 5 | 2 5 |

**Table 9.** Average measured water levels for selected observation wells, 1963–76, and model-calculated water levels for 1975 and 1989, and 2020 for the East Cape flow cell, Cape Cod Basin, Massachusetts

[Location of wells shown in figure 12 Change in model-calculated water levels between stress periods +, increase in the water level resulting from changing stress conditions, otherwise, a change is a decrease in water level]

| Well No | Model node | | | Average measured water level, in feet above sea level | Model-calculated water levels, in feet above sea level | | | | Change in model-calculated water level between stress periods, in feet | | |
|---|---|---|---|---|---|---|---|---|---|---|---|
| | | | | | | Year | | | | | |
| | Layer | Row | Column | | Predevelopment | 1975 | 1989 | 2020 | Predevelopment–1975 | 1975–1989 | 1989–2020 |
| BMW21 | 1 | 16 | 47 | 27 2 | 26 8 | 26 2 | 25 1 | 24 0 | 0 6 | 1 1 | 1 1 |
| BMW22 | 1 | 11 | 49 | 19 8 | 14 9 | 14 6 | 14 2 | 13 8 | 3 | 4 | 4 |
| BMW44 | 4 | 17 | 32 | 26 4 | 24 2 | 23 8 | 23 4 | 22 0 | 4 | 4 | 1 4 |
| BMW45 | 1 | 21 | 35 | 31 4 | 31 9 | 31 4 | 31 0 | 29 5 | 5 | 4 | 1 5 |
| CGW138 | 1 | 37 | 46 | 11 3 | 13 8 | 12 8 | 11 9 | 10 7 | 1 0 | 9 | 1 2 |
| CGW177 | 1 | 35 | 57 | 11 9 | 8 4 | 8 4 | 8 3 | 8 3 | 0 | 1 | 0 |
| DGW100 | 1 | 19 | 15 | 44 7 | 44 5 | 43 9 | 43 4 | 42 5 | 6 | 5 | 9 |
| DGW123 | 1 | 18 | 23 | 32 8 | 30 7 | 30 3 | 29 9 | 28 8 | 4 | 4 | 1 1 |
| DGW135 | 1 | 22 | 17 | 16 7 | 20 9 | 18 9 | 18 0 | 14 4 | 2 0 | 9 | 3 6 |
| DGW157 | 1 | 24 | 23 | 25 9 | 28 2 | 25 1 | 24 0 | 18 0 | 3 1 | 1 1 | 6 0 |
| DGW158 | 2 | 27 | 20 | 19 2 | 20 1 | 17 3 | 16 3 | 12 2 | 2 8 | 1 0 | 4 1 |
| DGW172 | 5 | 32 | 23 | 6 2 | 12 2 | 11 9 | 11 4 | 10 6 | 3 | 5 | 8 |
| DGW173 | 4 | 32 | 23 | 8 2 | 11 7 | 11 4 | 10 8 | 10 1 | 3 | 6 | 7 |
| HJW141 | 2 | 27 | 51 | 19 2 | 18 0 | 17 7 | 16 7 | 14 8 | 3 | 1 0 | 1 9 |
| HJW145 | 1 | 27 | 30 | 31 7 | 26 7 | 26 5 | 26 3 | 25 3 | 2 | 2 | 1 0 |
| HJW148 | 1 | 34 | 36 | 20 8 | 22 2 | 21 4 | 21 5 | 21 1 | 8 | + 1 | 4 |
| OSW22 | 1 | 9 | 61 | 4 2 | 3 0 | 3 2 | 3 3 | 3 5 | + 2 | + 1 | + 2 |
| OSW24 | 1 | 19 | 55 | 18 5 | 17 9 | 17 2 | 16 3 | 15 0 | 7 | 9 | 1 3 |
| OSW25 | 1 | 11 | 54 | 15 1 | 13 9 | 13 7 | 13 5 | 13 2 | 2 | 2 | 3 |

**Table 10** Measured and model-calculated pond levels for selected ponds in the West Cape flow cell, Cape Cod Basin, Massachusetts

[Change in model-calculated pond level between stress periods +, indicates an increase in the pond level resulting from changing stress conditions, otherwise, change is a decrease in the pond level]

| Pond name | Model node | | | Average measured pond level, in feet above sea level | Model-calculated pond levels, in feet above sea level | | | | Change in model-calculated pond level between stress periods, in feet | | |
|---|---|---|---|---|---|---|---|---|---|---|---|
| | | | | | | Year | | | | | |
| | Layer | Row | Column | | Predevelopment | 1975 | 1989 | 2020 | Predevelopment–1975 | 1975–1989 | 1989–2020 |
| **Town of Barnstable** | | | | | | | | | | | |
| Long | 1 | 35 | 57 | 51 0 | 45 6 | 45 6 | 45 0 | 43 1 | 0 | 0 6 | 1 9 |
| Lovells | 1 | 41 | 57 | 38 0 | 34 1 | 34 1 | 33 4 | 32 0 | 0 | 7 | 1 4 |
| Muddy | 1 | 36 | 60 | 45 0 | 42 5 | 42 5 | 41 7 | 39 8 | 0 | 8 | 1 9 |
| Mystic Lake | 1 | 33 | 64 | 44 0 | 44 3 | 44 3 | 43 6 | 41 7 | 0 | 7 | 1 9 |
| Middle | 1 | 35 | 64 | 42 0 | 44 3 | 44 3 | 43 6 | 41 7 | 0 | 7 | 1 9 |
| Hamblin | 1 | 36 | 65 | 42 0 | 44 3 | 44 3 | 43 6 | 41 7 | 0 | 7 | 1 9 |
| Shubael | 1 | 36 | 68 | 43 0 | 42 6 | 42 2 | 41 6 | 40 4 | 4 | 6 | 1 2 |
| Wequaquet | 1 | 39 | 79 | 34 0 | 33 1 | 33 8 | 34 0 | 33 3 | +7 | +2 | 7 |
| Long | 1 | 43 | 79 | 26 0 | 23 1 | 23 2 | 23 4 | 23 2 | +1 | +2 | 2 |
| Shallow | 1 | 38 | 83 | 34 0 | 33 2 | 33 8 | 34 0 | 33 4 | +6 | +2 | 6 |
| **Town of Falmouth** | | | | | | | | | | | |
| Long | 1 | 53 | 18 | 11 0 | 9 9 | 4 7 | 6 1 | 3 8 | 5 2 | +1 4 | 2 3 |
| Mares | 1 | 51 | 22 | 13 0 | 15 0 | 13 1 | 13 7 | 12 2 | 1 9 | +6 | 1 5 |
| Jenkins | 1 | 49 | 25 | 19 0 | 17 9 | 17 1 | 17 5 | 16 4 | 8 | +4 | 1 1 |
| Deep | 1 | 42 | 28 | 35 0 | 34 1 | 34 4 | 34 1 | 34 0 | +3 | 3 | 1 |
| Coonamessett | 1 | 44 | 30 | 33 0 | 32 9 | 33 1 | 32 9 | 32 9 | +2 | 2 | 0 |
| **Town of Mashpee** | | | | | | | | | | | |
| Ashumet | 1 | 41 | 37 | 44 0 | 41 8 | 42 1 | 42 0 | 41 9 | +0 3 | 0 1 | 0 1 |
| Johns | 1 | 43 | 40 | 38 0 | 34 9 | 35 1 | 35 0 | 34 9 | +2 | 1 | 1 |
| Wakeby | 1 | 31 | 50 | 55 0 | 53 4 | 53 5 | 53 3 | 52 5 | +1 | 2 | 8 |
| Mashpee | 1 | 35 | 48 | 55 0 | 52 8 | 52 9 | 52 7 | 51 9 | +1 | 2 | 8 |
| Santuit | 1 | 39 | 54 | 43 0 | 38 7 | 38 8 | 38 5 | 37 5 | +1 | 3 | 1 0 |
| **Town of Sandwich** | | | | | | | | | | | |
| Snake | 1 | 8 | 43 | 68 0 | 69 9 | 69 7 | 68 9 | 67 7 | 0 2 | 0 8 | 1 2 |
| Peters | 1 | 27 | 49 | 67 0 | 65 3 | 65 4 | 64 9 | 63 7 | +1 | 5 | 1 2 |
| Spectacle | 1 | 25 | 57 | 63 0 | 62 5 | 62 6 | 61 1 | 58 4 | +1 | 1 5 | 2 7 |
| Triangle | 1 | 8 | 57 | 62 0 | 60 3 | 60 4 | 59 3 | 56 4 | +1 | 1 1 | 2 9 |
| Lawrence | 1 | 27 | 60 | 61 0 | 56 4 | 56 4 | 55 5 | 53 0 | 0 | 0 9 | 2 5 |
| **Town of Yarmouth** | | | | | | | | | | | |
| Dennis | 1 | 35 | 98 | 25 0 | 20 0 | 19 7 | 19 1 | 16 3 | 0 3 | 0 6 | 2 8 |
| Horse | 1 | 44 | 99 | 16 0 | 13 7 | 13 3 | 12 1 | 10 0 | 4 | 1 2 | 2 1 |
| Plashes | 1 | 43 | 103 | 0 | 15 1 | 14 3 | 14 1 | 12 9 | 8 | 2 | 1 2 |
| Seine | 1 | 47 | 103 | 10 0 | 8 7 | 8 6 | 8 6 | 8 3 | 1 | 0 | 3 |
| Long | 1 | 44 | 108 | 7 0 | 11 1 | 10 1 | 10 8 | 10 8 | 1 0 | +7 | 0 |

**Table 11.** Measured and model-calculated pond levels for selected ponds in the East Cape flow cell, Cape Cod Basin, Massachusetts

[Change in model-calculated pond level between stress periods +, indicates an increase in the pond level resulting from changing stress conditions, otherwise, change is a decrease in the pond level]

| Pond name | Model node | | | Average measured pond level, in feet above sea level | Model-calculated pond levels, in feet above sea level | | | | Change in model-calculated pond level between stress periods, in feet | | |
|---|---|---|---|---|---|---|---|---|---|---|---|
| | | | | | Year | | | | | | |
| | Layer | Row | Column | | Predevelopment | 1975 | 1989 | 2020 | Predevelopment–1975 | 1975–1989 | 1989–2020 |
| **Town of Brewster** | | | | | | | | | | | |
| Cliff | 1 | 16 | 49 | 26 0 | 25 6 | 25 0 | 23 8 | 22 6 | 0 6 | 1 2 | 1 2 |
| Elbow | 1 | 24 | 26 | 30 0 | 28 5 | 27 6 | 26 9 | 24 4 | 9 | 7 | 2 5 |
| Cahoon | 1 | 24 | 45 | 31 0 | 31 7 | 30 9 | 29 9 | 27 9 | 8 | 1 0 | 2 0 |
| Flax | 1 | 14 | 49 | 24 0 | 25 6 | 25 0 | 23 8 | 22 6 | 6 | 1 2 | 1 2 |
| Greenland | 1 | 24 | 42 | 31 0 | 33 1 | 32 6 | 32 1 | 30 6 | 5 | 5 | 2 5 |
| Griffith | 1 | 18 | 35 | 33 0 | 30 9 | 30 4 | 30 0 | 28 5 | 5 | 4 | 1 5 |
| Little Cliff | 1 | 17 | 51 | 25 0 | 25 6 | 25 0 | 23 8 | 22 6 | 6 | 1 2 | 1 2 |
| Upper Mill | 1 | 20 | 29 | 26 0 | 28 3 | 27 6 | 26 8 | 24 6 | 7 | 8 | 2 2 |
| Lower Mill | 1 | 18 | 31 | 26 0 | 28 0 | 27 3 | 26 5 | 24 4 | 7 | 8 | 2 1 |
| Seymour | 1 | 23 | 33 | 29 0 | 31 2 | 30 7 | 30 3 | 28 8 | 5 | 4 | 1 5 |
| Sheep | 1 | 20 | 39 | 31 0 | 32 9 | 32 4 | 32 0 | 30 6 | 5 | 4 | 1 4 |
| Walkers | 1 | 22 | 27 | 26 0 | 28 5 | 27 6 | 26 8 | 24 5 | 9 | 8 | 2 3 |
| **Town of Chatham** | | | | | | | | | | | |
| Goose | 1 | 34 | 49 | 15 0 | 17 4 | 16 6 | 15 9 | 14 1 | 0 8 | 0 7 | 1 8 |
| Lovers Lake | 1 | 34 | 54 | 13 0 | 10 6 | 9 9 | 9 6 | 8 8 | 7 | 3 | 8 |
| School House | 1 | 34 | 52 | 13 0 | 13 5 | 12 2 | 11 7 | 9 9 | 1 3 | 5 | 1 8 |
| White | 1 | 37 | 53 | 14 0 | 9 0 | 8 2 | 8 0 | 7 6 | 8 | 2 | 4 |
| **Town of Dennis** | | | | | | | | | | | |
| Fresh | 1 | 33 | 19 | 7 0 | 6 2 | 6 2 | 5 9 | 6 3 | 0 | 0 3 | +0 4 |
| Scargo Lake | 1 | 15 | 16 | 12 0 | 15 3 | 15 4 | 15 1 | 15 4 | + 1 | 3 | + 3 |
| **Town of Harwich** | | | | | | | | | | | |
| Long (Brewster) | 1 | 24 | 38 | 31 0 | 32 9 | 32 5 | 32 0 | 30 6 | 0 4 | 0 5 | 1 4 |
| Bucks | 1 | 31 | 42 | 30 0 | 29 4 | 28 3 | 27 9 | 26 3 | 1 1 | 4 | 1 6 |
| Grass | 1 | 36 | 35 | 9 0 | 15 5 | 15 0 | 15 0 | 14 9 | 5 | 0 | 1 |
| Hawk's Nest | 1 | 27 | 43 | 32 0 | 32 9 | 32 3 | 31 7 | 30 1 | 6 | 6 | 1 6 |
| Hinckleys | 1 | 26 | 33 | 28 0 | 30 9 | 30 5 | 30 1 | 28 8 | 4 | 4 | 1 3 |
| Josephs | 1 | 31 | 41 | 30 0 | 29 7 | 28 5 | 28 2 | 26 8 | 1 2 | 3 | 1 4 |
| Reservoir | 1 | 33 | 27 | 7 0 | 7 8 | 7 8 | 7 1 | 7 8 | 0 | 7 | + 7 |
| Robbins | 1 | 27 | 29 | 28 0 | 27 1 | 26 7 | 26 4 | 23 8 | 4 | 3 | 2 6 |
| Sand | 1 | 32 | 28 | 13 0 | 12 8 | 12 8 | 12 8 | 12 7 | 0 | 0 | 1 |
| Walkers | 1 | 27 | 41 | 32 0 | 33 4 | 32 8 | 32 4 | 30 9 | 4 | 4 | 1 5 |
| **Town of Orleans** | | | | | | | | | | | |
| Crystal Lake | 1 | 13 | 58 | 15 0 | 9 5 | 9 2 | 8 7 | 8 4 | 0 3 | 0 5 | 0 3 |
| Pilgrim Lake | 1 | 16 | 58 | 8 0 | 8 7 | 8 1 | 7 7 | 7 0 | 6 | 4 | 7 |

**Table 12** Model-calculated streamflow for selected streams in the West Cape and East Cape flow cells, Cape Cod Basin, Massachusetts

[All streamflow values are in cubic feet per second, locations of streammodel-calculated water levels, in feet above sea level, are shown in figures 11 and 12]

| Stream (town) | Model-calculated water levels, in feet above sea level | | | | Decrease in model-calculated streamflow between stress periods | | |
|---|---|---|---|---|---|---|---|
| | Predevelopment | Year | | | Predevelopment–1975 | 1975–1989 | 1989–2020 |
| | | 1975 | 1989 | 2020 | | | |
| **West Cape Flow Cell** | | | | | | | |
| Coonamessett (Falmouth) | 99 | 98 | 92 | 89 | 01 | 06 | 03 |
| Childs (Falmouth) | 58 | 58 | 52 | 45 | 0 | 6 | 7 |
| Quashnet (Mashpee) | 136 | 136 | 135 | 126 | 0 | 1 | 9 |
| Mashpee (Mashpee) | 102 | 102 | 101 | 95 | 0 | 1 | 6 |
| Santuit (Barnstable) | 41 | 41 | 39 | 33 | 0 | 2 | 6 |
| Mill (Barnstable) | 41 | 40 | 32 | 24 | 1 | 8 | 8 |
| Centerville (Barnstable) | 14 | 14 | 14 | 14 | 0 | 0 | 0 |
| **East Cape Flow Cell** | | | | | | | |
| Herring (Harwich) | 47 | 43 | 40 | 26 | 04 | 03 | 14 |
| Stony Brook (Brewster) | 19 | 18 | 18 | 16 | 1 | 0 | 2 |

The 1975 recharge rates to the West Cape and East Cape flow cells specified in the MODFLOW models are 247 8 and 88 1 $ft^3/s$ (table 13), respectively, which are 93 and 97 percent, respectively, of the recharge rates specified for the two flow cells by Guswa and LeBlanc (1985) The small differences between recharge rates specified in these models and the Guswa and LeBlanc (1985) models likely are due to the finer discretization used in these models Most freshwater discharge from the flow cells occurs to coastal discharge areas for 1975, calculated freshwater discharge to coastal areas is 68 and 69 percent of the total freshwater discharge from the West Cape and East Cape flow cells, respectively Total 1975 groundwater discharge to streams in the freshwater-flow models is 51 5 $ft^3/s$ in the West Cape flow cell (or 21 percent of total freshwater discharge from the flow cell) and 12 1 $ft^3/s$ in the East Cape flow cell (or 14 percent of total freshwater discharge from the flow cell) The water budgets calculated by the SHARP and MODFLOW models for each flow cell are in close correlation For example, the steady-state volumetric flow rates of water moving between the five layers of the West Cape MODFLOW model for the 2020 pumping and recharge conditions ranged from 94 to 100 percent of those calculated for the same stress conditions by the SHARP model, for the East Cape flow models, the percentages ranged from 87 to 100 Differences between water budgets calculated by the SHARP and MODFLOW models result in part from differences in the method used to simulate the freshwater-saltwater interface in each model

As part of the calibration process, a sensitivity analysis was done to determine the response of the freshwater models to changes in recharge, horizontal hydraulic conductivity, vertical conductance, vertical

**Table 13** Model-calculated water budgets for the West Cape and East Cape flow cells, Cape Cod Basin, Massachusetts

[All values are in cubic feet per second]

| Budget item | Predevelopment | Year | | |
|---|---|---|---|---|
| | | 1975 | 1989 | 2020 |
| **West Cape Flow Cell (Area = $5.3 \times 10^9$ ft$^2$)** | | | | |
| Inflow | | | | |
| Recharge | | | | |
| Natural | 238 9 | 238 9 | 238 9 | 238 9 |
| Wastewater return flow | 0 | 8 9 | 13 0 | 22 2 |
| Release from storage | 0 | 0 | 0 | 0 |
| Total inflow | 238 9 | 247 8 | 251 9 | 261 1 |
| Outflow | | | | |
| Pumpage | 0 | 16 9 | 24 2 | 48 3 |
| Stream | 51 3 | 51 5 | 49 2 | 46 2 |
| Coastal discharge | 178 1 | 170 0 | 169 7 | 158 0 |
| Subsea discharge | 10 3 | 10 3 | 10 3 | 10 3 |
| Storage | 0 | 0 | 0 | 0 |
| Total outflow | 239 7 | 248 7 | 253 4 | 262 8 |
| Model error (inflow minus outflow) | –0 8 | –0 9 | –1 5 | –1 7 |
| **East Cape Flow Cell (Area = $2.4 \times 10^9$ ft$^2$)** | | | | |
| Inflow | | | | |
| Recharge | | | | |
| Natural | 83 2 | 83 2 | 83 2 | 83 2 |
| Wastewater return flow | 0 | 4 9 | 7 2 | 15 9 |
| Release from storage | 0 | 0 | 3 | 0 |
| Total inflow | 83 2 | 88 1 | 90 7 | 99 1 |
| Outflow | | | | |
| Pumpage | 0 | 6 5 | 11 4 | 22 7 |
| Streams | 12 7 | 12 1 | 11 5 | 9 5 |
| Coastal discharge | 61 4 | 60 6 | 58 9 | 58 1 |
| Subsea discharge | 9 1 | 9 1 | 9 1 | 9 1 |
| Storage | 0 | 0 | 0 | 0 |
| Total outflow | 83 2 | 88 3 | 90 9 | 99 4 |
| Model error (inflow minus outflow) | 0 | –0 2 | –0 2 | –0 3 |

leakance of seabed deposits, specific yield, and storage coefficient specified in the models The sensitivity analysis identifies those parameters to which the model results are most sensitive Each parameter was uniformly increased and decreased individually, while other model parameters were held constant The model-calculated water-table altitude, streamflow, and coastal discharge were most sensitive to increases and decreases in simulated recharge to the models and to decreases in the horizontal hydraulic conductivity and vertical conductance of the top layer of each model, however, changes in simulated vertical leakance of seabed deposits by an order of magnitude did not have a substantial effect on model-calculated results

## Response of the Freshwater-Saltwater Interface to Simulated Ground-Water Pumping and Recharge

Model-calculated changes in the position of the freshwater-saltwater interface were minimal between the two steady-state simulations, in which pumping and recharge rates were increased from predevelopment flow conditions to those projected to occur in the year 2020 For example, hydrogeologic sections in the north-south direction that show the interface location along one column of each model in which pumping is substantial indicate minimal change in the position of the interface for increased pumping and recharge rates (fig 14) Also, the rate of freshwater discharge to overlying saltwater zones of the aquifer did not change substantially with increased pumping and recharge rates The minimal change in the position of the freshwater-saltwater interface likely results from several factors First, much of the existing and proposed pumping occurs more than 1 mi landward of the coasts and from the top two layers of the models at depths less than 70 ft below sea level, the three-dimensional location of these withdrawals seems to place little landward stress on the interface

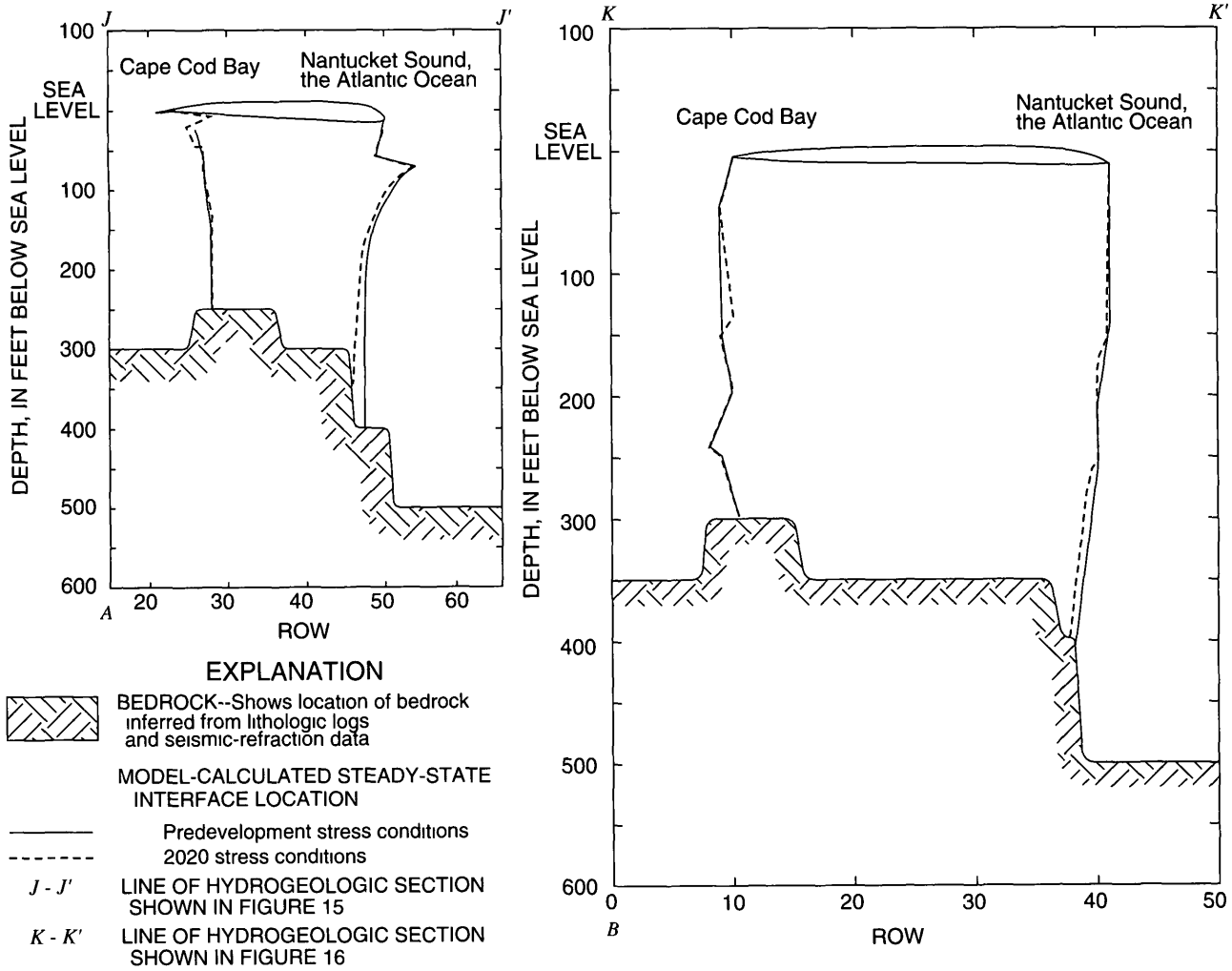


**Figure 14.** Steady-state location of the model-calculated freshwater-saltwater interface for predevelopment and projected 2020 stress conditions for (*A*) column 84 of the West Cape flow model and (*B*) column 40 of the East Cape flow model, Cape Cod Basin, Massachusetts

Second, the net stress on the interface due to pumping is decreased because a large percentage of the water pumped from the aquifer is returned to the aquifer through wastewater-return flow Finally, much of the wastewater-return flow from septic systems occurs near the coasts where residences are concentrated This recharge of wastewater near the coasts possibly helps to maintain a constant position of the interface with time

Areas of freshwater overlying saltwater and areas of freshwater that extend to bedrock, as determined by the SHARP models, are shown in figures 15 and 16 saltwater occurs between freshwater and underlying bedrock generally in areas that are within 1 to 2 mi landward of the coastline Exceptions to this generalization are in the East Cape flow cell (fig 16), where (1) water levels are low and saltwater underlies a substantial part of the northeast and southeast areas of the flow cell, and (2) freshwater discharge is through clay and silt deposits that underlie Cape Cod Bay, which forces the freshwater-saltwater interface seaward of the coastline saltwater generally is prevented from moving more than 1 to 2 mi landward of the coasts because of the high ground-water levels that occur in each flow cell This limitation is a reflection of the high recharge rates to the flow systems and thinness of the unconsolidated deposits

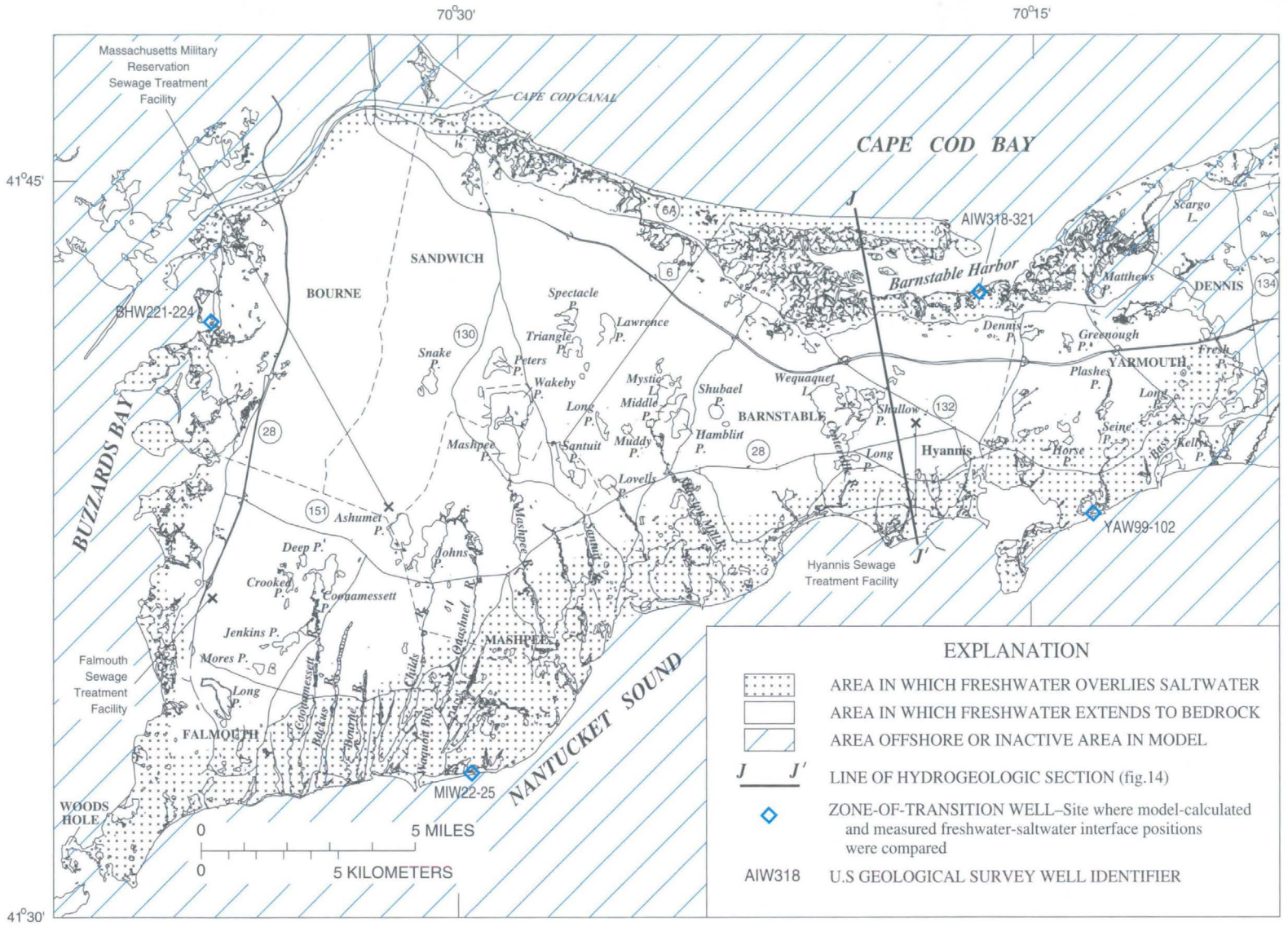


**Figure 15.** Model-calculated location of freshwater overlying saltwater and freshwater extending to bedrock for the West Cape flow cell, Cape Cod Basin, Massachusetts.

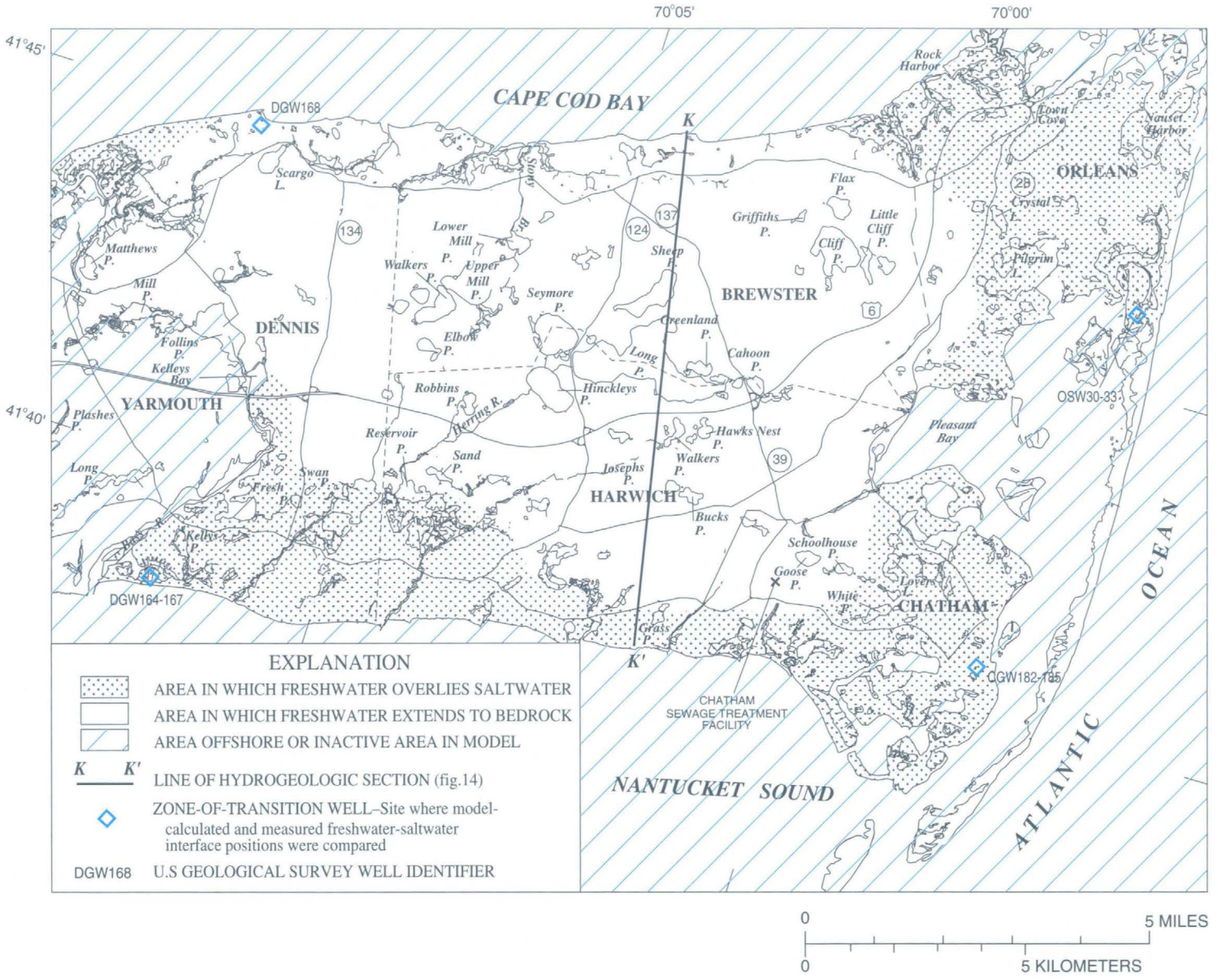


**Figure 16.** Model-calculated location of freshwater overlying saltwater and freshwater extending to bedrock for the East Cape flow cell, Cape Cod Basin, Massachusetts.

### Response of the Freshwater-Flow Systems to Simulated Ground-Water Pumping and Recharge

Because the SHARP models indicated little movement of the freshwater-saltwater interface for the stress conditions simulated, boundary conditions along the freshwater-saltwater interface were assumed to remain constant with time This assumption simplified the incorporation of the freshwater-saltwater boundary conditions calculated by the SHARP models within the MODFLOW models This was done to complete a detailed analysis of the freshwater-flow systems with the MODFLOW models, which are less computationally intensive than the SHARP models Boundary conditions along the freshwater-saltwater interface calculated for steady-state flow conditions using the projected 2020 pumping and recharge rates were used in the MODFLOW models The methodology for incorporating these boundary conditions was discussed in the section "Boundary Conditions" and shown schematically in figure 10

Despite the substantial increases in pumping with time, the average model-calculated decline in heads in the flow cells generally was a small percentage of the saturated thickness of the aquifer in each flow cell The model-calculated average decline in water-table altitude at the observation wells was 0 4 and 0 8 ft for the West Cape and East Cape flow cells, respectively, from predevelopment flow conditions to 1975 (tables 8 and 9) From 1975 to 1989, the average model-calculated declines in water-table altitudes at the observation wells were 0 3 and 0 6 ft for the West Cape and East Cape flow cells, respectively Model-calculated declines in water levels for the projected 2020 stress conditions are estimated to increase by an average of 1 1 ft in the West Cape and 1 5 ft in the East Cape flow cells between 1989 and 2020 Model-calculated total average declines in the water table at the observation wells are, then, 1 8 and 2 9 ft in the West Cape and East Cape flow cells, respectively, for the simulation period

Model-calculated pond levels declined an average of 0 4 and 0 5 ft in the West Cape and East Cape flow cells, respectively, from predevelopment flow conditions to 1975, and 0 4 and 0 5 ft, respectively, from 1975 to 1989 (tables 10 and 11) Model-calculated pond-level declines are estimated to increase by an average of 1 3 ft in the West Cape and East Cape flow cells between 1989 and 2020

Maps depicting model-calculated changes in the altitude of the water table (drawdowns) in the flow cells show areas of local stresses in response to changing pumping and recharge rates (figs 17–20) Model-calculated declines due to ground-water pumping were largest at the sites of major pumping centers—in the towns of Falmouth and Barnstable in the West Cape flow cell and in the towns of Dennis and Harwich in the East Cape flow cell In the West Cape flow cell, simulated pumping from Long Pond in Falmouth resulted in a 5 2-foot decline in pond-level altitude from predevelopment flow conditions to 1975 (table 10), however, the water table increased 6 ft beneath the wastewater-treatment facility in Hyannis during the same period due to simulated increased discharge rates at the facility Model-calculated declines in water levels from 1975 to 1989 in both flow cells were, on average, less than those that occurred from predevelopment flow conditions to 1975 For example, the level of Long Pond Reservoir in Falmouth actually increased by 1 4 ft from 1975 to 1989 as a result of the decrease in the average daily pumping rate from the reservoir from 2 7 to 2 0 Mgal/d The simulated increase from 1989 to projected 2020 pumping rates resulted in the largest drawdowns in both flow cells Model-calculated water levels in the eastern part of Barnstable showed declines as much as 8 ft near Mary Dunn Pond Model-calculated water-level declines in the East Cape flow cell are estimated to be the largest in Dennis, where pumping is projected to increase from 2 5 to 5 3 Mgal/d from 1989 to 2020

Model-calculated streamflow depletions from predevelopment flow conditions to 2020, as a percentage of model-calculated predevelopment flow conditions, are greatest for the Marston Mill, Santuit, and Childs Rivers in the West Cape flow cell Model-calculated streamflow depletions from predevelopment flow conditions to 2020 are least for the Quashnet, Mashpee, and Centerville Rivers in the West Cape flow cell The average streamflow for the nine streams simulated decreased between each of the three stress periods (predevelopment flow conditions to 1975, 1975–89, and 1989–2020) (table 12)

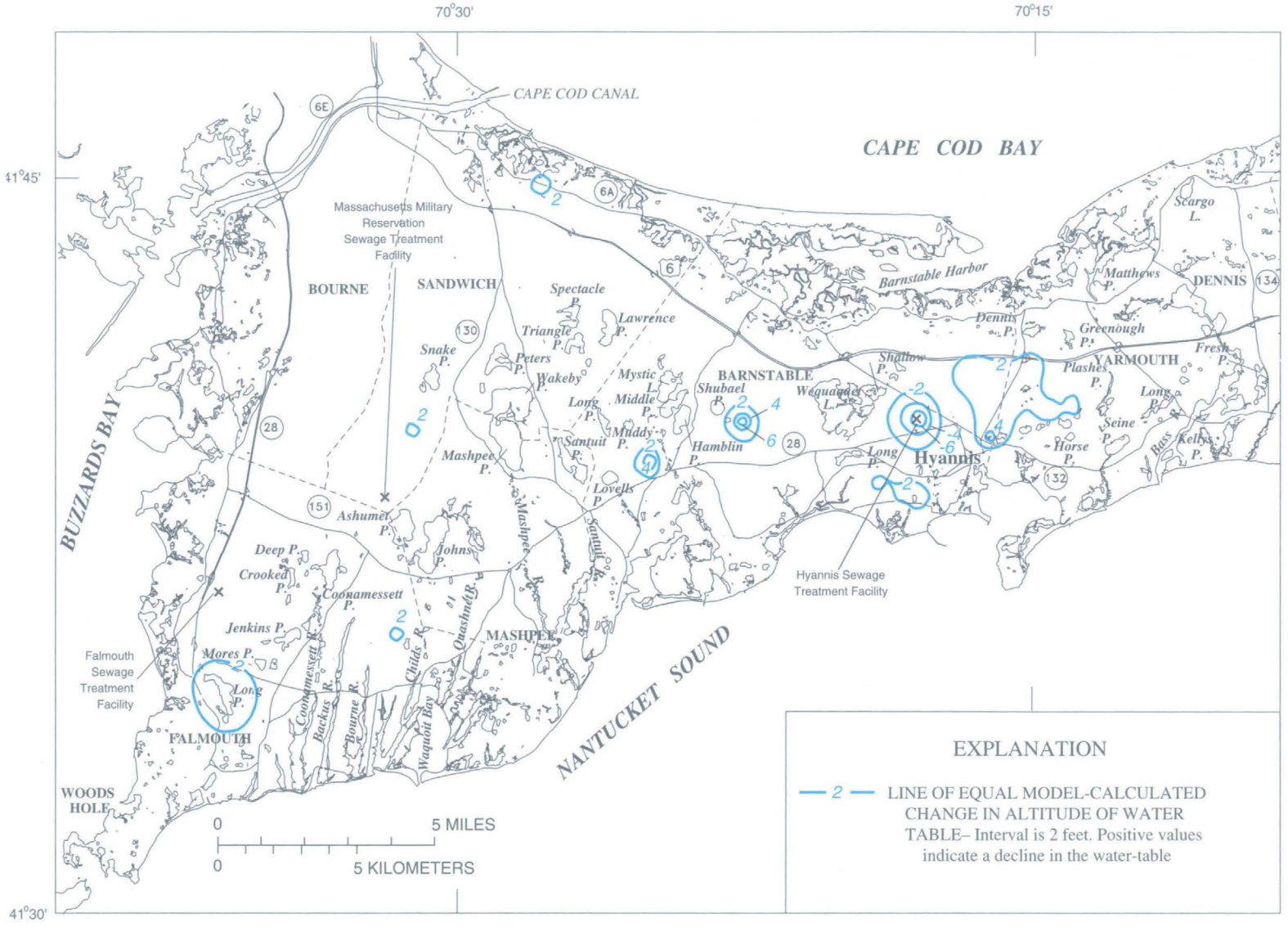


**Figure 17.** Model-calculated change in the altitude of the water-table configuration in the West Cape flow cell from predevelopment to 1989, Cape Cod Basin, Massachusetts.

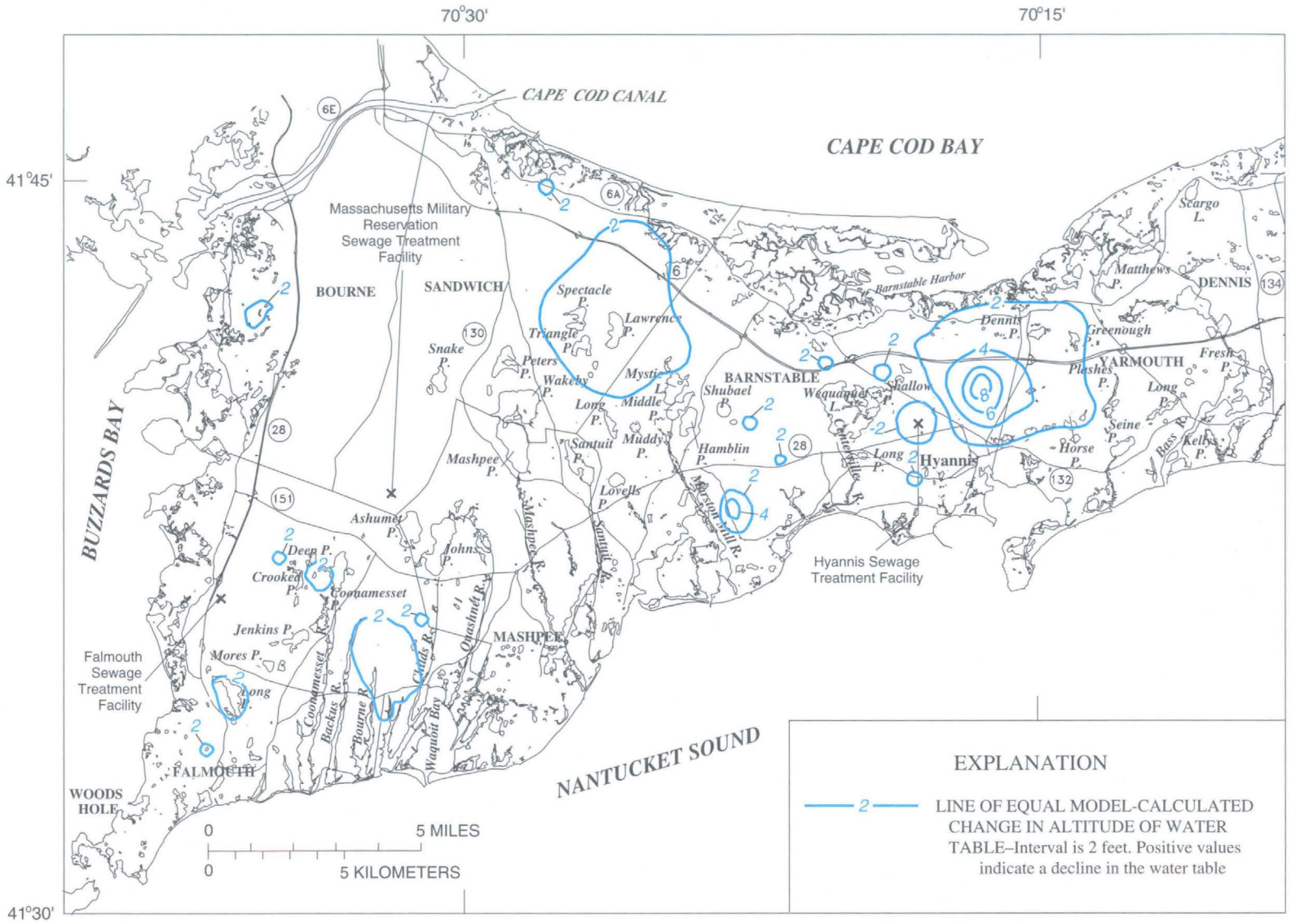


**Figure 18.** Model-calculated change in the altitude of the water-table configuration in the West Cape flow cell from 1989 to 2020, Cape Cod Basin, Massachusetts.

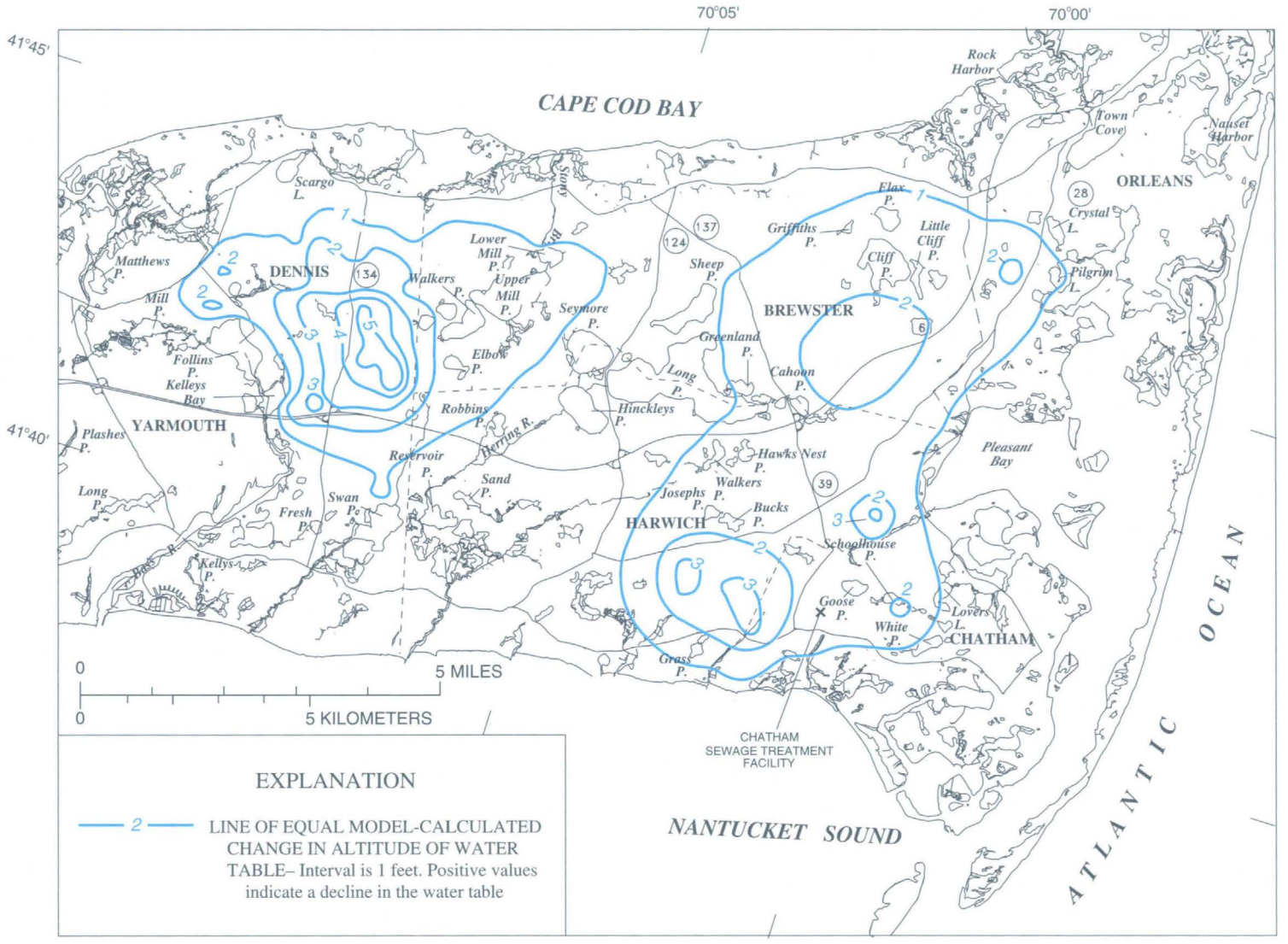


**Figure 19.** Model-calculated change in the altitude of the water-table configuration in the East Cape flow cell from predevelopment to 1989, Cape Cod Basin, Massachusetts.

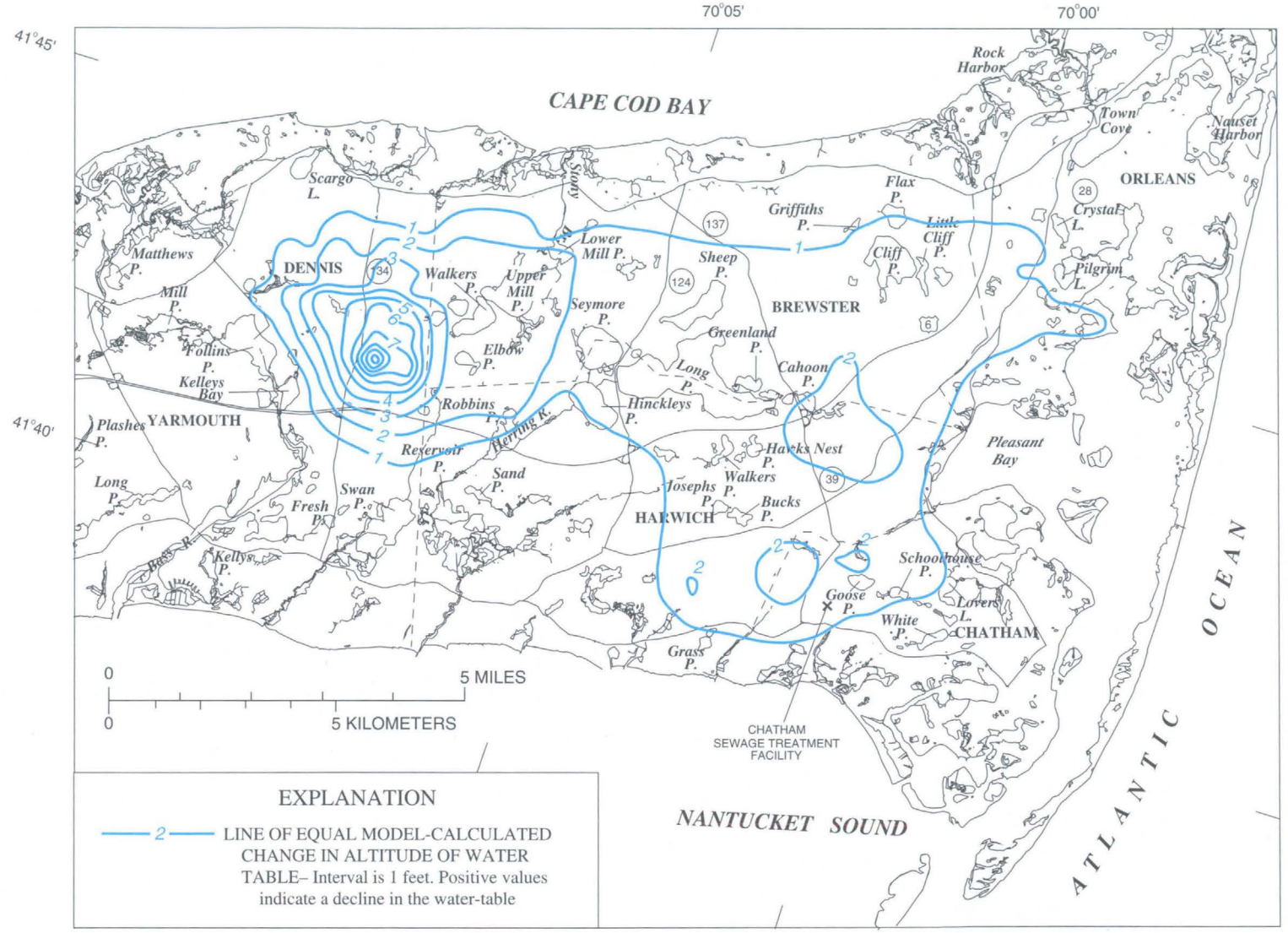


**Figure 20.** Model-calculated change in the altitude of the water-table configuration in the East Cape flow cell from 1989 to 2020, Cape Cod Basin, Massachusetts.

The average depletion in the model-calculated rate of streamflow at the gaging points for eight of the nine rivers during the simulation period is 14 percent of the model-calculated predevelopment streamflow in the rivers The Herring River in the East Cape flow cell was not included because of the significant contribution of surface-water outflow from Hinckleys Pond that could not be simulated

The total inflow and outflow of freshwater to each of the flow cells increases from predevelopment flow conditions to 2020 because the total pumping rate and wastewater-return flow to each system increases with time (table 13) The wastewater-return flow recharge rates specified in the West Cape and East Cape flow models were less than 90 percent of the total public-supply pumping rates Wastewater-return flow at coastal roads is lost to the flow cells because of the approximation of the coastline used for boundary conditions for the current model grid discretization The increased rate of ground-water pumping from the flow cells resulted in a decrease in water-table and pond altitudes in the flow cells and decreased rates of freshwater discharge to streams and saltwater boundaries For example, total ground-water discharge to streams in the West Cape flow cell decreases from 51 3 to 46 2 $ft^3/s$ from predevelopment flow conditions to 2020 (table 13), a decrease of nearly 10 percent of the total predevelopment ground-water discharge to streams The total model-calculated decrease in coastal discharge from predevelopment flow conditions to 2020 is 11 percent and 5 percent of total predevelopment coastal discharge in the West Cape and East Cape flow cells, respectively The changes in the altitude of the water table and pond levels were due to the removal of freshwater from storage (tables 8–11, figs 17–20)

Summer and winter pumping and recharge rates for 1989 were used to investigate the effects of a 30-percent decrease in recharge to the flow cells over a hypothetical 5-year period of drought Summer (in-season) and winter (off-season) pumping rates for 1989, shown in tables 3 and 4, were used in the simulations Total annual precipitation recharge to the flow cells was decreased by 30 percent, and all precipitation recharge was assumed to occur between September and May, an assumption that is consistent with the timing of precipitation recharge to the flow cells determined by LeBlanc and others (1986) and Barlow and Hess (1993) Simulated recharge from wastewater-return flow was assumed to occur uniformly throughout the year Two stress periods were simulated for each year—a 3-month summer stress period (June through August) and a 9-month winter stress period (September through May) Initial conditions used in the simulation were model-calculated water levels for 1989

The average model-calculated fluctuations in water-table altitudes between summer and winter stress conditions for the 32 observation wells in the West Cape flow cell and 19 observation wells in the East Cape flow cell were 2 0 and 1 7 ft, respectively Model-calculated fluctuations were greatest near the top of the ground-water mounds and least near the coast, where water levels are held nearly constant because of the ocean discharge boundary Changes in pumping and recharge rates resulted in a model-calculated average streamflow decrease of 36 percent between summer and winter stress conditions for the nine streams simulated Model-calculated coastal discharge decreased 14 percent in each flow cell, and the subsea discharge was negligible Water released from storage accounts for 90 and 84 percent of the total inflow to the ground-water systems for West Cape and East Cape flow cells, respectively, for the summer stress period, which resulted from the large decrease in recharge to the systems and doubling of ground-water pumping that occurred during the summer stress period

## Truro Flow Cell

The Truro flow cell is the smallest of the five flow cells of Cape Cod that are used for water supply The flow cell includes the town of Truro and extends from the Pamet River northward to Pilgrim Lake (fig 21) Although the sand and gravel aquifer that constitutes the flow cell extends to bedrock, the freshwater-flow system is bounded at depth (within the unconsolidated deposits) by the transition zone between freshwater and underlying saltwater (Guswa and LeBlanc, 1985, LeBlanc and others, 1986) Upconing of the freshwater-saltwater interface and contamination of public-water supplies by saltwater intrusion caused by pumping have been recorded at the Knowles Crossing well field, located 1,500 ft from Cape Cod Bay (LeBlanc and others, 1986) Evidence of saltwater intrusion precluded an assumption of a static interface between freshwater and saltwater, consequently, the ground-water-flow system was assessed using a freshwater-saltwater flow model

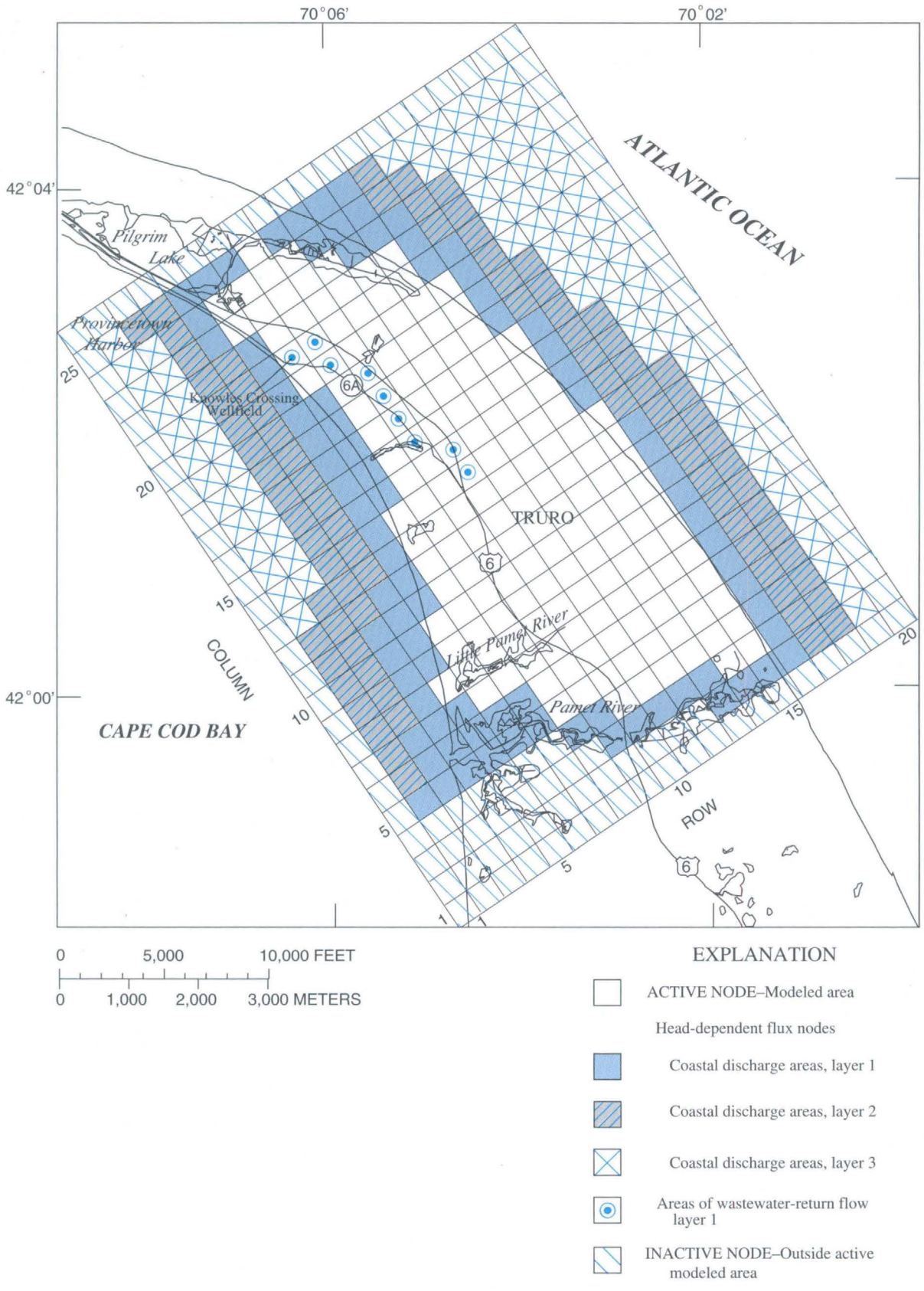


**Figure 21.** Grid and boundary conditions for the Truro flow cell, Cape Cod Basin, Massachusetts.

## Description of Model

### Grid

The finite-difference grid of the ground-water-flow system consists of uniform nodes 1,320 by 1,320 ft The grid contains 26 columns and 20 rows and has approximately the same orientation and extent as that used by Guswa and LeBlanc (1985) The aquifer was subdivided into seven layers to provide an adequate vertical representation of the aquifer's lithology The model extends from the water table to the contact between unconsolidated sediments and bedrock The bottom altitude of these layers ranges from 10 to 900 ft below sea level (table 7)

### Hydraulic Properties

The hydraulic properties used in the model of the Truro flow cell were assigned on the basis of available aquifer test, lithologic, and geologic information This ground-water-flow system has few deep wells, therefore, lithologic information necessary to determine hydraulic properties of the lower layers is limited Hydraulic properties of the aquifer material in areas with little or no lithologic information were estimated from an interpretation of the geologic processes that formed the sand and gravel deposits in the flow cell

Horizontal hydraulic conductivity assigned in the model ranged from 350 ft/d for medium sand to gravel to 50 ft/d for fine sand The ratio of vertical to horizontal hydraulic conductivity ranged from 1 3 for medium sand to gravel to 1 30 for fine sand The porosity was assumed to be constant throughout the aquifer and equal 0 30 A specific yield of 0 10 was used in the model, which was based on the results of aquifer tests done in the flow cell (Guswa and LeBlanc, 1985), a storage coefficient of $2\times10^{-4}$ was used in the model and was based on the results of Barlow and Hess (1993)

### Boundary Conditions

The Truro model is bounded on top by the water table, which is a free-surface boundary that receives spatially and temporally variable rates of recharge Recharge rates specified for the flow model are discussed in the next section, "Stresses and Stress Periods " The lower boundary of the model is the contact between the unconsolidated glacial sediments and the underlying bedrock, which was assumed to be impermeable Each model layer is bounded laterally by inactive nodes that separate modeled areas from unmodeled areas

Head-dependent flux boundary conditions were used to simulate saltwater discharge areas The head values specified at saltwater discharge boundaries are equivalent freshwater heads equal to the height of the column of saltwater overlying the seabed at the discharge boundary divided by 40 0, which is the ratio of the specific weight of freshwater (1 000 g/cm$^3$) to the difference between the specific weight of saltwater (1 025 g/cm$^3$) and freshwater (Essaid, 1990) The height of the column of saltwater in each node was determined from bathymetric maps of the area Head-dependent flux boundaries at saltwater discharge areas are in the top three model layers from 0 to 80 ft below sea level A vertical leakance of 20 day$^{-1}$ for the seabed deposits was used in the simulations, which corresponds to a 1-foot-thick sediment deposit on the seabed with a vertical hydraulic conductivity of 20 ft/d This value of vertical leakance of seabed deposits also was used by Guswa and LeBlanc (1985) in their model of the area

Ground-water discharge areas at Pilgrim Lake and Pamet River were simulated as head-dependent flux boundaries in the model Freshwater heads specified at these boundaries were determined from surface-water altitudes shown on topographic maps A small section along the northern boundary of the model where the Truro flow cell and the adjoining Provincetown flow cell meet was designated as an inactive node because no surface-water discharge bodies are present here

### Stresses and Stress Periods

The effects of changing stress conditions in the Truro flow cell were determined for predevelopment, 1975, 1989, and projected 2020 average daily demand stresses The response of the flow cell to fluctuations in pumping and recharge rates between in-season and off-season conditions was determined for current conditions based on 1989 in-season and off-season pumping and recharge rates

Recharge at the water table was estimated to be 19 8 in/yr by the method of Thornthwaite and Mather (1957), using 80 years of precipitation and air temperature data measured at the Provincetown weather station and an assumed soil-moisture capacity of the root zone of 4 in This precipitation recharge estimate is consistent with others previously made for the Cape Cod flow system (LeBlanc, 1984b, LeBlanc and others, 1986, Barlow and Hess, 1993)

Predevelopment conditions assumed that the public-supply well was not pumped, that no recharge entered the water table other than that from precipitation, and that change in aquifer storage was negligible The resulting head distribution and freshwater-saltwater interface position from the predevelopment simulation were used as the initial conditions for subsequent transient simulations

The 1975 stress condition assumed an average annual pumping rate for 1950 through 1981 Aquifer recharge during the 1975 stress condition equaled the average annual recharge rate of 19 8 in/yr added to wastewater-return flow from septic systems Wastewater-return flow from septic systems was estimated for the area between Route 6 and Route 6A that receives water supplies from the Provincetown Water Department The model nodes that received wastewater-return flow within the Truro flow cell are shown in figure 21

The rate of water supplied to the area between Route 6 and Route 6A is about 12 percent of the Provincetown Water Department's total pumpage during the winter (off-season) and about 25 percent of the total pumpage during the summer (in-season) (J M Cook, Provincetown Water Department, oral commun , 1992) The average annual rate of water supplied to this area then is assumed to be equal to 15 percent of the Water Department's annual ground-water pumpage A recharge rate from wastewater-return flow was calculated by dividing the total daily rate of water supplied to the area by the area of the grid nodes where the water supply is distributed To account for consumptive losses, only 90 percent of the total rate supplied to the area was returned to the simulated aquifer, as was done for the West and East Cape models Return flow from domestic and commercial septic systems in the areas not receiving water from the Provincetown Water Department was assumed to be equal to the amount of water withdrawn at each site, therefore, net ground-water recharge from wastewater-return flow was assumed to be zero The average annual 1975 pumping rate is 0 91 Mgal/d The simulated return-flow rate for the septic systems between Route 6 and Route 6A equaled 90 percent of the 15 percent of the total pumping, which equaled 0 12 Mgal/d

The 1989 stress condition was simulated by use of off-season and in-season pumping periods The off-season period was assumed to be from September through May All natural recharge was assumed to occur during this 9-month period The simulated return-flow recharge was 90 percent of the 12 percent of the ground-water pumpage distributed between Route 6 and Route 6A The in-season pumping period simulated 3 months of summer pumping rates with no natural recharge The only simulated recharge to the system during the in-season pumping period was that distributed between Route 6 and Route 6A (fig 21), which equaled 90 percent of the 25 percent of the total ground-water pumpage A hypothetical 5-year drought with a 30-percent decrease in natural recharge was simulated from 1989 to 1994 to determine the effects of a decrease in recharge on the water-table altitude and the position of the freshwater-saltwater interface The projected stress condition for 2020 was simulated based on the projected average daily pumping rates supplied by MOWR

#### Calibration

The Truro flow model was calibrated by comparing model-calculated water levels to measured water levels at 11 observation well locations (fig 22) Average water levels for 1963–76 were compared to water levels calculated by the model for 1975 Average water levels for 1963–76 were compared to calculated water levels for 1975 because (1) measured water-level information was not available for predevelopment flow conditions, (2) this time period is consistent with that used for model calibration by Guswa and LeBlanc (1985), and (3) water levels measured in these observation wells in 1975 are representative of long-term average water levels reported for the observation wells for the 1963–76 period (Letty, 1984)

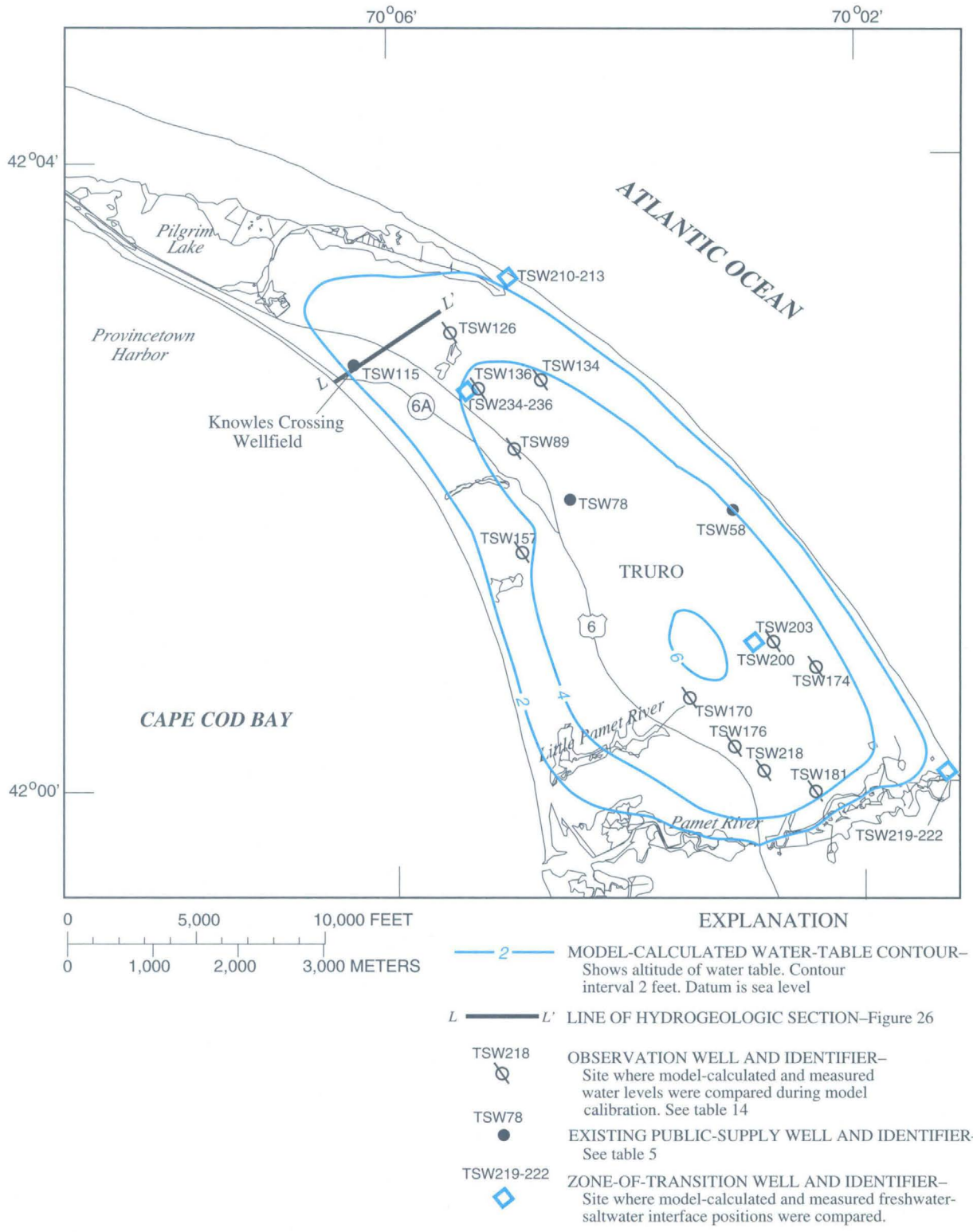


**Figure 22.** Model-calculated water-table configuration for the Truro flow cell, Cape Cod Basin, Massachusetts, 1989.

Initial estimates of horizontal hydraulic conductivity and vertical conductance were adjusted within reasonable limits during model calibration Generally, agreement between model-calculated and measured water levels at observation wells is close (table 14) The mean error between model-calculated water levels for 1975 and measured average water levels in observation wells was 0 3 ft, or 4 percent of the total relief of the water table in the Truro flow cell

Chloride data from four zone-of-transition wells (LeBlanc and others, 1986) were used for comparison to freshwater-saltwater interface locations calculated by the SHARP model in the Truro flow cell Model-calculated interface locations for 1975 were compared to measured chloride data collected between August 1973 and April 1976, results for the four wells are shown in figure 23 The measured data generally indicate that chloride concentrations increase with depth, a finding that is indicative of the transition between freshwater- and saltwater-flow systems The model-calculated vertical distribution of freshwater and saltwater at the four well sites generally agrees well with the chloride data and is considered adequate for the purpose of this investigation

Total inflow to the Truro flow cell ranges from 14 9 to 15 2 $ft^3/s$ from predevelopment flow conditions to 2020 (table 15) Most ground-water discharge is to coastal discharge areas, pumping constitutes a maximum of 11 percent of the freshwater discharge from the flow cell

**Table 14.** Average measured water levels for selected wells, 1963–76, and model-calculated water levels for 1975 and 1989, and 2020 for the Truro flow cell, Cape Cod Basin, Massachusetts

[Location of wells shown in figure 22 Change in model-calculated water level between stress periods +, increase in the water level resulting from changing stress conditions, otherwise, a change is a decrease in water level --, no value reported]

| Well No | Model node | | | Average measured water level, in feet above sea level | Model-calculated water levels, in feet above sea level | | | | Change in model-calculated water level between stress periods, in feet | | |
|---|---|---|---|---|---|---|---|---|---|---|---|
| | Layer | Row | Column | | Predevelopment | Year 1975 | Year 1989 | Year 2020 | Predevelopment–1975 | 1975–1989 | 1989–2020 |
| **Observation Wells** | | | | | | | | | | | |
| TSW157 | 7 | 8 | [1]13–14 | 44 | 42 | 39 | 38 | 35 | 03 | 01 | 03 |
| TSW176 | 7 | 10 | 5 | 56 | 52 | 52 | 52 | 51 | 0 | 0 | 1 |
| TSW170 | 7 | 10 | 7 | 59 | 58 | 57 | 57 | 56 | 1 | 0 | 1 |
| TSW89 | 7 | 10 | 16 | 44 | 49 | 46 | 45 | 43 | 3 | 1 | 2 |
| TSW136 | 7 | 10 | [1]18–19 | 43 | 44 | 44 | 42 | 42 | 0 | 2 | 0 |
| TSW126 | 7 | 10 | 20 | 42 | 40 | 39 | 39 | 39 | 1 | 0 | 0 |
| TSW218 | 7 | 11 | 4 | 52 | 51 | 50 | 50 | 50 | 1 | 0 | 0 |
| TSW181 | 7 | 12 | [1]3–4 | 47 | 48 | 48 | 48 | 48 | 0 | 0 | 0 |
| TSW134 | 7 | 12 | [1]17–18 | 49 | 44 | 43 | 42 | 41 | 1 | 1 | 1 |
| TSW203 | 7 | 13 | 8 | 57 | 58 | 57 | 57 | 56 | 1 | 0 | 1 |
| TSW174 | 7 | 14 | 6 | 56 | 48 | 48 | 47 | 47 | 0 | 1 | 0 |
| **Public-Supply Wells** | | | | | | | | | | | |
| TSW115 | 7 | 7 | 21 | -- | 28 | 09 | 22 | 22 | 19 | +13 | 0 |
| TSW78 | 7 | 10 | 14 | -- | 54 | 40 | 37 | 28 | 14 | 3 | 9 |
| TSW58 | 7 | 14 | 12 | -- | 49 | 47 | 44 | 41 | 2 | 3 | 3 |

[1]Where an observation well lies near the boundary of adjacent model cells, an average decline in levels is reported

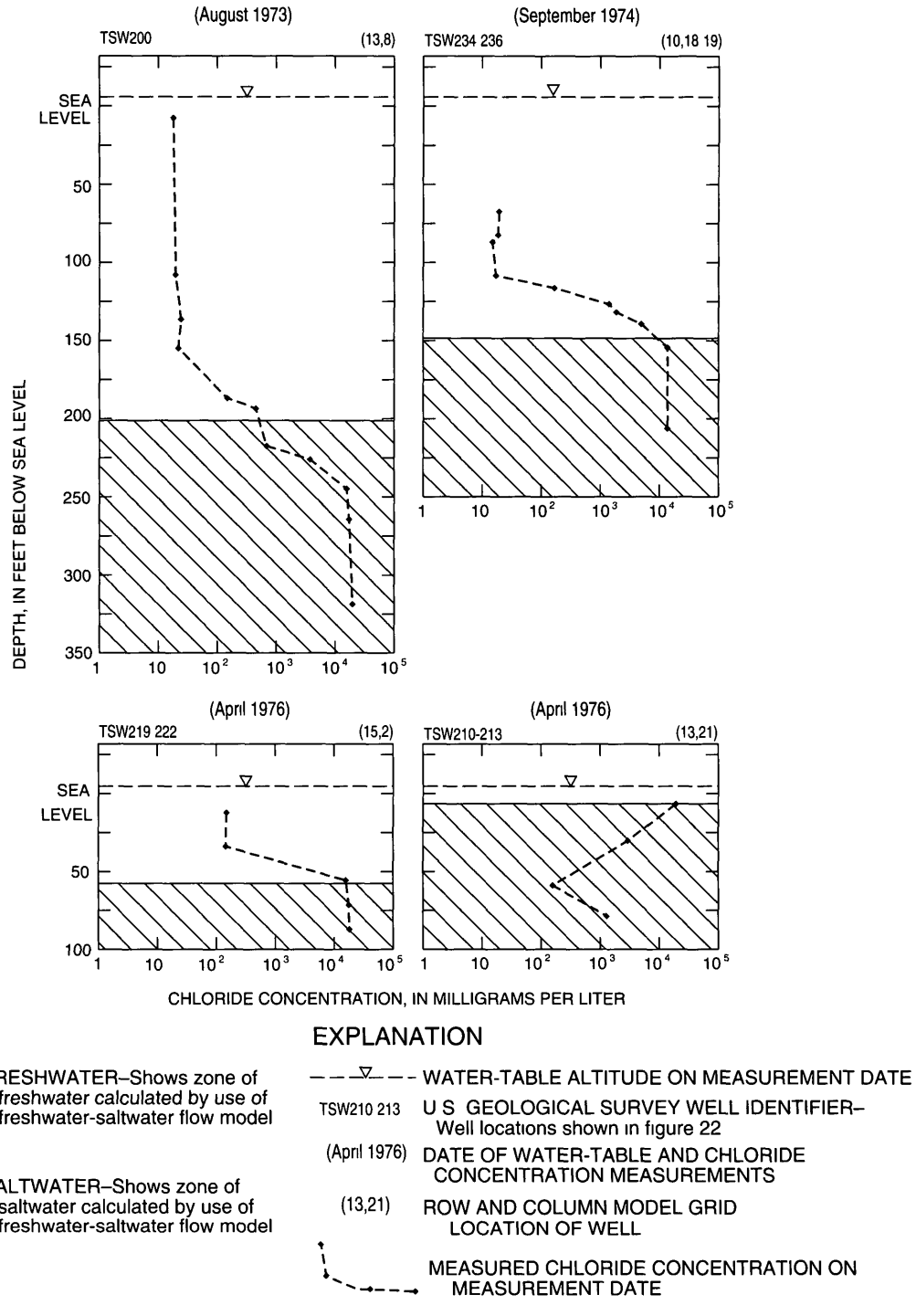


**Figure 23** Measured chloride concentrations and model-calculated freshwater and saltwater zones at selected observation wells in the Truro flow cell, Cape Cod Basin, Massachusetts

**Table 15** Model-calculated water budgets for the Truro flow cell, Cape Cod Basin, Massachusetts

[All values are in cubic feet per second]

| Budget item | Predevelopment | Year | | |
|---|---|---|---|---|
| | | 1975 | 1989 | 2020 |
| **Truro Flow Cell (Area = $2.7 \times 10^8$ ft$^2$)** | | | | |
| Inflow | | | | |
| Recharge | | | | |
| Natural | 14 9 | 14 9 | 14 9 | 14 9 |
| Wastewater return flow | 0 | 2 | 2 | 3 |
| Release from storage | 0 | 0 | 0 | 0 |
| Total inflow | 14 9 | 15 1 | 15 1 | 15 2 |
| Outflow | | | | |
| Pumpage | 0 | 1 4 | 1 2 | 1 7 |
| Coastal discharge | 12 3 | 11 6 | 11 9 | 11 3 |
| Subsea discharge | 2 1 | 2 0 | 1 9 | 1 9 |
| Storage | 0 | 0 | 0 | 0 |
| Total outflow | 14 4 | 15 0 | 15 0 | 14 9 |
| Model error (inflow minus outflow) | 0 5 | 0 1 | 0 1 | 0 3 |

## Response of the Freshwater-Saltwater Flow System to Simulated Ground-Water Pumping and Recharge

Changing stress conditions on the ground-water-flow system of the Truro flow cell show that both increases in ground-water pumping and decreases in the aquifer recharge result in (1) a decrease of discharge to coastal and subsea discharge boundaries (table 15), (2) a decline in water-table altitude (figs 24 and 25, table 14), and (3) a decrease in the depth to the freshwater-saltwater interface Pumping rates from the Truro flow cell generally increase from 1975 to 2020 because of projected increases in population and development Increases in pumping, however, had a negligible effect on the water-table altitude, position of the freshwater-saltwater interface, and coastal and subsea discharge despite the fact that about 86 percent of the total pumpage is lost from the ground-water-flow system The effect of changing stresses on the ground-water-flow system is most pronounced at the grid nodes where public-supply wells are located

The Knowles Crossing well field (TSW115) located 1,500 ft from the coast with a screen interval from 20 to 36 ft below sea level shows a model-calculated decline in water level of nearly 2 ft from the predevelopment flow conditions to 1975 The model-calculated depth to the freshwater-saltwater interface decreased from 90 to 35 ft below sea level, causing saltwater contamination at the well screen (fig 26) Simulated pumping rates were decreased for the 1975–89 stress condition, the model-calculated water-table altitude rose 1 3 ft, and the position of the interface increased from 35 to 65 ft below sea level during that period Although simulated pumping rates increased from 1989 to 2020, the model-calculated depth to the freshwater-saltwater interface had still increased, implying that the interface is still recovering from the 1975 stress condition

Seasonal fluctuations in the water budget show that during the off-season, aquifer recharge replenishes the ground water removed from storage during the increased stress of the in-season Although the seasonal change in aquifer recharge is significant, the effects on coastal and subsea discharge are minimal because of the changes in the storage component of the water budget Changes in the water-table altitude at the observation well nodes show an average seasonal fluctuation of 0 2 ft

The total simulated ground-water pumpage rates from the Truro flow cell from 1975 to the 1989 stress conditions were nearly equal Simulated pumping rates at Knowles Crossing, however, were decreased from 0 45 Mgal/d average daily demand in 1975 to 0 12 Mgal/d average daily demand in 1989 to reverse the effects of saltwater intrusion The simulated decrease in pumping at Knowles Crossing well field was offset by additional pumping of the public-supply well at the North Truro Air Force Base and increased pumping from the South Hollow well

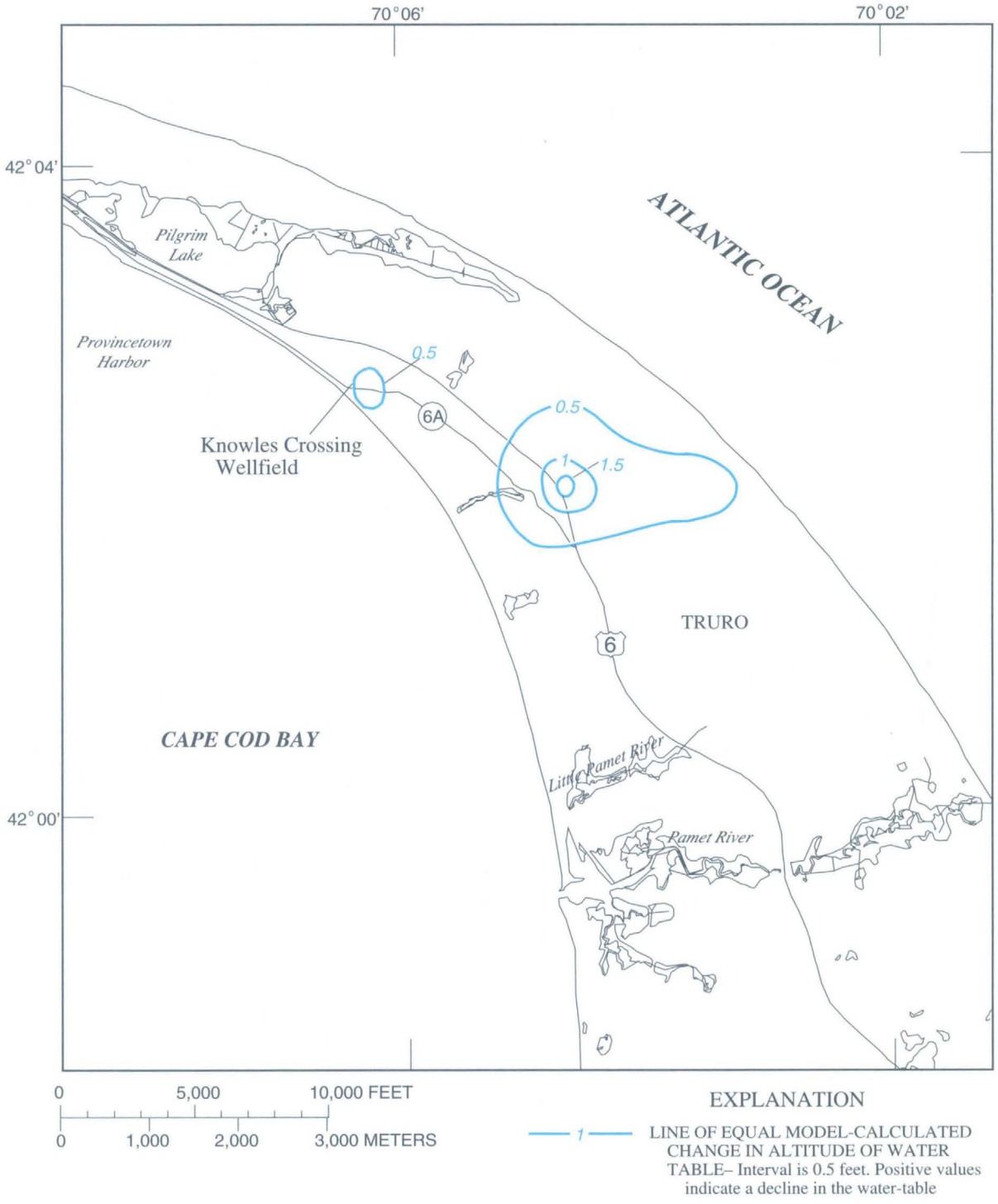


**Figure 24.** Model-calculated change in the altitude of the water-table configuration in the Truro flow cell from predevelopment to 1989, Cape Cod Basin, Massachusetts.

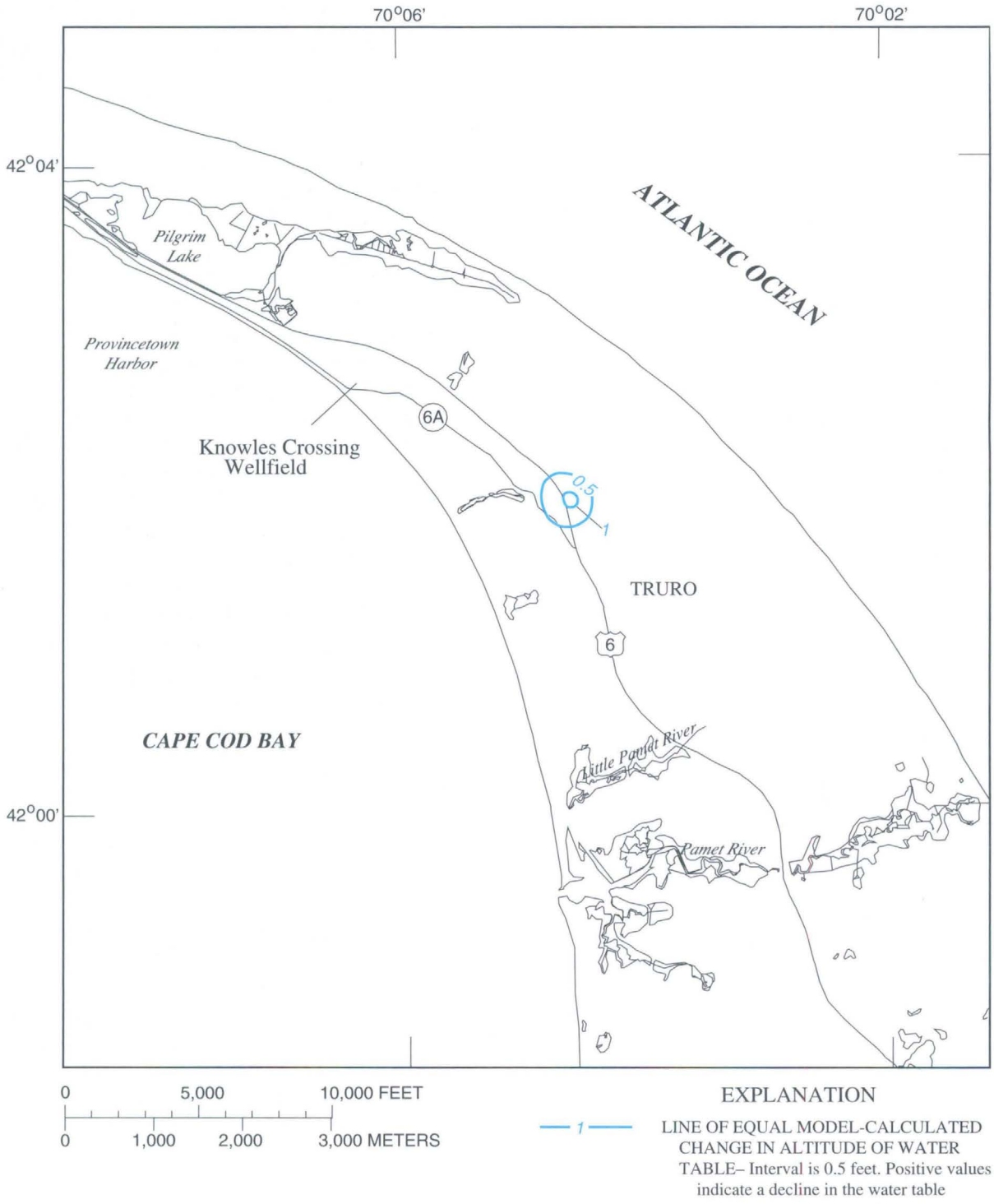


**Figure 25.** Model-calculated change in the altitude of the water-table configuration in the Truro flow cell from 1989 to 2020, Cape Cod Basin, Massachusetts.

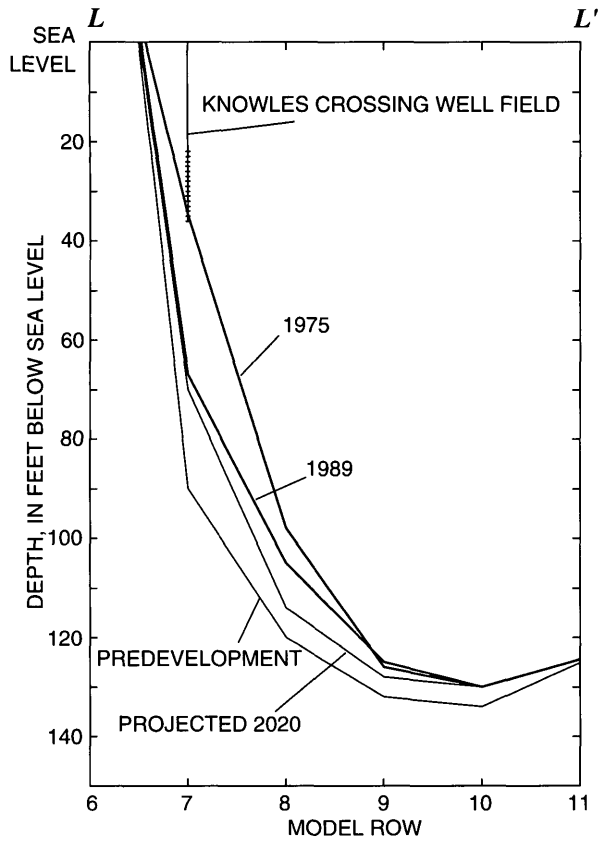


**Figure 26** Model-calculated position of the freshwater-saltwater interface along section *L-L'* (see fig 22) in the Truro flow cell for predevelopment,1975, 1989, and projected 2020 pumping and recharge rates, Cape Cod Basin, Massachusetts

The simulated redistribution of pumping from 1975 to 1989 resulted in an increase in the depth of the freshwater-saltwater interface at Knowles Crossing well field such that the well can continue to be used for public supply Water-quality data collected from water pumped from the well field showed a subsequent decrease in chloride concentration with a decrease in pumping rate at the well field (J M Cook, Provincetown Water District, written commun , 1992) The 1989 simulation shows that increased pumping at South Hollow well resulted in a model-calculated decrease in the depth to the freshwater-saltwater interface from 159 to 149 ft below sea level The change in simulated pumping at the North Truro Air Force Base resulted in a model-calculated decrease in the depth to the freshwater-saltwater interface from 175 to 167 ft below sea level No detectable response of the freshwater-saltwater interface to simulated fluctuations between in-season and off-season pumping and aquifer recharge rates occurred because the lag time in the response to changing stress conditions is longer than the simulated seasonal fluctuations

A hypothetical drought was simulated for a 5-year period for 1989–94 to determine the effects of a 30-percent decrease in natural recharge on the water budget, the head distribution, and the depth to the freshwater-saltwater interface The model-calculated effects of the simulated extended drought are most evident during the off-season pumping period During the off-season pumping periods for the nondrought simulation, aquifer recharge replenishes the water lost during the in-season pumping period During the off-season drought simulation, however, the rate of ground water replenished to storage decreased, resulting in a net loss of water to the system Coastal and subsea discharge decreased by 10 percent The comparison of the in-season nondrought to the in-season drought conditions shows that although the return-flow recharge is the same, the rate of ground water released from storage is decreased by nearly 11 percent during drought conditions As a result of the decreased recharge to the system, the coastal and subsea discharges were decreased by 7 to 20 percent

The average model-calculated decline in water level after 5 years of simulated drought conditions was about 0 5 ft, and the position of the freshwater-saltwater interface moved vertically upward by about 3 ft Model-calculated change in the position of the freshwater-saltwater interface was then compared at TSW89 for 15 years, from 1984 to 1999, for simulated nondrought and drought conditions (drought conditions were 1989–94) The model-calculated water-table altitude at the well responded quickly to the decrease in recharge and rose quickly at the end of the drought period as shown in figure 27 The model-calculated response of the interface to the decrease in aquifer recharge was much slower than that of the water table, and the interface position consequently took longer to recover after the simulated average (nondrought) recharge rate was resumed In addition, under simulated average in-season and off-season recharge conditions for this time period, a slight long-term decline in model-calculated ground-water altitude and a decrease in depth to the freshwater-saltwater interface were evident(fig 27) These findings may be attributed to the fact that on average, only 14 percent of the total simulated pumpage from the flow cell is returned to the aquifer

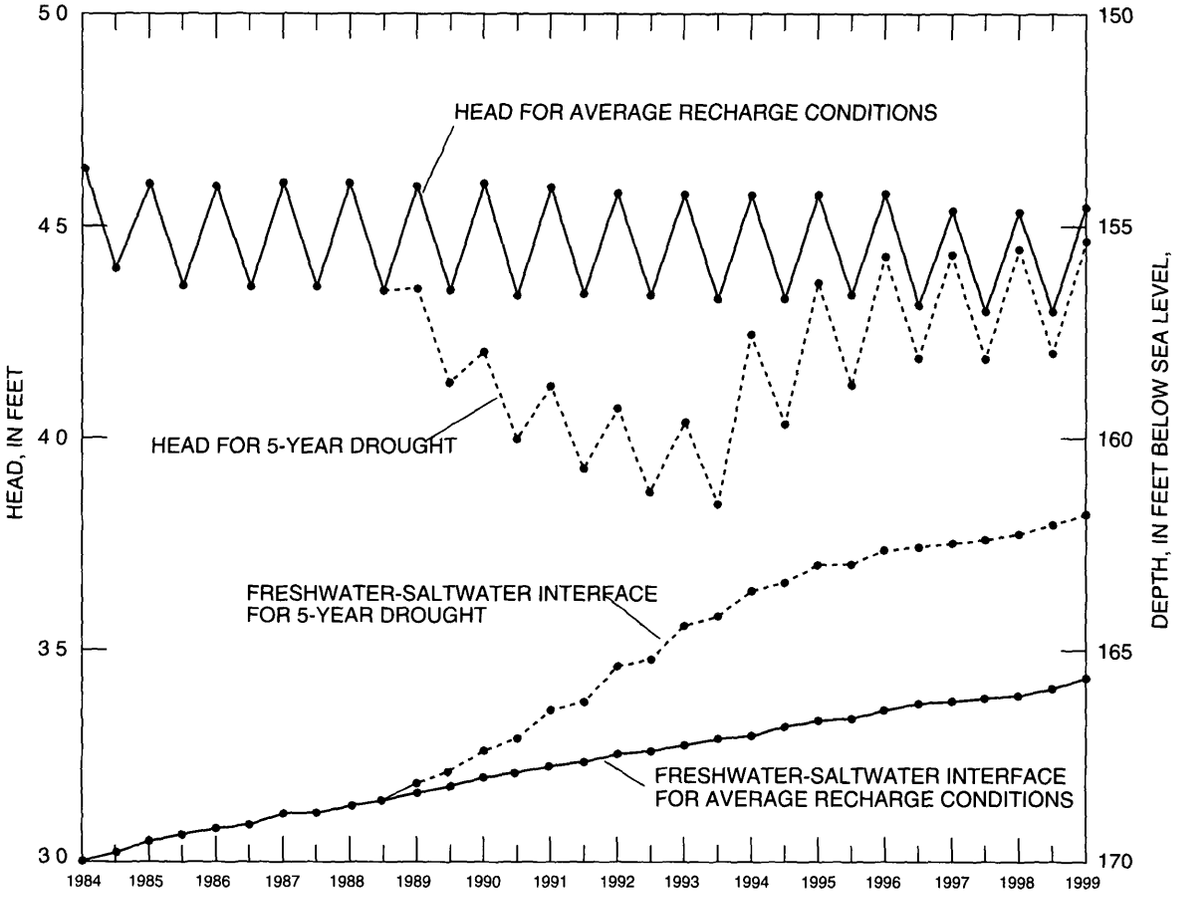


**Figure 27.** Model-calculated water-table altitude and position of the freshwater-saltwater interface at observation well TSW89 for average recharge conditions and for simulation of a 5-year drought and 1989 pumping rates, Cape Cod Basin, Massachusetts

Seasonal fluctuations in simulated pumping and recharge show that the cyclic depletion and replenishment of ground water in storage affects model-calculated water-table altitude and coastal and subsea discharge, yet has a minimal effect on the model-calculated position of the freshwater-saltwater interface The model-calculated freshwater-saltwater interface position was most sensitive to increases in simulated pumping rates as shown at the Knowles Crossing well

## Eastham and Wellfleet Flow Cells

The Eastham flow cell is south of the Wellfleet flow cell and north of the East Cape flow cell (fig 2) Blackfish Creek forms the ground-water divide separating the Eastham and Wellfleet flow cells

Town Cove and Boat Meadow River represent the ground-water divide between Eastham and East Cape Eastham is bounded on the west by Cape Cod Bay and to the east by the Atlantic Ocean and Nauset Harbor The unconsolidated sands, silts, and clays extend to the bedrock surface, which forms the lowest extent of the Eastham ground-water-flow system The general direction of ground-water flow is from the potentiometric high in the center of the flow cell toward the coastal discharge boundaries The ground-water divides at the northern and southern borders of the flow cell prevent ground-water flow between adjoining flow cells

The Wellfleet flow cell is south of the Truro flow cell and north of the Eastham flow cell as shown in figure 2 The Pamet River represents the ground-water divide separating the Wellfleet and Truro flow cells Wellfleet is bounded on the west by Cape Cod Bay and Wellfleet Harbor and to the east by the Atlantic Ocean

The water-table altitude ranges from 7 ft above sea level at the center of the flow cell to near 0 ft at the shore Under natural conditions, freshwater enters the ground-water-flow system at the water table as recharge from precipitation A series of small streams discharges to the Wellfleet Harbor The unconsolidated sand and gravel aquifer of the flow cell extends to the bedrock surface, which is assumed to be impermeable Results from Guswa and LeBlanc (1985) show, however, that the freshwater lens is bounded below by the freshwater-saltwater interface at about 280 ft below sea level

The analyses for the Eastham and Wellfleet flow cells were not as detailed as those of the East Cape, West Cape, and Truro flow cells because (1) no existing or projected large-capacity wells are planned for these areas, and (2) little hydrologic information has been collected for the Eastham flow cell since the work of Barlow (1994) and for the Wellfleet flow cell since the work of Guswa and LeBlanc (1985) Barlow (1994) recently updated the modeling work of Guswa and LeBlanc (1985) for the Eastham flow cell using hydrologic data collected since Guswa and LeBlanc (1985) A freshwater-flow model was constructed for the Wellfleet flow cell by updating the model data sets used by Guswa and LeBlanc (1985) and incorporating the freshwater-saltwater interface positions calculated by Guswa and LeBlanc (1985) into the updated model

## Description of Models

### Grids

The freshwater-flow model developed by Barlow (1994) for the Eastham flow cell consisted of 96 rows, 78 columns, and 5 layers (table 7, fig 28) The vertical discretization was based on available information on the lithology of the glacial deposits The horizontal discretization was finest in the areas of simulated hypothetical well sites to simulate the movement of water near these wells accurately

The Wellfleet freshwater-flow model consists of 26 rows, 32 columns, and 6 layers The Wellfleet flow model developed by Guswa and LeBlanc (1985) had seven layers (table 7, fig 29) The lowest layer was not part of the freshwater-flow system and therefore was not included in the freshwater model A uniform grid spacing of 1,320 by 1,320 ft was used

### Hydraulic Properties

The horizontal hydraulic conductivity and vertical conductance were estimated for the Eastham flow model developed by Barlow (1994) by comparing lithologic logs of test holes at 31 sites to generalized values of hydraulic conductivity for glacial sediments The transmissivity of layer 5 (0 4 $ft^2/d$) was determined by multiplying the average hydraulic conductivity of silt and clay sediment cores collected in Eastham by the total thickness of the layer Barlow's (1994) model did not simulate transient flow conditions, therefore, specific yield and storage coefficient for the flow system needed to be estimated for this analysis Assumed values of 0 15 for specific yield and of $2\times10^{-4}$ for storage coefficient were used, which are consistent with those used in the West Cape and East Cape models The range in horizontal hydraulic conductivity and vertical conductance used in the Eastham flow model for each layer is presented in table 7

The aquifer properties used for the Wellfleet flow model were derived by estimating values of hydraulic conductivity from available lithologic logs as discussed earlier in the section "Aquifer Hydraulic Properties " Specific yield and storage coefficient values were estimated from the average values used in the West Cape and East Cape flow models Specific yield was set at 0 15, and $2\times10^{-4}$ was used for the storage coefficient The range in horizontal hydraulic conductivity and vertical conductance used in the Wellfleet flow model for each layer is presented in table 7

### Boundary Conditions

The model boundaries for the Eastham flow model were determined by Barlow (1994) and are shown in figure 28 Specified head nodes were used to simulate saltwater bodies that surround much of the modeled area Equivalent freshwater heads were used at coastal saltwater boundaries to account for the higher density saltwater that overlies the freshwater system at the seabed discharge boundaries

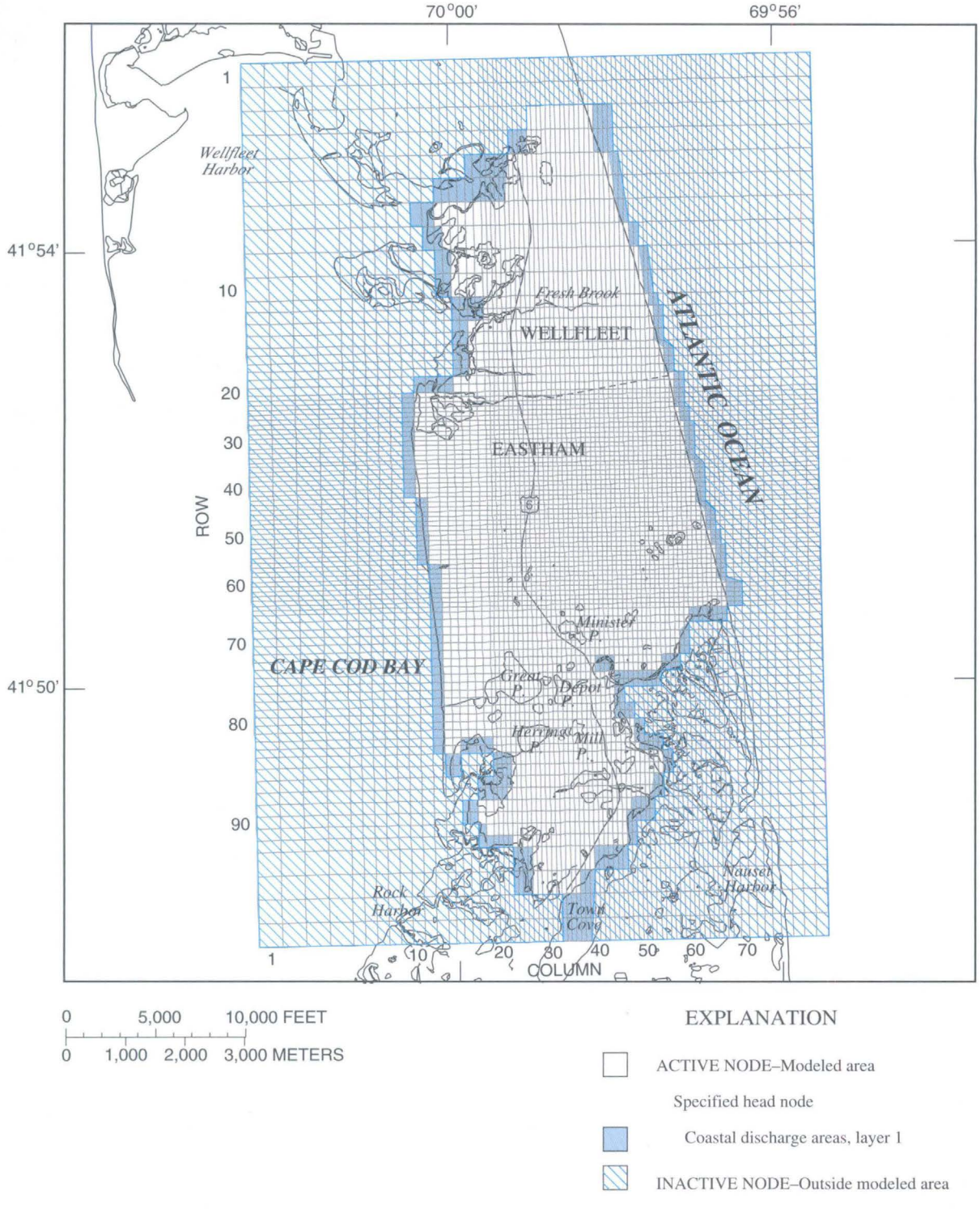


**Figure 28.** Grid and boundary conditions for the Eastham flow cell, Cape Cod Basin, Massachusetts. (Modified from Barlow, 1994.)

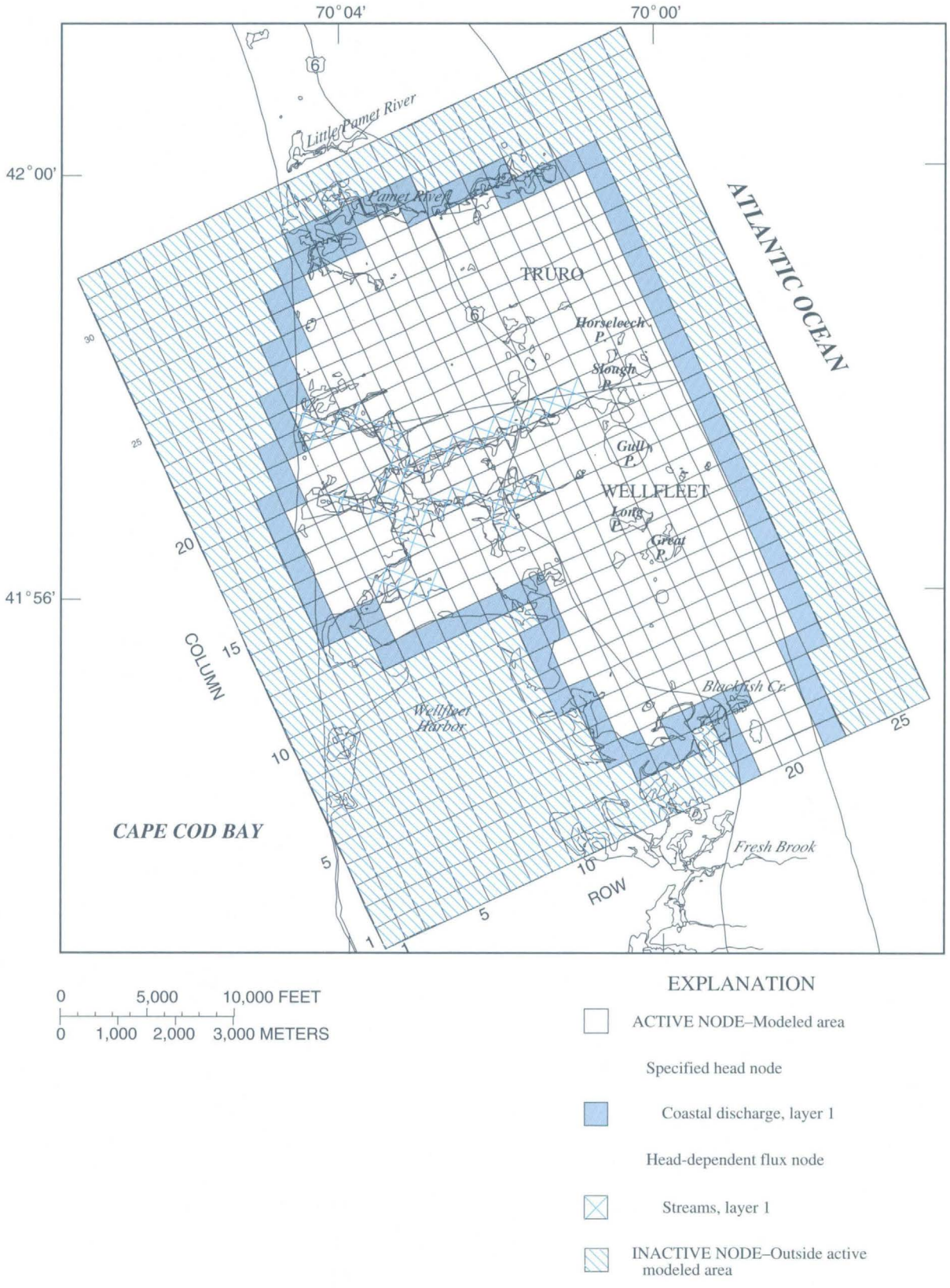


**Figure 29.** Grid and boundary conditions for the Wellfleet flow cell, Cape Cod Basin, Massachusetts.

These specified head nodes were used only in the top layer of the model The cells in layers 2 through 5 underlying these nodes remained active, and a no-flow boundary was used to simulate the freshwater-saltwater interface in the adjacent seaward nodes No-flow boundaries were specified at the northern and southern extents of the flow model to coincide with the natural divides separating Eastham from Wellfleet and East Cape The model is bounded above by the water table, which receives recharge at a rate equal to 18 3 in/yr, and below by a no-flow boundary that simulates the contact between glacial deposits and bedrock Ponds simulated in the top layer were assigned a recharge rate of only 12 4 in/yr to account for evaporation losses from the pond surfaces

The boundaries for the Wellfleet flow model were determined by Guswa and LeBlanc (1985) A no-flow (inactive) boundary was set at the ground-water divide that separates the Wellfleet flow cell from the Eastham flow cell The Pamet River, which forms the northern boundary of the flow cell, was simulated as a head-dependent flux boundary

Specified head nodes were used in the top layer to simulate the saltwater bodies of Cape Cod Bay, Wellfleet Harbor, and the Atlantic Ocean Although the movement of the freshwater-saltwater interface was not calculated, equivalent freshwater heads were used at the coastal saltwater boundaries to account for the higher density saltwater that overlies the freshwater system at the seabed discharge boundaries (fig 29) The equivalent heads used in the Wellfleet model were obtained from Guswa and LeBlanc (1985)

The freshwater-saltwater interface position calculated by Guswa and LeBlanc (1985) was used to determine the extent of the freshwater-flow system The freshwater-saltwater interface altitude represents the lowest extent of the freshwater-flow system and was specified as a no-flow boundary based on the results of Guswa and LeBlanc (1985) Nodes that were calculated to consist of freshwater were kept active, whereas those calculated to consist of saltwater were made inactive All subsequent nodes below a saltwater node were considered inactive

The Wellfleet model is bounded on top by the water table, which is a free-surface boundary that receives recharge at an estimated rate of 19 8 in/yr This recharge rate is equal to that determined for the adjoining Truro flow cell Nodes containing ponds were assigned a reduced recharge rate of 13 9 in/yr to account for evaporation at the pond surface A series of small streams and wetlands intersect the water table, drain the top layer, and discharge to the Wellfleet Harbor These surface-water bodies are ground-water fed and act as local sinks to the freshwater system Discharge in these nodes was determined by the difference between specified head in the stream and calculated head in the aquifer and by the calculated conductance between the streambed and the aquifer

#### Stresses and Stress Periods

Steady-state simulations for the Wellfleet and Eastham flow cells were made to determine the water budget and head distribution for long-term average recharge conditions The water-supply needs of the residents of the towns of Eastham and Wellfleet are met by private wells Wastewater is returned to the ground-water-flow system by on-site septic systems, the net loss to the system was assumed to be negligible The results from the steady-state simulations then were used as the initial conditions for a transient analysis, in which a 30-percent decrease in recharge was simulated for 5 years, as discussed for the West and East Cape freshwater-flow models

#### Calibration

The freshwater model for the Eastham flow cell was calibrated by comparing model-calculated water levels to measured water levels and pond altitudes at 21 locations in the flow cell measured in 1988 (fig 30) Barlow (1994) provides a more detailed discussion on the calibration of the Eastham flow model

The aquifer properties used in the Wellfleet flow model were calibrated by comparing the model-calculated water-table altitude at the five observation wells and five pond levels shown in figure 31 with measured water levels The water-table altitudes reported for the observation wells used in calibration represent the average water levels measured from 1963 to 1976 and are consistent with the water levels used for calibration for the West Cape, East Cape, and Truro flow models

Total inflows to the Eastham and Wellfleet flow cells calculated by the freshwater models for steady-state flow conditions are 22 1 and 27 5 $ft^3/s$, respectively (table 16) Nearly one-half of the freshwater discharge from the Wellfleet flow cell is to the several streams in the flow cell

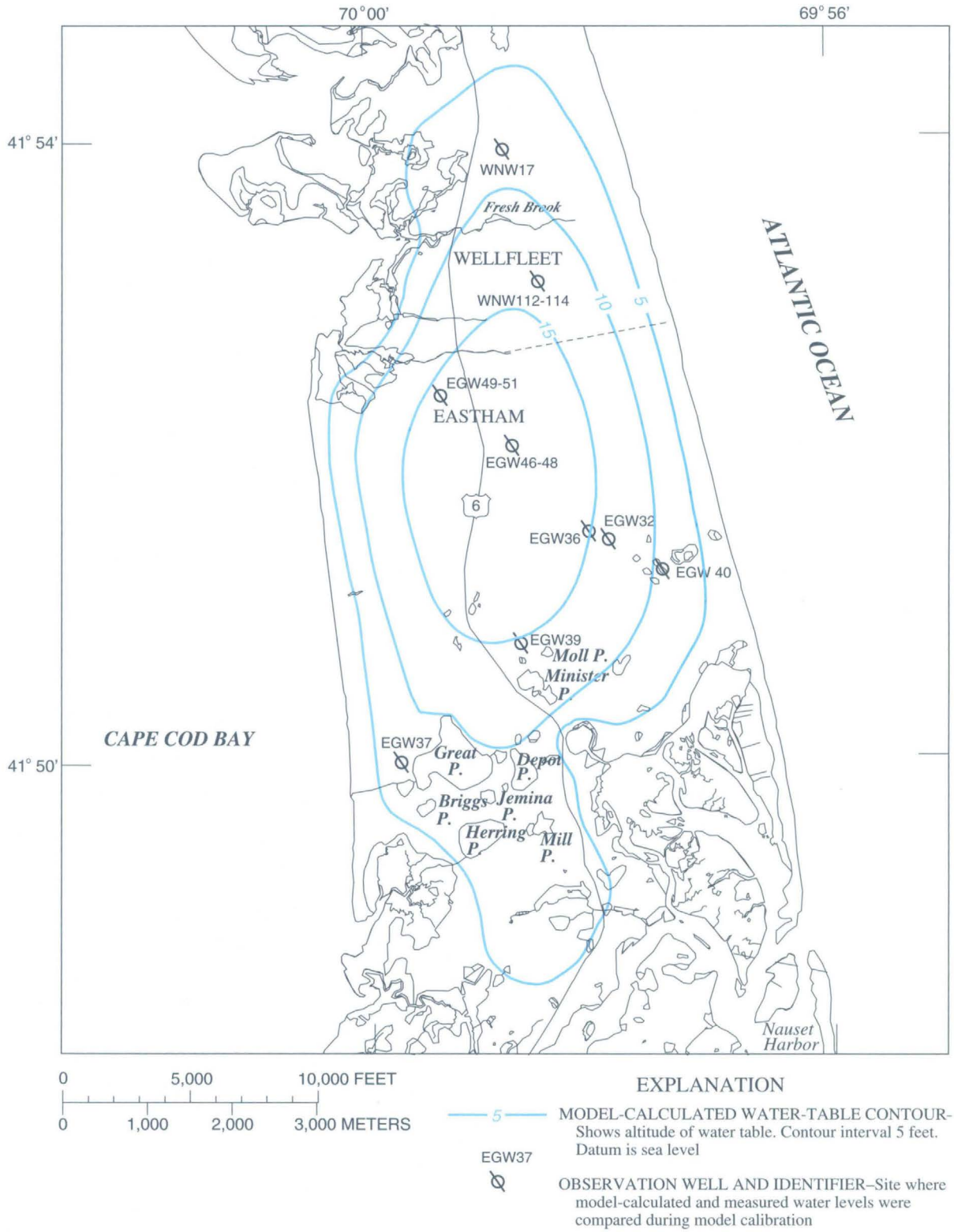


**Figure 30.** Model-calculated steady-state water-table configuration for the Eastham flow cell, Cape Cod Basin, Massachusetts.

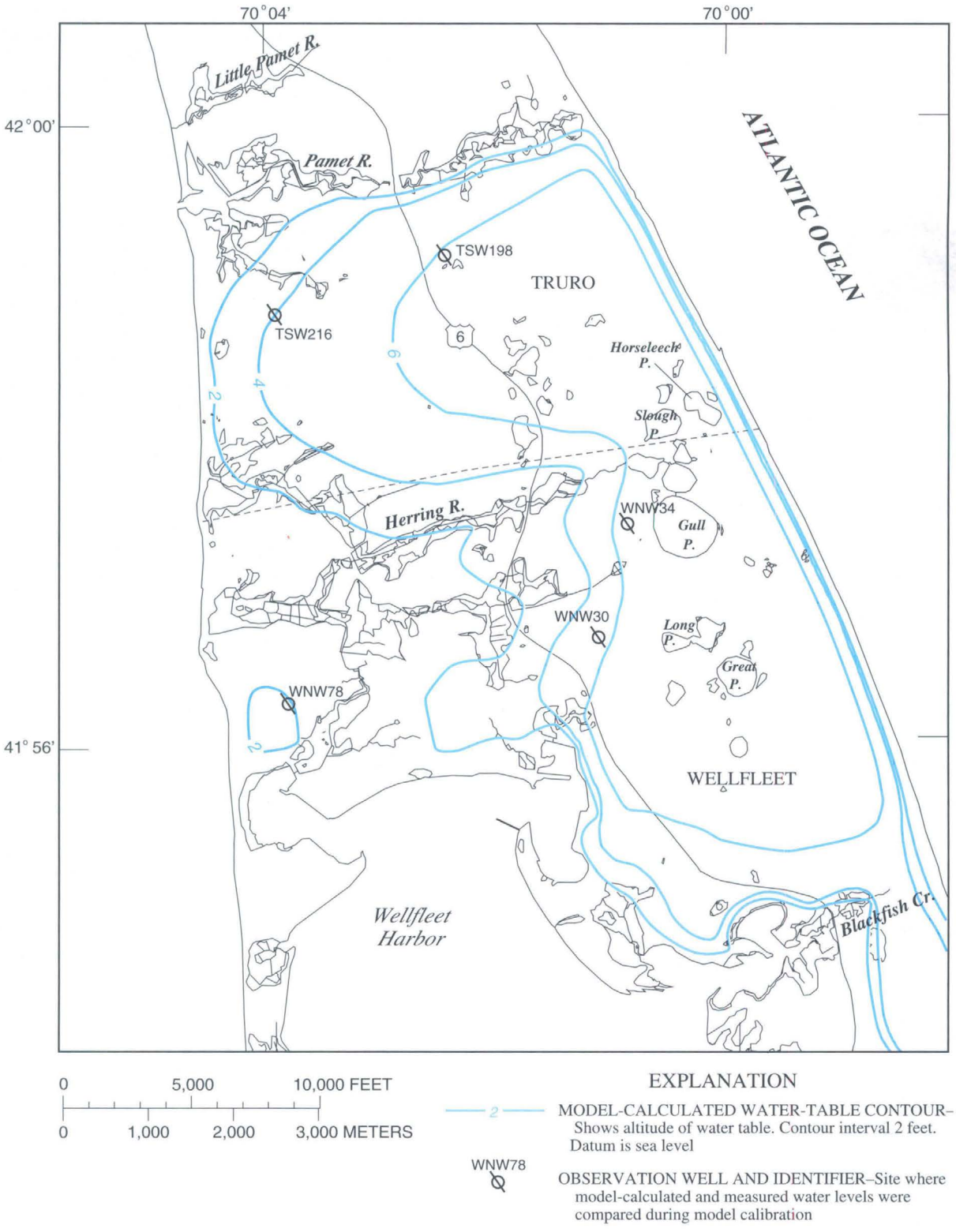


**Figure 31.** Model-calculated steady-state water-table configuration for the Wellfleet flow cell, Cape Cod Basin, Massachusetts.

**Table 16** Model-calculated water budgets for the Eastham and Wellfleet flow cells, Cape Cod Basin, Massachusetts

[All values are in cubic feet per second]

| Budget item | Average recharge conditions | 30-percent decrease in recharge after 5 years |
|---|---|---|
| **Eastham Flow Cell (Area = $4.6 \times 10^8$ ft$^2$)** | | |
| Inflow | | |
| Recharge | 22 1 | 15 4 |
| Release from storage | 0 | 1 1 |
| Total inflow | 22 1 | 16 5 |
| Outflow | | |
| Coastal discharge | 22 1 | 16 6 |
| Storage | 0 | 0 |
| Total outflow | 22 1 | 16 6 |
| Model error (inflow minus outflow) | 0 | - 1 |
| **Wellfleet Flow Cell (Area = $5.4 \times 10^8$ ft$^2$)** | | |
| Inflow | | |
| Recharge | 27 5 | 19 3 |
| Release from storage | 0 | 1 |
| Total inflow | 27 5 | 19 4 |
| Outflow | | |
| Streams | 13 0 | 9 1 |
| Coastal discharge | 14 6 | 10 5 |
| Storage | 0 | 0 |
| Total outflow | 27 6 | 19 5 |
| Model error (inflow minus outflow) | - 1 | - 1 |

### Simulation of the Freshwater-Flow Systems

The response of the Eastham and Wellfleet flow cells to simulated stress conditions was determined by comparing the model-calculated water-table altitude at the observation wells and ponds and the model-calculated water budgets for long-term average recharge conditions and for hypothetical drought conditions Drought conditions of a 30-percent decrease in the average recharge rate were simulated for 5 years

The change in model-calculated water-table altitude from unstressed to stressed conditions for each of the flow cells is shown in table 17 The model-calculated decline in water-table altitude is largest near the center of the flow cells and diminishes toward the coast The model-calculated average decline in water-table altitude was 2 6 ft for the Eastham flow cell and 1 0 ft for the Wellfleet flow cell Although the model-calculated decline in water levels in Eastham was nearly triple that of Wellfleet, the decline was 18 percent of the total relief of the water table in the Eastham flow cell and 14 percent of the total relief of the water table in the Wellfleet flow cell

Model-calculated streamflow was decreased 30 percent and model-calculated coastal discharge by 28 percent in the Wellfleet flow cell (table 16) for the simulated drought conditions In the Eastham flow cell, the model-calculated coastal discharge was decreased 25 percent Although simulated recharge into the Eastham flow cell was decreased by 30 percent, the total decrease in outflow for the flow cell was about 25 percent Water released from storage accounts for the additional source of water into the system in response to the simulated reduction of natural recharge If the simulated drought conditions were continued until the ground-water system was in equilibrium (no change in storage) then the 30-percent decrease in recharge (inflow) would ultimately result in a 30-percent decrease in outflow

## Limitations of the Numerical Modeling Analyses for the Cape Cod Basin

The Cape Cod flow models were developed to provide a regional assessment of the response of the ground-water-flow system to changing stress conditions These flow models were not designed to provide detailed analyses of the effects of local stresses on specific areas in the flow system

Four of the five flow models (not including Eastham) have uniform horizontal grid spacing of 1,320 by 1,320 ft with five to seven layers subdividing the system from the water table to the bedrock or freshwater-saltwater boundary The discretized estimates of hydraulic properties and the contact between the unconsolidated deposits and bedrock were intended to provide an approximation of the actual flow system and should not be used to provide exact values of these parameters at specific locations in the flow cells

Ground-water models developed for each of the flow cells assumed no-flow conditions in the underlying bedrock As a result, the possibility of vertical saltwater intrusion in locations in the West Cape and East Cape flow cells, where freshwater extends to the top of the bedrock, cannot be assessed

**Table 17** Average measured water levels for selected wells, 1963–76, measured pond levels, and model-calculated water levels and pond levels for average recharge rates and for a 5-year drought for the Eastham and Wellfleet flow cells, Cape Cod Basin, Massachusetts

| Well No or pond site | Model node | | | Average measured water level, in feet above sea level | Model-calculated water level, in feet above sea level | | Decrease in model-calculated water levels between average and 30-percent decrease in recharge, in feet |
|---|---|---|---|---|---|---|---|
| | Layer | Row | Column | | Average recharge conditions | 30-percent decrease in recharge, after 5 years | |
| **Eastham Flow Cell** | | | | | | | |
| **Observation Well** | | | | | | | |
| Town of Eastham | | | | | | | |
| EGS49 | 1 | 29 | 20 | 16 9 | 15 6 | 12 6 | 3 0 |
| EGW50 | 2 | 29 | 20 | 16 9 | 15 4 | 12 4 | 3 0 |
| EGW51 | 4 | 29 | 20 | 17 2 | 15 4 | 12 4 | 3 0 |
| EGW46 | 1 | 35 | 33 | 17 7 | 18 4 | 15 0 | 3 4 |
| EGW47 | 2 | 35 | 33 | 17 6 | 18 4 | 15 0 | 3 4 |
| EGW48 | 3 | 35 | 33 | 17 6 | 18 4 | 15 0 | 3 4 |
| EGW32 | 1 | 49 | 48 | 12 8 | 13 4 | 10 7 | 2 7 |
| EGW36 | 1 | 48 | 44 | 13 9 | 15 3 | 12 3 | 3 0 |
| EGW37 | 1 | 75 | 17 | 8 2 | 8 3 | 6 4 | 1 9 |
| EGW39 | 1 | 65 | 33 | 13 9 | 14 3 | 11 5 | 2 8 |
| EGW40 | 2 | 55 | 55 | 8 6 | 9 2 | 7 3 | 1 9 |
| Town of Wellfleet | | | | | | | |
| WNW17 | 2 | 9 | 32 | 8 5 | 9 3 | 7 2 | 2 1 |
| WNW112 | 1 | 14 | 37 | 13 8 | 13 5 | 10 7 | 2 8 |
| WNW113 | 2 | 14 | 37 | 13 7 | 13 5 | 10 7 | 2 8 |
| WNW114 | 3 | 14 | 37 | 13 7 | 13 5 | 10 7 | 2 8 |
| **Pond** | | | | | | | |
| Mill | 1 | 81 | 37 | 10 2 | 8 1 | 6 3 | 1 8 |
| Moll | 1 | 65 | 38 | 13 2 | 13 6 | 10 9 | 2 7 |
| Jemima | 1 | 78 | 29 | 9 1 | 9 7 | 7 6 | 2 1 |
| Briggs | 1 | 59 | 23 | 15 5 | 15 4 | 12 4 | 3 0 |
| Minister | 1 | 64 | 36 | 12 6 | 12 0 | 9 6 | 2 4 |
| Great | 1 | 75 | 23 | 8 6 | 10 0 | 7 9 | 2 1 |
| **Wellfleet Flow Cell** | | | | | | | |
| **Observation Well** | | | | | | | |
| Town of Wellfleet | | | | | | | |
| WNW78 | 1 | 6 | 16 | 2 7 | 2 3 | 1 9 | 0 4 |
| WNW30 | 1 | 15 | 14 | 6 6 | 5 4 | 4 3 | 1 1 |
| WNW34 | 1 | 18 | 17 | 8 0 | 6 9 | 5 4 | 1 5 |
| Town of Truro | | | | | | | |
| TSW216 | 1 | 11 | 26 | 4 1 | 4 5 | 3 5 | 1 0 |
| TSW198 | 1 | 16 | 26 | 7 6 | 6 1 | 4 9 | 1 2 |
| **Pond** | | | | | | | |
| Town of Wellfleet | | | | | | | |
| Gull | 1 | 19 | 16 | 6 0 | 7 7 | 6 0 | 1 7 |
| Great | 1 | 19 | 11 | 8 0 | 8 0 | 6 2 | 1 8 |
| Long | 1 | 18 | 13 | 8 0 | 7 9 | 6 1 | 1 8 |
| Town of Truro | | | | | | | |
| Horseleech | 1 | 21 | 19 | 6 0 | 7 3 | 5 7 | 1 6 |
| Slough | 1 | 20 | 19 | 6 0 | 7 3 | 5 7 | 1 6 |

In addition to hydraulic properties, streamflow and pumping well locations were controlled by model discretization Streams in the Cape Cod ground-water-flow system are typically shallow and narrow Representation of these streams results in coarse approximations in streambed conductance and streambed altitude, and, therefore, stream discharge Pumping wells simulated in the flow models are assumed to be pumping at the center of each node, and therefore, can provide only an approximation of the actual pumping locations

Pumping and recharge rates also are approximations of the actual stresses on the ground-water-flow system Three stress conditions were used to represent 71 years (1950–2020) of changing stresses on Cape Cod This coarse representation resulted in periods of overestimated stresses from 1950 to 1974, 1982 to 1988, and 2005 to 2019, and underestimated stresses from 1976 to 1981 and 1990 to 2004 As a result of this temporal discretization, the effects of changing stress conditions for specific years are not possible In-season and off-season stress periods were used to simulate seasonal fluctuations in pumping and recharge for the 1989 stress conditions For the in-season stress condition, the summer pumping rate was assumed to occur from June through August with the only recharge to the system from wastewater-return flow For the off-season stress period, the winter pumping rate was assumed to occur from September through May, and all natural recharge and off-season wastewater-return flow were assumed to occur Natural recharge, however, is assumed to occur only from November through May (Barlow and Hess, 1993) Therefore, to accurately represent seasonal fluctuations in both pumping and recharge, monthly stress periods would have to be implemented

Wastewater-return-flow estimates also provide a coarse approximation of the percentage and location of the actual artificial recharge to the ground-water-flow system from wastewater-treatment facilities and domestic septic systems The septic-system return-flow approximations were based on the total length of roads in each water district and do not account for housing/population density

The limitations of the boundary condition representing the freshwater-saltwater interface for the West and East Cape freshwater-flow models are discussed in the section "Boundary Conditions " This boundary condition, however, was based on the projected 2020 pumping rates discussed in the section "Ground-Water Pumping" and substantial changes in the projected pumping rates or well locations may produce results that are significantly different from those reported here

The boundaries simulating the freshwater-saltwater interface in the Wellfleet and Eastham flow models were simpler than those used in the West Cape and East Cape flow models The boundary used in the Wellfleet flow model was based on the freshwater-saltwater interface calculated by Guswa and LeBlanc (1985) The boundary separating the freshwater- and saltwater-flow systems in the Eastham model was assumed to coincide with the shoreline and extend to bedrock (Barlow, 1994) The freshwater-saltwater interface boundary for both flow models was assumed to be static and was not designed to account for increased stresses to the ground-water-flow system

## EFFECTS OF SIMULATED GROUND-WATER PUMPING AND RECHARGE ON MARTHA'S VINEYARD AND NANTUCKET ISLAND BASINS

The analysis of the effects of pumping on the ground-water-flow systems of Martha's Vineyard and Nantucket Island Basins was completed using two-dimensional, finite-difference flow models that calculate changes (or drawdowns) in ground-water levels resulting from pumping These "change models" do not calculate absolute water levels nor were they calibrated against measured water levels in the basins The models were used because available data were insufficient to construct three-dimensional, calibrated models that simulate flow in the basins The use of the models is an improvement over the use of simple analytical models to calculate drawdowns resulting from pumping, however, because (1) multiple pumping wells may be simulated simultaneously, and (2) surface-water boundaries may be represented easily in the finite-difference models

The change models assume that pumping rates and drawdowns are linearly related For linear flow systems, a change model produces valid results In nonlinear systems, however, the effects of stresses cannot be calculated independently (Reilly and others, 1987), that is, the effect of a stress depends on all the conditions occurring when the stress is applied The ground-water-flow systems simulated in this study are nonlinear because they include a water table and the transmissivity of the aquifer depends on the saturated thickness Nevertheless, the change-model approach was used as an approximation to the response of the basins to pumping Although predicted changes will

not be exact, they provide a reasonable basis for evaluating effects of pumping and recharge on the simulated ground-water-flow systems.

The model of the Martha's Vineyard Basin consists of 70 rows and 74 columns and has a uniform grid spacing of 590 ft. The model of the Nantucket Island Basin consists of 30 rows and 36 columns and has a uniform grid spacing of 810 ft. The grid spacing and model dimensions were selected arbitrarily to reference the location of the pumping wells and to cover a large enough area to minimize the effects of model boundaries on model-calculated drawdowns. Constant values of horizontal hydraulic conductivity and initial saturated thicknesses were used throughout the models. A horizontal hydraulic conductivity of 500 ft/d was used in the model of the Martha's Vineyard Basin, and a horizontal hydraulic conductivity of 240 ft/d was used in the model of the Nantucket Island Basin. The initial water-table altitude was set at 0 ft in each flow model. A bottom altitude of 70 ft was set for the model of Martha's Vineyard (representative of the primary sand and gravel aquifer), and -250 ft was set for the model of Nantucket Island (representative of the shallow aquifer). These horizontal hydraulic conductivities and initial saturated thicknesses were assigned on the basis of average transmissivities of 35,000 and 60,000 $ft^2/d$ estimated from aquifer tests at pumping wells in the Martha's Vineyard and Nantucket Island Basins, respectively. A uniform value of 0.2 for specific yield was used for both models. Specified-head nodes with constant water levels of 0 ft were used at saltwater boundaries; no-flow boundaries were used to separate modeled from unmodeled areas of the flow basins. Simulations of each basin were made for conditions of 180 days of no recharge and pumping rates equal to those projected to occur in 2020 during summer (in-season) months.

Model-calculated changes in the altitude of the water table for the projected pumping rates and 180 days of no recharge are shown for the modeled areas of Martha's Vineyard and Nantucket Island Basins in figures 32 and 33, respectively. The changes in water-table altitudes were not superimposed on the current water-table maps (figs. 6 and 7) because of the limited number of observation wells used in the development of the maps for each basin. Maximum declines occur at pumping wells and decrease with distance from the wells. The largest model-calculated declines for the Martha's Vineyard Basin were 2.7 ft near the proposed Manter site in Tisbury. The largest model-calculated declines for the Nantucket Island Basin were 1.6 ft near the proposed Wannacomet well. These maximum declines represent a small percentage of the total simulated saturated thickness in each flow basin.

Model-calculated changes in the altitude of the water table near Tisbury, Oak Bluffs, and Edgartown public-supply wells on Martha's Vineyard (fig. 32) show interference from nearby wells

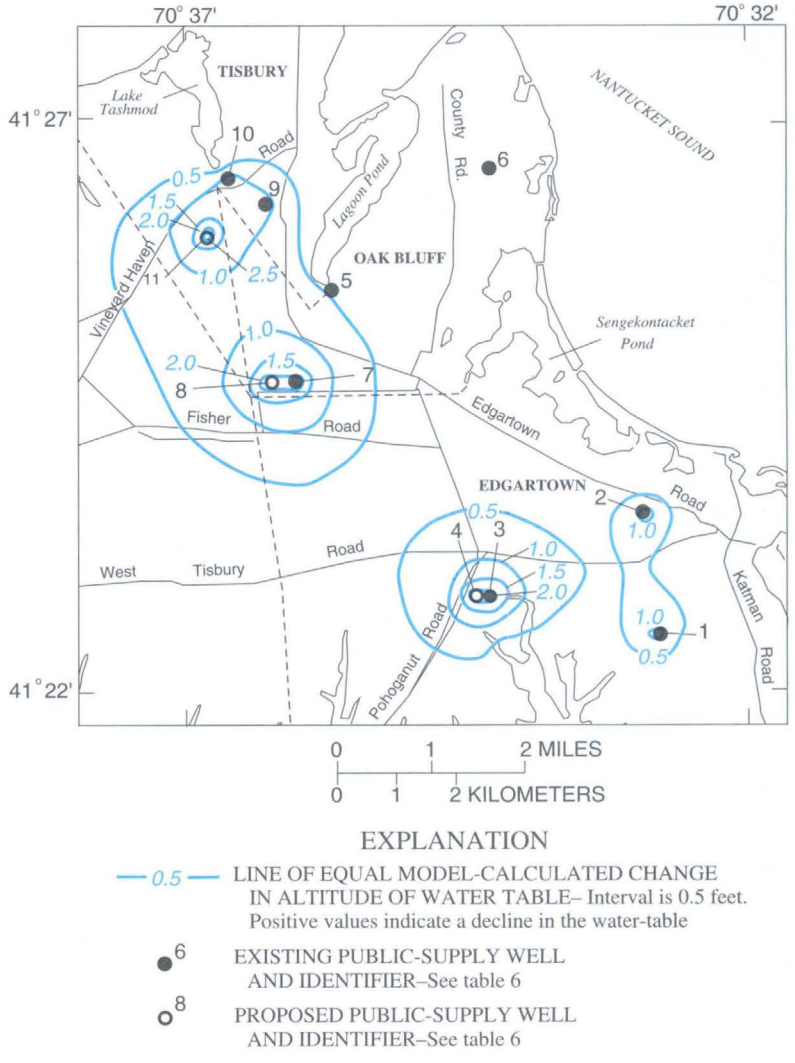


**Figure 32.** Model-calculated change in the altitude of the water-table configuration for projected 2020 summer pumping rates and 180 days of zero recharge in the Martha's Vineyard Basin, Massachusetts.

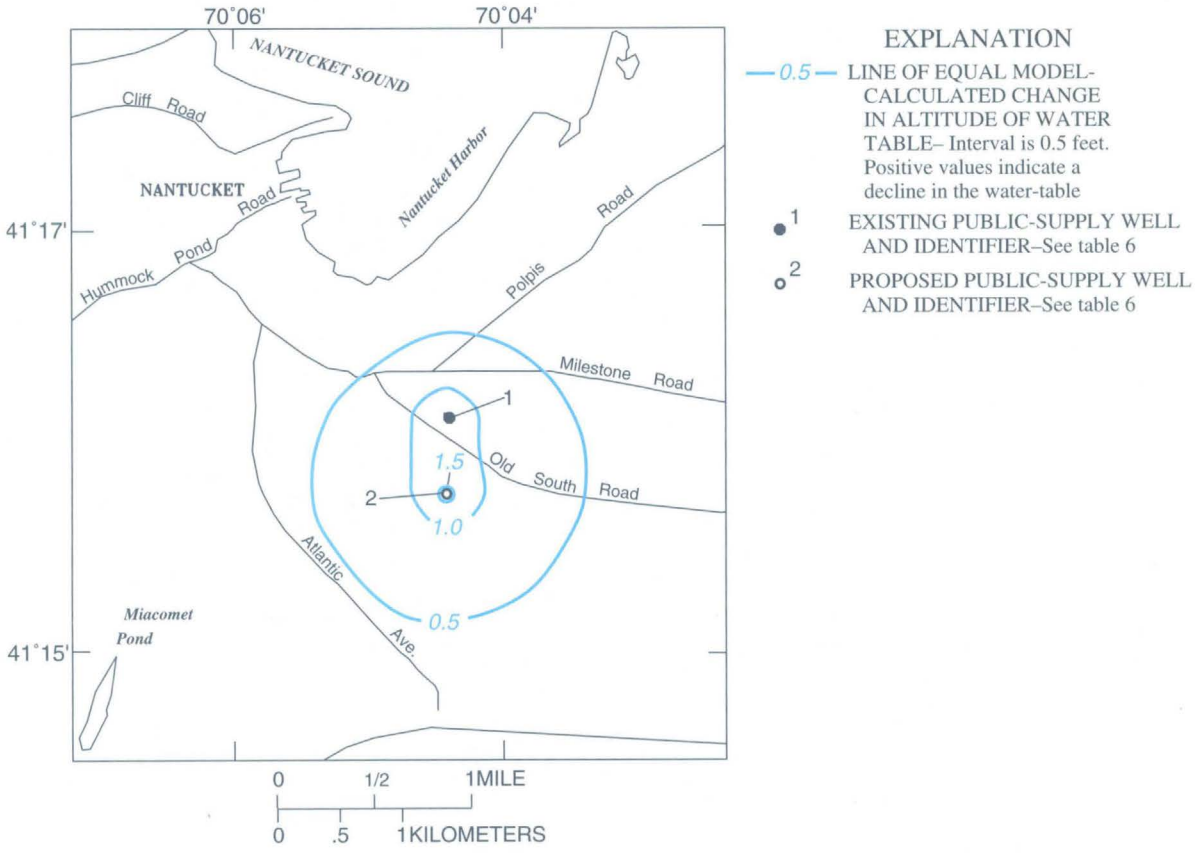


**Figure 33.** Model-calculated change in the altitude of the water-table configuration for projected 2020 summer pumping rates and 180 days of zero recharge in the Nantucket Island Basin, Massachusetts.

because of the proximity of the simulated pumping wells to one another and simulated high (in-season) pumping rates. Simulated well interference also occurs between the existing and proposed Wannacomet wells in the Nantucket Island Basin (fig. 33). Model-calculated declines are likely to be somewhat greater than those that would actually occur in the real flow basins because no-flow boundaries were simulated around the edge of each model. In the real flow basins, declines would be less at these boundaries than those calculated by the models because of the availability of water to the wells from areas of the aquifers outside of the modeled areas. Also, the simulations indicate that pumping at the wells would cause a flow of water from surrounding saltwater boundaries to the flow basins. Although these results must be viewed in light of the limitations of the change-model approach, the possible movement of the interface between freshwater and saltwater resulting from increased pumping from the flow basins should be considered in future evaluations of water-resource management options in the basins.

Metcalf and Eddy, Inc. (1972) estimated the total inflow to the Martha's Vineyard Basin aquifer system to be 58 Mgal/d. Total simulated in-season pumping from the aquifer in the year 2020 was 6.5 Mgal/d, or 11 percent of the estimated total inflow. Horsley Witten Hegemann, Inc. (1990) estimated the total inflow to the Nantucket Island Basin aquifer system to be 56 Mgal/d. Total simulated in-season pumping from the aquifer in the year 2020 was 2.4 Mgal/d, or 4 percent of simulated total inflow. Most of the water pumped from the flow basins is returned to them as wastewater-return flow through septic systems and wastewater-treatment facilities. Consequently, little water is actually lost from the flow basins through consumptive use. The analysis used in this investigation does not consider wastewater-return flow as a source of water to the flow basins; therefore, model-calculated changes in water-table altitudes are likely to be larger than if wastewater-return flow had been included.

## SUMMARY AND CONCLUSIONS

The management and protection of water resources of Cape Cod, Martha's Vineyard, and Nantucket Island water-resource planning basins are of concern to Massachusetts State and local officials because ground water is the sole source of drinking water in the basins Significant growth in the number of summer and permanent residents has resulted in an increase in water use during the last 30 years and has placed stresses on the ground-water resources In particular, the extent of long-term declines in ground-water and pond levels and in the quantity of streamflow, as well as in the possibility of saltwater intrusion from the surrounding ocean, are matters of concern Effects of simulated ground-water pumping and recharge on the ground-water-flow system were assessed for the Cape Cod, Martha's Vineyard, and Nantucket Island Basins Five of the six flow cells of Cape Cod Basin were assessed—the West Cape, East Cape, Eastham, Wellfleet, and Truro flow cells These effects are reported as (1) changes in water-table altitudes in the three basins, (2) changes in pond altitudes and streamflow for selected ponds and streams of Cape Cod Basin, (3) changes in the sources and sinks of water in Cape Cod Basin, and (4) changes in the position of the freshwater-saltwater interface in the West Cape, East Cape, and Truro flow cells of Cape Cod Basin The water quality in the Provincetown flow cell is such that it is not used for public-water supplies, and no analysis of this flow cell was provided

Cape Cod Basin consists of glacial deposits ranging in size from clay to boulders deposited during the glaciation of the Pleistocene Epoch The glacial deposits overlie an igneous and metamorphic basement complex whose surface ranges in depth from 100 ft below sea level near Cape Cod Canal to more than 900 ft below sea level near Provincetown The bedrock is overlain by a thin veneer of basal till, which is overlain in turn by thick deposits of fine sand, silt, and clay that were deposited in a proglacial lake Extensive deposits of stratified drift were deposited on the fine-grained lake deposits by meltwater streams

The coarse-grained stratified-drift deposits are generally highly transmissive to water and have high storage capacities Horizontal hydraulic conductivity of the stratified drift ranges from about 40 ft/d for fine sand and silt to about 380 ft/d for medium to coarse sand and gravel Estimates of the ratio of vertical to horizontal hydraulic conductivity range from 1 1 to 1 30 for all sediment classes except fine sand and silt, for which the estimated ratio is 1 50 There is a general increase in horizontal hydraulic conductivity with increase in grain size Specific yield of the stratified drift, as estimated from 10 aquifer tests, ranges from 0 02 to 0 25, and is consistent with estimates made by previous investigators

The Cape Cod ground-water-flow system consists of six flow cells that are hydraulically distinct under present-day hydrologic conditions These flow cells are bounded laterally by saltwater bodies that include Cape Cod Bay, Nantucket Sound, and the Atlantic Ocean freshwater in the aquifers is separated from surrounding saltwater by a zone of transition in which the two waters mix The interface between freshwater and saltwater zones of the flow cells can move landward or seaward of its current position as a result of changes in the rate of recharge to the flow cells and changes in the rate of water pumped from the flow cells No known contamination of public-water supplies by saltwater intrusion due to ground-water pumping has occurred except where upconing of the freshwater-saltwater interface caused by ground-water pumping has been documented at the Knowles Crossing well field in Truro

The flow cells are recharged by precipitation and wastewater-return flow from such sources as septic systems and wastewater-treatment facilities Estimates of precipitation recharge to the flow cells range from about 45 to 48 percent of annual precipitation, or from about 18 to 22 in/yr Surface-water runoff is assumed to be negligible because of the highly permeable soils of Cape Cod

The water-table configuration in the Cape Cod Basin is characterized by six oblong mounds, one in each flow cell, and water-table contours approximately follow the shape of the coast Ground water flows radially from the center of the peninsula toward coastal discharge areas The altitude of the water table fluctuates by a range of 0 to 7 ft because of seasonal changes in aquifer recharge and ground-water pumping

Public-water supply systems have been in operation on Cape Cod since 1893 Average daily demand of water in 1989 in the West Cape and East Cape flow cells, the largest and most populated of the six flow cells of Cape Cod, was 24 2 and 11 4 $ft^3/s$, respectively

Transient, three-dimensional, finite-difference models were developed to simulate freshwater and saltwater flow in the West Cape, East Cape, and Truro flow cells Results of simulations indicate little change in the position of the freshwater-saltwater interface in the West Cape and East Cape flow cells for pumping and recharge conditions similar to those that occurred in the basin from predevelopment flow conditions (assumed to have ended in 1950) to 1989 and for those anticipated to occur from 1989 to 2020 Possible reasons for the small displacement of the interface for the increased pumping and recharge rates are that (1) most pumping in the flow cells is more than 1 mi inland of the coasts and from depths less than 70 ft below sea level, (2) a large percentage of water withdrawn from the ground-water-flow system is returned through wastewater return flow from septic systems and at wastewater-treatment facilities, and there is little consumptive use of the pumped water, and (3) much of the wastewater return flow from septic systems is near the coasts

Increases in pumping in the Truro flow cell also had a negligible effect on the position of the freshwater-saltwater interface except near the three areas of pumping in the cell The most pronounced effect of pumping on the Truro flow cell occurred in response to a simulated 5-year (30 percent) decrease in natural recharge The model-calculated water table responded rapidly to the increased stress and recovered quickly afterward The model-calculated freshwater-saltwater interface responded more slowly to the drought stress than the water table and subsequently recovered more slowly than the water table once the stress was removed

Transient, three-dimensional, finite-difference models also were developed to simulate freshwater flow in the West Cape, East Cape, Wellfleet, and Eastham flow cells from predevelopment flow conditions to 2020 Locally, areas such as the Mary Dunn Pond in the West Cape flow cell are significantly affected by changes in pumping and recharge conditions Total average declines in the water table at 32 observation wells in the West Cape flow cell and 19 observation wells in the East Cape flow cell are 1 8 and 2 9 ft, respectively, for the simulation period Declines in the average water levels of ponds during the simulation period are less than those at observation wells because of the greater storage capacity of the ponds than of the surrounding aquifers The average depletion in the rate of streamflow at the gaging points of eight of the largest streams in the West Cape and East Cape flow cells simulated in the models in 2020 was 14 percent of the model-calculated predevelopment streamflow in the streams

Total sources and sinks of freshwater to the West Cape and East Cape flow cells calculated by the models increase from predevelopment flow conditions to 2020 because the total rate of simulated ground-water pumping and wastewater-return flow to each system increases with time The model-calculated source of the increased ground-water pumping from the flow cells is freshwater removed from storage in the flow cells and decreased rates of freshwater discharge to streams and saltwater boundaries of the flow cells Sources and sinks of water to the Eastham and Wellfleet flow cells have not changed and are not projected to change significantly with time because no large-scale pumping occurs in the flow cells for public supply

The effects of fluctuations in seasonal pumping and recharge rates for 1989 stress conditions were determined for long-term average recharge rates and for a hypothetical, simulated 5-year (30 percent) decrease in natural recharge Simulated seasonal fluctuations in stresses resulted in a model-calculated 2 0 and 1 7 ft average range in water-table altitudes at the observation wells for the West Cape and East Cape flow cells, respectively Model-calculated fluctuations were greatest near the top of the ground-water mounds and least near the coast where water levels are held nearly constant because of the ocean discharge boundary

Numerical models developed for the Cape Cod Basin ground-water-flow cells provide information regarding regional-scale characteristics of the hydrology of the flow cells, including regional movement of the interface separating the freshwater- and saltwater-flow systems Although detailed analyses of local hydrologic conditions were beyond the scope of the investigation, the flow models may serve as starting points for more detailed investigations of smaller areas of the flow cells The results of the simulations are limited by the availability and accuracy of data on the hydraulic properties of the flow cells and streambeds, by the discretization of the models, which affects the representation of the geologic framework of the aquifer, of pumping wells, and of boundary conditions of the aquifers, and by the accuracy of available data and the temporal discretization of pumping and recharge stresses on the system

Martha's Vineyard Basin consists of more than 600 ft of Cretaceous- and Tertiary-aged coastal-plain deposits that overlie crystalline bedrock and are mantled by Pleistocene glacial formations The principal flow system of Martha's Vineyard consists of a primary and a secondary aquifer, both of which consist of glacial deposits within the upper 160 ft of saturated material

Nantucket Island Basin consists of nearly 1,500 ft of Cretaceous- and Tertiary-aged coastal-plain deposits overlying crystalline bedrock The coastal-plain deposits are mantled by 150 to 250 ft of sand and gravel deposited by Pleistocene-aged glaciers The hydrologic system of Nantucket Island can be divided into a shallow and a deep aquifer The upper 250 ft of sand and gravel outwash deposits of the shallow aquifer constitutes the principal flow system The thick wedge of fine sand, silt, and clay of the underlying coastal-plain deposits contains the deeper flow system, which is poorly understood A wide range of hydraulic conductivity values for the coastal-plain deposits that are generally much lower than the overlying glacial deposits has been reported

Martha's Vineyard and Nantucket Island Basins are surrounded by saltwater that forms the outer boundary of the fresh ground-water-flow systems Precipitation is the sole source of natural recharge to the basins Precipitation recharge to the flow systems is estimated to be 22 in/yr in the Martha's Vineyard Basin and 19 in/yr in the Nantucket Island Basin Ground water is the principal source of drinking water for the residents of Martha's Vineyard and Nantucket Island Basins

Simulated effects of ground-water pumping on Martha's Vineyard and Nantucket Island Basins were analyzed by use of uncalibrated, two-dimensional, finite-difference flow models that do not calculate absolute ground-water levels These simple flow models, called change models, were used to calculate the change in water-table altitude resulting from changes in pumping rates in the two basins They were used because insufficient data were available for the basins to construct three-dimensional, calibrated flow models (1) the change models were used to calculate changes in the altitude of the water table in each basin resulting from 180 days of pumping at projected maximum (in-season) pumping rates for 2020, with no aquifer recharge, (2) calculated changes were largest at the pumping centers The largest declines in the altitude of the water table for the Martha's Vineyard Basin were 2 7 ft near the proposed Manter site in Tisbury The largest declines for the Nantucket Island Basin were 1 6 ft near the proposed Wannacomet well The results of simulations made with the change models provide a preliminary estimate of the effects of ground-water pumping to the flow systems

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